SCS ENGINEERS

March 16, 2011 File No. 09207049.04

Mr. Douglas Hyman, P.E.
Environmental Resource Management
Florida Department of Environmental Protection, Southwest District
13051 N. Telecom Parkway
Temple Terrace, Florida 33637-0926

Subject:

Statement of Completion, Transfer to Operations Phase and Record Drawings

Citrus County Phase 3 Expansion Project

Citrus County, Florida

Permit Number 09-0291076-001

Dear Mr. Hyman:

On behalf of the Citrus County Solid Waste Management Division (SWMD), SCS Engineers (SCS) is submitting the required documentation in compliance with General Condition 13 and Specific Condition 8 of the Environmental Resource Permit No. 09-0291076-001 for the Citrus County Central Landfill Phase 3 Expansion Project.

Attachment 1 of this letter contains two originals of the "Environmental Resource Permit As-Built Certification by a Registered Professional" (Form No. 62-343.900(5), F.A.C.) signed by Dominique H. Bramlett, a Registered Professional Engineer in the State of Florida as required by General Condition 13 of the permit.

Attachment 2 of this letter contains two originals of the "Request for Transfer of Environmental Resource Permit Construction Phase to Operation Phase" (Form No. 62-343.900(7), F.A.C.) signed by Casey Stephens the director of Citrus County Solid Waste Management Department, the operating entity as required by General Condition 13 of the permit.

Attachment 3 of this letter contains two originals of the Record Drawings signed and sealed by Dominique H. Bramlett, a Registered Professional Engineer in the State of Florida as required by Specific Condition 8 of the permit. All components of the surface water management system were constructed in general accordance with the plans. The Record Drawings also include the As-Built Survey that has been signed and sealed by a Registered Surveyor in the State of Florida.

were constructed in general and As-Built Survey that has been signed and sealed by a regular to this letter contains two originals of the Box Culvert and Endwalls Report. In lieu of the precast structure as shown on the drawings the structure was poured in place. The Report contains the calculation worksheets and drawings signed and sealed by Registered Professional Engineer in the State of Florida.

Mr. Douglas Hyman, P.E. March 16, 2011 Page 2

If you have any questions, please feel free to contact us at 813-621-0080.

Sincerely,

Dominique H. Bramlett, P.E. Senior Project Engineer

SCS ENGINEERS

C. Ed Hilton, P.E.

Ed Hitos

Vice President

SCS ENGINEERS

DHB/CEH:dhb

cc: Steve Morgan, P.E., FDEP Tampa

Casey Stephens, Citrus County Solid Waste Management Division

FLORIDA DEPARTMENT OF ENVIRONMENTAL PROTECTION MAR 17 ZUII SOUTHWEST DISTRICT TAMPA

Attachment 1

"Environmental Resource Permit As-Built Certification by a Registered Professional" (Form No. 62-343.900(5), F.A.C.)

Form #62-343.900(5), F.A.C. Form Title: As-Built Certification by a Registered Professional Effective Date: October 3, 1995

ENVIRONMENTAL RESOURCE PERMIT AS-BUILT CERTIFICATION BY A REGISTERED PROFESSIONAL

Permit Number: 09-0291076-001

Project Name: Citrus County Central Landfill, Phase 3 Expansion Project

FLORIDA DEPARTMENT OF ENVIRONMENTAL PROTECTION

I hereby certify that all components of this surface water management system have been built substantially in accordance with the approved plans and specifications and are ready for inspection. Any substantial deviations (noted below) from the approved plans and specifications will not prevent the system from functioning as designed when properly maintained and operated. These determinations are based upon onsite observation of the system conducted by me or by my designee under my direct supervision and/or my review of as-built plans certified by a registered professional or Land Surveyor licensed in the State of Florida.

Dominique H. Bramlett, P.E.

4041 Park Oaks Blvd., Suite 100

Name (please print)

Signature of Professional

SCS Engineers

Company Name

Florida Registration Number

61829

Company Address

Joinpany Address

Date

Tampa, Florida 33610

City, State, Zip Code

(813) 621-0080

Telephone Number

(Affix Seal)

Substantial deviations from the approved plans and specifications:

All components of the surface water management system were constructed in general accordance with the plans.

(Note: attach two copies of as-built plans when there are substantial deviations)

Within 30 days of completion of the system, submit two copies of the form to:

Florida Department of Environmental Protection Southwest District Office 13051 North Telecom Parkway Temple Terrace, Florida 33637-0926

62-343.900(5) On-Line Document Formatted 12/01/97 kag



Attachment 2

"Request for Transfer of Environmental Resource Permit Construction Phase to Operation Phase" (Form No. 62-343.900(7), F.A.C.)

Form #: 62-353.900(7)F.A.C. Form Title: Request for Transfer to

Operation Phase

Effective Date: September 25, 1995

Request for Transfer of Environmental Resource Permit Construction Phase to Operation Phase

(To be completed and submitted by the operating entity)

1	Florida	Department of Environm	ental Protectio	n	٦	FLORIDA DEPARTMENT OF ENVIRONMENTAL PROTECTION MAR 1 7
	130511	est District Office North Telecom Parkway Terrace, Florida 33637-	0926			MAR 17 ZUII SOUTHWEST DISTRICT TAMPA
	of a sur	uested that Department P face water management s ermittee to the operation	ystem for the b	elow	mention project be transfer	construction and operation red from the construction
	Project:	Citrus County Central I	andfill, Phase	3 Exp	oansion Project	
	From:	Name: Citrus County S		nage	ment Division	
		Address: P.O. Box 340 City: Lecanto	State: FL	Zip:	34460	
	To:	Name: Citrus County S Address: P.O. Box 340		nage	ment Division	
		City: Lecanto	State: FL	Zip:	34460	
	with the	e engineers certification a rating entity. Enclosed is n areas on which the surf been previously approve	nd as outlined a copy of the ace water mana	in the docur		that if the operating entity
	any, hav		derstood and ar	e her	aditions of the permit and s eby accepted. Any propose	ubsequent modifications, if ed modifications shall be
	Operati	ng Entity: Citrus County	Solid Waste N	1anag	ement Division	
(Name	. Case Jay	lu_		Title: <u>Director</u>	
	Telepho	one: (352) 527-7670				
	Enclosu	ıre				
	☐ Cop	y of recorded transfer of y of plat(s)				anto of importanting
		y of recorded restrictive	covenants, arti	cies c	f incorporation, and certification	Late of incorporation.

ENVIRONMENTAL PROTECTION

MAR 17 2011

SOUTHWEST DISTRICT

Attachment 3

Record Drawings (Under Separate Cover)



Attachment 4

Box Culvert and Endwalls Report



CALCULATION WORKSHEETS

FOR

CITRUS CENTRAL LANDFILL PHASE 3 EXPANSION PROJECT BOX CULVERT AND ENDWA

LECANTO, FL

Prepared By: Mark E. Schröder, P.E. Florida License #67585 JAN 1 1 2011

December 2010

Project No. 10-154

Box Culvert Program: Estimate of Quantities

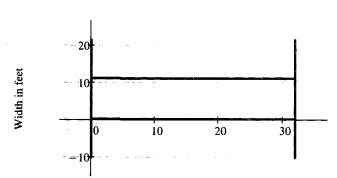
© 2002 Florida Department of Transportation

Project = "CCLandfill Box Culvert"

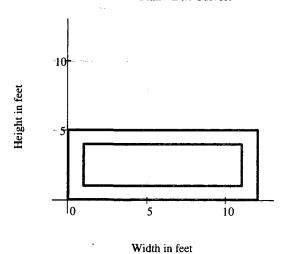
DesignedBy = "Mark E Schroder, P.E."

