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March 25, 2013

Ms. Susan Pelz, P.E. Florida Department of Environmental Protection 13051 North Telecom Parkway Temple Terrace, Florida 33637-0926

Subject:

Assessment Report for the Pasco Resource Recovery Facility South Stormwater

Detention Pond Subsidence Features

Dear Ms. Pelz:

CDM Smith Inc., on behalf of Pasco County, is providing the enclosed report of investigation of the subsidence feature that occurred in the immediate vicinity of the south detention pond at the above-referenced facility. Two hard copies of the report and a compact disc with a PDF version of the report are transmitted with this letter.

The results of the investigation confirmed that the subsidence features formed during or immediately after Tropical Storm Debby as a result of the heavy rainfall and resulting hydraulic loading in the pond where limestone is present near the land surface. The results of the investigation also indicated that the subsidence is very localized, does not pose a threat to the adjacent landfill cells or to the north detention pond, and that there is no increased threat of additional subsidence or contamination of groundwater associated with the subsidence event. Therefore, no further investigation, remediation or other measures to mitigate or monitor further subsidence are recommended.

Please let me know if you have any questions or require additional information.

Sincerely.

William T. Beeson, P.G.

CDM Smith Inc.

Enclosure

cc: John Power (Pasco County)

Mr. Richard Tedder, P.E. (FDEP Tallahassee)

Dept. Of Environmental Protection



Pasco County, Florida

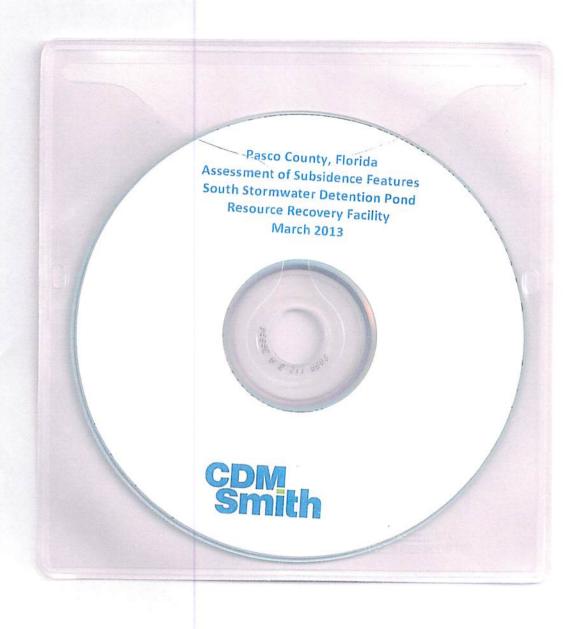
Assessment of Subsidence Features South Stormwater Detention Pond Resource Recovery Facility

March 2013

Prepared by:







In accordance with Section 471 and 492 of Florida Statutes, this Report of the Assessment of Subsidence Features in the South Detention Pond at the West Pasco Resource Recovery Facility in Pasco County, Florida has been prepared under the direct supervision of a licensed Professional Geologist and licensed Professional Engineer in the State of Florida.

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Executive Summary



CDM Smith Inc. (CDM Smith) was retained by Pasco County Utilities (the County or Pasco County) to perform assessment investigations and prepare remedial action plans for subsidence features that developed at the Pasco County Resource Recovery Facility (PCRRF). The facility is located at 14230 Hays Road in Spring Hill, Pasco County, Florida.

As a result of heavy rain associated with Tropical Storm Debby, subsidence features formed in the south stormwater detention pond east of Ash Cell A-3 at the West Pasco Class I Landfill on June 26, 2012. The Florida Department of Environmental Protection (FDEP) was notified of the occurrence of the depressions and inspected the facility on June 27, 2012. In response to the FDEP request, a temporary fence was installed around the subsidence features and a temporary berm was placed in the detention pond to isolate the subsidence area from the remainder of the detention pond.

The objectives of the assessment are to evaluate the potential for additional subsidence to occur in the south detention pond and to obtain sufficient information to develop plans to remediate the existing subsidence features in the south detention pond and to mitigate potential risks to the landfill. In order to safely perform the investigation, filling of the subsidence features was necessary. Initial filling included placement of Controlled Low Strength Material (CLSM) into eight of the subsidence features that were determined to possibly be open to subsurface conduits. After the CLSM cured, the remaining depression spaces in the subsidence features were filled using common fill and the area was returned to the approximate pre-event grade.

This assessment was performed in accordance with the approved work plan and in a manner consistent with the protocols established by the FDEP Landfill Sinkhole Technical Advisory Group. The assessment included the following five tasks:

- Task 1 Establishment of the site grid and coordinate system,
- Task 2 Electromagnetic (EM-31) survey (over 21,000 linear feet),
- Task 3 Geotechnical/geological control borings (45 borings averaging 45 feet deep),
- Task 4 Ground penetrating radar (GPR) survey (over 48,000 linear feet), and,
- Task 5 Geotechnical/geological investigations of existing subsidence features.

The objective of the geophysical (EM-31 and GPR) surveys was to identify anomalous subsurface conditions (anomalies) that appear consistent with potential subsidence features. The geophysical anomalies provided guidance in targeting locations for subsurface exploration and testing in the most probable areas for sinkhole development, as well as providing better insight into the geological conditions at the site. The EM-31 survey indicated that there was substantial variation in the electrical conductivity of the shallow sediments at the site. Both low and high conductivity areas were tested with SPT borings in order to evaluate and confirm the causes for the variations in terrain conductivity.



The anomalies identified in the GPR survey included areas where there were interrupted signal reflectors, areas of increased penetration of the radar waves, and areas where reflectors dipped toward a central location were observed. The anomalies most likely to reflect sinkhole development were tested by SPT borings.

Data from SPT borings were used to verify the geophysical data from the GPR and EM surveys and characterizing geological conditions. The data from SPT borings were used to characterize the karst and sinkhole conditions. The SPT data indicate that both sinkhole development and sinkhole activity (as defined in the Florida Statutes) or the potential for future subsidence exists within a focused area of the south detention pond area (The Subsidence Event Area). Sinkhole activity was identified through the presence of weakening of sediments with depth, which is indicative of raveling in the southern part of the south detention pond in the vicinity of the Subsidence Event Area.

The larger sinkholes that developed in the Subsidence Event Area are clearly cover-collapse sinkholes formed by hydraulic loading in the stormwater pond during Tropical Storm Debby. The origin of the larger sinkholes in the Subsidence Event Area occurred in a shallow rock layer with apparent limited areal extent. The smaller, satellite sinkholes peripheral to the cover-collapse sinkholes are small suffosion sinks where sand raveled into pipes that lead to the main conduit of the cover-collapse sinks. Such satellite sinkholes are common around larger, cover collapse features. These filled features are unlikely to re-form as the CLSM and fill material has replaced the failed cover and filled any remaining open void space.

Based on the data collected, it does not appear that the raveling conditions associated with the formation of the sinkhole features in the Subsidence Event Area extend to the west, toward the landfill. Although sinkhole activity, or the potential for subsidence feature development, was also identified in the northeastern corner of the southern detention pond, this area poses no risk to the landfill cells or to the northern detention pond.

Neither additional investigation of the area nor remedial or preventative measures are warranted to mitigate potential future subsidence and/or adverse impacts to the landfill cells. However, securing the area around the event area is recommended to mitigate the potential for injury in the unlikely event of additional subsidence. If the areas continue to subside, additional fill should be placed in the depressions to prevent ponding of rain water.



Section 1

Introduction

1.1 Authorization

CDM Smith Inc. (CDM Smith) was retained by Pasco County Utilities (the County or Pasco County) to perform assessment investigations and prepare remedial action plans for subsidence features that developed at the Pasco County Resource Recovery Facility (PCRRF). The facility is located at 14230 Hays Road in Spring Hill, Pasco County, Florida.

As a result of heavy rain associated with Tropical Storm Debby, subsidence features formed in the south stormwater detention pond east of Ash Cell A-3 at the West Pasco Class I Landfill on June 26, 2012. The Florida Department of Environmental Protection (FDEP) was notified of the occurrence of the depressions and inspected the facility on June 27, 2012. Representatives of Pasco County and CDM Smith met with FDEP on June 27, 2012. In addition to requiring Pasco County to take immediate actions to mitigate the potential for future subsidence, FDEP requested that Pasco County develop a work plan for geological, geotechnical and geophysical investigations of the stormwater detention ponds. This assessment was performed in accordance with the approved work plan and this report is being submitted in accordance with the FDEP request. In addition, the investigation was performed in a manner consistent with the protocols established by the FDEP Landfill Sinkhole Technical Advisory Group.

1.2 Objectives of Assessment

The objectives of the assessment are to evaluate the potential for additional subsidence to occur in the south detention pond and to obtain sufficient information to develop plans to remediate the existing subsidence features in the south detention pond and to mitigate potential risks to the landfill. The assessment was performed in general accordance with a work plan dated October 19, 2012 and approved by FDEP on October 31, 2012, after clarifications regarding the plan were submitted to FDEP (Appendix A).

1.3 Organization of Assessment Report

This assessment report presents and describes the results and methods for the investigation, including interpretations of data. Included are discussions regarding the regional and site-specific geology and hydrogeology, geophysical surveys, and geotechnical investigations. This report is organized as follows:

- Section 1 Introduction
- Section 2 Results of Investigation
- Section 3 Methods of Investigation
- Section 4 Conclusions and Recommendations
- Section 5 References





1.4 Background Information

1.4.1 Site Location and Description

The PCRRF is located at 14230 Hays Road. Approximately four miles southeast of Spring Hill, Pasco County, Florida within Sections 25 and 26, Township 24 South, and Range 17 East as shown on the United States Geological Survey (USGS) Fivay Junction Quadrangle 7.5-Minute Series Topographic map (Figure 1-1). The south detention pond is located in the eastern part of the facility in Section 25 immediately east of the Ash Cell A-1, A-2 and A-3. The location of the South Detention Pond is shown on Figure 1-2.

1.4.2 Physical Setting

There are four physiographic provinces in Pasco County (White, 1970). These provinces are the Coastal Swamps, the Gulf Coastal Lowlands, the Brooksville Ridge, and the Tsala Apopka Plain. The site is located in the northwestern part of the county in the Gulf Coastal Lowlands physiographic province.

The Spring Hill area is located in an area with a generally flat landscape typical of the Gulf Coastal Lowlands. The Gulf Coastal Lowlands lie adjacent to the Gulf of Mexico coastline. Fluctuating sea levels sculpted the lowlands through alternating depositional and erosional cycles. Varying thicknesses of unconsolidated to indurated sand, silt and clay overlie limestone throughout this physiographic region. The terrain is flat to gently undulating. Karst features are common throughout the area. The Gulf Coastal Lowlands range in elevation from sea level to approximately 70 feet above sea level although higher elevations occur in limited areas. Drainage varies from poorly drained in the low-lying areas to well drained in the higher elevations.

1.4.3 Regional Geology and Hydrogeology

1.4.3.1 Introduction

The regional geology and hydrogeology were evaluated using information contained in the published reports prepared by the Florida Geological Survey (FGS) and the U.S. Geological Survey (USGS). The terminology applied to the regional and local geology and hydrogeology conforms to that given in FGS Special Publication 28 entitled Hydrogeological Units of Florida (Vecchioli et al., 1986) and the Geologic Map of the State of Florida (Scott et al., 2001).

Western Pasco County is an area influenced by geomorphic features, structure, and karst. In general, the area has a veneer of predominantly siliciclastic sediments near surface overlying a section of Cenozoic carbonate sediments. According to Fretwell (1988), there are three primary hydrogeologic units present beneath Pasco County and the Spring Hill area. These units are: 1) the Surficial Aquifer System (SAS), 2) the Intermediate Confining Unit /Intermediate Aquifer System (ICU/IAS), and 3) the Floridan Aquifer System (FAS).

1.4.3.2 Undifferentiated Sediments, the Surficial Aquifer System, and the Intermediate Aquifer System or Intermediate Confining Unit

A series of undifferentiated, predominantly siliciclastic sediments are present from land surface to the top of the underlying Hawthorn Group, if present, or the uppermost carbonate unit of the FAS. These sediments were deposited during the high sea stands and repeatedly eroded and altered by exposure during low sea stands of the Pliocene and/or Pleistocene epochs. Accumulation of wind-blown sand dune deposits and soil formation processes further altered the landscape during the Holocene. This sand mantle ranges in thickness from zero to approximately 100 feet in Pasco County (Fretwell,





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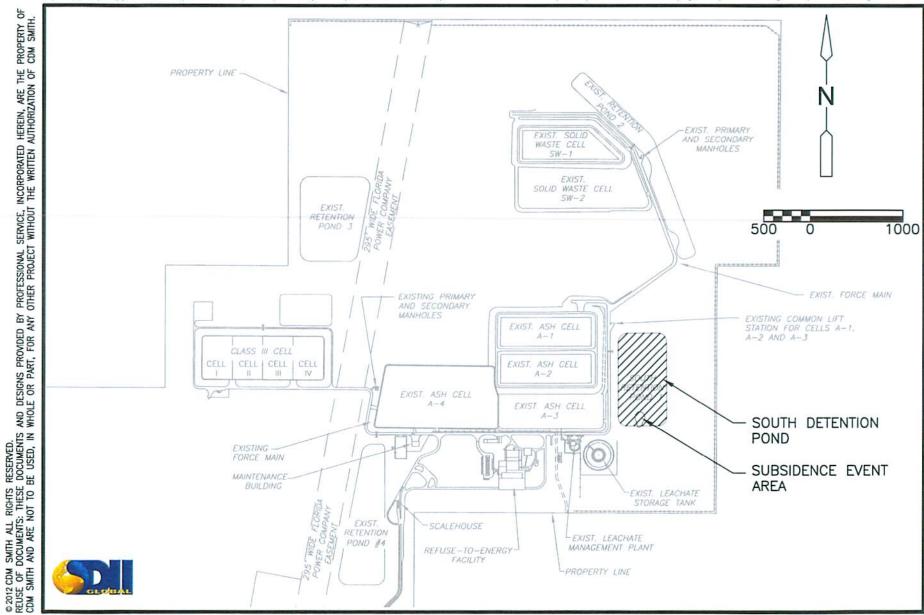




Figure No. 1-2
Pasco County Resource Recovery Facility
Site Plan Showing Location of South Detention Pond

1988). The unit typically is very heterogeneous with extreme variations in lithology from nearly pure quartz sand to pure clay and admixtures of these and other components, such as organics and phosphate. These sediments comprise the SAS, also termed the surficial aquifer, where there is a sufficient thickness of the deposits and sufficient hydraulic conductivity to define it as an aquifer. In the Spring Hill area, these deposits vary in thickness from being absent to more than 25 feet. In many areas of the region, the sand deposits directly overlie the uppermost carbonate unit of the FAS.

Recharge to the SAS occurs primarily from direct rainfall where the vertical hydraulic conductivity is sufficient to allow infiltration of rainfall. Where clayey soils occur at land surface or where man-made impervious surfaces occur, direct recharge is minimal or non-existent.

The IAS/ICU is defined as all rocks that lie between and collectively retard the exchange of water between the FAS and SAS (Vecchioli, et. al., 1986). In the Spring Hill area, where the Hawthorn Group is only present in remnants or is reworked, the clay layers within the remnants of the Hawthorn Group and in undifferentiated sediments locally function as the ICU and retard exchange of groundwater between the SAS and FAS.

1.4.3.3 Carbonate Units and the Floridan Aquifer System

Carbonate strata of Miocene age and older uncomfortably underlie the ICU where the ICU is present. In ascending order, the freshwater-bearing part of the FAS is the Upper Floridan aquifer, which consists of the Avon Park Formation, Ocala Limestone, Suwannee Limestone, and the Tampa Member of the Hawthorn Group (Arthur et.al. 2008). The uppermost carbonate unit varies locally and may be the Tampa Member of the Hawthorn Group, the Suwannee Limestone, or the Ocala Limestone (Arthur et.al. 2008). In the Spring Hill area, the uppermost carbonate unit of the FAS is the Suwannee Limestone. Also, in the Spring Hill area, the top of the FAS is often at or near land surface. The thickness of the Upper Floridan aquifer in Pasco County ranges from slightly less than 700 feet in the north-central part of the county to about 1,050 feet in the southwestern part of the county (Fretwell, 1988 and Miller, 1986).

Recharge to the FAS occurs by direct entry through sinkholes, by vertical leakage through the confining beds, and by drainage of hydraulically connected beds within the SAS and ICU (Sinclair et al., 1985; Trommer, 1987). The regional direction of groundwater movement in the Upper Floridan aquifer in the Spring Hill area is generally from east to west-northwest (Ortiz, 2007, 2008, 2009 and 2011).

1.4.4 Initial Response Actions

Immediately following the discovery of the subsidence features in the south stormwater detention pond on June 26, 2012, FDEP was notified. FDEP visited the site on June 27, 2012. The site visit included an inspection of the area of the subsidence features, inspection of erosion and temporary repairs to the side slopes of the landfill, and inspection of other surface water detention areas. Eighteen subsidence features were identified by FDEP in the southwestern part of the south detention pond. Global Positioning System (GPS) coordinates were obtained by FDEP using a hand-held GPS instrument. No other subsidence features or evidence of possible additional subsidence was found. Following the site visit, FDEP requested that Pasco County monitor groundwater levels and implement measures to isolate the subsidence features from the detention pond.

In response to the FDEP request, a temporary fence was installed around the subsidence features and a temporary berm was placed in the detention pond to isolate the subsidence area from the remainder



of the detention pond. Prior to placement of the temporary berm, a soil boring program was performed along the proposed location of the temporary berm to identify potential locations where additional subsidence might occur. The locations of the subsidence features and the temporary berm are shown on **Figure 1-3**.

The area was inspected by engineers and geologists of the CDM Smith Team and representatives of Pasco County on September 14, 2012. Observations made at the time indicated that the subsidence features occurred in an area where limestone was present near the surface.

In order to safely perform the investigation, filling of the subsidence features was necessary. Prior to filling the subsidence features, the locations were surveyed and the sizes and approximate volumes were measured. Twenty-three subsidence features were mapped and are designated A through W (Figure 1-3). The volumes of the subsidence features varied from 442 cubic yards (A) to less than 0.1 cubic yard (T, U, V, and W).

Brief plans to fill the subsidence features were submitted to the FDEP on September 21, 2012. The initial filling plans included initial placement of Controlled Low Strength Material (CLSM) into eight of the subsidence features that were determined to possibly be open to subsurface conduits. The CLSM consisted of a cement mixture with a compressive strength of approximately 300 pounds per square inch. Subsidence features that received CLSM and the approximate amounts of CSLM installed are identified in **Table 1-1**. After the CLSM was given approximately two days to cure, the remaining depression spaces in the subsidence features were filled using common fill and the area was returned to the approximate pre-event grade.





Table 1-1 Summary of Subsidence Feature Information Pasco County Resource Recovery Facility

Subsidence Feature Designation on Figure 1-3	Amount of CLSM Installed (C.Y.)	Approximate Area of Subsidence Feature (FT ²)	Approximate Maximum Depth of Subsidence Feature (FT)	Approximate Volume of Subsidence Feature (C.Y.)
Α	-	1688.0	7.0	257.0
В	-	294.0	2.0	13.6
С	-	36.0	1.0	1.3
D	-	575.0	4.0	52.4
Е	-	328.0	2.0	16.1
F	13.4	557.0	4.0	49.0
G	1.9	28.0	1.0	1.0
Н	13.6	136.0	1.0	5.0
I	-	238.0	1.0	9.0
J	11.6	180.0	1.0	4.4
К	0.1	480.0	1.0	17.8
L	0.6	53.0	1.0	1.0
М	0.3	1480.0	1.0	31.0
N		10.0	0.5	0.1
0	6.0	10.0	0.5	0.1
Р	0.3	15.0	0.5	0.2
Q	1.7	15.0	0.5	0.2
R	0.4	23.0	0.5	0.3
S	0.1	6.0	0.5	0.1
Т	-	12.0	0.5	0.2
U	-	2.0	0.5	0.0
V	×=	8.0	0.5	0.2
W .	-	4.5	0.5	0.1

Locations of Subsidence Features Shown on Figure 1-3

CLSM = Controlled Low Strength Material

C.Y. = Cubic Yard or

Yards

FT = Foot or Feet



Section 2

Results of Investigations

2.1 Introduction

Assessment investigations were initiated by the CDM Smith team in November 2012. The specific objectives of the assessment were (1) to evaluate the potential for additional subsidence activity to occur in the south detention pond, (2) to obtain sufficient information to develop plans to remediate the existing subsidence features in the south detention pond, if necessary, and (3) to evaluate and mitigate, if necessary, potential risks to the landfill. The assessment included the following five tasks:

- Task 1 Establishment of the site grid and coordinate system,
- Task 2 Electromagnetic (EM-31) survey (over 21,000 linear feet),
- Task 3 Geotechnical/geological control borings (45 borings averaging 45 feet deep),
- Task 4 Ground penetrating radar (GPR) survey (over 48,000 linear feet), and,
- Task 5 Geotechnical/geological investigations of existing subsidence features.

Locations of the EM survey transects, GPR transects are shown on **Figure 2-1** and **Figure 2-2**, respectively. Standard Penetration Test (SPT) boring locations are also shown on Figures 2-1 and 2-2. Results of the investigations including interpretations of the data are discussed in the following sections.

2.2 Geophysical and Geological Investigations

2.2.1 Introduction

The objective of the geophysical (EM-31 and GPR) surveys was to identify anomalous subsurface conditions (anomalies) that appear consistent with potential subsidence features. In doing this, the geophysical anomalies provide guidance for optimizing the geotechnical testing program by providing targeted locations for subsurface testing in the most probable areas for sinkhole development, as well as providing better insight into the geological conditions at the site.

Prior to performing the geophysical surveys, a grid with 50-foot centers was established in and adjacent to the south stormwater detention pond. The grid lines have a true (State Plane) orientation of due North-South and due East-West. All data generated during the geophysical and geotechnical testing was geo-referenced to the site geographic coordinate system.

In accordance with the approved plan, grid nodes were staked every 200 feet around the perimeter of the grid system and at 100-foot intervals along the northern and southern boundaries of the pond at the base of the berm. In the northern part of the pond, nodes were staked at 200-foot intervals from north to south and at 100-foot intervals from east to west. Grid nodes were staked at 100-foot intervals in the southern part of the pond. Each stake was marked with the appropriate site coordinates and elevation. The grid is shown on Figures 2-1 and 2-2.



2.2.2 Results of Geophysical Surveys

Two different geophysical survey technologies were used to identify areas most likely to show signs of sinkhole development and where significant variations in subsurface conditions exist. Anomalous areas were evaluated and utilized to guide locations for SPT borings. The results were also utilized to interpolate geological conditions between SPT borings for construction of the geologic cross-sections and maps depicting thicknesses of different geologic materials presented below. Data from the geophysical surveys are in **Appendix B**.

The EM-31 instrument is used to map average variations of electrical conductivity at depths between zero and 10 to 20 ft. below land surface (bls). As indicated on **Figure 2-3**, the EM-31 survey indicated that there was substantial variation in the electrical conductivity of the shallow sediments at the site. Low conductivity areas, identified by the orange areas on Figure 2-3, reflect lower terrain conductivity attributed to sandy sediments while the blue areas reflect higher conductivity attributed to more clayrich sediments. As indicated in Figure 2-3, both low and high conductivity areas were tested with SPT borings in order to evaluate and confirm the causes for the variations in terrain conductivity.

GPR uses pulses of electromagnetic energy in the radar range to image changes in the physical properties of shallow sediments. Unlike the ERM-31, GPR data present a vertical section of the distribution of subsurface materials with differing dielectric constants. The anomalies identified in the GPR survey are indicated in **Figure 2-4**. The anomalies included areas where there were interrupted signal reflectors, areas of increased penetration of the radar waves, and areas where reflectors dipped toward a central location were observed. The anomalies most likely to reflect sinkhole development were tested by SPT borings (Figure 2-4).

As demonstrated by the GPR, and by the control and anomaly SPT borings, the site is located within an area of karst development. The general sediment profile at the site consists of sand overlying clay-rich sediments with varying amounts of sand. These sandy and clayey sediments constitute the sediment "cover" package that overlies karstic limestone. Limestone underlies the clayey sediment. Lateral variations in the nature and thicknesses of the geologic strata that comprise the cover are common. For example, depth to limestone (see geologic cross sections **Figures 2-5** through **2-14**) varies from essentially cropping out at the land surface to greater than 80 feet bls. The depth to more clay-rich sediments range from exposure at the land surface to approximately 45 feet bls. Much of this variation is a result of development of the karst at the top and within the limestone.

The uppermost part of the limestone has been subjected to weathering, which has resulted in small pinnacles and depressions in the limestone surface. This weathering has resulted in the mixing of clayrich sediments with fragments of limestone and localized subsidence feature development. Many of these variations in depth, thickness, and weathering of the strata associated with karst terrain can be discerned using the geophysical data.

The local presence of shallow clayey limestone or clay is reflected by a corresponding local increase in ground electrical conductivity in the EM-31 data set coupled with minimal penetration of radar waves in the GPR profiles. Thick and shallow sands, in comparison, correlate with decreased electrical conductivity and significant increases in the associated depth of GPR penetration. Therefore, the variations in terrain conductivity shown in Figure 2-3 reflect the variations in depth to clay and/or limestone. Conductivity varies over short distances at the site. The GPR data cannot be used to identify many of these small-scale, horizontal spatial variations, but it is excellent for identification the spatial distribution of vertical structures. GPR anomalies identified on Figure 2-4 are dominated by vertical,



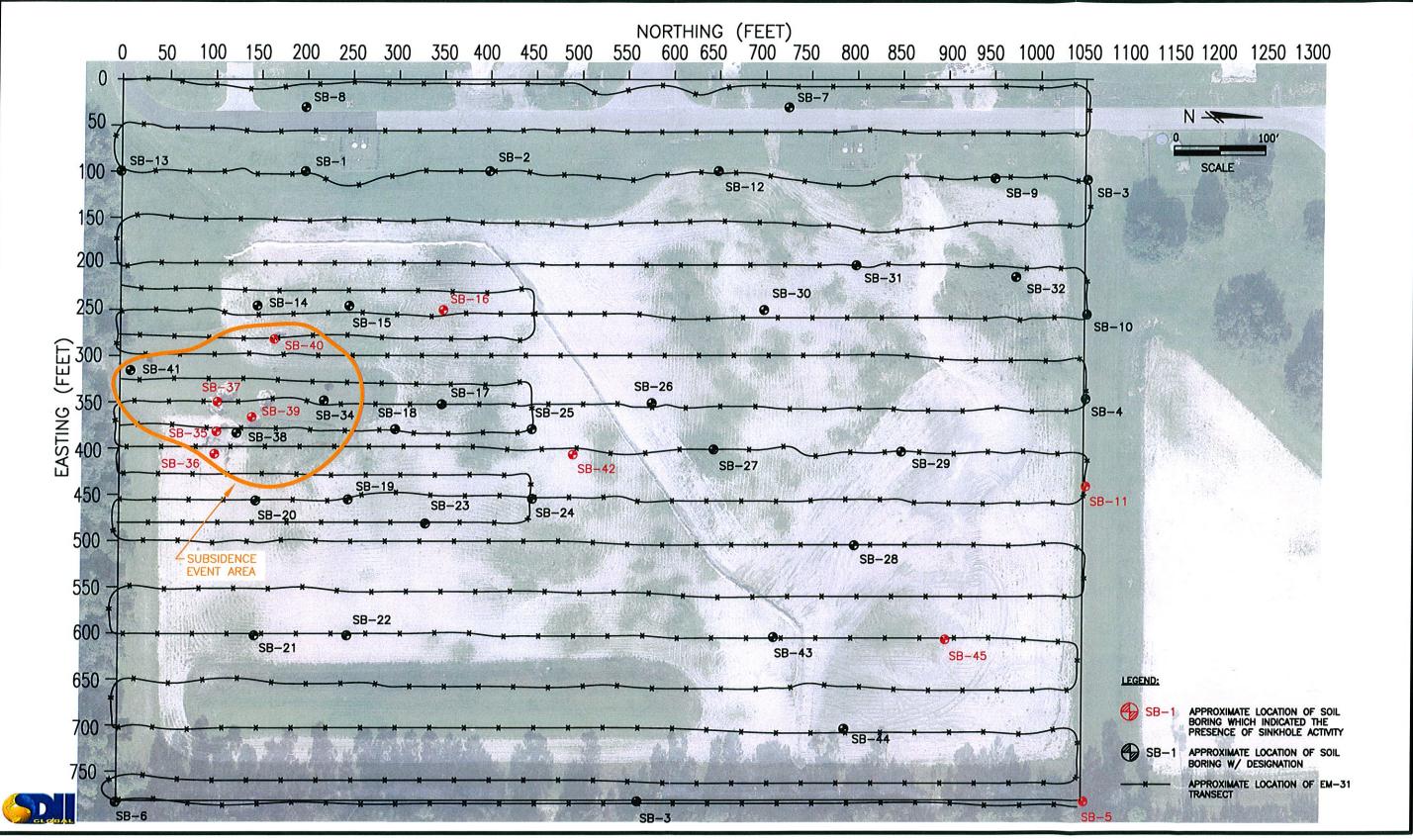




Figure No. 2-1
Pasco County Resource Recovery Facility
Electromagnetic Survey Transect Map and
Standard Penetration Test Boring Locations

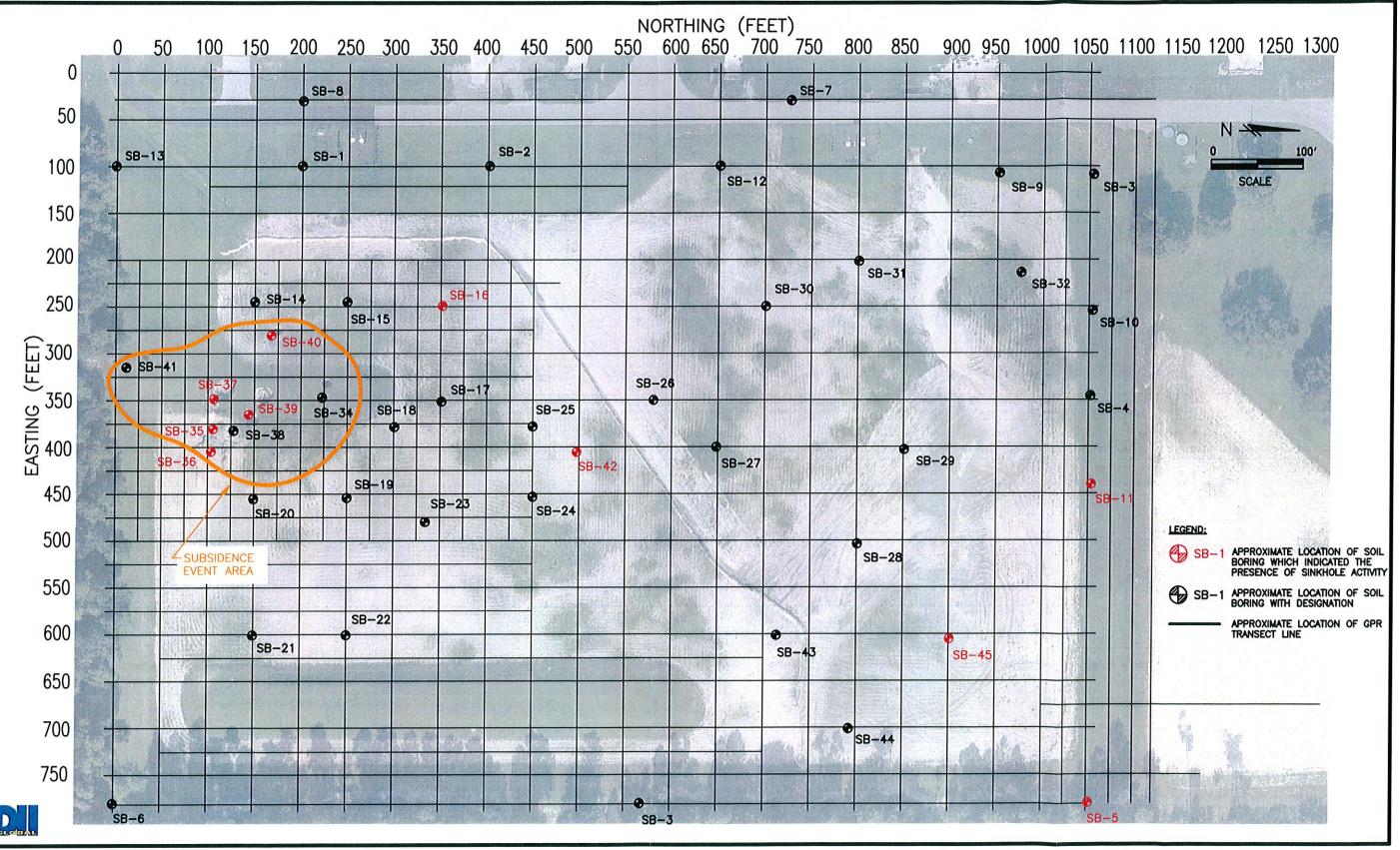




Figure No. 2-2
Pasco County Resource Recovery Facility
Ground Penetrating Radar Survey Transect Map and
Standard Penetration Test Boring Locations



Figure No. 2-3
Pasco County Resource Recovery Facility
Electromagnetic Survey Results Map

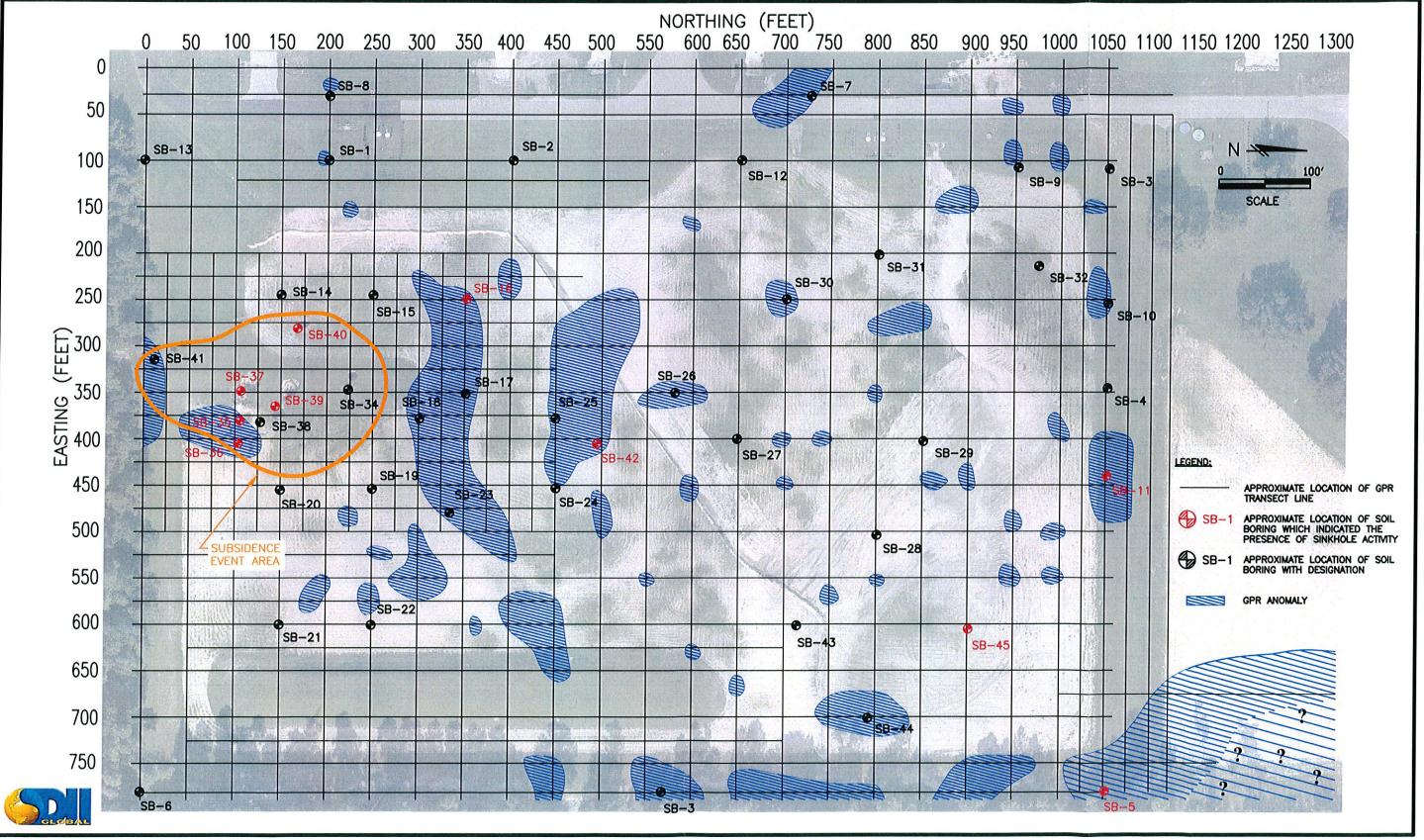






Figure No. 2-5
Pasco County Resource Recovery Facility
Geologic Cross Section Location Map

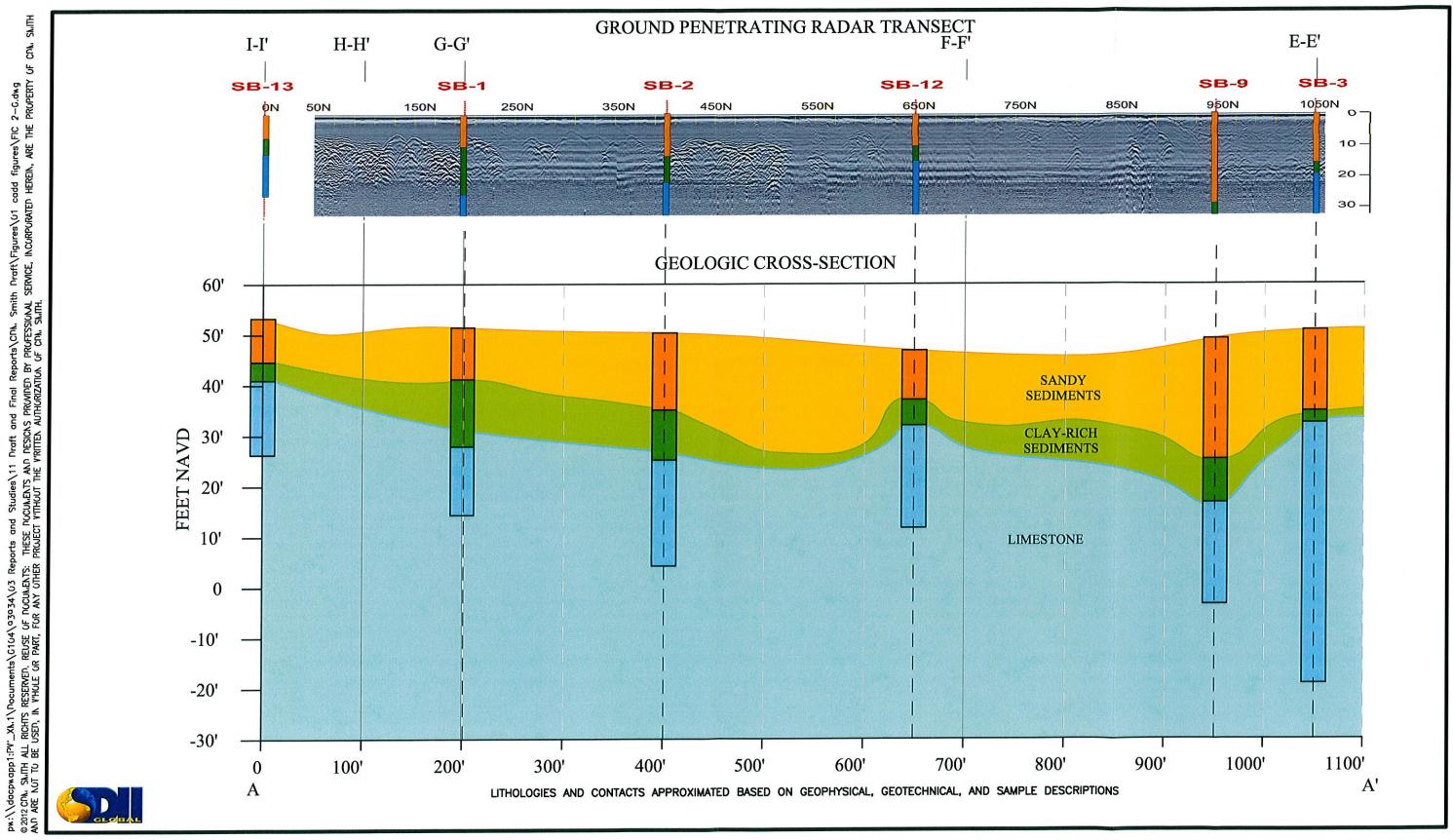




Figure No. 2-6
Pasco County Resource Recovery Facility
Ground Penetrating Radar and Geologic Cross Section A-A'

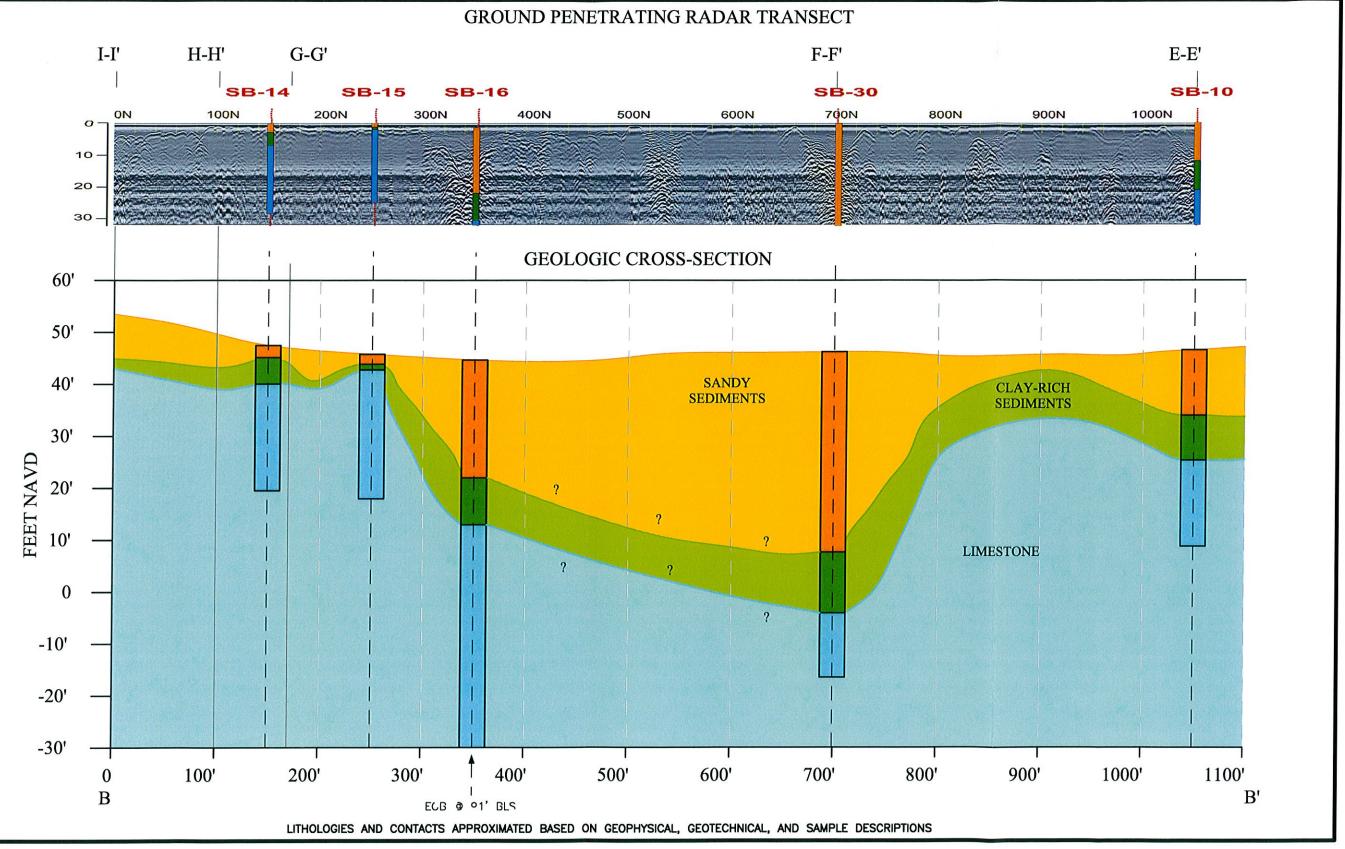
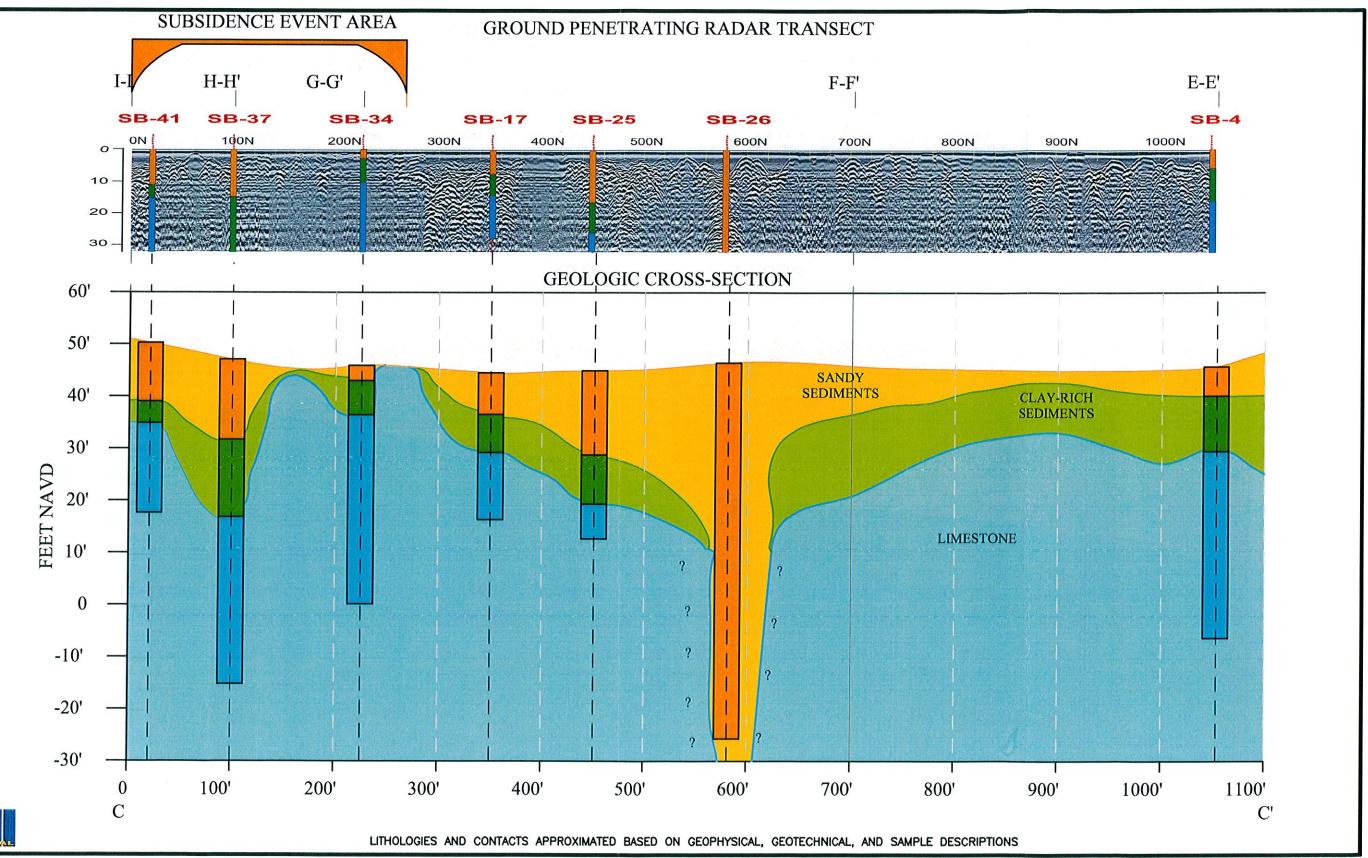




Figure No. 2-7
Pasco County Resource Recovery Facility
Ground Penetrating Radar and Geologic Cross Section B-B'