CheckedBy = "MES"

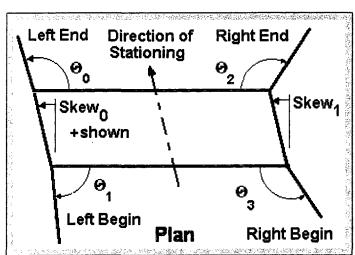
Comment = "Single Cell, 0 deg Skew, Wingwalls Parallel to Centerline"

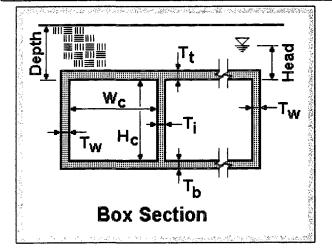


Length in feet Plan - Box Culvert



Cross Section - Box Culvert





Box Dimensions

HydraulicOpening := $W_c \cdot H_c \cdot NoOfCells$ HydraulicOpening = 30 ft^2

SoilHeight $= 0.2 \, ft$

NoOfCells = 1

 $W_c = 10 \, ft$

 $H_c = 3 ft$

 $L_c = 32 \, ft$

 $\theta^{T} = (90 \ 90 \ 90 \ 90) \cdot \text{deg}$

Head = 0 ft

 $T_t = 12 \cdot in$

 $T_b = 12 \cdot in$

 $T_w = 12 \cdot in$

 $T_i = 12 \cdot in$

Cover = $2 \cdot in$

Depth = $1.17 \, \text{ft}$

Cutoff wall and Headwall Dimensions

 $Skew_{left} = 0 \cdot deg$

 $B_{lhw} = 16 \cdot in$

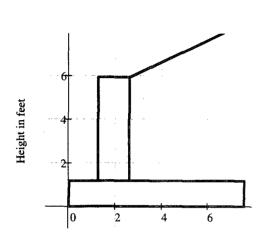
 $H_{lhw} = 21 \cdot in$

 $B_{lcw} = 12 \cdot in$

 $H_{lcw} = 24 \cdot in$

Wingwall Dimensions

CurrentDataFile = "\Data Files\CCLandFillBox1.dat"



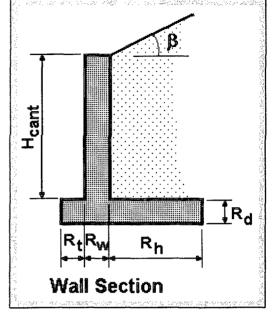
Width in feet

Cross Section - First Wingwall

$$R_{t} = \begin{pmatrix} 15 \\ 15 \\ 15 \\ 15 \\ 15 \end{pmatrix} \cdot \text{in}$$

$$\mathbf{R_w} = \begin{pmatrix} 16 \\ 16 \\ 16 \\ 16 \end{pmatrix} \cdot \mathbf{ir}$$

$$R_{h} = \begin{pmatrix} 60 \\ 60 \\ 60 \\ 60 \end{pmatrix} \cdot in$$



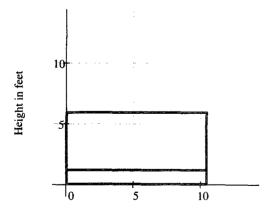
Elevation

$$\mathbf{R_d} = \begin{pmatrix} 14 \\ 14 \\ 14 \\ 14 \end{pmatrix} \cdot \mathbf{in}$$

Lww

Hstan

$$R_{t} = \begin{pmatrix} 15 \\ 15 \\ 15 \\ 15 \end{pmatrix} \cdot \text{in} \qquad R_{w} = \begin{pmatrix} 16 \\ 16 \\ 16 \\ 16 \end{pmatrix} \cdot \text{in} \qquad R_{h} = \begin{pmatrix} 60 \\ 60 \\ 60 \\ 60 \end{pmatrix} \cdot \text{in} \qquad R_{d} = \begin{pmatrix} 14 \\ 14 \\ 14 \\ 14 \end{pmatrix} \cdot \text{in} \qquad \beta = \begin{pmatrix} 26.57 \\ 26.57 \\ 26.57 \\ 26.57 \end{pmatrix} \cdot \text{deg}$$



Width in feet

Elevation - First Wingwall

$$\mathbf{H}_{end} = \begin{pmatrix} 4.75 \\ 4.75 \\ 4.75 \\ 4.75 \end{pmatrix} \mathbf{ft} \qquad \mathbf{H}_{start} = \begin{pmatrix} 4.75 \\ 4.75 \\ 4.75 \\ 4.75 \end{pmatrix} \mathbf{ft} \qquad \mathbf{L}_{ww} = \begin{pmatrix} 10.42 \\ 10.42 \\ 10.42 \\ 10.42 \end{pmatrix} \mathbf{ft} \qquad \theta = \begin{pmatrix} 90 \\ 90 \\ 90 \\ 90 \end{pmatrix} \cdot \deg$$

$$\mathbf{H_{start}} = \begin{pmatrix} 4.75 \\ 4.75 \\ 4.75 \\ 4.75 \\ 4.75 \end{pmatrix} \mathbf{ft}$$

$$L_{ww} = \begin{pmatrix} 10.42 \\ 10.42 \\ 10.42 \\ 10.42 \end{pmatrix} \text{ft}$$

$$\theta = \begin{pmatrix} 90 \\ 90 \\ 90 \\ 90 \end{pmatrix} \cdot \deg$$

Hend

Summary of Concrete Quantities

$$Vol_{cw,left} = 0.44 \cdot yd^3$$

$$Vol_{cw,right} = 0.44 \cdot yd^3$$

$$Vol_{bot,slab} = 15.56 \cdot yd^3$$
 $Vol_{walls} = 7.11 \cdot yd^3$

$$Vol_{walls} = 7.11 \cdot yd^3$$

$$Vol_{top.slab} = 14.22 \cdot yd^3$$

$$Vol_{hw.left} = 0.44 \cdot yd^3$$

$$Vol_{hw.right} = 0.44 \cdot yd^3$$

$$Vol_{wall} = \begin{pmatrix} 2.44 \\ 2.44 \\ 2.44 \\ 2.44 \\ 2.44 \end{pmatrix} \cdot yd^{3}$$

$$Vol_{wall} = \begin{pmatrix} 2.44 \\ 2.44 \\ 2.44 \\ 2.44 \end{pmatrix} \cdot yd^{3} \qquad Vol_{ww.cowall} = \begin{pmatrix} 0.3216 \\ 0.3216 \\ 0.3216 \\ 0.3216 \end{pmatrix} \cdot yd^{3} \qquad Vol_{footing} = \begin{pmatrix} 3.41 \\ 3.41 \\ 3.41 \\ 3.41 \end{pmatrix} \cdot yd^{3}$$

$$Vol_{footing} = \begin{pmatrix} 3.41 \\ 3.41 \\ 3.41 \\ 3.41 \end{pmatrix} \cdot yd^{3}$$

$$TotalVol_{wingwall} = \begin{pmatrix} 6.18 \\ 6.18 \\ 6.18 \\ 6.18 \end{pmatrix} \cdot yd^{3}$$

$$Vol_{box} = 38.73 \cdot yd^3$$

$$\sum Vol_{\text{wall}} = 9.78 \cdot \text{yd}^3$$

$$\sum \text{Vol}_{\text{wall}} \approx 9.78 \cdot \text{yd}^3$$
 $\sum \text{TotalVol}_{\text{footing}} = 14.94 \cdot \text{yd}^3$

TotalVolume =
$$63.45 \cdot \text{yd}^3$$

Summary of Soil and Miscellaneous Values

$$f_c = 3.4 \cdot \text{ksi}$$

Extension
$$= 0$$

$$Env = 1$$

Environmental Class

$$F_v = 60 \cdot ksi$$
 $n_{mod} = 9$

ConsiderLLSurcharge_{ww} = 1
$$\frac{0 - No}{1 - Yes}$$
 ConsiderLL_{hw} = 1 $\frac{0 - No}{1 - Yes}$ BarrierDL_{hw} = $0 \cdot \frac{\text{kip}}{\text{ft}}$

Consider
$$LL_{hw} = 1$$

BarrierDL_{hw} =
$$0 \cdot \frac{k}{2}$$

$$\gamma_{soil} = 110 \cdot \frac{lbf}{ft^3} \qquad k_s = 100000 \cdot \frac{lbf}{ft^3} \qquad \varphi = 32 \cdot deg \qquad q_{nom} = 2000 \cdot \frac{lbf}{ft^2}$$

$$k_s = 100000 \cdot \frac{1bt}{ft^2}$$

$$\phi = 32 \cdot \deg$$

$$q_{nom} = 2000 \cdot \frac{10}{ft^2}$$

Summary of Reinforcement Check Values

$$Check_{cw} = "OK"$$

$$Check_{hw} = "OK"$$

$$BarSize_{slabs} = \begin{pmatrix} 5\\5\\5\\5\\5 \end{pmatrix}$$

$$S_{slabs} = \begin{pmatrix} 8 \\ 8 \\ 8 \\ 8 \end{pmatrix} \cdot in$$

$$BarSize_{long} = \begin{pmatrix} 4 \\ 4 \\ 4 \\ 4 \\ 4 \\ 4 \end{pmatrix}$$

$$BarSize_{slabs} = \begin{pmatrix} 5 \\ 5 \\ 5 \\ 5 \\ 5 \end{pmatrix}$$

$$S_{slabs} = \begin{pmatrix} 8 \\ 8 \\ 8 \\ 8 \end{pmatrix} \cdot in$$

$$top slab, top mat top slab, bot mat bot slab, both m.$$

$$BarSize_{walls} = \begin{pmatrix} 4 \\ 4 \end{pmatrix} \qquad S_{walls} = \begin{pmatrix} 12 \\ 12 \end{pmatrix} \cdot in \qquad \begin{array}{c} interior \ wall(s) \\ exterior \ walls \end{array}$$

$$S_{\text{walls}} = \begin{pmatrix} 12 \\ 12 \end{pmatrix} \cdot \text{in}$$

BarSize_{corners} =
$$\begin{pmatrix} 5 \\ 5 \end{pmatrix}$$
 $S_{corners} = \begin{pmatrix} 8 \\ 8 \end{pmatrix}$ in

$$S_{corners} = {8 \choose 8} \cdot in$$

$$BarSize_{cw} = \begin{pmatrix} 4 \\ 4 \\ 4 \\ 4 \\ 4 \end{pmatrix}$$

$$Num_{cw} = \begin{pmatrix} 2 \\ 2 \\ 2 \\ 2 \\ 2 \end{pmatrix}$$

$$BarSize_{cw} = \begin{pmatrix} 4 \\ 4 \\ 4 \\ 4 \\ 4 \end{pmatrix} \qquad Num_{cw} = \begin{pmatrix} 2 \\ 2 \\ 2 \\ 2 \\ 2 \end{pmatrix} \qquad \begin{array}{l} top \ bar, \ left \ cw \\ bot \ bar, \ left \ cw \\ top \ bar, \ right \ cw \\ bot \ bar, \ right \ cw \\ \end{array}$$