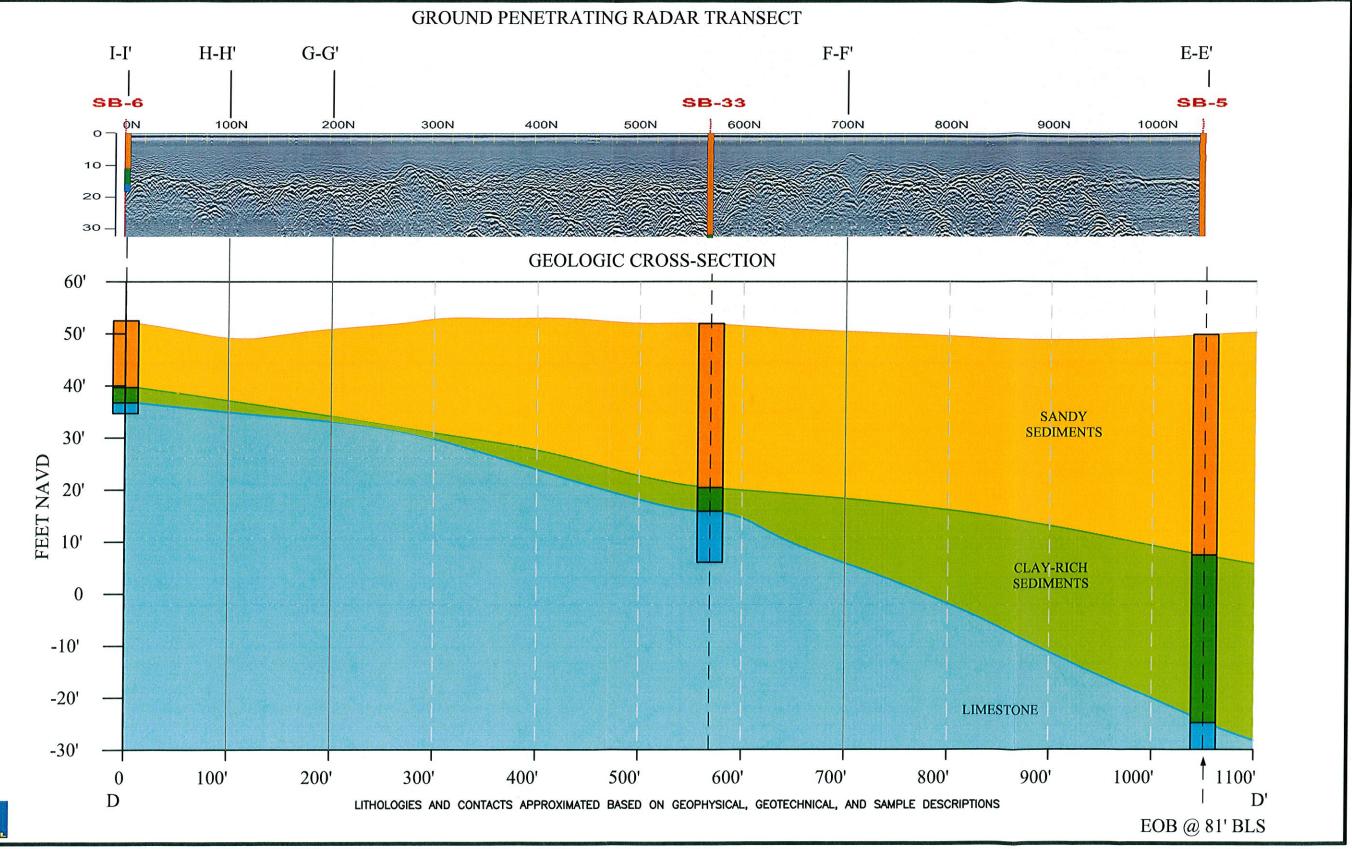
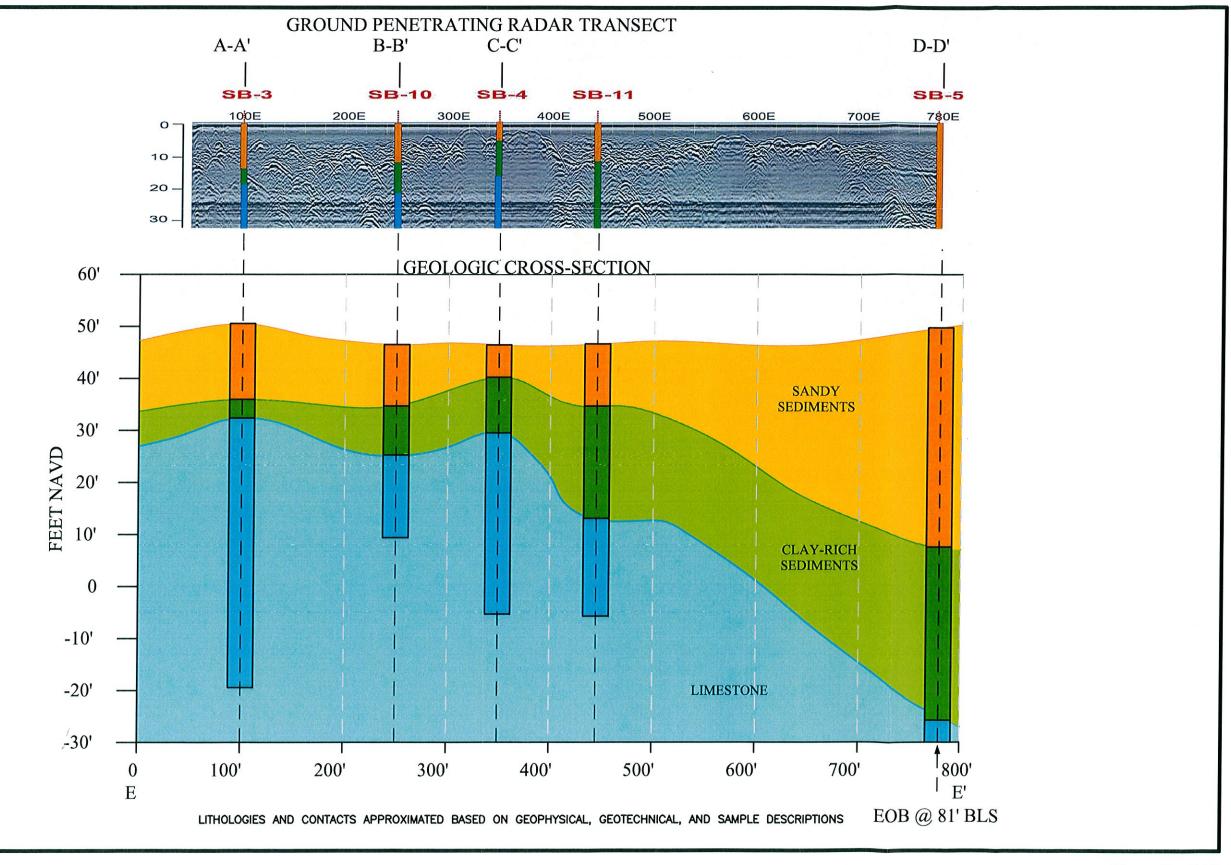
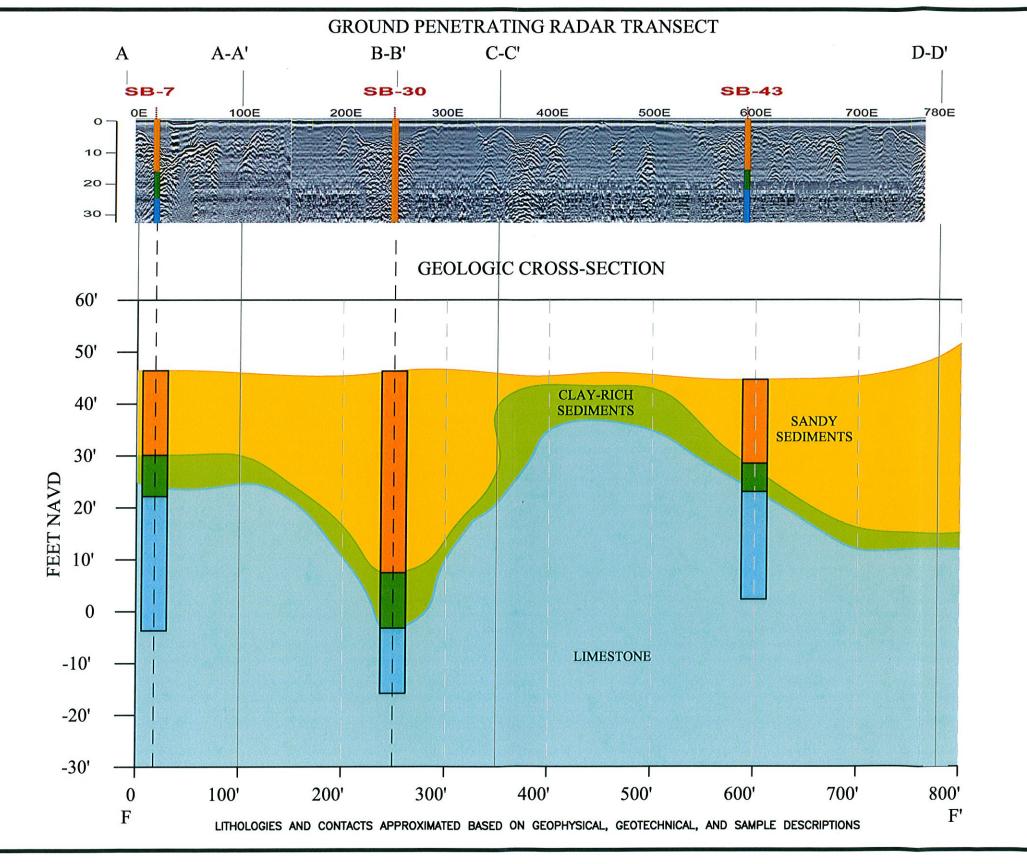




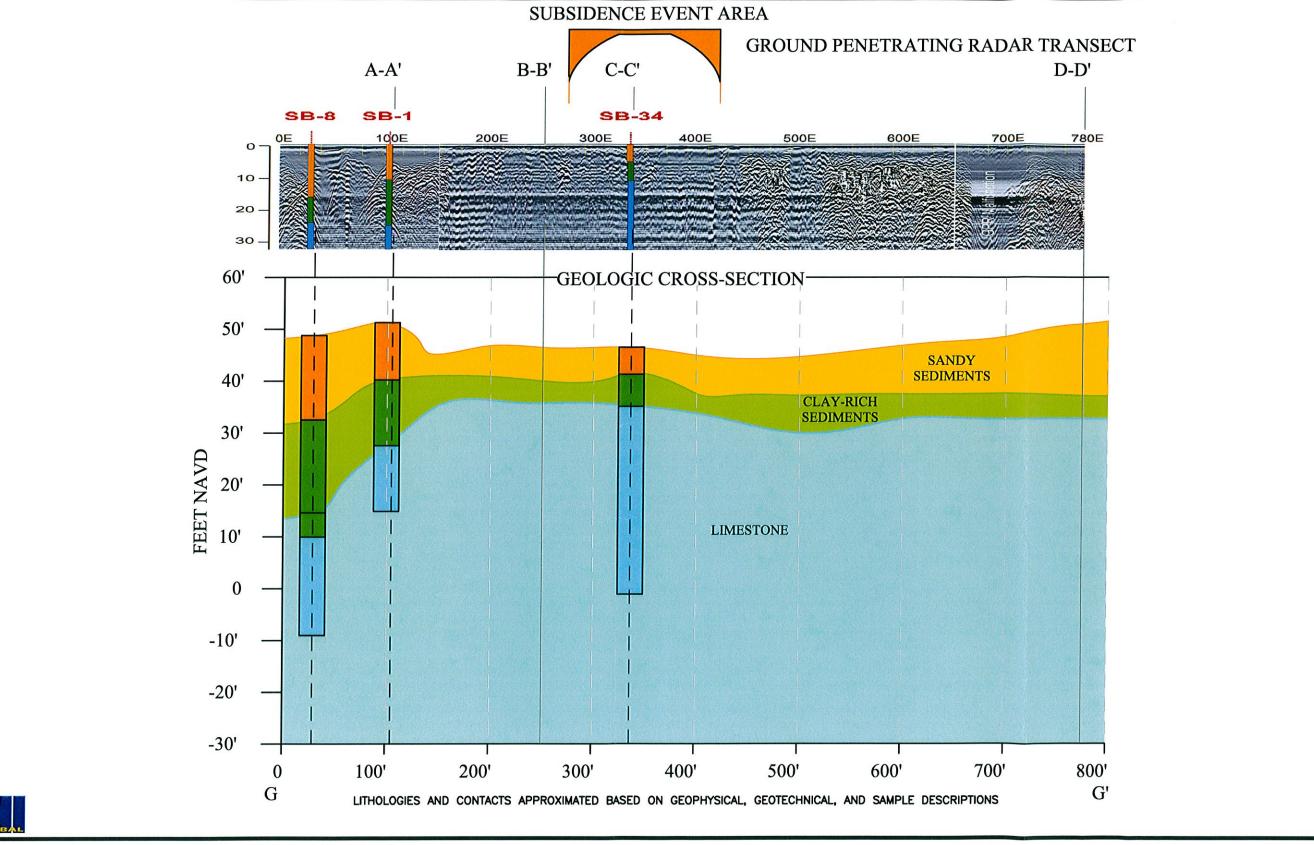
Figure No. 2-9
Pasco County Resource Recovery Facility
Ground Penetrating Radar and Geologic Cross Section D-D'













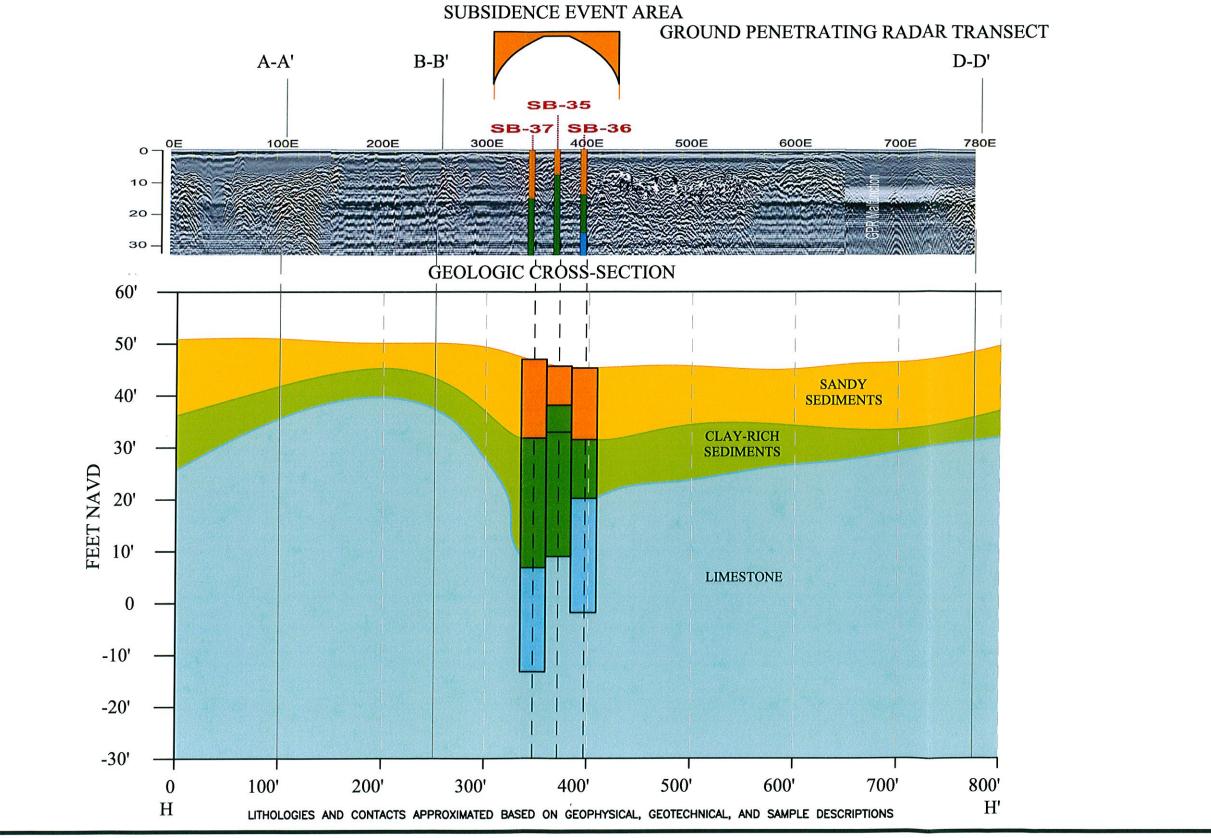
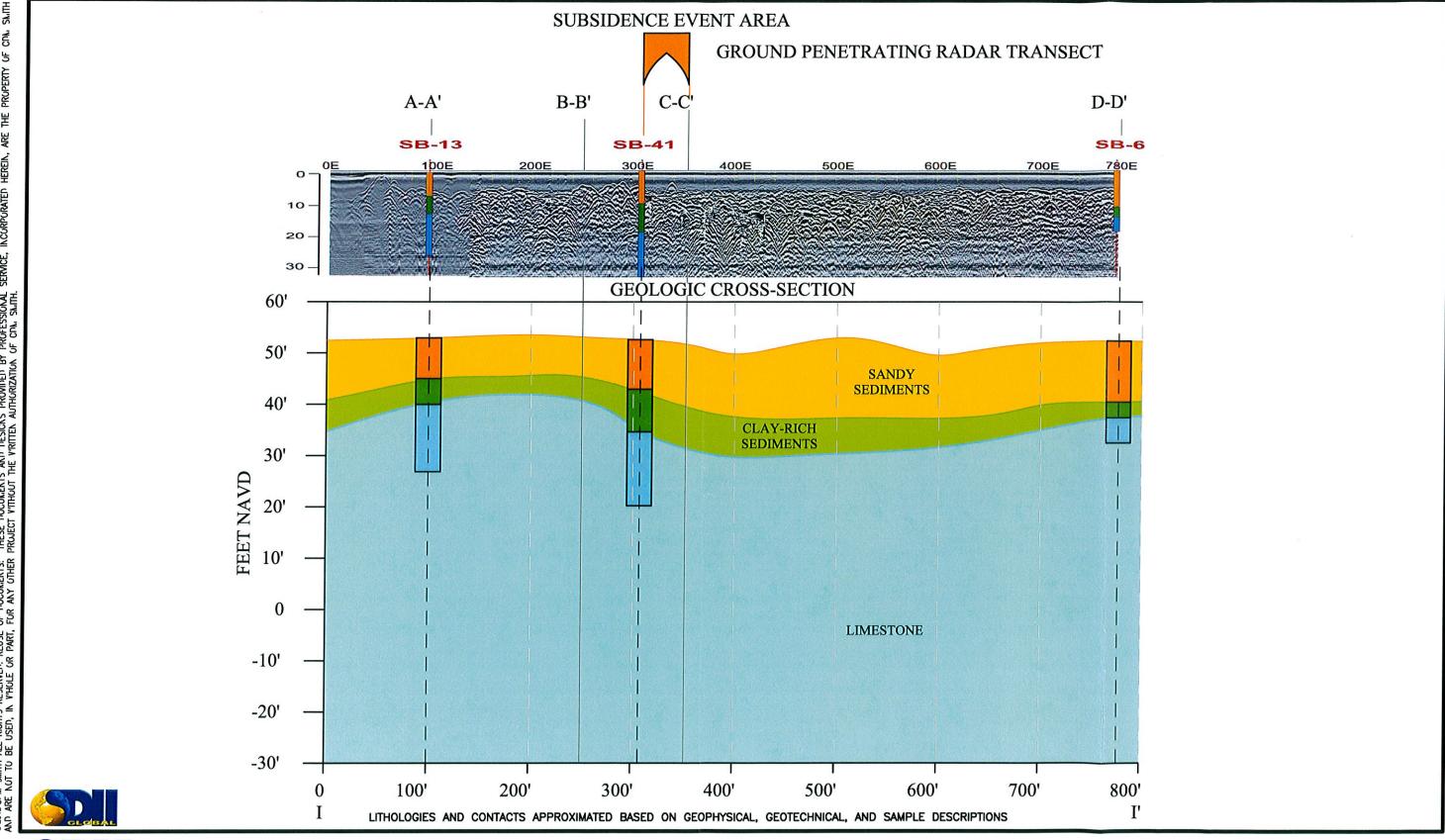




Figure No. 2-13
Pasco County Resource Recovery Facility
Ground Penetrating Radar and Geologic Cross Section H-H'





chimney-like subsurface structures that are predominantly sand-filled as seen in the anomaly borings advanced during the geotechnical phase of this investigation.

In the northeast corner of the south stormwater detention pond, the GPR identified an anomaly that is a large basin-shaped subsurface feature (Figure 2-4). As indicated by SPT boring SB-5, this depression is characterized by deep limestone and a thick volume of sand and clay. Examination of the predevelopment survey of the site indicates that this anomaly had a surface expression prior to site development.

Excluding the northeast corner of the investigation area, the northern half of the detention pond exhibits numerous, smaller diameter vertical sand-filled structures indicative of "chimney sinks" (narrow, near circular, open or in-filled shafts in the underlying limestone and cover materials) that have been in-filled by overlying sand- and clay-rich cover sediments. In the southern half of the detention pond, the GPR anomalies have a larger footprint and, on the GPR cross sections, have an appearance characterized by overlapping or coalesced smaller structures.

In the vicinity of the recent surface depressions (features within the "Subsidence Event Area"), there is a small area of decreased electrical conductivity. The decreased electrical conductivity is consistent with the presence of shallow sand fill material and CLSM fill used to backfill the depressions. The GPR data in the Subsidence Event Area showed mixed results.

The subsidence features in the Subsidence Event Area are not all isolated, individual features. They reflect a network of drainage features associated with a smaller number of chimneys in the underlying limestone. Some GPR anomalies in the Subsidence Event Area clearly indicate vertical, sand-filled chimney structures. These tended to be observed in the larger depressions. The other, smaller depressions in the Subsidence Event Area, however, showed only shallow indications ("diffractions" or highly curved reflections) of voids in the cover and very shallow limestone. In these areas, short lateral conduits in the cover at the top of the limestone combined some associated clay content or interbedding have attenuated or interrupted the GPR signal and reduced GPR penetration. With the reduced penetration, the GPR is only imaging discontinuities in the cover and very shallow limestone surface.

The GPR data were combined with the SPT boring data to construct geologic cross sections in both east-west and north-south directions at the site (Figure 2-5). The cross sections (Figures 2-6 through 2-14) include a GPR profile at the top of the figures. Superimposed on the GPR profiles are the results of the SPT borings intersected by the profiles. The SPT data depict the distribution and thickness of the sand, clay, and limestone along each profile for the purpose of ground truthing the GPR data. Below the GPR profiles are geologic cross sections showing the SPT data with land surface profiles and interpolated contacts between the lithologic strata. The interpolations were constructed by first connecting the lithologic contacts from borehole to borehole. The GPR profiles and kriged contour maps show the elevations of the tops of the clay and limestone strata with respect to the North American Vertical Datum of 1988 (NAVD 88). These data were used with geologic descriptions from SPT borings to prepare geologic cross-sections.

2.2.3 Site Geology

The geology of the site and immediate vicinity were evaluated using data from the SPT borings and, to a lesser extent, the geophysical data. Boring logs are included in **Appendix C**. The geologic cross-sections and SPT data were used to develop a conceptual model of the stratigraphy and karst at the site.



The uppermost stratum is a mantle of Pleistocene marine terrace sediment; primarily sand. The thickness of the near-surface sandy sediments, which were predominantly sand, silty sand, and/or clayey sand, ranged from near zero at SPT boring SB-27 to greater than 70 feet at SPT boring SB-26. Based on the standard penetration tests completed at each boring location, "N values" of this stratigraphic unit indicated the sediment exhibited very loose to medium dense relative strengths. Sandy sediments are not cohesive and are easily transported by wind, water, and gravity. As a result, it is this material that ravels or migrates into cavities in the underlying limestone when sinkholes develop.

Below the sandy sediment is a horizon of clay-rich sediment. This material is residuum from the Miocene Hawthorn Group. The clays have been reworked and weathered. As a result, the clay is often mixed with sand or limestone fragments from the underlying limestone. Reworking and development of sinkholes in the geologic past have caused many of the karst features observed at the site to be filled with clay, which renders the chimney sinks to be inoperative.

The top of the sandy clay and clay stratum was present generally between elevations +45 feet and more than -35 feet NAVD88 (**Figure 2-15**). Thickness varied from less than 5 feet to approximately 20 feet (**Figure 2-16**). The clay and sandy clay was relatively soft to very stiff. The clays of the clay-rich stratum are cohesive. As a result, they resist transport including raveling into karst void features. At the site, the clay-rich stratum locally serves to isolate the stormwater pond from subsidence feature development during ponding events.

The clay-rich stratum is thickest and has fewer penetrations in portions of the northern half of the site (Figure 2-16). This is significant because the clay-rich stratum reduces the risk of sinkhole activity in areas where it has fewer penetrations and is thicker.

The uppermost limestone unit consists of the Oligocene Suwannee Limestone. The Suwannee is a poorly to well lithified, relatively pure limestone with both primary (depositional) and secondary (fracture- and karst-related) porosity. The uppermost portion of the limestone has been subjected to weathering during periods of subaerial and subaqueous exposure and groundwater flow. As a result, the limestone surface is highly irregular and has more than 75 feet of relief at the site as shown on **Figure 2-17**.

In addition to the large-scale weathering and erosional features, small pinnacles and depressions with relief of less than one foot to over three feet are locally present on the weathered surface as observed in several of the depressions in the Subsidence Event Area. In addition, weathering has resulted in the karst conduits known as chimney sinks. This upper, highly weathered zone of the limestone, which is called the epikarst, has relatively high permeability unless sand and clay from the overlying sediments fill the pore space. Where voids exist in the epikarst, water can move laterally and vertically into karst conduits, causing sinkhole development. The subsidence features in the Subsidence Event Area reflect this movement, with subsidence features developed over the chimneys and as peripheral features where water moved from the stormwater impoundment laterally into the main subsidence feature.

The top of the epikarst was present between elevation +40 feet and more than -35 feet NAVD88 and ranged in thickness from less than one foot to greater than 50 feet. The sediment of the epikarst was predominantly clay-rich with limestone fragments and was relatively soft to hard. While subsidence feature development was observed in the Subsidence Event Area around SB-35 through SB-39, the epikarst-related features observed within the remainder of the southern detention pond area showed no signs of reactivation.





Figure No. 2-15
Pasco County Resource Recovery Facility
Estimated Top of Clay-Rich Sediment
(10-Foot Contour Interval)



Figure No. 2-16
Pasco County Resource Recovery Facility
Estimated Clay-Rich Sediment Thickness
(5-Foot Contour Interval)



Figure No. 2-17
Pasco County Resource Recovery Facility
Estimated Top of Limestone Including Epikarst
(10-Foot Contour Interval)

The main body of the karstic Suwannee Limestone was encountered between elevations +30 feet and greater than -35 feet NAVD88. This more-massive limestone ranged in relative strength from weak to refusal strength (SPT N values greater than 50).

Sandy horizons within the limestone reflect infilling of paleo-karst features such as karst conduits, relict subsidence features and other solution cavities. There are no known syndepositional sandy beds within the limestone.

As part of the geologic investigation, a photolineament map was prepared to help evaluate potential sinkhole-prone areas near the site. Photolineament maps are commonly prepared as part of a karst investigation because the photolineaments may reflect fractures and karst conduits in the underlying limestone. Note that photolineaments are simply features, such as depressions, linear soil tones, plant communities, and stream segments that are aligned on aerial photographs or topographic maps. One should not conclude that photolineaments depicted on an aerial photograph reflect fractures without ground truthing them.

Figure 2-18 is the photolineament map on a pre-development aerial photograph from 1941. The photolineament map suggests that one northeast-southwest photolineament encroaches on the southern part of the site. However, no major photolineaments are evident in the western part of the detention pond or in the vicinity of the landfill cells.

2.3 Geotechnical Investigation

In addition to providing verification for the geophysical data from the GPR and EM surveys and characterizing geological conditions, data from SPT borings were used to characterize the karst and sinkhole conditions. Early in the geotechnical phase of this investigation, both the EM-31 conductivity anomaly map (Figure 2-3) and the positions of the GPR anomalies (Figure 2-4) were reviewed, and SPT boring locations were selected based in large part on the geophysical results, thus satisfying the main objective of the geophysical investigation and providing sufficient data to evaluate sinkhole activity and sinkhole risk.

Split-spoon sampling was conducted continuously within at least the top 10 feet of each boring, and at 5-foot intervals thereafter. Split-spoon samples were collected in accordance with ASTM D1586 (2-inch-diameter sampler driven 24 inches by blows from a 140-pound hammer falling freely for a 30-inch drop). The number of blows required to drive the sampler each 6-inch increment were recorded. The Standard Penetration Resistance (N-value) was calculated as the sum of the blows over the second and third 6-inch increments of penetration.

In Florida, statutory definitions (Chapter 627.706 Florida Statutes) for property insurance have resulted in a dual classification of sinkholes. Sinkholes are landforms or depressions in the land surface. Sinkhole activity is defined by the pattern of systematic weakening of sediments as they migrate into the underlying void. Since the onset of raveling may not be immediately accompanied by development of a depression in the land surface, sinkhole activity is considered as a precursor to actual development of a sinkhole landform.

There is a pattern of systematic weakening of the cover materials with depth even though there is no landform. The depressions in the Subsidence Event Area are sinkholes using this dual classification and are known as cover-collapse and suffosion sinkholes.



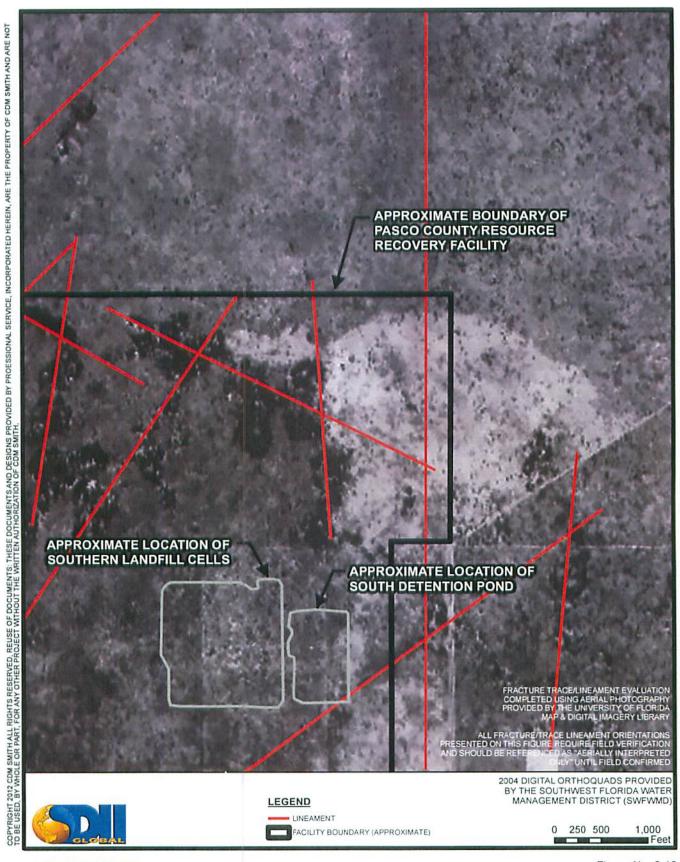




Figure No. 2-18
Pasco County Resource Recovery Facility
Fracture Trace/Lineament Evaluation
February 15, 1941 USDA (CTT-9B-28)

Cover-collapse sinkholes, as developed in the south detention pond, are common throughout central and north Florida. They form when there is a void near the top of the limestone that has been bridged over by clay- and sand-rich sediments. As noted above, the cover consists of a layer of clay-rich sediments overlain by a layer of sand. Since the clay is normally cohesive and capable of supporting the load generated by the cover materials, most voids are bridged. When a soil or rock bridge fails or collapses, the overlying, easily transported sandy sediments migrate (ravel) into the void space forming a cover-collapse sinkhole. Raveling is the process of migration of soils into the voids within the underlying limestone. Since sinkhole activity is a "bottom-up" process, raveling is identified by systematic weakening of the soils with depth.

The process of raveling should be expected to result in an organized pattern of weakening of the soils with depth. Individual horizons of weak soil sediment separated by stronger soils do not constitute a pattern reflecting sinkhole development. A pattern of systematic weakening is generally considered to exist when there is a fairly regular decrease in soil strength with depth. However, there are situations where sediment strength systematically decreases with depth as a result of common geological conditions other than sinkhole activity, such as at erosional or weathering surfaces.

In addition to sinkholes developing by failure of the roof of the void, they can form by direct raveling of the non-cohesive cover materials into void space in the limestone without failure of a void-roof. This transport by running water or soil creep creates what is known as a suffosion sinkhole.

Identification of sinkholes is much easier than identifying sinkhole activity. Sinkholes are depressions that can be assumed to have karst origins if there is no man-made void, pipeline, or other feature that could have caused the collapse. The presence of voids in the limestone and isolated artifacts of SPT drilling that do not fit the pattern of cover raveling should not be construed to indicative of sinkhole activity. Patterns in the SPT blow count data that reflect the raveling process must be present to identify sinkhole activity.

Identification of sinkhole activity and/or the potential for development of subsidence features in the immediate vicinity of the south detention pond, was performed by evaluating the following in the proper geologic context and pattern of sinkhole development:

- Pattern of declining N values over more than three sampling intervals in the sandy portion of the cover;
- Loss of drilling fluid circulation that is above the limestone and epikarst; and
- If the SPT is near the throat of the developing sinkhole the pattern of declining N values is likely to culminate in either weight-of-rod or -hammer strength materials (N values of 0).

Where these lines of evidence fit the expected pattern of raveling, one can conclude that sinkhole activity is present. These patterns may not be evident in GPR and EM-31 data. However, disruption of GPR reflectors and sudden decreases in terrain conductivity may reflect locations where the sediments are beginning to ravel.

In order to determine the risk of reoccurrence and/or future subsidence-related issues within the south stormwater detention pond and to evaluate the potential subsidence-related risks associated with the landfill cells to the west of the Subsidence Event Area and to the north, SPT borings were advanced to evaluate the subsurface conditions for raveling associated with sinkhole activity.



Locations for the SPT borings were determined through evaluation of geophysical and topographical data, as well as visual observation.

Within the vicinity of the subsidence event area, the following SPT borings indicated the presence of sinkhole activity: SB-16, SB-35, SB-36, SB-37, SB-39 and SB-40. Evidence of sinkhole activity was also observed in SB-42, which is located due north of the area of sinkhole development, as well as, SB-5, SB-11, and SB-45, within the northeastern corner of the southern detention pond. Figure 2-4 shows the locations of the SPT borings and identifies those borings that were located in the Subsidence Event Area or were located where sinkhole activity may be occurring. Descriptions of these borings follow.

SB-5: At this location, the extent of the cohesive material above the limestone and the total depth to the limestone surface indicate that long term settlement in the immediate vicinity of this boring would be likely, and rapid development of a sinkhole is unlikely. After fluctuating slightly to 15.5 feet bls through poorly graded sand, the N values in SPT boring SB-5 generally declined to 35.5 feet bls through poorly graded sand, including weight-of-hammer strength materials from 31 to 31.5 feet bls and from 34.5 to 35 feet bls. The SPT N values then increased to 45.5 feet bls before declining through interbedded sand and clay, including weight-of-hammer strength material from 51 to 52 feet bls. There were consecutive weight-of-rod events from approximately 54 to 76 feet bls within clay and a thin interbedded layer of sand, which was encountered from 66.5 to 67.5 feet bls. Below 75 feet bls, N values increased through poorly graded sand and limestone, which was encountered at 76.2 feet bls. The boring was terminated at 81 feet bls within limestone. Rapid losses of drilling fluid circulation occurred at 54 and 68 feet bls. Advancing the casing allowed drilling to be completed without borehole collapse. The systematic decline in N values above the clay, accompanied by a total of 23 feet of consecutive sampling intervals with weight of rod/hammer events within the clay and the rapid losses of circulation above the limestone, provide evidence for sinkhole activity in SB-5.

SB-11: At this location, the extent of the cohesive material above the limestone indicates that long term settlement in the direct vicinity of this boring would be likely and that the rapid development of a sinkhole is unlikely. In SPT boring SB-11, the N values increased to 11.5 feet bls within poorly graded sand and then declined systematically within clay culminating in weight-of-hammer from 25 to 27 feet bls and weight-of-rod conditions from 30 to 31.5 feet bls. A weight-of-rod event occurred at 20 feet bls within the seating interval; however, per ASTM D1586, this is not utilized in determining the relative sediment strength of the encountered sample. A rapid loss of circulation occurred at 29 feet bls. The N values increased within limestone, which was encountered at 33.5 feet bls. The boring was terminated in refusal strength limestone at 52 feet bls. The systematic decline in N values within the clay, accompanied by a total of 3.5 feet of weight-of-rod/hammer strength material and a rapid loss of drilling fluid circulation, all above the limestone, provide evidence for sinkhole activity in SB-11. The limited extent of stiff clay is insufficient to prevent sagging of the cohesive materials.

SB-16: Data from this boring does not indicate upward propagation of loosening conditions and may only represent a potential for localized long term settlement rather than the rapid development of a sinkhole in the near-term. However, because of the intervals of weight-of-rod/hammer materials and repetitive rapid losses of drilling fluid circulation, lateral raveling of the sediments cannot be eliminated as a potential cause of the intervals of very loose or very soft sediments. The N values in SPT boring SB-16 generally increased through poorly graded sand and a thin clay layer, which was encountered from 14 to 15.5 feet bls, to approximately 21.5 feet bls. The N values then declined through clay-rich sediment and fluctuated to the top of limestone, which was encountered at 61.5 feet bls. A weight-of-hammer event occurred within the seating interval at 30 feet bls. This weight-of-



hammer event is considered to be an artifact of the drilling process and is not considered important. Significant weight-of-hammer conditions were encountered from 40 to 41.5 feet bls, 46 to 46.5 feet bls, and 56 to 57 feet bls. A weight-of-rod event also occurred within the seating interval at 50 feet bls. The N values fluctuated, but generally increased, from 61.5 to 91 feet bls through poorly graded sand and limestone, where the boring was terminated within refusal-strength limestone. Weight-of-hammer strength limestone was encountered from 75.5 to 77 feet bls. Rapid losses of drilling fluid circulation occurred at 27, 35, and 42.5 feet bls. However, advancing casing allowed drilling to be completed without borehole collapse. Based on the location of the boring relative to SB-15 and SB-17, where limestone was encountered at 2.5 and 16 feet bls, respectively, SB-16 is representative of a paleo-sinkhole feature within which materials have not had enough time to settle and consolidate over geologic time.

SB-35: SPT boring SB-35 was advanced in close proximity to the western edge of subsidence feature A. The formation of subsidence feature A is the evidence of sinkhole activity at this location. The N values in SPT boring SB-35 declined through poorly graded sand and clayey sand to approximately 9.5 feet bls, where clay was encountered. The N values then increased to 16.5 feet bls through clay and into limestone, which was encountered from 12.5 to 17 feet bls. A rapid loss of drilling fluid circulation occurred at 13.5 feet bls within limestone. The N values declined within an interbedded clay unit, encountered from 17 to 25 feet bls, before increasing again within a lower limestone unit, which was penetrated at 25 feet bls. A weight-of-hammer event within a seating interval occurred at 20 feet bls. The boring was terminated at 37 feet in strong limestone. SPT boring SB-35 was advanced in close proximity to the lowest point of Feature A, which was approximately 8 to 9 feet deep. The decline in N values in sandy soils is solely within backfilled material. The increase in N values at 11.5 feet in clay and then again at 16.5 feet in limestone indicates that the soils did not move vertically downward at this location, rather they had been transported laterally into the limestone.

SB-36: SPT boring SB-36 was advanced in close proximity to the eastern edge of subsidence feature A. The decline in N values accompanied by a total of 2.5 feet of weight of rod/hammer strength materials and a rapid loss of drilling circulation above the limestone, as well as the proximity to subsidence feature A, provide evidence for the existence of sinkhole activity in the immediate vicinity of SPT boring SB-36. The N values in SPT boring SB-36 fluctuated, but remained relatively consistent within poorly graded sand, silty sand, and clay to approximately 17.5 feet bls. The N values then declined through clay and clayey sand to the top of limestone, which was encountered at 25 feet bls. Weight-of-rod/hammer conditions were encountered from 19.5 to 22 feet bls. A rapid loss of drilling fluid circulation occurred at 20 feet bls. Below 25 feet bls, the N values fluctuated within a sequence of limestone and interbedded clay, including weight-of-rod strength material from 30 to 31.5 feet bls. The boring was terminated within stiff clay at 47 feet bls.

SB-37: SPT boring SB-37 was advanced in close proximity to the southeastern edge of subsidence feature D. The decline in N values accompanied by weight-of-rod/hammer strength materials, rapid losses of drilling circulation above the limestone, and proximity to subsidence features B and D are evidence that sinkhole activity exists in the immediate vicinity of SPT boring SB-37. After initially increasing to 3.5 feet bls, the N values in SPT boring SB-37generally declined and remained soft to very soft or very loose through poorly graded sand, silty sand, clay, and clayey sand to the top of limestone, which was encountered at 40 feet bls. This decline included weight-of-rod events from 20 to 22 feet bls and 25 to 26 feet bls and weight-of-hammer events from 6.5 to 7 feet bls, 16.5 to 17 feet bls, 25 to 25.5 feet bls and 36 to 36.5 feet bls. The N values fluctuated, but generally increased, through limestone and included weight-of-hammer strength material from 46.5 to 47 feet bls and weight-of-hammer strength material from 46.5 to 47 feet bls and weight-of-



rod strength material from 50 to 51 feet bls. The boring was terminated within limestone at 62 feet bls. Rapid losses of drilling fluid circulation occurred at 15.5 and 34 feet bls; however, advancing casing during drilling allowed borehole completion without collapse.

SB-39: SPT boring SB-39 was advanced in close proximity to the southeastern edge of subsidence feature E and approximately five feet to the northwest of the edge of feature F. In SPT boring SB-39, the N values remained low through a stratigraphic sequence comprised of layers of poorly graded sand, silty sand, clay, and limestone to 20.5 feet bls. Limestone was consistently encountered below 20.5 feet bls, where N values fluctuated, but generally increased. Weight-of-hammer events occurred from six to eight feet bls, nine to 9.5 feet bls, 13 to 14 feet bls, 15.5 to 16 feet bls and 17 to 17.5 feet bls. Weight-of-rod events occurred from eight to 8.5 feet bls, within the seating interval, 14 to 15.5 feet bls, and 16 to 17 feet bls. Rapid losses of drilling fluid circulation occurred at 8 and 11 feet bls; however, advancing of casing allowed drilling to be completed without borehole collapse. SPT boring SB-39 was terminated in refusal strength limestone at 32 feet bls. Limestone was observed within two feet of the land surface on the sidewalls of subsidence feature F. The decline in N values accompanied by weightof-rod/hammer strength materials, rapid losses of drilling circulation above the limestone, proximity to subsidence features E and F, and the difference in depth to limestone from what was observed approximately five to eight feet away; indicate that sinkhole activity exists in the vicinity of the boring. Limestone was identified in nearby SPT boring SB-38 at a depth of six feet bls and just a few inches under the surface on the sidewalls of Features N, Q, and O. These shallow limestone surfaces could represent pinnacles of limestone where sediments could potentially ravel if excessive stormwater loading occurs.

SB-40: SPT boring SB-40 was advanced in close proximity to the eastern edge of subsidence feature I. The systematic decline in N values accompanied by weight-of-rod/hammer strength material, a rapid loss of drilling circulation above the limestone, and proximity to subsidence feature I indicate that sinkhole activity exists in the immediate vicinity of SPT boring SB-40. Systematically declining N values were measured through poorly graded sand and clayey sand in SPT boring SB-40 from land surface to the top of limestone. The N-value decline included weight-of-rod/hammer events from 10 to 11 feet bls, 12.5 to 13 feet bls, and 14 to 17 feet bls. A rapid loss of drilling fluid circulation occurred at 13.5 feet bls. Limestone was encountered at 18 feet bls where the N values increased and then fluctuated through limestone and an infilled clay unit to the termination of the boring at 47 feet bls. A weight-of-rod event occurred within the infilled clay horizon within the seating interval at 35 feet bls.

SB-42: The systematic decline in N values in non-cohesive sandy soils accompanied by weight-of-hammer strength materials above the limestone and proximity to subsidence feature I provide evidence for the existence of sinkhole activity in the immediate vicinity of SPT boring SB-42. Systematically declining N values were measured in SPT boring SB-42. This decline in N values, which were through poorly graded sand to 9.5 feet bls, included weight-of-hammer strength materials from 6.5 to seven feet bls and 8.5 to 9.5 feet bls. Below 9.5 feet bls, the N values fluctuated, but generally increased through clay and limestone, which was penetrated at 17 feet bls. A weight-of-hammer event occurred within the seating interval at 15.5 feet bls within clay. A rapid loss of drilling fluid circulation occurred within limestone at 18 feet bls. The boring was terminated at 37 feet bls in refusal strength limestone.

<u>SB-45</u>: Systematically declining N values were measured in SPT boring SB-45 from approximately two feet bls to the top of limestone, which was encountered at 17 feet bls. This interval, which was through poorly graded sand and clay, included weight-of-rod/hammer events from 10 to 15 feet bls. The N



values generally increased within limestone to the termination of the boring at 32 feet bls. Drilling fluid circulation was maintained while advancing the boring.

In summary, the SPT data indicate that both sinkhole development and sinkhole activity or the potential for future subsidence exists within a focused area of the south detention pond area. Sinkhole activity was identified through the presence of weakening of sediments with depth, which is indicative of raveling in SPT borings SB-16, SB-35, SB-36, SB-37, SB-39, SB-40 and SB-42 in the southern part of the southern detention pond in the vicinity of the Subsidence Event Area. SPT borings SB-35, SB-36, SB-37, SB-39, and SB-40 were in close proximity to the Subsidence Event Area. The limestone surface within the Subsidence Event Area is pinnacled and, in some areas, it crops out at the land surface.

Conditions in SPT boring SB-16, just northwest of the Subsidence Event Area, indicate that long-term settlement in the immediate vicinity of this boring would be likely, but slow to occur. This is the nearest location to the landfill cells where the potential for subsidence feature development was identified.

The potential of rapid development of a sinkhole near SB-42, which is north of the Subsidence Event Area, cannot be eliminated. While there was no rapid loss of circulation above the limestone surface and/or evidence of a large subsurface void within the limestone, the decline of N values within the weak material overlying limestone is indicative of sinkhole activity.

Sinkhole activity, or the potential for subsidence feature development, was also identified in SB-5, SB-11, and SB-45 in the northeastern corner of the southern detention pond. The locations of SPT SB-5, SB-11, and SB-45 correspond to the large GPR anomaly in the northeastern part of the site. Conditions in SPT borings SB-5 and SB-11 indicate that long-term settlement in the vicinity of these borings would be likely, but slow to occur, and rapid development of a sinkhole is unlikely. Like SB-42, the potential of rapid development of a sinkhole near SB-45 cannot be eliminated. There was no rapid loss of circulation above the limestone and or evidence of a large subsurface void in the close proximity to SB-45. Also, limestone is not present near land surface.

In addition, results of the investigation demonstrated that the larger sinkholes that developed in the Subsidence Event Area are clearly cover-collapse sinkholes formed by hydraulic loading in the stormwater pond during Tropical Storm Debby. The smaller, satellite sinkholes peripheral to the cover-collapse sinkholes are small suffosion sinks where sand raveled into pipes that lead to the main conduit of the cover-collapse sinks. Such satellite sinkholes are common around larger, cover collapse features.

Geotechnical, geophysical, and visual data collected as part of this investigation demonstrate that cover materials failed (collapsed) and the cover and overlying sediments were rapidly transported (raveled) into void space within the limestone in the southern stormwater detention pond. The cover failure in the south detention pond is most likely the result of the sudden and substantial loading of the stormwater pond as a result of rainfall events associated with Tropical Storm Debby. Once the cover materials failed, the large amount of water within the pond quickly emptied into the aquifer through the breach in the cover, transporting sediments into the void space within the limestone. This surge of sediment and water into the aquifer in turn created a localized, sudden change of the water pressure within the aquifer, which apparently induced additional and proximal cover collapses. The water collecting and migrating toward the cover failure piped through the sand resulting in the small, satellite, suffosion sinks.



Breaching of the cover in the Subsidence Event Area has allowed a substantial amount of sediment into the subsurface within the area of shallow, karstic limestone. As several high rainfall events occurred after the formation of the sinkhole features and progression or enlargement of the features was negligible, it is likely that a significant proportion of the available void space was filled as the features formed. Conduits into the limestone that were visible within the features were filled with CLSM. These filled features are unlikely to re-form as the CLSM and fill material has replaced the failed cover and filled any remaining open void space. There is no increased risk of subsidence under the landfill cells or for adverse impacts to groundwater quality as a result of the development of the subsidence features in the south detention pond.



Section 3

Method of Investigations

3.1 Electromagnetic Survey

The EM survey was performed using an EM31 Terrain Conductivity (Geonics Ltd.) meter. This meter is a non-contact means of determining the bulk ground electrical conductivity (inverse of electrical resistivity). The Trackmaker option was used to display a position grid on the instrument screen to ensure the operator maintained position along the lines at the surveyed transects.

Instrument calibration was checked on a daily basis using internal testing protocols defined by the manufacturer. Prior to use, the operator allowed the system to come to equilibrium by operating and observing stability of readings for a minimum 5-minute period before commencing data acquisition. Data were edited and geo-referenced using the Trackmaker software, and a conductivity contour map was prepared utilizing software program SURFER.

3.2 Ground Penetrating Radar Survey

A GPR survey was performed concurrently with completion of the geotechnical/geological control borings. Additional GPR transects were performed based on interpretations of geophysical and geotechnical data.

A Geophysical Survey Systems Inc. (GSSI) Model SIR2000 digital radar system with a 200 MHz dipole antenna and calibrated survey wheel was used as the data acquisition system. The antenna was towed behind a vehicle except where steep ground slopes of the berms caused safety issues. For such segments, the instrument was hand towed.

The operator established instrument record settings prior to commencing data acquisition by recording test profiles at several different range settings at several different locations across the Site to optimize recording of subsurface reflectors which may vary in depth from one portion of the Site to another. These locations were coordinated with the locations of geotechnical/geological control borings.

The survey wheel was calibrated prior to initial data acquisition. This was accomplished using the internal calibration protocol of the SIR2000 system along a measured distance along a line representative of the terrain over most of the property.

Data were initially acquired on the north-south transects. Data were then acquired on east-west transects. In-fill GPR lines were completed in the Subsidence Event Area to further define the observed anomalies.

3.3 Geotechnical Borings

Geotechnical/geological control soil borings, including standard penetration tests (SPTs), were completed to provide geotechnical and geological control for field interpretation of geophysical data.

Most soil borings were terminated approximately 10 feet below the surface of the limestone. Split spoon samples were collected continuously from most borings at the ends of the grid lines and from



selected locations in the bottom of the pond. Where continuous sampling was not performed, split spoon samples were collected continuously from the upper 12 feet of soil and at five-foot intervals thereafter until total boring depth. SPTs/split spoon testing was performed in accordance with ASTM D 1586. Boreholes were advanced using mud rotary drilling. After completion, each boring was grouted to land surface using neat Portland Type II cement.



Section 4

Conclusions and Recommendations

4.1 Conclusions

The following conclusions are based on results of the assessment.

- From published data (Sinclair et al., 1985;Trommer, 1987; Fretwell, 1988; and SWFWMD, 1988) and site-specific data, the Site is located within an area of karst development. The general sediment profile at the Site consists of sand overlying clay-rich sediments with varying amounts of sand. Lateral variations in the nature and thicknesses of the geologic strata that comprise the cover are common.
- One northeast-southwest photolineament visible on the 1941 pre-development aerial photograph encroaches on the southern part of the south detention pond. However, no major photolineaments are evident in the western part of the south detention pond or in the vicinity of the landfill cells.
- The subsidence features in the Event Area are not all isolated, individual features. They reflect a network of drainage features associated with a smaller number of chimneys in the underlying limestone.
- Based on the data collected, it does not appear that the raveling conditions associated with the formation of the sinkhole features in the Subsidence Event Area extend to the west, toward the landfill.
- Sinkhole activity, or the potential for subsidence feature development, was also identified in SB-5, SB-11, and SB-45 in the northeastern corner of the southern detention pond. This area poses no risk to the landfill cells.
- The larger sinkholes that developed in the Subsidence Event Area are clearly cover-collapse sinkholes formed by hydraulic loading in the stormwater pond during Tropical Storm Debby.
- The subsidence features in and adjacent to the south detention pond formed due to hydraulic loading in an isolated part of the detention pond where limestone is present near the land surface.
- The smaller, satellite sinkholes peripheral to the cover-collapse sinkholes are small suffosion sinks where sand raveled into pipes that lead to the main conduiting of the cover-collapse sinks.
 Such satellite sinkholes are common around larger, cover collapse features.
- Breaching of the cover in the Event Area has allowed a substantial amount of sediment into the subsurface within the area of shallow, karstic limestone. Conduits into the limestone that were visible within the features were filled with CLSM. These filled features are unlikely to re-form as the CLSM has replaced the failed cover and filled any remaining open void space.