StirSize_{cw} =
$$\begin{pmatrix} 3 \\ 3 \end{pmatrix}$$
 $S_{\text{stirrup.cw}} = \begin{pmatrix} 15 \\ 15 \end{pmatrix} \cdot \text{in}$

$$S_{\text{stirrup.cw}} = {15 \choose 15} \cdot in$$

$$\mathbf{BarSize_{hw}} = \begin{pmatrix} 5 \\ 5 \\ 5 \\ 5 \\ 5 \end{pmatrix} \qquad \mathbf{Num_{hw}} = \begin{pmatrix} 4 \\ 4 \\ 4 \\ 4 \\ 4 \end{pmatrix} \qquad \begin{array}{l} top \ bar, \ left \ hw \\ bot \ bar, \ left \ hw \\ top \ bar, \ right \ hw \\ bot \ bar, \ right \ hw \end{array} \qquad \mathbf{StirSize_{hw}} = \begin{pmatrix} 3 \\ 3 \\ 3 \end{pmatrix} \qquad \mathbf{S_{stirrup.hw}} = \begin{pmatrix} 10 \\ 10 \\ \end{pmatrix} \cdot \mathbf{in}$$

Reinforcement List - Main Box

	П	0	1	2
÷	0	"Bar Location"	"Size"	"Desig"
	1	"top face, top slab"	5	101
	2	"bot face, top slab"	5	102
	3	"top face, bot slab"	5	103
	4	"bot face, bot slab"	5	104
	5	"top ext corner"	5	105
$Reinf_{box} =$	6	"bot ext corner"	5	106
	7	"inside face, ext wall"	4	108
	8	"long top face, bot slab"	4	109
	9	"long top face, top slab"	4	110
	10	"long bot face, top slab"	4	111
	11	"long bot face, bot slab"	4	112
	12	"long each face, ext wall"	4	113
	13	"long each face, ext wall"	4	

Reinforcement Lists - Left Begin and Left End Wingwalls

	"Bar Location"	"Size"	"Desig"	"Len"	"Num"	"Type"	"A"	"G"	"B"	"C"	"D"	"E"	"F"	"H"	"J"	"K"	"N"
	"wall vert, soil side"	5	401	4.5	20	1	0	0	4.5	0	0	0	0	0	0	0	0
	"wall horiz, front side"	4	402	10.09	6	1	0	0	10.09	0	0	0	0	0	0	0	0
	"wall horiz, soil side"	4	404	10.09	6	1.	0	0	10.09	0	0	0	0	0	0	0	0
	"wall vert, front side"	4	406	4.5	12	1	0	0	4.5	0	0	0	0	0	0	0	0
$Rw_0 =$	"wall vert, soil side"	5	407	6.08	14	10	0	0	2.25	3.83	0	0	0	0	0	0	0
	"top footing heel"	3	409	7.25	14	1	0	0	7.25	0	0	0	0	0	0	0	0
	"bot footing toe"	3	410	7.25	14	1	0	0	7.25	0	0	0	0	0	0	0	0
	"temp footing"	3	411	10.09	20	1	0	0	10.09	0	0	0	0	0	0	0	0
	"wall to box ties"	5	412	21.17	6	1	0	0	10.09	11.09	0	0	0	0	0	0	0
,	0	0	0	21.17	0	0	0	0	10.09	11.09	0	0	0	0	0	0	0

	"Bar Location"	"Size"	"Desig"	"Len"	"Num"	"Type"	"A"	"G"	"B"	"C"	"D"	"E"	"F"	"H"	"J"	"K"	"N"
	"wall vert, soil side"	5	501	4.5	20	1	0	0	4.5	0	0	0	0	0	0	0	0
	"wall horiz, front side"	4	502	10.09	6	1	0	0	10.09	0	0	0	0	0	0	0	0
	"wall horiz, soil side"	4	504	10.09	6	1	0	0	10.09	0	0	0	0	0	0	0	0
	"wall vert, front side"	4	506	4.5	12	1	0	0	4.5	0	0	0	0	0	0	0	0
$Rw_1 =$	"wall vert, soil side"	5	507	6.08	14	10	0	0	2.25	3.83	0	0	0	0	0	0	0
	"top footing heel"	3	509	7.25	14	1	0	0	7.25	0	0	0	0	0	0	0	0
	"bot footing toe"	3	510	7.25	14	1	0	0	7.25	0	0	0	0	0	0	0	0
	"temp footing"	3	511	10.09	20	1	0	0	10.09	0	0	0	0	0	0	0	0
	"wall to box ties"	5 .	512	21.17	6	1	0	0	10.09	11.09	0	0	0	0	0	0	0
	0	0	0	21.17	0	0	0	0	10.09	11.09	0	0	0	0	0	0	0

Reinforcement Lists - Right Begin and Right End Wingwalls

	"Bar Location"	"Size"	"Desig"	"Len"	"Num"	"Type"	"A"	"G"	"B"	"C"	"D"	"E"	"F"	"H"	"J"	"K"	"N"
	"wall vert, soil side"	5	601	4.5	20	1	0	0	4.5	0	0	0	0	0	0	0	0
	"wall horiz, front side"	4	602	10.09	6	1	0	0	10.09	0	0	0	0	0	0	0	0
	"wall horiz, soil side"	4	604	10.09	6	1	0	0	10.09	0	0	0	0	0	0	0	0
	"wall vert, front side"	4	606	4.5	12	1	0	0	4.5	0	0	0	0	0	0	0	0
$Rw_2 =$	"wall vert, soil side"	5	607	6.08	14	10	0	0	2.25	3.83	0	0	0	0	0	0	0
	"top footing heel"	3	609	7.25	14	1	0	0	7.25	0	0	0	0	0	0	0	0
	"bot footing toe"	3	610	7.25	14	1	0	0	7.25	0	0	0	0	0	0	0	0
	"temp footing"	3	611	10.09	20	1	0	0	10.09	0	0	0	0	0	0	0	0
	"wall to box ties"	5	612	21.17	6	1	0	0	10.09	11.09	0	0	0	0	0	0	0
	0	0	0	21.17	0	0	0	0	10.09	11.09	0	0	0	0	0	0	0

	"Bar Location"	"Size"	"Desig"	"Len"	"Num"	"Type"	"A"	"G"	"B"	"C"	"D"	"E"	"F"	"H"	"J"	"K"	"N"
	"wall vert, soil side"	5	701	4.5	20	1	0	0	4.5	0	0	0	0	0	0	0	0
	"wall horiz, front side"	4	702	10.09	6	1	0	0	10.09	0	0	0	0	0	0	0	0 .
	"wall horiz, soil side"	4	704	10.09	6	1	0	0	10.09	0	0	0	0	0	0	0	0
	"wall vert, front side"	4	706	4.5	12	1	0	0	4.5	0	0	0	0	0	0	0	0
$Rw_3 =$	"wall vert, soil side"	5	707	6.08	14	10	0	0	2.25	3.83	0	0	0	0	0	0	0
	"top footing heel"	3	709	7.25	14	1	0	0	7.25	0	0	0	0	0	0	0	0
	"bot footing toe"	3	710	7.25	14	1	0	0	7.25	0	0	0	0	0	0	0	0
	"temp footing"	3	711	10.09	20	1	0	0	10.09	0	0	0	0	0	0	0	0
	"wall to box ties"	5	712	21.17	6	1	0	0	10.09	11.09	0	0	0	0	0	0	0
	0	0	0	21.17	0	0	0	0	10.09	11.09	0	0	0	0	0	0	0