 There is no increased risk of subsidence under the landfill cells or for adverse impacts to groundwater quality as a result of the development of the subsidence features in the south detention pond.

4.2 Recommendations

The following recommendations are based on results of the assessment:

- Neither additional investigation nor remedial or preventative measures are warranted.
 However, securing the area around the event area is recommended to mitigate the potential for injury in the unlikely event of additional subsidence.
- If the areas continue to subside, additional fill should be placed in the depressions to prevent ponding of rain water.



Section 5

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Appendix A

Approved Work Plan

Pasco County Utilities

West Pasco Resource Recovery Facility
South Stormwater Detention Pond Subsidence Feature
Assessment Work Plan

October 2012

William T. Beeson, P.G.TATE OF Florida Professional Geologist No. 1214

CDM Smith

1715 North Westshore Blvd., Suite 875

Tampa, FL 33607

West Pasco

Assessment Work Plan

Introduction

As a result of heavy rain associated with Tropical Storm Debby, subsidence features formed in and around the southwestern part south stormwater detention pond east of Cell A-3 at the West Pasco Class I Landfill on June 26, 2012. The Florida Department of Environmental Protection (FDEP) was notified of the occurrence of the subsidence features and inspected the facility on June 27, 2012. Representatives of Pasco County and CDM Smith met with FDEP at the site on June 27, 2012. In addition to requiring Pasco County to take immediate actions to mitigate the potential for future subsidence, FDEP requested that Pasco County develop a work plan for geological, geotechnical and geophysical investigations of the stormwater detention ponds.

The following plan for assessment of the south stormwater detention pond is submitted in accordance with the FDEP request and subsequent conversations with representatives of FDEP. A plan for assessing the potential for the occurrence of subsidence features in and around the north stormwater detention pond will be provided if results of the south detention pond assessment indicate that the potential for subsidence to the north exists.

The objectives of the assessment are to evaluate the potential for additional geologic activity to occur in and around the south detention pond and to obtain sufficient information to develop plans to remediate the existing subsidence features in the south detention pond and to mitigate potential risks to the landfill. The assessment will include five tasks. These are:

- Task 1 Establish the site grid and coordinate system,
- Task 2 Electromagnetic (EM) survey,
- Task 3 Geotechnical/geological control borings,
- Task 4 Ground penetrating radar (GPR) survey, and,
- Task 5 Geotechnical/geological investigations of existing subsidence features.

Results of the assessment will be presented in a brief report. The report will include a discussion of the results with respect to available information regarding the local and regional hydrogeology, an evaluation of potential risks for future subsidence that may adversely impact adjacent landfill cells, a focused feasibility analysis of potential remedial action strategies, remedial action plans for implementing the most cost-effective remedial action strategy, and a schedule for implementing the remedial action plans.

The assessment will be initiated upon receipt of approval of the work plan by FDEP and after the subsidence features have been filled and the security fence has been removed. Completion of the assessment and submittal of the report and remedial action plans is expected to take approximately three months.



Task 1 – Establish Site Grid and Coordinate System

A grid with 50-foot centers will be established in and adjacent to the south stormwater detention pond. The grid lines will have a true (State Plane Coordinate System) orientation of due North-South and due East-West. All data generated during the geophysical and geotechnical testing will be georeferenced to the site and/or State Plane geographic coordinate system.

Grid nodes will be staked every 200 feet around the perimeter of the grid system and at 100-foot intervals along the northern and southern boundaries of the pond at the base of the berm. In the northern part of the pond, nodes will be staked at 200-foot intervals from north to south and at 100-foot intervals from east to west. Grid nodes will be staked at 100-foot intervals in the southern part of the pond. The nodes that are staked may be revised pending field conditions. Each stake will be marked with the appropriate site coordinates. Selected stakes will also be marked with the land surface elevation. Approximate locations of the grid and staked nodes are shown on **Figure 1**.

Task 2 – Electromagnetic Survey

The EM survey will be performed using an EM31 Terrain Conductivity (Geonics Ltd.) meter. This meter is a non-contact means of determining the bulk ground electrical conductivity (inverse of electrical resistivity). The instrument is a man-portable device with an integrated GPS location system (Trackmaker Option) that will determine the bulk conductivity to a depth of approximately 16 feet below land surface with readings (conductivity and latitude/longitude) being taken on one-second intervals as the operator walks along the grid of transect lines. The Trackmaker option displays a position grid on the instrument screen to ensure the operator maintains position along the lines at the (approximate) desired line spacing. Recording conductivity and positioning data in one file enables production of conductivity contour maps for detection of anomalous areas that may be associated with subsidence features.

The operator will check instrument calibration on a daily basis using internal testing protocols defined by the manufacturer. The operator will allow the system to come to equilibrium by operating and observing stability of readings for a minimum 5-minute period before commencing data acquisition.

Data will be acquired on a series of parallel north-south transects having maximum length of 950 feet and an inter-line spacing (optimal) of 50 feet. Acquisition will proceed by starting in the SW corner of the grid and proceeding north along the westernmost line. Acquisition will be paused at the northern end of the line, and the operator will move to the northern end of the second line and begin acquisition of the second line proceeding to the south. This zig-zag acquisition pattern will be continued until all north-south grid lines have been acquired. Additional in-fill EM survey lines of appropriate length to further define the observed anomalies

Data will be edited and geo-referenced using the Trackmaker software, and a conductivity contour map will be prepared utilizing the SURFER software program. It may be necessary to convert the internal GPS positioning format to Florida State Plane coordinates prior to final map generation.

Task 3 – Geotechnical/Geological Control Borings

Prior to the performance of the ground penetrating radar (GPR) survey, geotechnical/geological control soil borings, including standard penetration tests (SPTs), will be completed to provide geotechnical and geological control for field interpretation of geophysical data. Figure 1 shows the proposed locations of 24 control borings. As shown on Figure 1, the control borings are more tightly spaced in the southern part of the pond in the vicinity of the subsidence features. Locations of the



geotechnical/geological control borings may be modified based on results of the EM survey or other site conditions.

Each soil boring will be terminated approximately 10 feet below the surface of competent rock. Split spoon samples will be collected continuously from borings at the ends of the grid lines and from selected locations in the bottom of the pond. Where continuous sampling is not performed, split spoon samples will be collected continuously from the upper 10 feet of soil and at five-foot intervals thereafter until refusal. SPTs/split spoon testing will be performed in accordance with ASTM D 1586. If split-spoon samples cannot be collected, then drill cuttings will be described. Boreholes will be advanced using hollow stem augers or, if necessary, rotary drilling methods.

If results of SPTs indicate that soft or incompetent materials are present in the subsurface, then additional testing using a flat dilatometer may be performed. Flat Dilatometer testing (ASTM D6635) may be utilized to investigate the strength and compressibility of selected zones that exhibit negligible Standard Penetration Resistance. The Flat Dilatometer test is a refined tool that enables better characterization of such zones and to distinguish between strata that may simply be weak or less consolidated versus cavity or raveling zones that would have an elevated potential for subsidence.

After completion, each boring will be grouted to land surface using neat Portland Type II cement. All drilling and sampling will be supervised by a qualified geologist or engineer.

Task 4 – Ground Penetrating Radar Survey

A GPR survey will be performed after most of the geotechnical/geological control borings have been completed. A Geophysical Survey Systems Inc. (GSSI) Model SIR2000 digital radar system with a 200 MHz dipole antenna and calibrated survey wheel as the data acquisition system will be used. The antenna will be towed behind a vehicle except where steep ground slopes of the berms cause safety issues. For such segments, the instrument will be hand towed.

The operator will establish instrument record settings prior to commencing data acquisition by recording test profiles at several different range settings (e.g. 100 ns, 150 ns, and 200 ns) at several different locations across the property to optimize recording of subsurface reflectors which may vary in depth from one portion of the site to another. These locations will be coordinated with the locations of geotechnical/geological control borings.

The survey wheel will be calibrated prior to initial data acquisition. This will be accomplished using the internal calibration protocol of the SIR2000 system along a measured distance along a line representative of the terrain over most of the property. Calibration line length must be at least 700 feet in length.

Data will be initially acquired on the north-south transects (see Figure 1). Data will then be acquired on east-west transects. Acquisition will proceed by starting in the SW corner of the grid and proceeding north along the westernmost line. Acquisition will be closed at the northern end of the line, and the operator will move to the northern end of the second line and begin acquisition of the second line proceeding to the south. This zig-zag acquisition pattern will be continued until all north-south grid lines have been acquired. The GPR crew will then start on a similar acquisition scheme for the staked east-west lines. Note that at any time during data acquisition anomalous subsurface conditions may be observed on the gridded GPR profiles. The crew chief will mark (as possible) the approximate locations of such anomalies and add in-fill GPR lines of appropriate length to further define the observed anomalies. SPT borings, and if appropriate, flat dilatometer testing, will be performed at the locations of anomalies to confirm GPR data.



GPR profiles will be in final form for analysis without further data processing steps. Data will be analyzed and georeferenced using the surveyed grid and stake coordinates as measured by others. A final GPR anomaly map will be prepared based on analysis of the GPR profiles.

Task 5 – Geotechnical/Geological Investigations of Existing Subsidence Features

SPT borings will be performed in and in the immediate vicinity of the existing subsidence features in the south detention pond. In addition to the approximate centers of the largest subsidence features that are accessible for the drilling rig, other locations in the immediate vicinities of the subsidence features will be selected based on results of the GPR and EM surveys. Borings and testing will be performed as described in described in Task 2. If possible, continuous split-spoon sampling will be performed. If split-spoon samples cannot be collected, then drill cuttings will be described. A light, track-mounted drill unit (Diedrich D-25) will be used in and in the vicinity of the subsidence features. This rig is capable of drilling at least 100 ft.

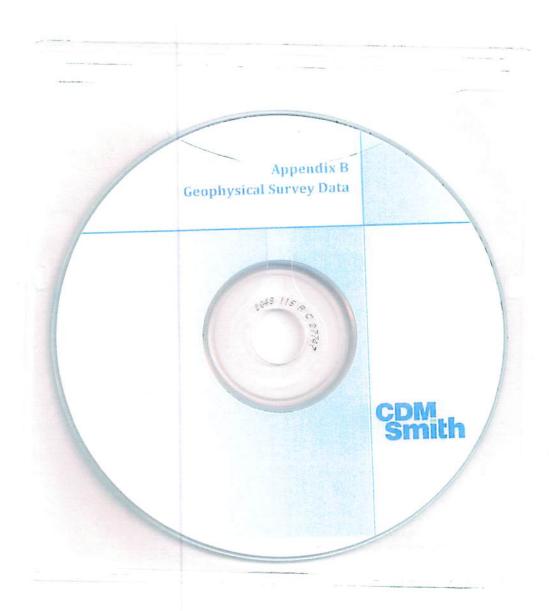




Figure No. 1 West Pasco Resource Recovery Facility South Detention Pond Grid and Control Locations

Appendix B

Geophysical Survey Data



Appendix C Boring Logs

DEPTH (ft) BLS	DESCRIPTION	PENETRATION blows per foot	BLOW	N VALUE	COMMENTS Water Table Drilling Fluid Losses ft. = ft. bls (typical)
5—	POORLY GRADED SAND (SP): Light-brownish-gray (10YR 6/2) fine to medium quartz sand Hand augered from 0-6 ft.				Hand auger boring from 0-6 ft.
- 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1	POORLY GRADED SAND (SP): Loose to medium dense Light-yellowish-brown (10YR 6/4) fine to medium quartz sand Spooned from 6-10 ft.		2/3/4/5	7	NoEV from 6 ft. to end of boring EFR @ 6 ft. @ 12 gpm
10	CLAY (CL/CH): Stiff Sandy clay Spooned from 10-12 ft.		5/5/6/7	15	Casing set to 10 ft. when at 12 ft.
	CLAY (CL/CH): Soft to stiff Mottled greenish-gray (5G 5/1) and yellow (10YR 7/6) clay Spooned from 12-18.5 ft.		2/4/5/7	9	
15 —		0 20 40 60 80	3/3/3/5	6	Casing set to 15 ft. when at 18 ft.
	CLAY (CL/CH): Medium stiff Light-yellowish-brown (10YR 6/4) calcareous clay Spooned from 18.5-19 ft.		2/2/3/5	5	
20	CLAY (CL/CH): Mottled light- greenish-gray (5G 7/1) and yellow (10YR 7/6) clay		3/4/2/4	6	



SPT BORING LOG

SITE NAME: PCRRF - South Stormwater Pond

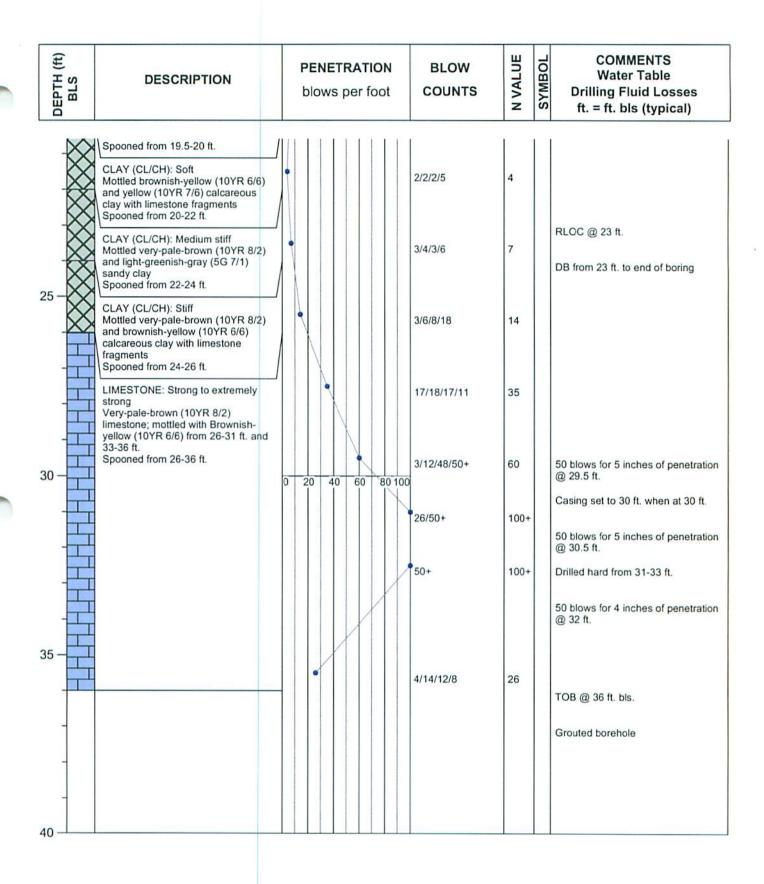
SITE LOCATION: Spring Hill, Florida

DESIGNED BY: CHECKED BY: **JMW** DRAWN BY: MVK

PROJECT NO: 8053622 BORING DATE: 11/07/12 PAGE NO: 1 of 2

BORING

SB-1





SPT BORING LOG

SITE NAME: PCRRF - South Stormwater Pond SITE LOCATION: Spring Hill, Florida

DESIGNED BY: DR PROJ CHECKED BY: JMW BORIN

MVK

DRAWN BY:

PROJECT NO: 8053622 BORING DATE: 11/07/12 PAGE NO: 2 of 2

DEPTH (ft) BLS	DESCRIPTION	PENETRATION blows per foot		N VALUE	COMMENTS Water Table Drilling Fluid Losses ft. = ft. bls (typical)
0	POORLY GRADED SAND (SP): Brown (10YR 5/3) fine to medium quartz sand with limestone gravel Hand augered from 0-2 ft.				Hand auger boring from 0-6 ft.
5-	POORLY GRADED SAND (SP): Loose Light-gray (10YR 7/2) fine to medium quartz sand; mottled with grayish- brown (10YR 5/2) from 3.75-4 ft. Hand augered from 0-6 ft., spooned from 6-8 ft.				
- -		•	3/3/3/4	6	NoEV from 6-18 ft. EFR @ 6 ft. @ 12 gpm
10 —	SILTY SAND (SM): Loose to medium dense Light-brownish-gray (10YR 6/2) silty fine to medium quartz sand Spooned from 8-12 ft.		2/3/5/7	8	
	CLAYEY SAND (SC): Mottled light- gray (10YR 7/1) and yellow (10YR 7/6) clayey fine to medium quartz sand Spooned from 12-14 ft.		4/4/5/6	9	
15 -	CLAY (CL/CH): Medium stiff Mottled light-greenish-gray (5G 7/1) and light-gray (10YR 7/2) sandy clay Spooned from 14-16 ft.	0 20 40 60 8	0 100 3/3/4/5	7	Casing set to 15 ft. when at 16 ft.
	CLAY (CL/CH): Mottled light- greenish-gray (5G 7/1) clay and brownish-yellow (10YR 6/6) Spooned from 16-18 ft.	•	2/3/3/2	6	RLOC @ 18 ft.
20 -	CLAY (CL/CH): Very soft Mottled light-greenish-gray (5G 7/1) and brown (10YR 5/3) sandy clay with limestone gravel from 21.5-22 ft. Spooned from 18-22 ft.	•	wон/wон/wон/ woн	woh	FAS/WOH from 18-20 ft. FAS/WOR from 20-21 ft., then SLO/WOH from 21-21.5 ft.
	51	1 1 1 1 1 1 1 1 1	CDT	BOD	ING LOG



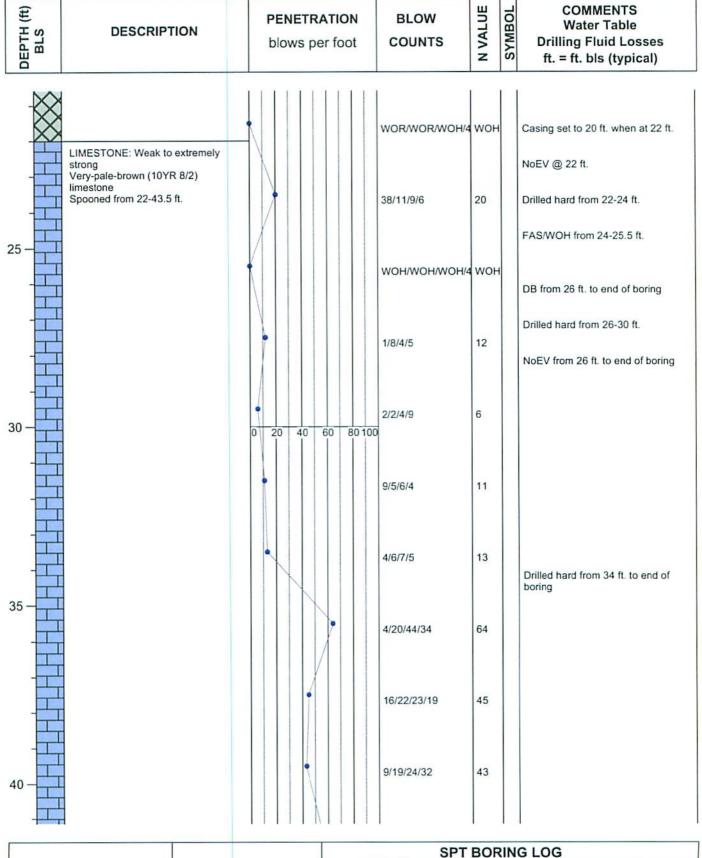
SPT BORING LOG

SITE NAME: PCRRF - South Stormwater Pond

SITE LOCATION: Spring Hill, Florida

DESIGNED BY: CHECKED BY: JMW DRAWN BY: MVK

PROJECT NO: 8053622 BORING DATE: 11/06/12 PAGE NO: 1 of 3



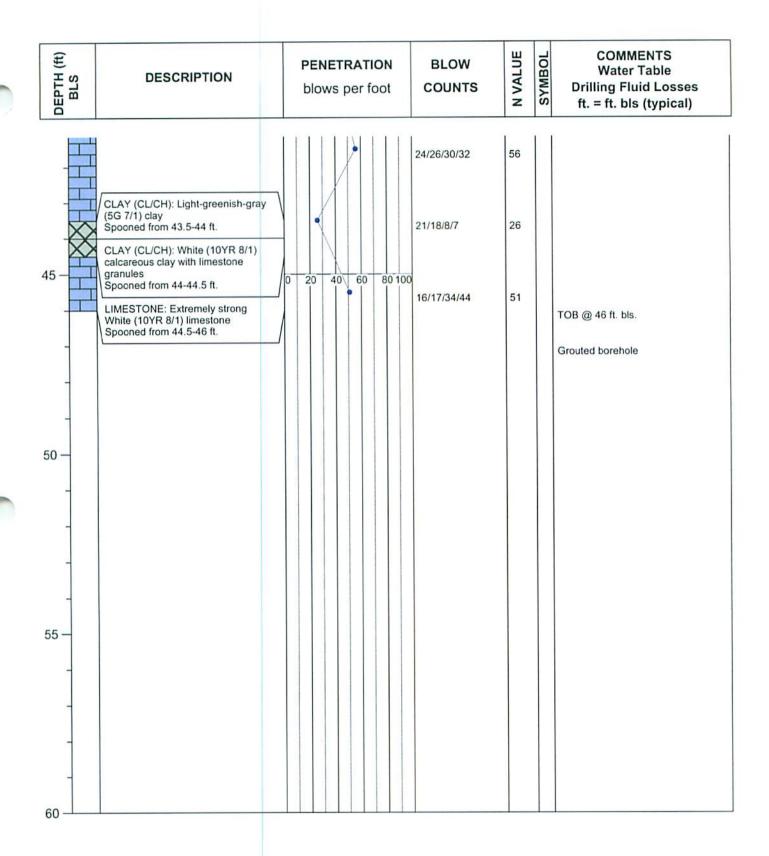


SITE NAME: PCRRF - South Stormwater Pond

SITE LOCATION: Spring Hill, Florida

DESIGNED BY: CHECKED BY: JMW DRAWN BY: MVK

PROJECT NO: 8053622 **BORING DATE: 11/06/12** PAGE NO: 2 of 3





SPT BORING LOG

SITE NAME: PCRRF - South Stormwater Pond

SITE LOCATION: Spring Hill, Florida

DESIGNED BY: DR
CHECKED BY: JMW
DRAWN BY: MVK

PROJECT NO: 8053622 BORING DATE: 11/06/12 PAGE NO: 3 of 3

DEPTH (ft) BLS	DESCRIPTION	PENETRATION blows per foot	BLOW	N VALUE	COMMENTS Water Table Drilling Fluid Losses ft. = ft. bls (typical)
0	POORLY GRADED SAND (SP): Light-brownish-gray (10YR 6/2) fine to medium quartz sand Hand augered from 0-6 ft.				Hand auger boring from 0-6 ft.
5—					NoEV from 4 ft. to end of boring
	SILTY SAND (SM): Very loose Light-brownish-gray (10YR 6/2) silty fine to medium quartz sand, mottled with grayish-brown (10YR 5/2) from 8-10 ft. Spooned from 6-10 ft.		1/1/1/1	2	EFR @ 6 ft. @ 12 gpm
10	POORLY GRADED SAND (SP): Loose to medium dense Gray (10YR 6/1) fine to medium quartz sand, mottled with brown (10YR 5/3) from 10-12 ft. Spooned from 10-16 ft.		2/2/3/6	5	
			3/6/6/10	12	
15 —	CLAY (CL/CH): Stiff Mottled gray (10YR 6/1) and brownish-yellow (10YR 6/6) clay	0 20 40 60 80	5/8/13/15	21	Casing set to 15 ft. when at 16 ft.
	Spooned from 16-19 ft.		5/5/4/5	9	MRR from 18-19 ft.
20	CLAY (CL/CH): Mottled light- greenish-gray (5G 7/1) and light-gray (10YR 7/1) clay with limestone gravel		2/2/2/5	4	RLOC @ 19 ft. Casing set to 20 ft. when at 20 ft.