Reinforcement Lists - Headwalls and Cutoff Walls

$$\mathbf{Rh_1} = \begin{pmatrix} \text{"Bar Location" "Size" "Desig" "Len" "Num" "Type" "A" "G" "B" "C" "D" "E" "F" "H" "J" "K" "N" \\ \text{"top"} & 5 & 801 & 11.67 & 4 & 1 & 0 & 0 & 11.67 & 0 & 0 & 0 & 0 & 0 & 0 \\ \text{"bottom"} & 5 & 802 & 11.67 & 4 & 1 & 0 & 0 & 11.67 & 0 & 0 & 0 & 0 & 0 & 0 & 0 \\ \text{"stirrups"} & 3 & 803 & 5.13 & 15 & 27 & 0 & 0 & 1.36 & 0.5 & 0.5 & 0.42 & 0.95 & 0.83 & 0.83 & 0 & 0 \\ \end{pmatrix}$$

$$Rh_2 = \begin{pmatrix} \text{"Bar Location" "Size" "Desig" "Len" "Num" "Type" "A" "G" "B" "C" "D" "E" "F" "H" "J" "K" "N" \\ \text{"top"} & 5 & 804 & 11.67 & 4 & 1 & 0 & 0 & 11.67 & 0 & 0 & 0 & 0 & 0 & 0 & 0 \\ \text{"bottom"} & 5 & 805 & 11.67 & 4 & 1 & 0 & 0 & 11.67 & 0 & 0 & 0 & 0 & 0 & 0 & 0 \\ \text{"stirrups"} & 3 & 806 & 5.13 & 15 & 27 & 0 & 0 & 1.36 & 0.5 & 0.5 & 0.42 & 0.95 & 0.83 & 0.83 & 0 & 0 \\ \end{pmatrix}$$

$$Rc_{1} = \begin{pmatrix} \text{"Bar Location" "Size" "Desig" "Len" "Num" "Type" "A" "G" "B" "C" "D" "E" "F" "H" "J" "K" "N" \\ \text{"top"} & 4 & 807 & 11.67 & 2 & 1 & 0 & 0 & 11.67 & 0 & 0 & 0 & 0 & 0 & 0 \\ \text{"bottom"} & 4 & 808 & 11.67 & 2 & 1 & 0 & 0 & 11.67 & 0 & 0 & 0 & 0 & 0 & 0 & 0 \\ \text{"stirrups"} & 3 & 809 & 4.9 & 10 & 7 & 0 & 0 & 1.61 & 0.67 & 0.5 & 0.5 & 0 & 0 & 0 & 0 & 0 \\ \end{pmatrix}$$

$$Rc_{2} = \begin{pmatrix} \text{"Bar Location" "Size" "Desig" "Len" "Num" "Type" "A" "G" "B" "C" "D" "E" "F" "H" "J" "K" "N" \\ \text{"top"} & 4 & 810 & 11.67 & 2 & 1 & 0 & 0 & 11.67 & 0 & 0 & 0 & 0 & 0 & 0 & 0 \\ \text{"bottom"} & 4 & 811 & 11.67 & 2 & 1 & 0 & 0 & 11.67 & 0 & 0 & 0 & 0 & 0 & 0 & 0 \\ \text{"stirrups"} & 3 & 812 & 4.9 & 10 & 7 & 0 & 0 & 1.61 & 0.67 & 0.5 & 0.5 & 0 & 0 & 0 & 0 & 0 \end{pmatrix}$$

No variables are modified in this file CurrentDataFile = "\Data Files\CCLandFillBox1.dat"

```
OuickWall 7.1 - RETAINING WALL ANALYSIS AND DESIGN
________
Job ID : CC Central Landfill
                                      Designed By : Mark Schroder, PE
Job Description : Headwalls
________
FOOTING DESIGN METHOD: Ultimate Strength ACI 318-08
STEM DESIGN METHOD : Ultimate Strength ACI 318-08 (Concrete)
WALL TYPE : Cantilever Retaining Wall
RETAINING WALL DIMENSIONS:
_______
Wall Stem Height = 6.00 ft. Stem Thickness @ Top = 12.00 in. Stem Thickness @ Bottom = 12.00 in.
Footing Thickness = 12.00 in.
Heel Width = 2.50 ft.
Toe Width
                         = 1.50 ft.
Stem Bar Size = \# 5 at 12.00 in. o.c.
Heel Bar Size = \# 5 at 12.00 in. o.c.
Toe Bar Size = \# 5 at 12.00 in. o.c.
Footing Key Depth = 0.00 \text{ ft.}
Footing Key Width = 0.00 \text{ ft.}
BackFill Slope (Vert/Horiz) = 0.00 :12
RETAINING WALL LOADS:
Horizontal Equivalent Fluid Pressure = 35.00 pcf. (Load Case = Soil)
Backfill Height
                                      = 6.00 \text{ ft.}
Equivalent Fluid Pressure Angle
                                     = 0.00 \text{ deg.}
                                     = 220 psf. (Load Case = Soil)
Vertical Surcharge on Backfill
                                         0 psf. (Load Case = Live)
Horizontal Surcharge
                                      =
                                            0 psf. (Load Case = Soil)
Vertical Surcharge on Toe
                                      =
                                      = 0 psf. (Load Case = Wind)
Wind Load on Fence
                                       = 0.00 ft.
Fence Height
Line Ld. Type Magnitude Dist. (x) Load Case No. (H or V) (plf) (ft.)
  1
 2
 3
 4
 5
 6
 7
 8
 9
 10
```

Notes: 1. "H" = Horizontal loads. "V" = Vertical loads.

2. Vertical loads are positive down.

ULTIMATE STRENGTH LOAD COMBINATIONS (Concrete Design):

```
1.4D + 1.4H

1.2D + 1,6L + 1.6H + 0.5R

1.2D + 1.6R + 1.0L

1.2D + 1.6R + 0.8W

1.2D + 1.6W + 1.0L + 0.5R

1.2D + 1.0E + 1.0L + 0.2R

0.9D + 1.6W + 1.6H

0.9D + 1.0E + 1.6H
```

WORKING STRESS LOAD COMBINATIONS (Stability Checks and Masonry Design):

```
D + L + R + H
D + L + W + H
D + L + W + 0.5R + H
D + L + R + 0.5W + H
D + L + R + E/1.4 + H
D + E/1.4 + H
```

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RETAINING WALL RESISTING FORCES:

Allowable Soil Pressure = 2,000 psf. Passive Equivalent Fluid Press. = 300.00 pcf. Passive Soil Height = 1.00 ft. Coefficient of Friction = 0.50 Cohesion = 0 psf.

Use Vertical Surcharge as Resisting Wt.? = No

Overturning Safety Factor = 2.50 Sliding Safety Factor = 1.50 Limit Reaction to Mid 1/3? = No

MATERIAL DATA:

Concrete Strength, f'c = 3.40 ksi.Steel Yield Strength, Fy = 60.00 ksi.

Concrete Unit Weight = 150.00 pcf.Soil Unit Weight = 110.00 pcf.Fence Weight = 0.00 psf.

REINFORCING STEEL DATA:

Concrete cover to center of steel:

Wall Inside Face = 3.00 in.

Footing Heel (Top Face) = 3.00 in.

Footing Toe (Bottom Face) = 3.00 in.

Minimum Ratios for Shrinkage and Temperature Reinf:

Vertical Stem Reinf. = 0.0018

Horizontal Stem Reinf. = 0.0020

Footing Reinforcement = 0.0018

3 of 8

DIMENSIONS:

Stem Height =	= 6.00	ft.	Heel Length	=	2.50 ft.
Stem Thick. @ Top =	= 12.00	in.	Toe Length	=	1.50 ft.
Stem Thick. @ Base =	= 12.00	in.	Total Ftg. Width, B	=	5.00 ft.
Footing Thickness =	= 12.00	in.	Key Depth	==	0.00 ft.
			Key Width	=	0.00 ft.

ANALYSIS RESULTS:

Max Brg Press. @ Toe	=	1,267 psf. O.K.	Sliding Force	= 1,348	3 Lb
@ Heel	=	119 psf. O.K.	Resisting Force	= 2,183	3 Lb
Allowable Brg. Press.	=	2,000 psf.	F.O.S. =	1.62 O.K	•

Resultant Loc From (C.L.=	0.69	ft.	Overturn.	Moment	=	3,716	ft-lb
Kern Point Loc., E	B/6 =	0.83	ft.	Resisting	Moment	==	9,986	ft-lb
Limit Resultant To M	Mid 1/3	? = 1	1 0		F.O.S.	=	2.69	O.K.