SPT BORING LOG

BORING

SB-3

SITE NAME: PCRRF - South Stormwater Pond SITE LOCATION: Spring Hill, Florida

DESIGNED BY: DR PROJECT NO: 8053622 CHECKED BY: **JMW** BORING DATE: 11/07/12 PAGE NO: 1 of 4 DRAWN BY: MVK

DEPTH (ft) BLS	DESCRIPTION	PENETRATION blows per foot	BLOW	N VALUE	SYMBOL	COMMENTS Water Table Drilling Fluid Losses ft. = ft. bls (typical)
\bigotimes	Spooned from 19-21 ft. POORLY GRADED SAND (SP): Mottled gray (10YR 6/1) and yellow (10YR 7/6) fine to medium quartz sand Spooned from 21-22 ft.		5/7/5/7	12		EFR @ 20 ft. @ 12 gpm
	CLAY (CL/CH): Medium stiff Light-greenish-gray (5G 7/1) clay with limestone gravel Spooned from 22-24 ft.		2/4/3/2	7		
25	LIMESTONE: Moderately strong White (10YR 8/1) limestone Spooned from 24-26 ft.		5/8/4/5	12		DI 00 @ 25 4
	CLAY (CL/CH): Soft Mottled light-greenish-gray (5G 7/1) and brownish-yellow (10YR 6/6) clay Spooned from 26-29 ft.	•	2/2/2/1	4		RLOC @ 26 ft.
	CLAYEY SAND (SC): Pale-brown (10YR 6/3) clayey fine to medium quartz sand Spooned from 29-29.5 ft.	•	2/2/2/3		0	DB from 29-42 ft.
30	LIMESTONE: White (10YR 8/1) limestone Spooned from 29.5-30 ft. CLAYEY SAND (SC): Very loose	0 20 40 60 80100		4		
	Light-brownish-gray (10YR 6/2) Spooned from 30-33 ft.		2/1/1/1	2		
-	LIMESTONE: Moderately strong to strong White (10YR 8/1) limestone Spooned from 33-40 ft.		2/4/10/18	14		
35			5/9/12/6	21		
			6/4/3/5	7		
40	CLAY (CL/CH): Medium stiff to very		5/6/12/6	18		
	stiff Mottled light-greenish-gray (5G 7/1)					



SPT BORING LOG

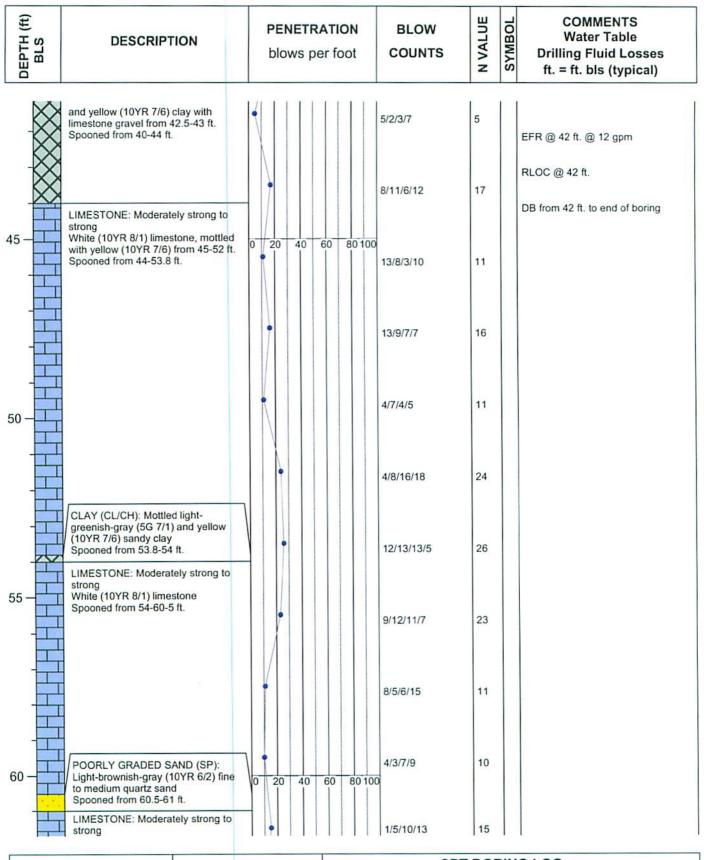
SITE NAME: PCRRF - South Stormwater Pond

SITE LOCATION: Spring Hill, Florida

 DESIGNED BY:
 DR
 PROJECT NO:
 8053622

 CHECKED BY:
 JMW
 BORING DATE:
 11/07/12

 DRAWN BY:
 MVK
 PAGE NO:
 2 of 4



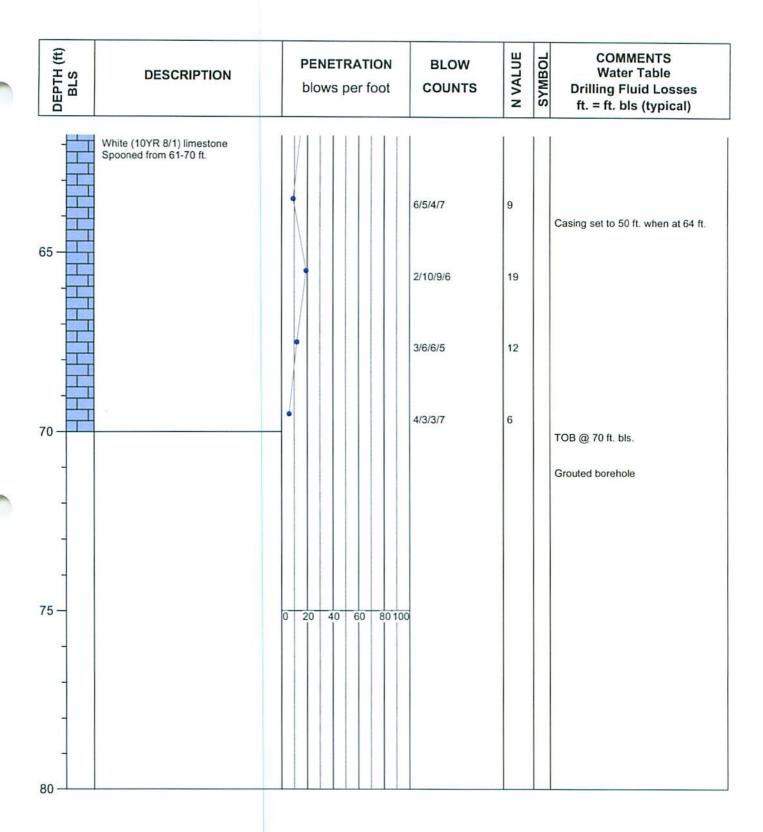


SPT BORING LOG

SITE NAME: PCRRF - South Stormwater Pond SITE LOCATION: Spring Hill, Florida

DESIGNED BY: DR
CHECKED BY: JMW
DRAWN BY: MVK

PROJECT NO: 8053622 BORING DATE: 11/07/12 PAGE NO: 3 of 4 SB-3





SPT BORING LOG

SITE NAME: PCRRF - South Stormwater Pond

SITE LOCATION: Spring Hill, Florida

DESIGNED BY: DR PROJECT
CHECKED BY: JMW BORING
DRAWN BY: MVK PAGE

PROJECT NO: 8053622 BORING DATE: 11/07/12 PAGE NO: 4 of 4 SB-3

POORLY GRADED SAND (SP): Very loose to loose Very-pale-brown (10YR 7/3) fine to medium quartz sand, mottled with light-gray (10YR 7/2) from 0-2 ft. and light-gray (10YR 7/1) from 2-4 ft. Spooned from 0-6 ft. NoEV from 0-4 ft. 2/2/3/4 5 EFR @ 2 ft. @ 12	
5/1/2/2 3 FAS/WOH from 4	4.5-5 ft.
CLAYEY SAND (SC): Very loose Very-pale-brown (10YR 7/3) clayey fine to medium quartz sand Spooned from 6-7.5 ft. 1/1/1/2 2	ft.
CLAY (CL/CH): Mottled light- greenish-gray (5G 7/1) and grayish- brown (10YR 5/2) clay Spooned from 7.5-9 ft. CLAY (CL/CH): Stiff to very stiff Mottled light-greenish-gray (5G 7/1)	
and brownish-yellow (10YR 6/6) sandy clay Spooned from 9-15.5 ft. 4/6/9/7 15	ft. when at 12 ft.
2/6/10/5	
15	n 15-16 ft.
CLAY (CL/CH): Soft to medium stiff Greenish-gray (5G 5/1) clay with	end of boring
limestone gravel and granules, mottled with brownish-yellow (10YR 6/6) from 18-22 ft. Spooned from 18-24 ft.	

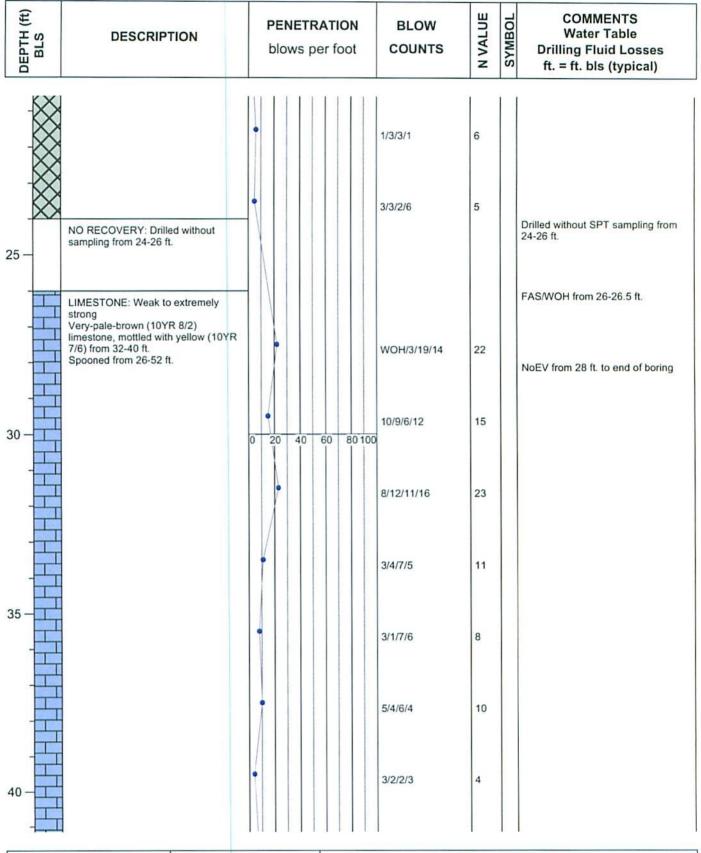


SPT BORING LOG

SITE NAME: PCRRF - South Stormwater Pond SITE LOCATION: Spring Hill, Florida

DESIGNED BY: DR
CHECKED BY: JMW
DRAWN BY: MVK

PROJECT NO: 8053622 BORING DATE: 11/07/12 PAGE NO: 1 of 3





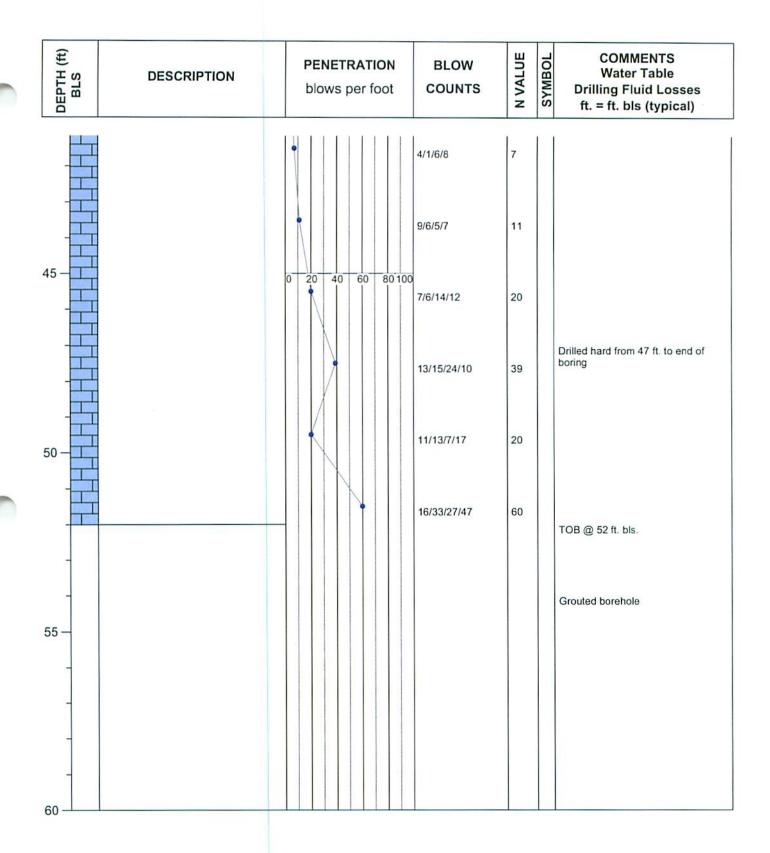
SPT BORING LOG

SITE NAME: PCRRF - South Stormwater Pond

SITE LOCATION: Spring Hill, Florida

DESIGNED BY: DR PROJECT NO: 8053622
CHECKED BY: JMW BORING DATE: 11/07/12
DRAWN BY: MVK PAGE NO: 2 of 3

SB-4





SPT BORING LOG

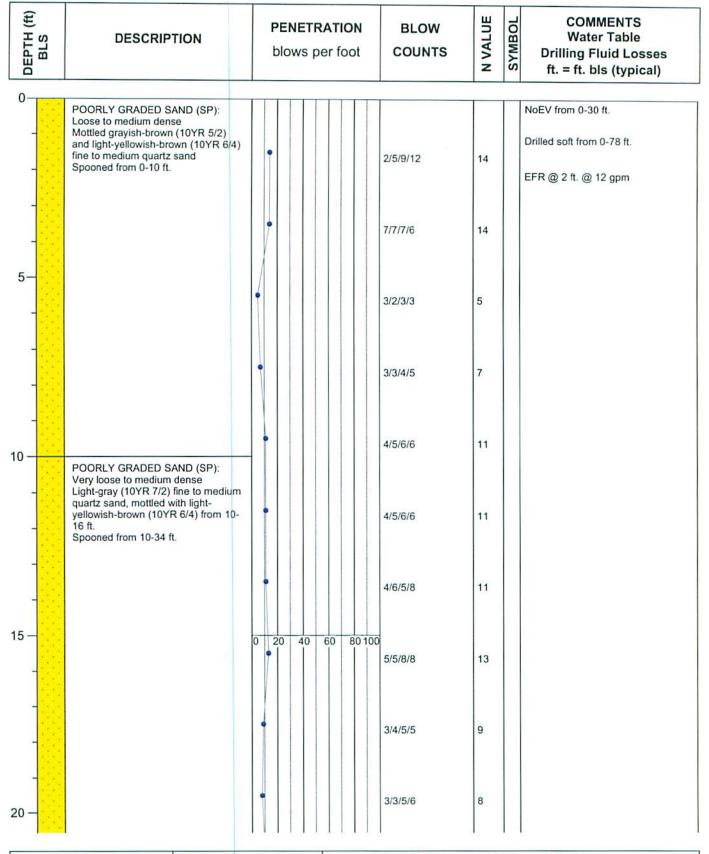
SITE NAME: PCRRF - South Stormwater Pond

SITE LOCATION: Spring Hill, Florida

 DESIGNED BY:
 DR
 PROJECT NO:
 8053622

 CHECKED BY:
 JMW
 BORING DATE:
 11/07/12

 DRAWN BY:
 MVK
 PAGE NO:
 3 of 3



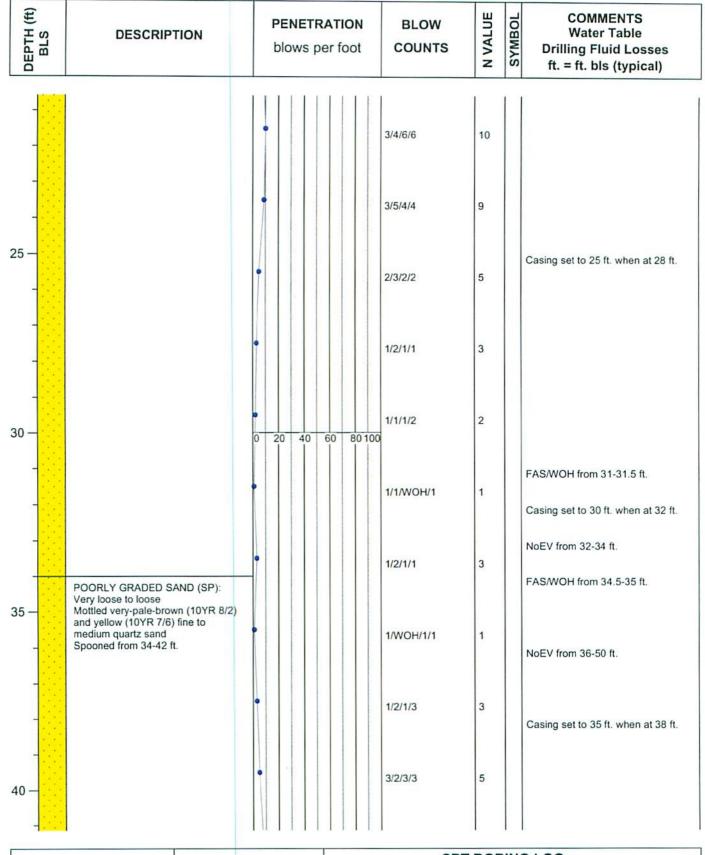


SPT BORING LOG

SITE NAME: PCRRF - South Stormwater Pond

SITE LOCATION: Spring Hill, Florida

DESIGNED BY: DR PROJECT NO: 8053622
CHECKED BY: JMW BORING DATE: 11/08/12
DRAWN BY: MVK PAGE NO: 1 of 4





SPT BORING LOG

SITE NAME: PCRRF - South Stormwater Pond SITE LOCATION: Spring Hill, Florida

DESIGNED BY: DR PROJECT NO: 8053622 CHECKED BY: **JMW BORING DATE: 11/08/12** PAGE NO: 2 of 4 DRAWN BY: MVK

DEPTH (ft) BLS	DESCRIPTION	PENETRATION blows per foot	BLOW	N VALUE	COMMENTS Water Table Drilling Fluid Losses ft. = ft. bls (typical)
	CLAY (CL/CH): Medium stiff to very	•	2/3/5/7	8	Casing set to 40 ft. when at 42 ft.
	stiff Mottled light-gray (10YR 7/1) and brownish-yellow (10YR 6/6) clay Spooned from 42-45.5 ft.	•	3/2/4/4	6	
45	POORLY GRADED SAND (SP): Medium dense Light-gray (10YR 7/1) fine to medium	0 20 40 60 80 100	3/6/12/11	18	
-	quartz sand Spooned from 45.5-48.5 ft. CLAY (CL/CH): Very soft Mottled light-gray (10YR 7/1) and	•	3/5/8/8	13	Casing set to 45 ft. when at 48 ft.
50	yellow (10YR 7/6) sandy clay Spooned from 48.5-49.5 ft. CLAYEY SAND (SC): Mottled light- gray (10YR 7/1) and yellow (10YR 7/6) clayey fine to medium quartz		8/1/1/3	2	
	sand Spooned from 49.5-50 ft. CLAY (CL/CH): Very soft to medium stiff Mottled light-gray (10YR 7/1) and		1/1/WOH/WOH	1	FAS/WOH from 51-52 ft. Casing set to 50 ft. when at 52 ft.
	yellow (10YR 7/6) sandy clay Spooned from 50-54 ft.		2/2/3/3	5	NoEV from 52-54 ft. RLOC @ 54 ft.
55 -	Light-gray (10YR 7/1) clay Spooned from 54-62 ft.		WOR/WOR/WOR/ WOR	WOR	DB from 54-58 ft. FAS/WOR from 54-56 ft.
			WOR/WOR/WOR/ WOR	WOR	FAS/WOR from 56-58 ft. Casing set to 55 ft. when at 58 ft.
60 -		0 20 40 60 8010	WOR/WOR/WOR/ WOR	WOR	EFR @ 58 ft. @ 12 gpm FAS/WOR from 58-60 ft.
$\stackrel{\sim}{\otimes}$			wor/wor/wor/	WOR	FAS/WOR from 60-62 ft.
			SPT	BOR	ING LOG



SPI BURING LUG

SITE NAME: PCRRF - South Stormwater Pond SITE LOCATION: Spring Hill, Florida

DESIGNED BY: DR PROJECT NO: 8053622
CHECKED BY: JMW BORING DATE: 11/08/12

HECKED BY: JMW BORING DATE: 11/08/12
DRAWN BY: MVK PAGE NO: 3 of 4

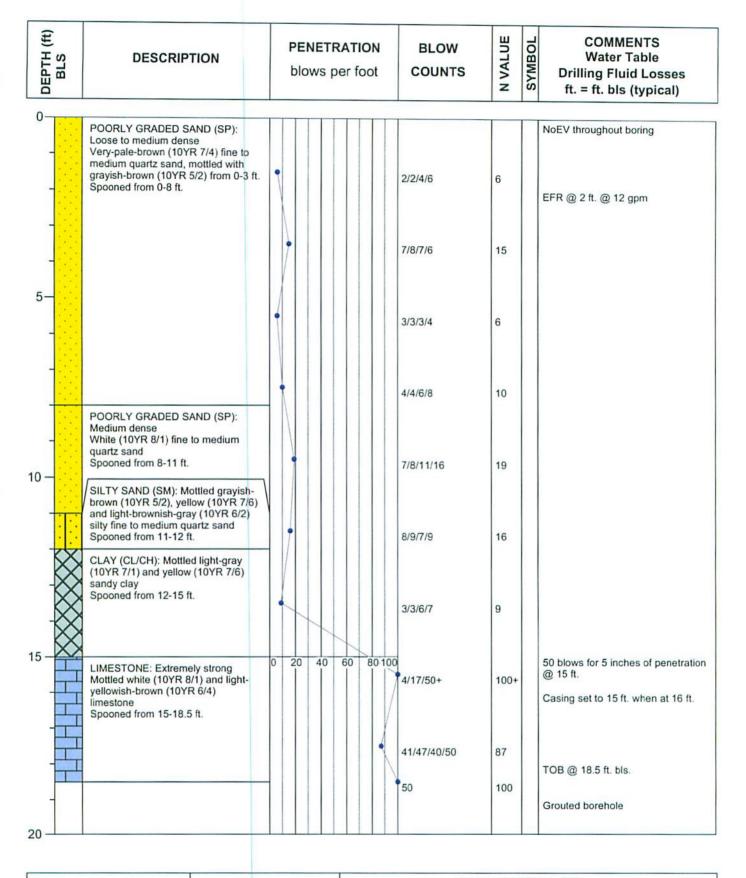
DEPTH (ft) BLS	DESCRIPTION	PENETRATION blows per foot	BLOW	N VALUE	SYMBOL	COMMENTS Water Table Drilling Fluid Losses ft. = ft. bls (typical)
	CLAY (CL/CH): Very soft Mottled light-gray (10YR 7/1) and yellow (10YR 7/6) sandy clay Spooned from 62-64 ft.		WOR/WOR/WOR/ WOR	WOR		Casing set to 60 ft. when at 62 ft. FAS/WOR from 62-64 ft. FAS/WOR from 64-66 ft.
65 –	CLAY (CL/CH): Very soft Mottled light-gray (10YR 7/1) and yellow (10YR 7/6) clay Spooned from 64-66.5 ft. POORLY GRADED SAND (SP): Very loose Light-gray (10YR 7/2) fine to medium quartz sand		WOR/WOR/WOR/ WOR	wor		FAS/WOR from 66-68 ft.
	CLAY (CL/CH): Mottled light-gray (10YR 7/1) and yellow (10YR 7/6) clay		WORWOR/WOR/ WOR	WOR		RLOC @ 68 ft.
70 -	Spooned from 67.5-68 ft. CLAY (CL/CH): Very soft Light-gray (10YR 7/2) sandy clay Spooned from 68-75 ft.		WOR/WOR/WOR/ WOR	wor		DB from 68-78 ft. FAS/WOR from 68-70 ft.
			WOR/WOR/WOR/ WOR	WOR		FAS/WOR from 70-75 ft.
75	POORLY GRADED SAND (SP): Mottled light-gray (10YR 7/2) and yellow (10YR 7/6) fine to medium quartz sand Spooned from 75-76.2 ft.	0 20 40 60 80100				FAS/WOR from 75-76 ft.
	LIMESTONE: Moderately strong to strong Light-gray (10YR 7/2) limestone Spooned from 76.2-81 ft.	•	WOR/WOR/7/6	7		NoEV from 77 ft. to end of boring
			2/3/3/3	6		EFR @ 78 ft. @ 12 gpm
80			4/6/13/10	19		TOB @ 81 ft. bls.
						Grouted borehole