DECICN DECILIES. Decima Method Otems 1905 ACT 210 00 (Consents)

DESIGN RESULTS: Design Method, Stem: USD, ACI 318-08 (Concrete)
Ftg.: Ultimate Strength ACI 318-08

		Mu	Vu	Phi Vn	Shr	As Reqd.	As Furn.	Astl.	Devel.
		(ft-k)	(kip)	(kip)	Chk.	(in^2)	(in^2)	Chk.	Lgth.
Stem	:	4.03	1.68	9.45	O.K.	0.303	0.310	O.K.	O.K.
Toe	:	1.72	1.22	9.45	O.K.	0.259	0.310	O.K.	O.K.
Heel	:	2.57	1.76	9.45	O.K.	0.259	0.310	O.K.	O.K.
Key	:	0.00	0.00	0.00		0.000	0.000		

- Notes: 1. Stem moments are positive if they cause tension on the soil face.

 Negative if they cause tension on the outside face.

 Stem shear is positive to the left as measured on a section cut below the top of wall.
 - 2. Heel moments are positive if they cause tension in the top of the footing. Heel shear is positive up as measured on a section cut to the right of the end of the heel.
 - 3. Toe moments are positive if they cause tension in the bottom of the footing. Toe shear is positive up as measured on a section cut to the left of the end of the toe.

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Stability Analysis: Governing Combination = D + L + R + H

------RESISTING FORCES-----
Element Weight x Arm = Moment Element Force x Arm = Moment

Element Weight x Arm = Moment Element Force x Arm = Moment Soil 1,815 6,311 R at Top
Ftg. 750 2.50 1,875 R at Bot.
Stem 900 2.00 1,800 Horiz. EFP 858 2.33 2,001
Vert Sur Vert Sur 490 3.50 1,715
Vert EFP Horiz Sur Wind
Fence Wt.
V. line Vert. line

Sum WT = 3,465 MR = 9,986 Sum F = 1,348 MOT = 3,716

Friction Force = 1,733 Lb F.O.S. Sliding = RF / F = 1.62 Passive Pressure = 450 Lb F.O.S. Overturn. = MR / MOT = 2.69 Cohesion = 0 Lb Coef. Vert. Surcharge or Line Load

Resist. Force, Sum RF = 2,183 Lb to Horiz. = EFP / Soil Dens. = 0.318

Resultant Loc From Toe, X = (MR - MOT) / Sum WT = 1.81 ft.Eccentricity From Ftg. C.L., e = (B / 2) - X = 0.69 ft.Soil Pressure @ Toe = (WT / B) * (1 + 6e/B) = 1,267 psf.Soil Pressure @ Heel = (WT / B) * (1 - 6e/B) = 119 psf.

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DETAILED DESIGN REPORT

STEM DESIGN: Steel Design Comb = 1.2D + 1.6L + 1.6H + 0.5RShr Strength @ Base, Phi Vn = 9.45 kip

Dist From	d	Mu	Vu	As Flex.	As Min.	As T+S	As Reqd	Comb
Top (ft)	(in.)	(ft-k)	(kip)	(in^2)	(in^2)	(in^2)	(in^2)	
0.60	9.00	0.02	0.08	0.001	0.001	0.259	0.259	2
1.20	9.00	0.10	0.17	0.002	0.003	0.259	0.259	2
1.80	9.00	0.24	0.29	0.006	0.008	0.259	0.259	2
2.40	9.00	0.45	0.43	0.011	0.015	0.259	0.259	2
3.00	9.00	0.76	0.59	0.019	0.025	0.259	0.259	2
3.60	9.00	1.16	0.77	0.029	0.038	0.259	0.259	2
4.20	9.00	1.68	0.96	0.042	0.056	0.259	0.259	2
4.80	9.00	2.32	1.18	0.058	0.077	0.259	0.259	2
5.40	9.00	3.10	1.42	0.077	0.103	0.259	0.259	2
6.00	9.00	4.03	1.68	0.101	0.134	0.303	0.303	2

Vertical Stem Reinforcement:

Shear-Friction Steel Added at Stem Base (ACI 08 11.6), Avf = 0.044 in^2 Available Length for Hook Embedment into Footing = 9.50 in. Available Length for Straight Embedment into Stem = 70.00 in.

	Development	Length			
	Straight	Hook	Percent	Spac.	50% Cut Off
	(in.)	(in.)	Develop.	(in.)	(in.)
#4	12.35	7.20	100.00	7.91	72.00
#5	15.43	9.00	100.00	12.26	72.00
#6	18.52	10.80	87.93	15.30	72.00
#7	27.01	12.61	75.37	17.88	72.00
#8	30.87	14.41	65.95	18.00	72.00
#9	34.82	16.25	58.46	18.00	72.00
#10	41.49	18.30	51.93	18.00	72.00
#11	51.14	20.31	46.77	18.00	72.00

Horizontal Stem Reinforcement:

Area of steel for Shrinkage and Temp. Reinforcement = 0.288 in^2

	Spacing,	in	Total	Bars
	I.F. Only	EA. Face	I.F. Only	EA. Face
#4	8.33	16.67	10.00	6.00
#5	12.92	18.00	7.00	5.00
#6	18.00	18.00	5.00	5.00
#7	18.00	18.00	5.00	5.00
#8	18.00	18.00	5.00	5.00
#9	18.00	18.00	5.00	5.00
#10	18.00	18.00	5.00	5.00
#11	18.00	18.00	5.00	5.00

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TOE DESIGN:

- * Steel Design Comb. = 0.9D + 1.6W + 1.6H
- * Thickness Design Comb. = 0.9D + 1.6W + 1.6H
- * Available Length for Hook Embedment into Stem = 9.50 in.
- * Available Length for Straight Embed. into Toe = 16.00 in.

								
d	Mu	Vu	Phi Vn	As Flex.	As Min.	As T+S	As Req'd	
(in.)	(ft-k)	(kip)	(kip)	(in^2)	(in^2)	(in^2)	(in^2)	
9.00	1.72	1.22	9.45	0.043	0.057	0.259	0.259	

#4 #5 #6 #7	Development Straight (in.) 12.35 15.43 18.52 27.01	Hook (in.) 7.20 9.00 10.80 12.61	Percent Develop. 100.00 100.00 86.38 59.24	Spac. (in.) 9.26 14.35 17.60 16.45
#8	30.87	14.41	51.83	18.00
#9 #10 #11	34.82 41.49 51.14	16.25 18.30 20.31	45.95 38.56 31.28	18.00 18.00 18.00

HEEL DESIGN:

- * Steel Design Comb. = 1.2D + 1.6L + 1.6H + 0.5R
- * Thickness Design Comb. = 0.9D + 1.6W + 1.6H
- * Available Length for Straight Embedment into Toe = 28.00 in.
- * Available Length for Straight Embedment into Heel = 28.00 in.

d	Mu	Vu	Phi Vn	As Flex.	As Min.	As T+S	As Req'd	
(in.)	(ft-k)	(kip)	(kip)	(in^2)	(in^2)	(in^2)	(in^2)	
9.00	2.57	1.76	9.45	0.064	0.085	0.259	0.259	

	Development	Length		
	Straight	Hook	Percent	Spac.
	(in.)	(in.)	Develop.	(in.)
#4	12.35	7.20	100.00	9.26
#5	15.43	9.00	100.00	14.35
#6	18.52	10.80	100.00	18.00
#7	27.01	12.61	100.00	18.00
#8	30.87	14.41	90.70	18.00
#9	34.82	16.25	80.41	18.00
#10	41.49	18.30	67.48	18.00
#11	51.14	20.31	54.75	18.00

LONGITUDINAL FOOTING REINFORCEMENT (TEMP & SHRINK ONLY):

	Spacing (in.)
#4	9.26
#5	14.35
#6	20.37
#7	27.78
#8	36.57
#9	46.30
#10	58.80
#11	72.22

CITRUS COUNTY SOLID WASTE MANAGEMENT DIVISION

CENTRAL LANDFILL PHASE 3 EXPANSION PROJECT BOX CULVERT AND HEADWALLS

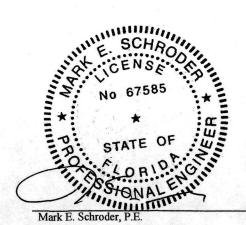


DRAWING INDEX

DRAWING NO.	DRAWING TITLE
1	COVER SHEET
2	BOX CULVERT DATA
3	BOX CULVERT - INDEX 289, SHEET 1
4	BOX CULVERT - INDEX 289, SHEET 2
5	BOX CULVERT - INDEX 289, SHEET 3
6	BOX CULVERT - INDEX 289, SHEET 5
7	24" RCP HEADWALL DETAILS

Kings Bay Engineering

FL Certificate #28555 9478 W Marquette Lane Crystal River, FL 34428 (352) 564-8017



Florida License #67585
DATE: JAN 1 1 2011

REV 1: Issued For County Permit - 12/10/10

UCTURE																			
DIDAE L					BOX								HEADWA	LL AND	CUTOF	F WALL			
RIDGE IMBER	Wc(ft)	Hc(ft)	Tt	Tw	ТЬ	Ti	#cells	Lc(ft)	Cover	Blhw	Hlhw	Brhw	Hrhw	Blcw	HICW	Brcw	Hrcw	SL (deg)	SR(deg
BC1	10	3	12	12	12	na	1	32	2	16	21	16	21	12	24	12	24	0	0
M	BER	BER Wc(ft)	BER Wc(ft) Hc(ft)	BER Wc(ft) Hc(ft) Tt	BER Wc(ft) Hc(ft) Tt Tw	BER Wc(ft) Hc(ft) Tt Tw Tb	BER Wc(ft) Hc(ft) Tt Tw Tb Ti	BER Wc(ft) Hc(ft) Tt Tw Tb Ti #cells	BER Wc(ft) Hc(ft) Tt Tw Tb Ti #cells Lc(ft)	BER Wc(ft) Hc(ft) Tt Tw Tb Ti #cells Lc(ft) Cover	BER Wc(ft) Hc(ft) Tt Tw Tb Ti #cells Lc(ft) Cover Blhw	BER Wc(ft) Hc(ft) Tt Tw Tb Ti #cells Lc(ft) Cover Blhw Hlhw	BER Wc(ft) Hc(ft) Tt Tw Tb Ti #cells Lc(ft) Cover Blhw Hlhw Brhw	BER Wc(ft) Hc(ft) Tt Tw Tb Ti #cells Lc(ft) Cover Blhw Hlhw Brhw Hrhw	BER Wc(ft) Hc(ft) Tt Tw Tb Ti #cells Lc(ft) Cover Blhw Hlhw Brhw Hrhw Blcw	BER Wc(ft) Hc(ft) Tt Tw Tb Ti #cells Lc(ft) Cover Blhw Hlhw Brhw Hrhw Blcw Hlcw	BER Wc(ft) Hc(ft) Tt Tw Tb Ti #cells Lc(ft) Cover Blhw Hlhw Brhw Hrhw Blcw Hlcw Brcw	BER Wc(ft) Hc(ft) Tt Tw Tb Ti #cells Lc(ft) Cover Blhw Hlhw Brhw Hrhw Blcw Hlcw Brcw Hrcw	BER Wc(ft) Hc(ft) Tt Tw Tb Ti #cells Lc(ft) Cover Blhw Hlhw Brhw Hrhw Blcw Hlcw Brcw Hrcw SL(deg)

	Date /-	ole Date	Tal			wise)	n other	ss show	es unles	LE (inche	TA TAB	LLS DA	WINGWA	T SIDE	LEF				
				NDWALL	BEGIN E	LEFT I						L	VINGWAL	T END W	LEF				STRUCTURE
le(ft) Hs(ft)	is(ft)	Hs(ft)	He(ft)	β(deg)	SW(deg.	Rd	Rh	Rw	Rt	Lw(ft)	Hs(ft)	He(ft)	β(deg)	SW(deg	Rd	Rh	Rw	Rt	/BRIDGE NUMBER
.75 4.75	1.75 1	4.75	4.75	26.57	90	14	60	16	15	10.42	4.75	4.75	26.57	90	14	60	16	15	BC1
!	1.75 4	.75	4	26.57	90	14	60	16	15	10.42	4.75	4.75	26.57	90	14	60	16	15	BC1

				RIGH	IT SIDE	WINGW	ALLS DA	ATA TAE	BLE (incl	nes unle	ess sho	vn othe	rwise)			Та	ble Date	7-01-09
STRUCTURE				RIG	HT END	WINGW	ALL						RIGHT	BEGIN E	NDWALL			
/BRIDGE NUMBER	Rt	Rw	Rh	Rd	SW(deg	β(deg)	He(ft)	Hs(ft)	Lw(ft)	Rt	Rw	Rh	Rd	SW(deg.	β(deg)	He(ft)	Hs(ft)	Lw(ft)
BC1	15	16	60	14	90	26.57	4.75	4.75	10.42	15	16	60	14	90	26.57	4.75	4.75	10.42

							E	STIMATI	ED CONG	CRETE (QUANTIT	TIES (C)	Y)						To	ble Date	7-01-09
STRUCTURE				В	OX I					EFT EN			FT BEG VINGWAL			IGHT EI			HT BE		Culver
/BRIDGE NUMBER	Left Cutoff Wall	Right Cutoff Wall	Bottom Slab	Walls	Top Slab	Left Head Wall	Right Head Wall	Sub Total	Footing	Wall	Sub Total	Footing	Wall	Sub Total	Footing	Wall	Sub Total	Footing	Wall	Sub Total	Total
BC1	0.44	0.44	15.56	7.11	14.22	0.44	0.44	38.73	3.41	2.76	6.18	3.41	2.76	6.18	3.41	2.76	6.18	3.41	2.76	6.18	63.45

						MAIN	STEEL	REINFL	RCEMEN	IT SPAC	ING (in	ches)					To	oble Date	-01-09
STRUCTURE				BO)X											HEAD	WALLS	CUTOFF	WALL
/BRIDGE NUMBER	101	102	103	104	105	106	107	108	109	110	111	112	113	114	115, 116	803	806	809	812
BC1	8	8	8	8	8	8	na	12	12	12	12	12	12	12	na	10	10	15	15

										WIN	GWALL	STEEL	REINFOR	RCEMEN	T SPACI	NG (inc	hes)									Ta	ble Date	7-01-09
STRUCTURE	STRUCTURE LEFT END WINGWALL						LEFT BEGIN WINGWALL						RIGHT END WINGWALL						RIGHT BEGIN WINGWALL									
/BRIDGE NUMBER	401 407(8)	402 (403)	404 (405)	406	409	410	411	501 507(8)	502 (503)	504 (505)	506	509	510	511	601 607(8)	602 (603)	604 (605)	606	609	610	611	701 707(8)	702 (703)	704 (705)	706	709	710	711
BC1	10	12	12	12	10	10	10	10	12	12	12	10	10	10	10	12	12	12	10	10	10	10	12	12	12	10	10	10

WINGWALL NOTE: Bar designations in "()" are only required for variable height wingwalls.

MAIN STEEL REINFORCING SIZE TABLE														ABLE B	Y MES				
STRUCTURE /BRIDGE		ВОХ															VALLS	CUTOFF	WALLS
NUMBER	101	102	103	104	105	106	107	108	109	110	111	112	113	114	115	803	806	809	812
BC1	5	5	5	5	5	5	na	4	4	4	4	4	4	4	na	3	3	3	3

										WING	WALL 5	TEEL R	EINFOR	CING S	IZE											1	TABLE B	Y MES
STRUCTURE	LEFT END WINGWALL							LEFT BEGIN WINGWALL						RIGHT END WINGWALL							RIGHT BEGIN WINGWALL							
/BRIDGE NUMBER	401	402	404	406	409	410	411	501	502	504	506	509	510	511	601	602	604	606	609	610	611	701	702	704	706	709	710	711
BC1	5	4	4	4	3	3	3	5	4	4	4	3	3	3	5	4	4	4	3	3	3	5	4	4	4	3	3	3

NOTES:

1. Environmental Class: 1

2. Reinforcing Steel, Grade: 60

3. Concrete Class: II f'c = : 3.4 ksi

4. Soil Properties:

Friction Angle: 32

Modulus of Subgrade Reaction : 9 Nominal Bearing Resistance : 2,000psi

5. Total Estimated Quantity of Reinforcing Steel: n/a lbs

6. Work this Drawing with Design Standards Index No. 289 and Sheet Nos. :1,2,3 & 5

7. n/a

FLORIDA DEPARTMENT OF ENVIRONMENTAL PROTECTION

MAR 1'7 2011

SOUTHWEST DISTRICT

ENGINEER'S NOTES:

1. THE DESIGN HAS BEEN PERFORMED USING THE FDOT LRFD BOX CULVERT DESIGN SOFTWARE.

2. SINCE NO SOIL DATA WAS PROVIDED THIS DESIGN CONSIDERS THE FOLLOWING SOIL CONDITIONS:

2.1. MAXIMUM BEARING CAPACITY OF 2,000PSF;

2.2. ENVIRONMENTAL CLASS 1 OR SLIGHTLY AGGRESSIVE SOIL

5. REINFORCING STEEL SHALL BE GRADE 60 DEFORMED BILLET STEEL
6. MINIMUM 2 INCH STEEL COVER REQUIRED, 3 INCHES WHERE

SURFACE WILL BE IN CONTACT WITH SOIL.