SPT BORING LOG

SITE NAME: PCRRF - South Stormwater Pond SITE LOCATION: Spring Hill, Florida

DESIGNED BY: DR PROJECT NO: 8053622 CHECKED BY: JMW BORING DATE: 11/08/12 DRAWN BY: MVK PAGE NO: 4 of 4



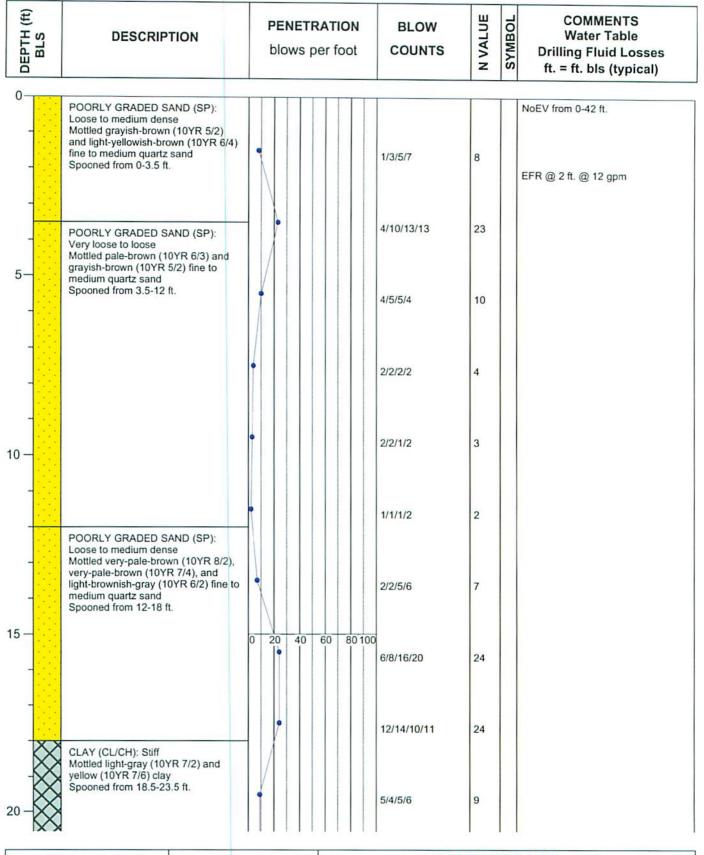


SPT BORING LOG

SITE NAME: PCRRF - South Stormwater Pond

SITE LOCATION: Spring Hill, Florida

DESIGNED BY: PROJECT NO: 8053622 CHECKED BY: **JMW BORING DATE: 11/09/12** DRAWN BY: MVK PAGE NO: 1 of 1





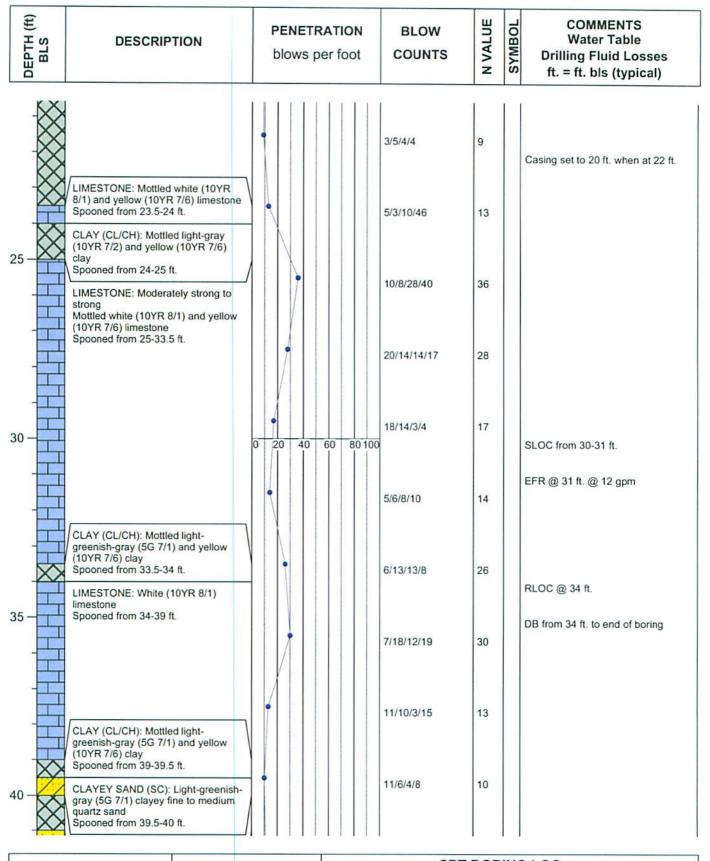
SPT BORING LOG

SITE NAME: PCRRF - South Stormwater Pond

SITE LOCATION: Spring Hill, Florida

DESIGNED BY: DR PROJECT NO: 8053622
CHECKED BY: JMW BORING DATE: 11/09/12
DRAWN BY: MVK PAGE NO: 1 of 3

53622 BORING /09/12 SB-7





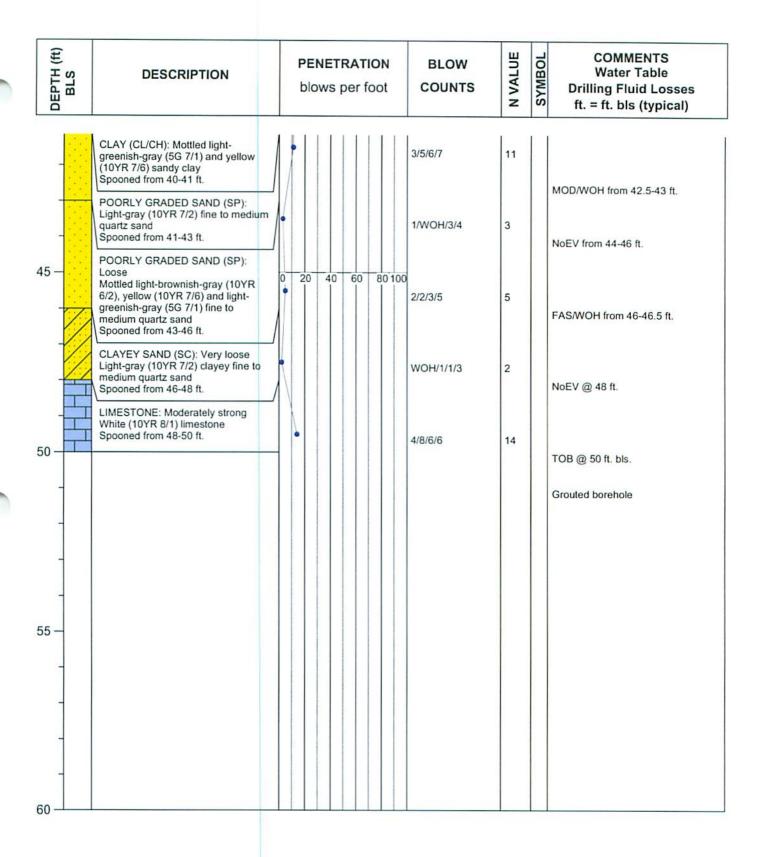
SPT BORING LOG

SITE NAME: PCRRF - South Stormwater Pond

SITE LOCATION: Spring Hill, Florida

DESIGNED BY: CHECKED BY: **JMW** DRAWN BY: MVK

PROJECT NO: 8053622 **BORING DATE: 11/09/12** PAGE NO: 2 of 3





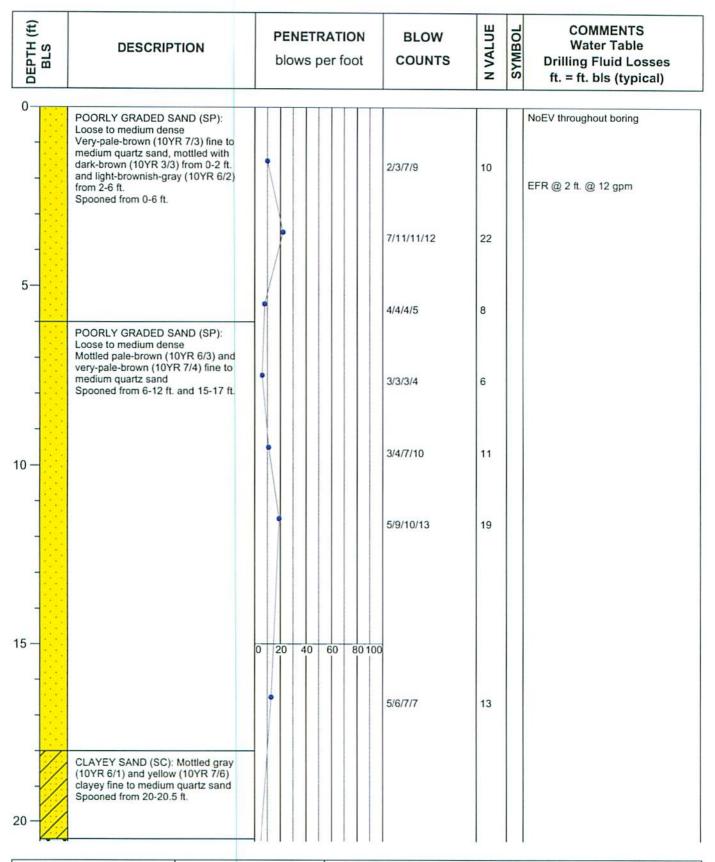
SPT BORING LOG

SITE NAME: PCRRF - South Stormwater Pond

SITE LOCATION: Spring Hill, Florida

DESIGNED BY: CHECKED BY: **JMW** DRAWN BY: MVK

PROJECT NO: 8053622 **BORING DATE: 11/09/12** PAGE NO: 3 of 3





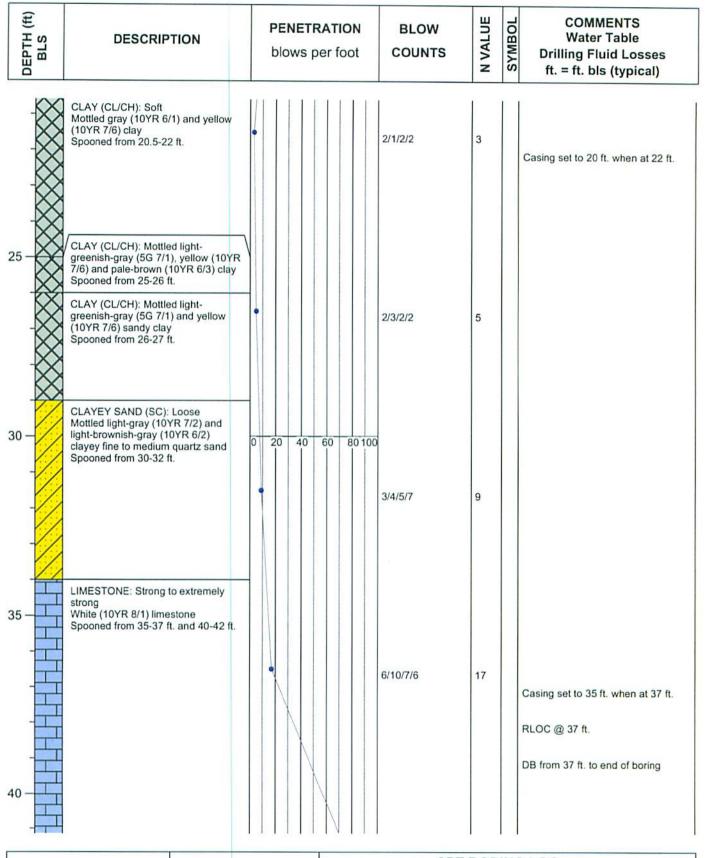
SPT BORING LOG

SITE LOCATION: Spring Hill Florida

SITE LOCATION: Spring Hill, Florida

DESIGNED BY: DR
CHECKED BY: JMW
DRAWN BY: MVK

PROJECT NO: 8053622 BORING DATE: 11/09/12 PAGE NO: 1 of 3





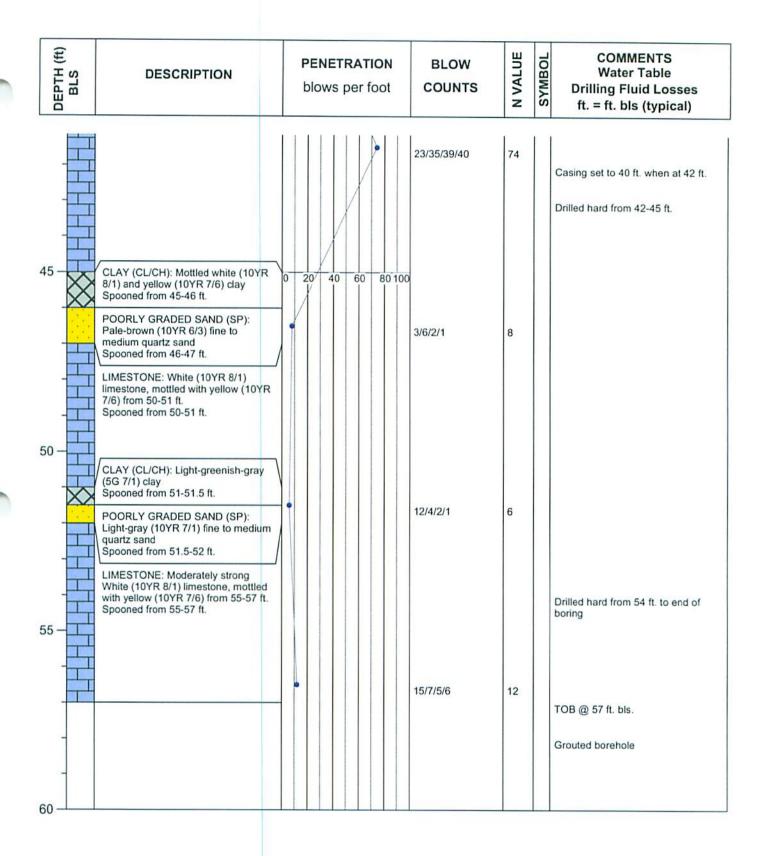
SPT BORING LOG

SITE NAME: PCRRF - South Stormwater Pond

SITE LOCATION: Spring Hill, Florida

DESIGNED BY: DR PR
CHECKED BY: JMW BOTO
DRAWN BY: MVK

PROJECT NO: 8053622 BORING DATE: 11/09/12 PAGE NO: 2 of 3





SPT BORING LOG

SITE NAME: PCRRF - South Stormwater Pond

SITE LOCATION: Spring Hill, Florida

DESIGNED BY: DR PROJECT NO: 8053622
CHECKED BY: JMW BORING DATE: 11/09/12
DRAWN BY: MVK PAGE NO: 3 of 3

BORING

DEPTH (ft) BLS	DESCRIPTION	PENETRATION blows per foot	BLOW	N VALUE	SYMBOL	COMMENTS Water Table Drilling Fluid Losses ft. = ft. bls (typical)
0						
- -	POORLY GRADED SAND (SP): Loose Pale-brown (10YR 6/3) fine to medium quartz sand, mottled with brown (10YR 5/3) from 2-4 ft. Spooned from 0-4 ft.		2/2/5/4	7		NoEV from 0-30 ft. EFR @ 2 ft. @ 12 gpm
	POORLY GRADED SAND (SP): Very loose to medium dense	-	2/2/3/3	5		
5	Grayish-brown (10YR 5/2) fine to medium quartz sand Spooned from 4-8 ft.		2/1/1/2	2		
			2/3/9/9	12		
10 -	POORLY GRADED SAND (SP): Loose Mottled light-brownish-gray (10YR 6/2) and light-gray (10YR 7/2) fine to medium quartz sand Spooned from 8-12 ft.		4/5/4/5	9		
-		• 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1	3/3/2/3	5		
15 —	SILTY SAND (SM): Medium dense Pale-brown (10YR 6/3) silty fine to medium quartz sand Spooned from 15-17 ft.	0 20 40 60 80100	4/4/7/8	11		
20	POORLY GRADED SAND (SP):	-				
					8	

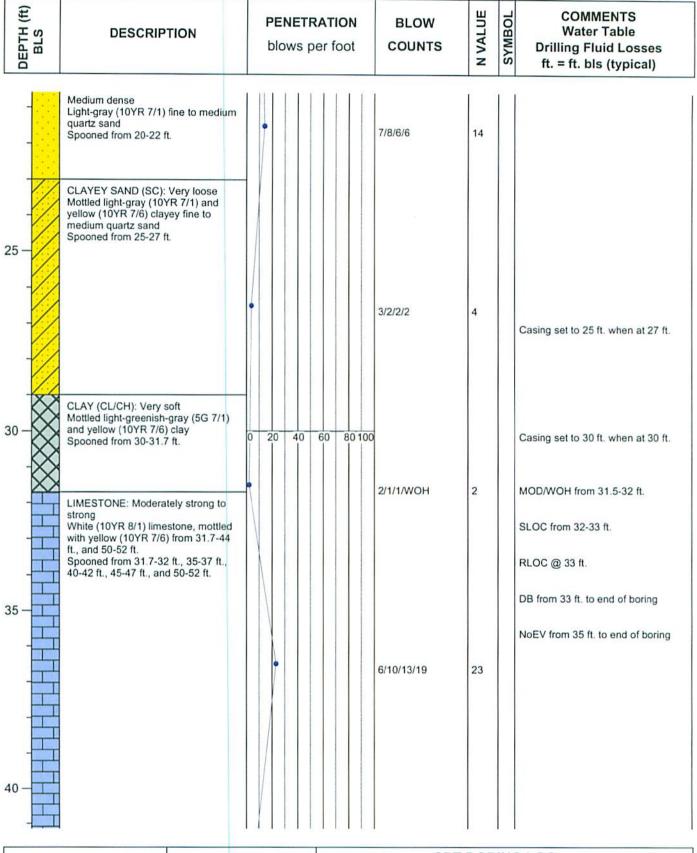


SPT BORING LOG

SITE NAME: PCRRF - South Stormwater Pond SITE LOCATION: Spring Hill, Florida

DESIGNED BY: CHECKED BY: JMW DRAWN BY: MVK

PROJECT NO: 8053622 BORING DATE: 11/12/12 PAGE NO: 1 of 3





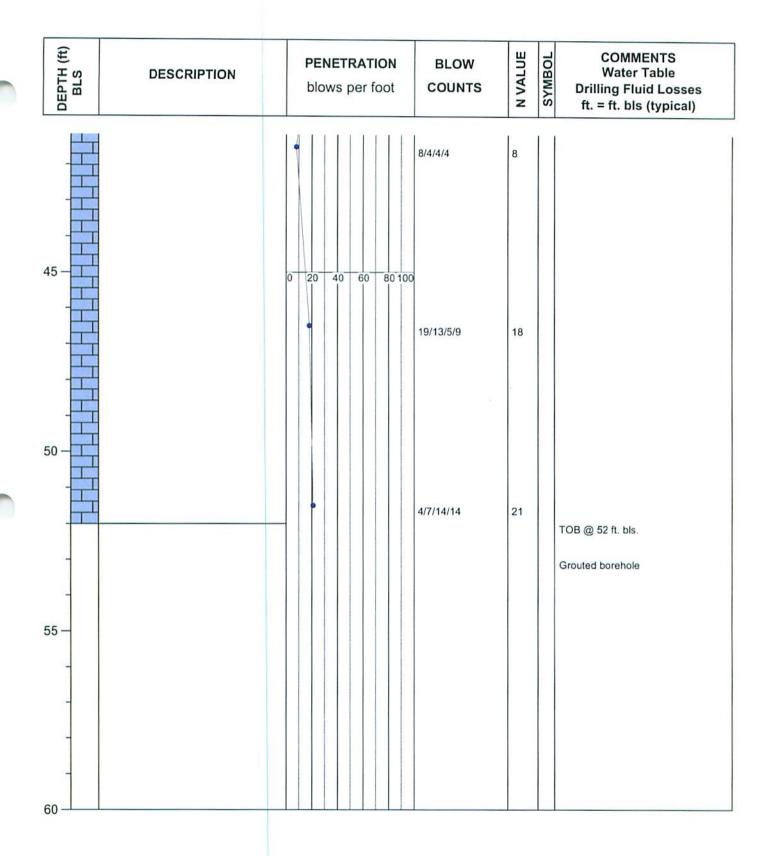
SPT BORING LOG

SITE LOCATION: Spring Hill Florida

SITE LOCATION: Spring Hill, Florida

DESIGNED BY: DR PROJE
CHECKED BY: JMW BORING
DRAWN BY: MVK PA

PROJECT NO: 8053622 BORING DATE: 11/12/12 PAGE NO: 2 of 3





SPT BORING LOG

SITE NAME: PCRRF - South Stormwater Pond

SITE LOCATION: Spring Hill, Florida

DESIGNED BY: DR
CHECKED BY: JMW
DRAWN BY: MVK

PROJECT NO: 8053622 BORING DATE: 11/12/12 PAGE NO: 3 of 3

DEPTH (ft) BLS	DESCRIPTION	PENETRATION blows per foot	BLOW	N VALUE	SYMBOL	COMMENTS Water Table Drilling Fluid Losses ft. = ft. bls (typical)
	4			-	_	ACTION III PO
0	POORLY GRADED SAND (SP): Medium dense Very-pale-brown (10YR 7/3) fine to medium quartz sand Spooned from 0-2 ft.		2/5/6/7	11		NoEV from 0-22 ft. EFR @ 0 ft. @ 12 gpm
5—	POORLY GRADED SAND (SP): Loose Light-brownish-gray (10YR 6/2) fine to medium quartz sand, mottled with yellow (10YR 7/6) from 2-4 ft. and very-pale-brown (10YR 7/3) from 4-8 ft. Spooned from 2-8 ft.		4/4/4/6	8		
3-			3/3/4/4	7		
	POORLY GRADED SAND (SP):	-	3/4/4/6	8		
10 -	Medium dense Mottled light-gray (10YR 7/2) and light-yellowish-brown (10YR 6/4) fine to medium quartz sand Spooned from 8-11.5 ft.		6/8/10/14	18		
	CLAY (CL/CH): Mottled light- greenish-gray (5G 7/1) and light- yellowish-brown (10YR 6/4) clay Spooned from 11.5-12 ft.		7/6/5/7	11		Casing set to 10 ft. when at 12 ft.
15 - 2						Drilled hard from 12-22 ft.
	CLAY (CL/CH): Medium stiff Mottled very-pale-brown (10YR 7/3) and yellow (10YR 7/6) sandy clay Spooned from 15-17 ft.	0 20 40 60 80100	5/4/4/4	8		Coping set to 15 ft, when at 17 ft
20	CLAY (CL/CH): Mottled light- greenish-gray (5G 7/1) and yellow (10YR 7/6) clay					Casing set to 15 ft. when at 17 ft.



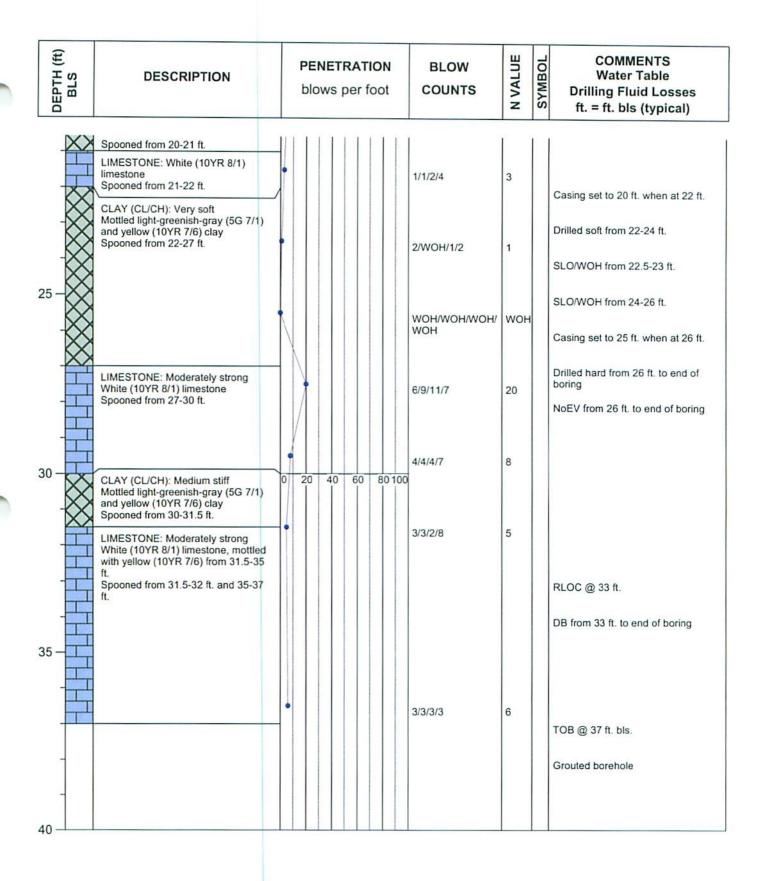
SPT BORING LOG

SITE NAME: PCRRF - South Stormwater Pond

SITE LOCATION: Spring Hill, Florida

DESIGNED BY: DR CHECKED BY: **JMW** DRAWN BY: MVK

PROJECT NO: 8053622 BORING DATE: 12/05/12 PAGE NO: 1 of 2





SPT BORING LOG

SITE NAME: PCRRF - South Stormwater Pond

SITE LOCATION: Spring Hill, Florida

DESIGNED BY: DR
CHECKED BY: JMW
DRAWN BY: MVK

PROJECT NO: 8053622
BORING DATE: 12/05/12
PAGE NO: 2 of 2

DEPTH (ft) BLS	DESCRIPTION	PENETRATION blows per foot	BLOW	N VALUE	COMMENTS Water Table Drilling Fluid Losses ft. = ft. bls (typical)
0	POORLY GRADED SAND (SP): Very loose Light-brownish-gray (10YR 6/2) fine to medium quartz sand Spooned from 0-2 ft. POORLY GRADED SAND (SP): Very loose to medium dense		2/2/2/2	4	NoEV from 0-20 ft. EFR @ 2 ft. @ 12 gpm
-	Mottled very-pale-brown (10YR 7/3) and yellow (10YR 7/6) fine to medium quartz sand Spooned from 2-11.9 ft.		2/1/1/1	2	
5—			1/1/1/WOH	2	
			1/2/2/2	4	
10 —			2/3/4/7	7	
	CLAY (CL/CH): Light-greenish-gray (5G 7/1) sandy clay Spooned from 11.9-12 ft.		7/9/6/6	15	
15	CLAY (CL/CH): Very soft to stiff Mottled ight-greenish-gray (5G 7/1) and yellow (10YR 7/6) clay Spooned from 15-17 ft., and 20-22 ft.	0 20 40 60 80100	6/5/5/5	10	Casing set to 15 ft. when at 17 ft.
20-					SLOC from 19-22 ft. MOD/WOH from 20-20.5 ft.