7. ALL STEEL INTERSECTIONS SHALL BE TIED. 8. MINIMUM SPLICE LENGTH SHALL BE 30 INCHES.

9. MINIMUM BENDING RADIUS IS 5 INCHES.

10. CONCRETE SHALL BE CLASS II - 3,400 PSI 28 DAY MINIMUM STRENGTH.

11. THE CONTRACTOR IS PERMITTED TO USE CONCRETE WITH A MAXIMUM
7 INCH SLUMP WITH THE USE OF A HIGH RANGE WATER REDUCER.

12. NO CHANGES MAY BE MADE TO THIS DESIGN WITHOUT CONSULTING
THE DESIGN ENGINEER INCLUDING BUT NOT LIMITED TO ADDITIONAL
STEEL, LARGER STEEL THAN SPEC'D, STRONGER CONCRETE THAN
SPEC'D AND THICKER CROSS-SECTIONS.

2011 B NAME: CC CENTRAL I

Wark E. Schroder, P.E.

1) Issued for County Permit

DRAWN BY: MES

BRAINBER: 10-154

9478 W Marquete Lane Crystal River, FL 3428 (352) 564-8017

BOX CULVERT DATA

AL LANDFILL

2 John 308

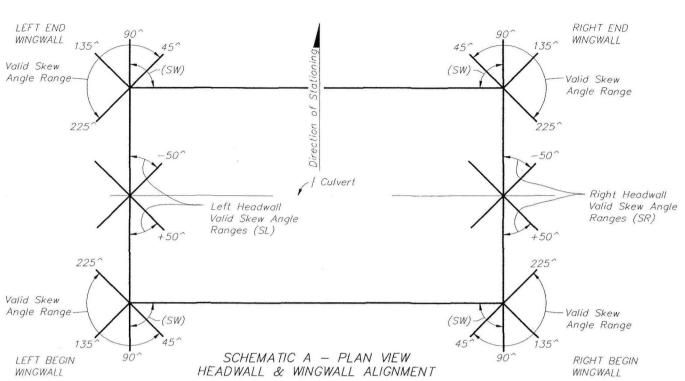
INDEX CULVERT BOX

SHEET

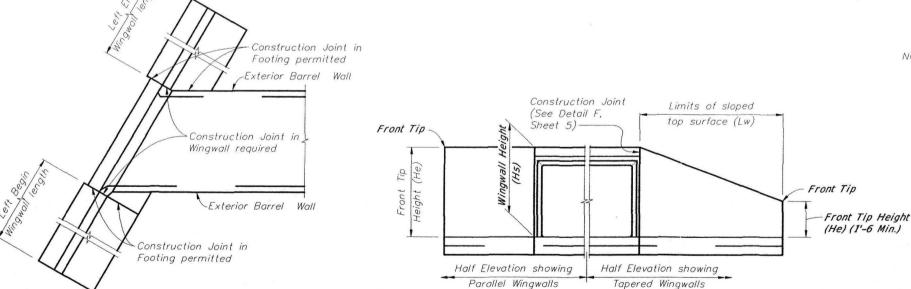
289,

CENTRAL LANDFILL \equiv

NAME 2011



NOTE: All headwall and culvert skew angles are measured in degrees from a line perpendicular to the centerline of culvert (counter-clockwise positive), see Schematic B.



END ELEVATION OF CULVERT

PART PLAN SHOWING PARALLEL WINGWALLS AND LOCATION OF CONSTRUCTION JOINTS

NOTE: Construction Joints in wingwalls and footings are located as follows: For non-skewed wingwalls they are located adjacent to the exterior face of the exterior barrel wall; when the | of wingwall and | of exterior barrel wall results in an acute angle see Left End Wingwall above, and when the angle is obtuse see Left Begin Wingwall above and Detail C (Sheet 5).

GENERAL NOTES:

DESIGN SPECIFICATIONS: AASHTO LRFD Bridge Design Specifications, 3rd Edition.

LIVE LOAD: HL-93.

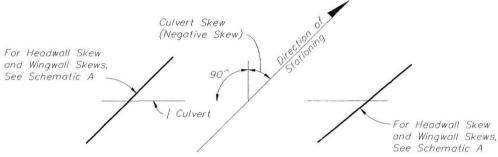
CONSTRUCTION LOADING: It is the construction Contractor's responsibility to provide for supporting construction loads that exceed AASHTO HL-93, and any construction load applied prior to 2 feet of compacted fill placed above the top slab.

SURFACE FINISH: All concrete surfaces shall receive a general surface finish.

SKEWED CONSTRUCTION JOINTS: Construction joints in barrels of culverts with skewed wingwalls may be placed parallel to the headwalls and the reinforcing steel, and the slabs may be cut provided that the cut reinforcing steel extends beyond the construction joint enough for splices to be made in accordance with Table 1 on this sheet. The cost of construction joints and additional reinforcing shall be at the expense of the Contractor.

CULVERT EXTENSIONS: For cut backs and ties into existing concrete box culverts see Sheet 6 of 7.

REINFORCING STEEL: ASTM A615, see the Box Culvert Data Tables in the Contract Plans for grade and bar spacing. See the Reinforcing Bar List in the Contract Plans for bar sizes and bar bending details.



SCHEMATIC B - PLAN VIEW CULVERT ALIGNMENT

NOTE: For Culvert Skew see Contract Plans.

TAE	BLE 1 - N	AINIMUM E	BAR SF	PLICE LEN	IGTHS
	FOR LO	NGI TUDIN i	AL REII	VFORCING	
BAR	SPLICE (CLASS B)	BAR	SPLICE (CLASS B)
SIZE	CLASS II	CLASS IV	SIZE	CLASS 11	CLASS IV
_	(3400 psi)	(5500 psi)		(3400 psi)	(5500 psi)
#3	1'-0	1'-0	#8	3'-6	2'-9
#4	1'-4	1'-4	#9	4'-5	3'-6
#5	1'-8	1'-8	#10	6'-7	4'-5
#6	1'-11	1'-11	#11	7'-10	6'-5
#7	2'-8	2'-3			

TABLE 1 NOTE: Splice lengths are based on an AASHTO Class B tension lap splice for the Specification Section 346 concrete class shown.

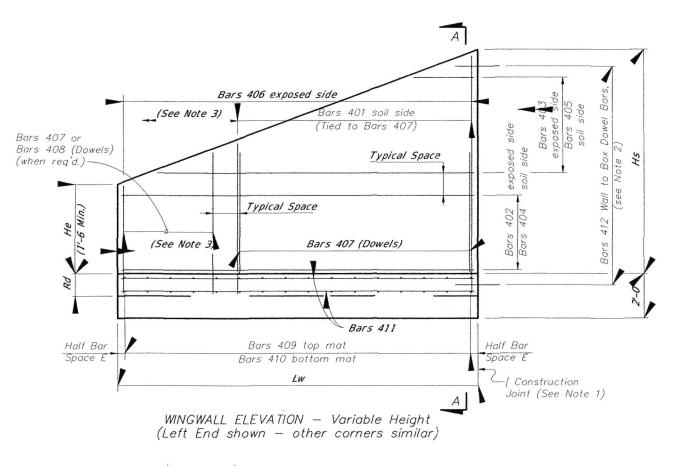
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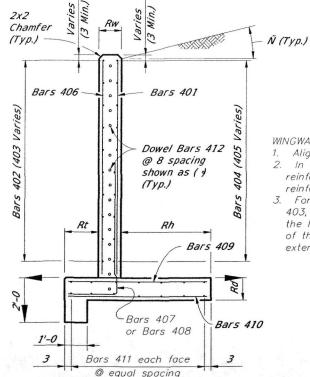
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SOUTHWEST DISTRICT TAMPA

CULVERT BARREL NOTES:

- Space Bars 110 and 112 with a bar in each corner, and at the | of interior walls (for multiple barrel culverts only), and the remaining bars placed at equal spacing shown in the Contract Plans. Adjust last bar spacing when required.
- 2. Place Bars 113 and 114 at spacing shown in the Contract Plans evenly between Bars 109 and 111.
- 3. Locate the first transverse bar from the ends of the culvert at one half the bar spacing, but provide the minimum reinforcement cover and not greater than 4 clear.





WINGWALL SECTION A-A

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SOUTHWEST DISTRICT TAMPA

WINGWALL NOTES:

- Align construction joint perpendicular to wingwall.
 In the vicinity of the construction joint, field bend
- In the vicinity of the construction joint, field bend reinforcement as necessary to maintain minimum reinforcement cover.
- 3. For constant height wingwalls, variable length Bars 403, 405 & 408 are not required, and as such the limits of Bars 401 & 407 extend the full length of the wingwall, and the limits of Bars 402 & 404 extend to the full height of the wingwall.

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PHASE III EXPSNSION PROJECT

SHEET

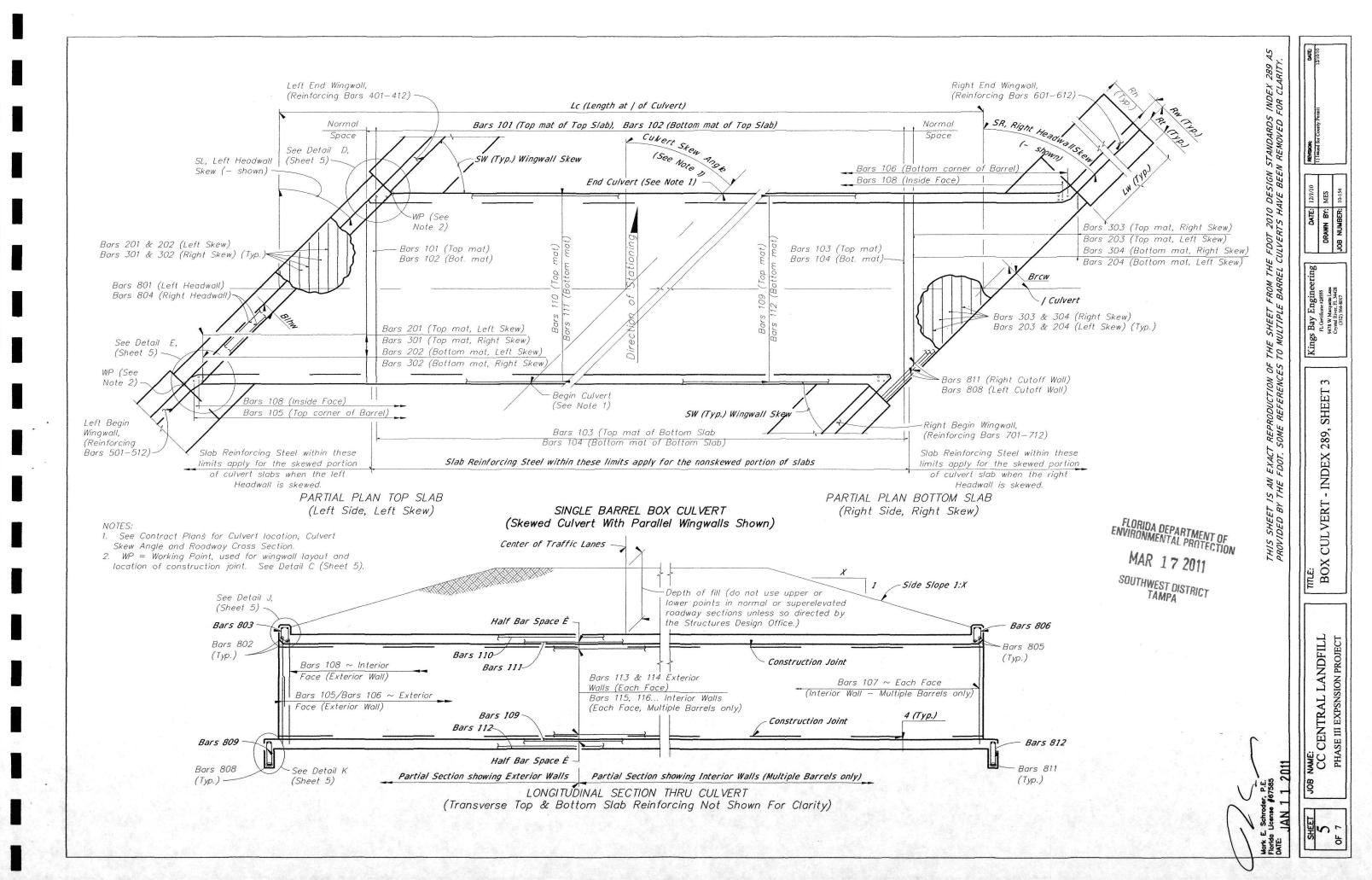
BOX CULVERT - INDEX 289,

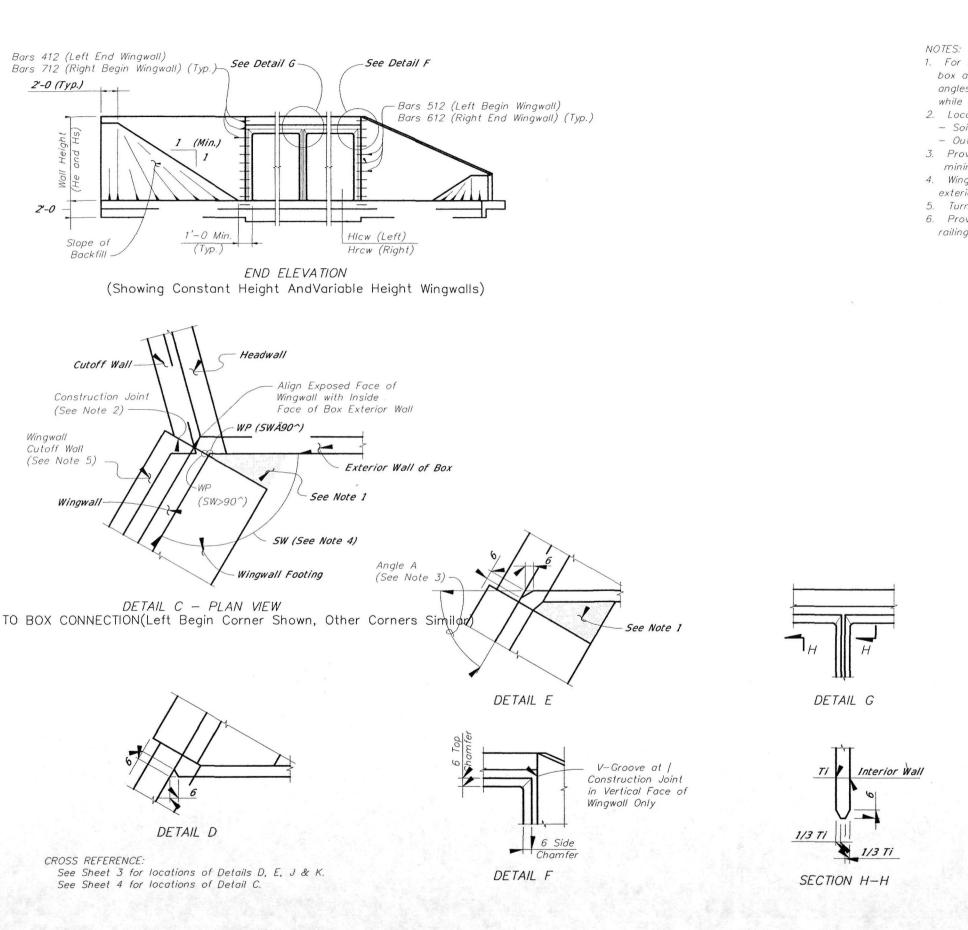
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THIS SHEET IS AN PROVIDED BY THE

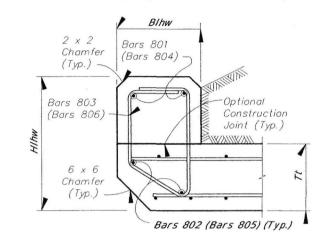
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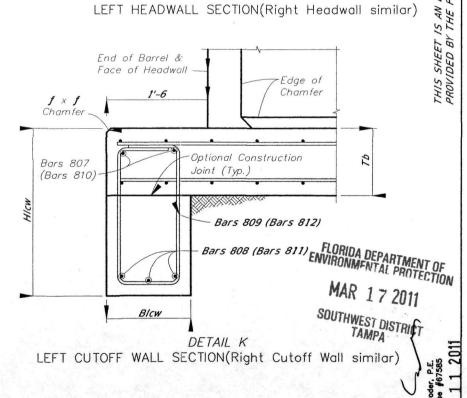




- 1. For small angles, the Contractor may elect to fill the area between the box and the wingwall footing with unreinforced concrete. For wingwall skew angles less then 90 degrees, field bend wingwall reinforcement as necessary while maintaining cover. No additional payment will be made for this work.
- 2. Location of Construction Joint determined by WP at theoretical intersection of: - Soil side face of Headwall and outside face of Box Exterior Wall, for SWA90^;
- Outside face of Wingwall and outside face of Box Exterior Wall, for\SW>90^.
- 3. Provide 6 chamfer when angle A is greater than 45^. Maintain minimum wall thickness. Field adjust reinforcing to maintain cover.
- 4. Wingwall Skew Angles (SW) are measured from the adjacent box exterior wall to the wingwall.
- 5. Turn or extend Wingwall Cutoff Wall as necessary to meet Box Cutoff Wall.
- 6. Provide additional reinforcement in the top of the top slab below traffic railings to ensure a minimum area of 0.80 sq. in./ft. transverse reinforcing.



DETAIL J LEFT HEADWALL SECTION(Right Headwall similar)



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ASE III EXPSNSION PROJECT

FOR CLARITY.

THE SHEET FROM THE FOOT 2010 DESIGN STANDARDS TO MULTIPLE BARREL CULVERTS HAVE BEEN REMOVED !

SOME REFERENCES

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