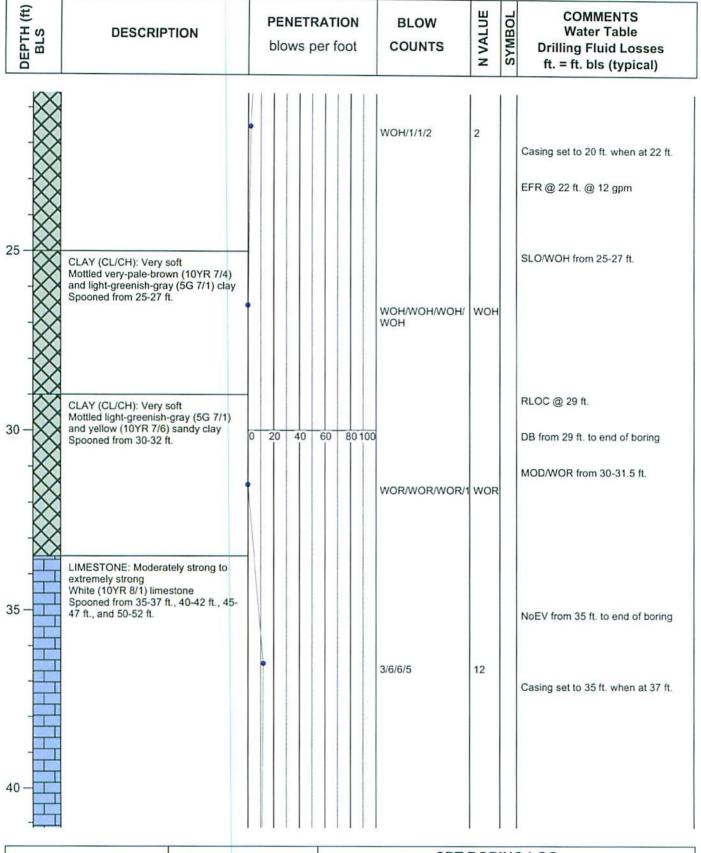
SPT BORING LOG

SITE NAME: PCRRF - South Stormwater Pond

SITE LOCATION: Spring Hill, Florida

DESIGNED BY: DR PROJECTION PROJEC

PROJECT NO: 8053622 BORING DATE: 11/12/12 PAGE NO: 1 of 3





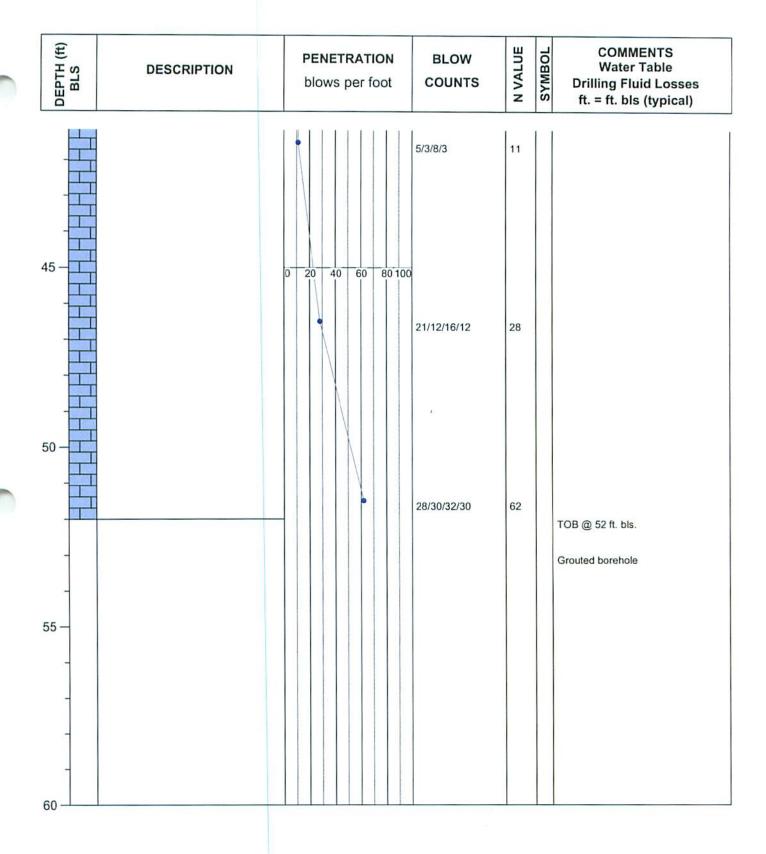
SPT BORING LOG

SITE NAME: PCRRF - South Stormwater Pond

SITE LOCATION: Spring Hill, Florida

DESIGNED BY: CHECKED BY: JMW DRAWN BY: MVK

PROJECT NO: 8053622 **BORING DATE: 11/12/12** PAGE NO: 2 of 3





SPT BORING LOG

SITE NAME: PCRRF - South Stormwater Pond

SITE LOCATION: Spring Hill, Florida

DESIGNED BY: DR
CHECKED BY: JMW
DRAWN BY: MVK

PROJECT NO: 8053622 BORING DATE: 11/12/12 PAGE NO: 3 of 3

DEPTH (ft) BLS	DESCRIPTION	PENETRATION blows per foot					BLOW	N VALUE	SYMBOL	COMMENTS Water Table Drilling Fluid Losses ft. = ft. bls (typical)
0									_	
	SILTY SAND (SM): Loose Mottled light-brownish-gray (10YR 6/2), brown (10YR 5/3), brown (10YI 4/3) and yellow (10YR 7/6) silty fine to medium quartz sand Spooned from 0-2 ft.	R	•				3/5/4/5	9		NoEV from 0-20 ft. EFR @ 2 ft. @ 12 gpm
	CLAYEY SAND (SC): Medium dens Mottled light-brownish-gray (10YR 6/2) and light-gray (10YR 7/1) clayer fine to medium quartz sand Spooned from 2-4 ft.					111	4/7/11/9	18		a.v. @ 2 tt @ v2 gpm
5-	POORLY GRADED SAND (SP): Medium dense Mottled very-pale-brown (10YR 7/3) and light-gray (10YR 7/1) fine to medium quartz sand Spooned from 4-6 ft.		•				4/6/7/7	13		
	CLAY (CL/CH): Stiff Mottled very-pale-brown (10YR 7/3) and yellow (10YR 7/6) sandy clay Spooned from 6-7.5 ft.						4/6/6/8	12		
10	POORLY GRADED SAND (SP): Mottled very-pale-brown (10YR 7/3), light-gray (10YR 7/1) and yellow (10YR 7/6) fine to medium quartz sand Spooned from 7.5-8 ft.	,					3/4/4/4	8		
	SILTY SAND (SM): Loose Mottled light-gray (10YR 7/2), brown (10YR 5/3) and dark-grayish-brown (10YR 4/2) silty fine to medium quartz sand Spooned from 8-10 ft.						3/3/3/4	6		
	CLAY (CL/CH): Medium stiff Mottled light-greenish-gray (5G 7/1), yellow (10YR 7/6) and dark-brown (10YR 3/3) clay Spooned from 10-12 ft.									RLOC @ 14 ft.
15 —	CLAY (CL/CH): Soft Light-gray (10YR 7/2) clay with limestone gravel	C	2	20	40 6	0 80100	į			DB from 14-15 ft.
\otimes	Spooned from 15-17 ft.						3/1/2/2	3		EFR @ 15 ft. @ 12 gpm
$\stackrel{\cdot}{\otimes}$							processor forms.—			RLOC @ 16 ft.
\otimes										DB from 16-20 ft.
. 💢	CLAY (CL/CH): Very soft Mottled light-greenish-gray (5G 7/1) and yellow (10YR 7/6) clay with									Casing set to 15 ft. when at 17 ft.
20	limestone gravel									Casing set to 20 ft. when at 20 ft.
			_					000		

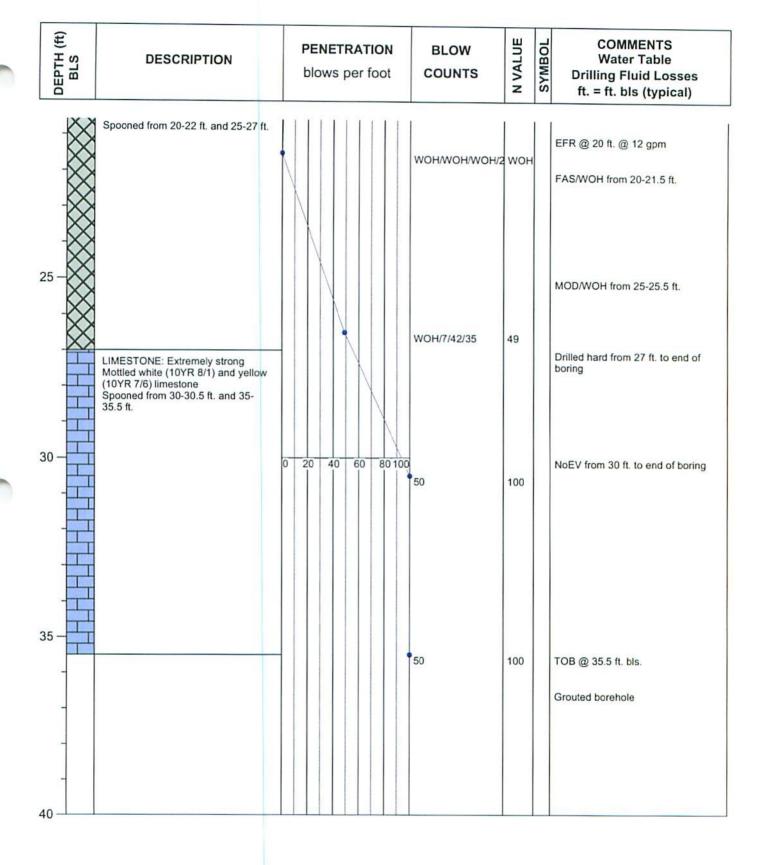


SPT BORING LOG

SITE NAME: PCRRF - South Stormwater Pond SITE LOCATION: Spring Hill, Florida

DESIGNED BY: CHECKED BY: JMW DRAWN BY:

PROJECT NO: 8053622 **BORING DATE: 11/12/12** PAGE NO: 1 of 2





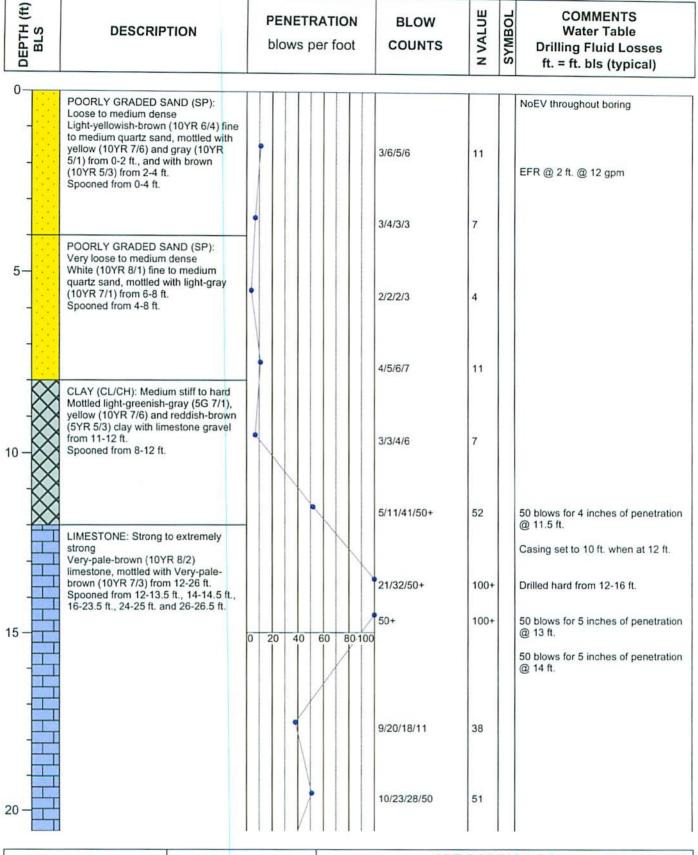
SPT BORING LOG

SITE NAME: PCRRF - South Stormwater Pond

SITE LOCATION: Spring Hill, Florida

DESIGNED BY: CHECKED BY: **JMW** DRAWN BY:

PROJECT NO: 8053622 **BORING DATE: 11/12/12** PAGE NO: 2 of 2



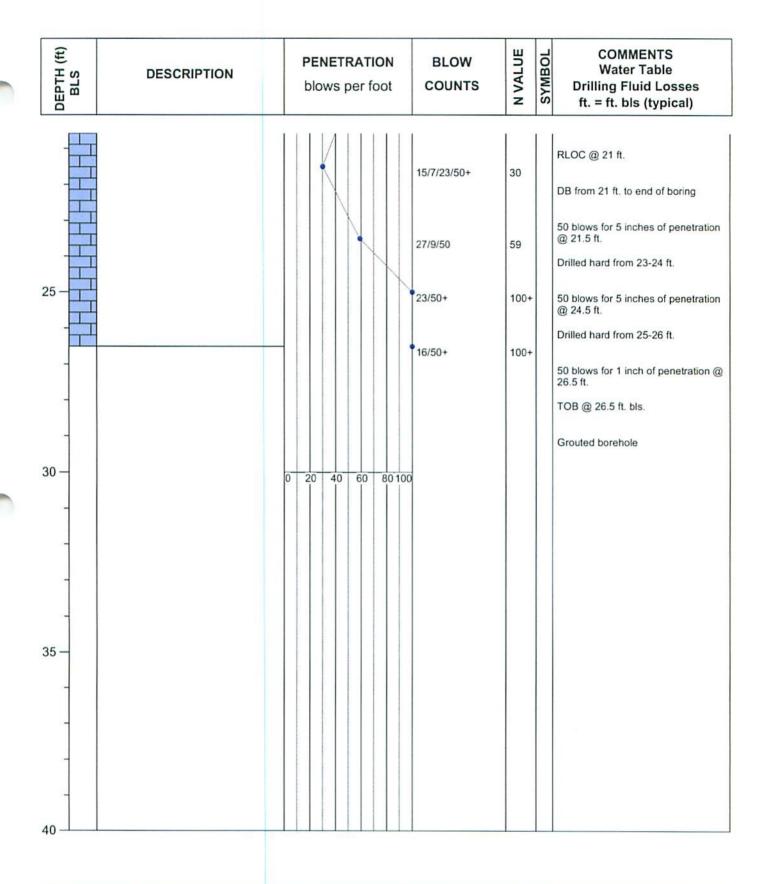


SPT BORING LOG

SITE NAME: PCRRF - South Stormwater Pond SITE LOCATION: Spring Hill, Florida

DESIGNED BY DB

DESIGNED BY: DR PROJECT NO: 8053622
CHECKED BY: JMW BORING DATE: 11/13/12
DRAWN BY: MVK PAGE NO: 1 of 2





SPT BORING LOG

SITE NAME: PCRRF - South Stormwater Pond

SITE LOCATION: Spring Hill, Florida

DESIGNED BY: CHECKED BY: JMW DRAWN BY: MVK

PROJECT NO: 8053622 **BORING DATE: 11/13/12** PAGE NO: 2 of 2

DEPTH (ft) BLS	DESCRIPTION	PENETRATION blows per foot	BLOW	N VALUE	COMMENTS Water Table Drilling Fluid Losses ft. = ft. bls (typical)
0	POORLY GRADED SAND (SP): Mottled brownish-yellow (10YR 6/6), yellow (10YR 7/6) and grayish-brown (10YR 5/2) fine to medium quartz sand Spooned from 0-1 ft.		2/2/3/4	5	NoEV throughout boring EFR @ 2 ft. @ 12 gpm
	CLAY (CL/CH): Medium stiff Mottled light-greenish-gray (5G 7/1) and yellow (10YR 7/6) clay Spooned from 1-6 ft.		3/3/3/6	6	2 к. д. 12 дрн
5-	CLAY (CL/CH): Mottled light-gray (10YR 7/2), yellow (10YR 7/6) and light-greenish-gray (5G 7/1) clay Spooned from 6-7 ft.		2/2/4/7	6	RLOC @ 6 ft.
- 1	LIMESTONE: Weak Very-pale-brown (10YR 8/2) limestone Spooned from 7-10 ft.		1/4/24/7	28	DB from 6-12 ft.
10	CLAY (CL/CH): Medium stiff Mottled light-greenish-gray (5G 7/1)		3/3/1/1	4	
	and yellow (10YR 7/6) clay Spooned from 10-12 ft.		2/3/3/4	6	Casing set to 10 ft. when at 12 ft.
	LIMESTONE: Very strong to extremely strong Mottled very-pale-brown (10YR 8/2) and yellow (10YR 7/6) limestone Spooned from 15-17 ft., @ 20 ft. and				EFR @ 12 ft. @ 12 gpm Drilled hard from 13-15 ft.
15 —	from 25-27 ft.	0 20 40 60 8010	00		SLOC from 14.5-17 ft. 50 blows for 5 inches of penetration
			9/5/16/50+	21	@ 16.5 ft. Casing set to to 15 ft. when at 17 ft. EFR @ 17 ft. @ 12 gpm
20 —			50.	100	Drilled hard from 17-19.5 ft.
			J ⁵⁰⁺	100+	RLOC @ 17.5 ft.

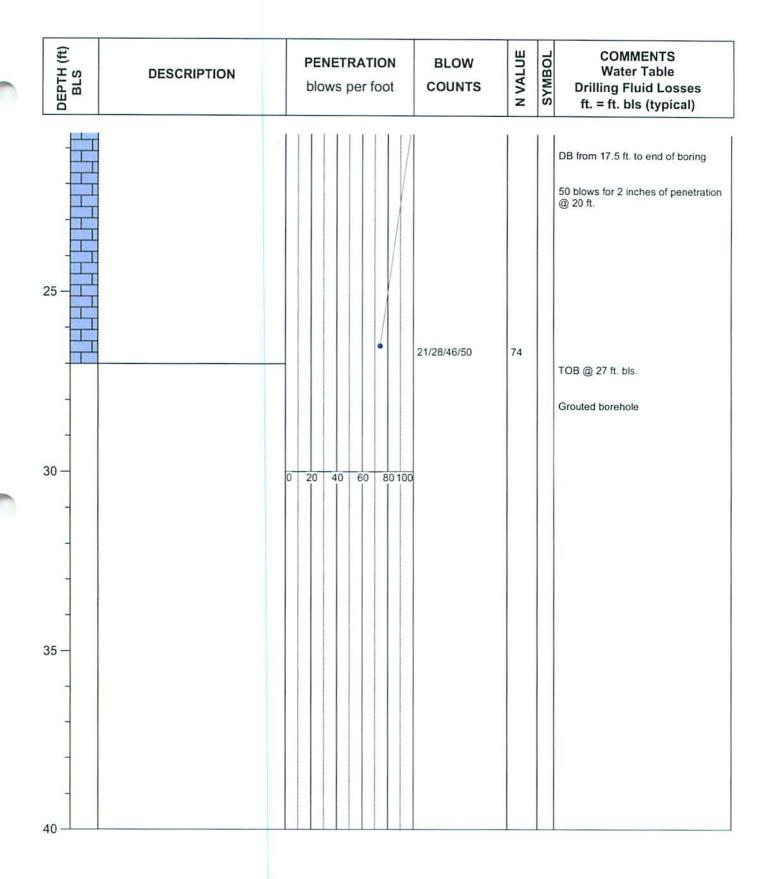


SPT BORING LOG

SITE NAME: PCRRF - South Stormwater Pond SITE LOCATION: Spring Hill, Florida

DESIGNED BY: DR PROJECT NO: 8053622

CHECKED BY: JMW BORING DATE: 11/13/12
DRAWN BY: MVK PAGE NO: 1 of 2





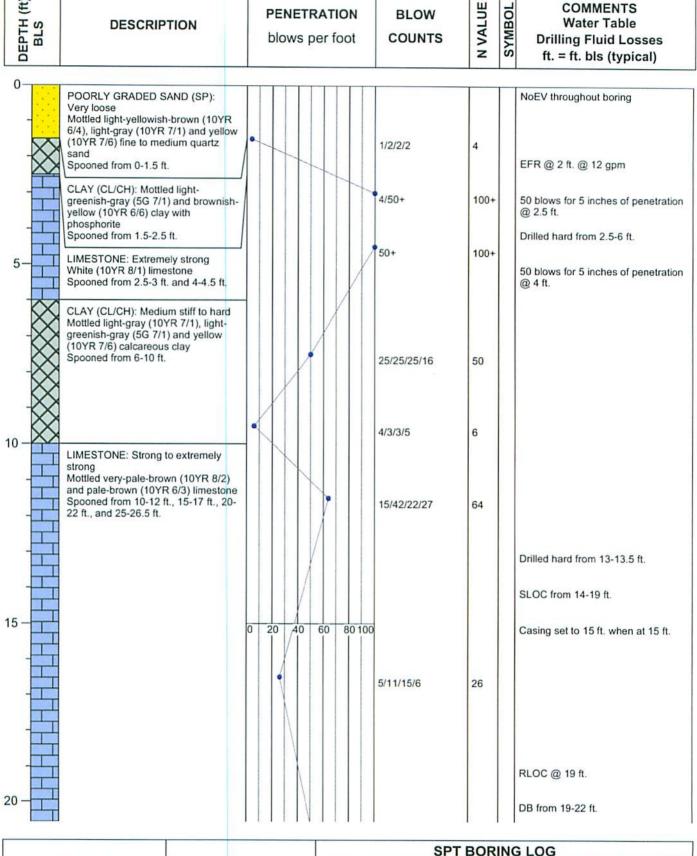
SPT BORING LOG

SITE NAME: PCRRF - South Stormwater Pond

SITE LOCATION: Spring Hill, Florida

DESIGNED BY: DR
CHECKED BY: JMW
DRAWN BY: MVK

PROJECT NO: 8053622 BORING DATE: 11/13/12 PAGE NO: 2 of 2



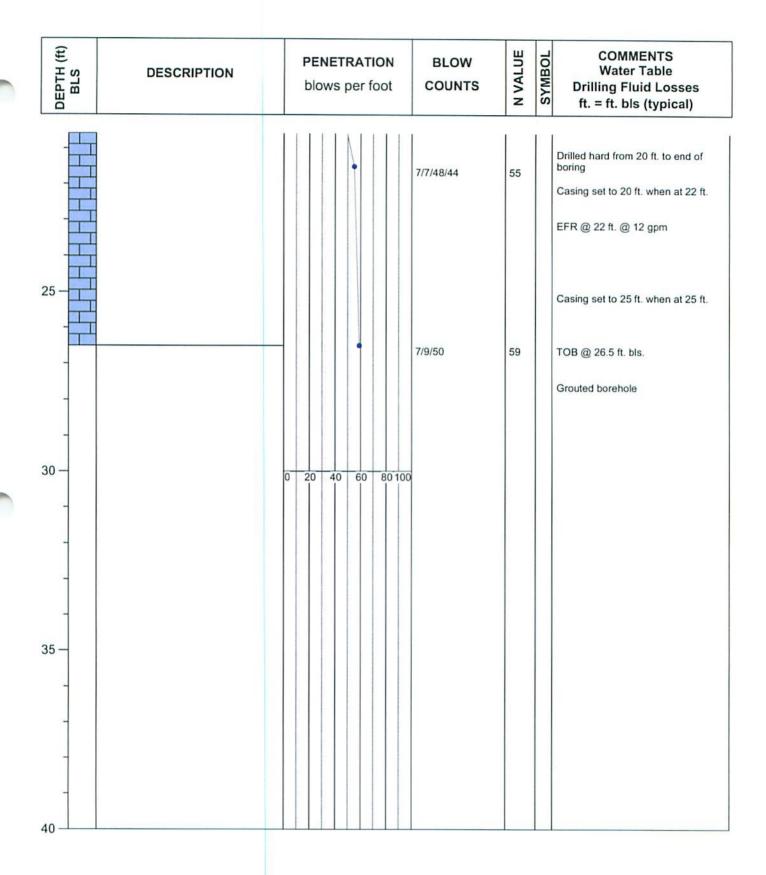


SITE NAME: PCRRF - South Stormwater Pond

SITE LOCATION: Spring Hill, Florida

DESIGNED BY: CHECKED BY: **JMW** DRAWN BY: MVK

PROJECT NO: 8053622 **BORING DATE: 11/14/12** PAGE NO: 1 of 2





SPT BORING LOG

SITE NAME: PCRRF - South Stormwater Pond

SITE LOCATION: Spring Hill, Florida

DESIGNED BY: DR
CHECKED BY: JMW
DRAWN BY: MVK

PROJECT NO: 8053622 BORING DATE: 11/14/12 PAGE NO: 2 of 2

DEPTH (ft) BLS	DESCRIPTION	PENETRATION blows per foot	BLOW	N VALUE	SYMBOL	COMMENTS Water Table Drilling Fluid Losses ft. = ft. bls (typical)
0	POORLY GRADED SAND (SP): Loose to medium dense Mottled very-pale-brown (10YR 7/4) and yellow (10YR 7/6) fine to medium quartz sand Spooned from 0-6 ft.		2/4/7/7	11		NoEV from 0-30 ft. EFR @ 2 ft. @ 12gpm
5-			6/6/7/7	13		
	SILTY SAND (SM): Medium dense Mottled yellowish-brown (10YR 5/4) and dark-brown (10YR 3/3)) silty fine to medium quartz sand Spooned from 6-8 ft. POORLY GRADED SAND (SP): Medium dense		5/5/7/10	12		
10 -	Mottled very-pale-brown (10YR 7/4) and grayish-brown (10YR 5/2) fine to medium quartz sand Spooned from 8-12 ft.		6/10/11/14	21		
-			6/6/8/10	14		
15 -	CLAY (CL/CH): Light-gray (10YR 7/2) sandy clay Spooned from 15-15.5 ft. POORLY GRADED SAND (SP): Medium dense White (10YR 8/1) fine to medium quartz sand Spooned from 15.5-17 and 20-22 ft.	0 20 40 60 80 100	7/9/15/18	24		
20 -						



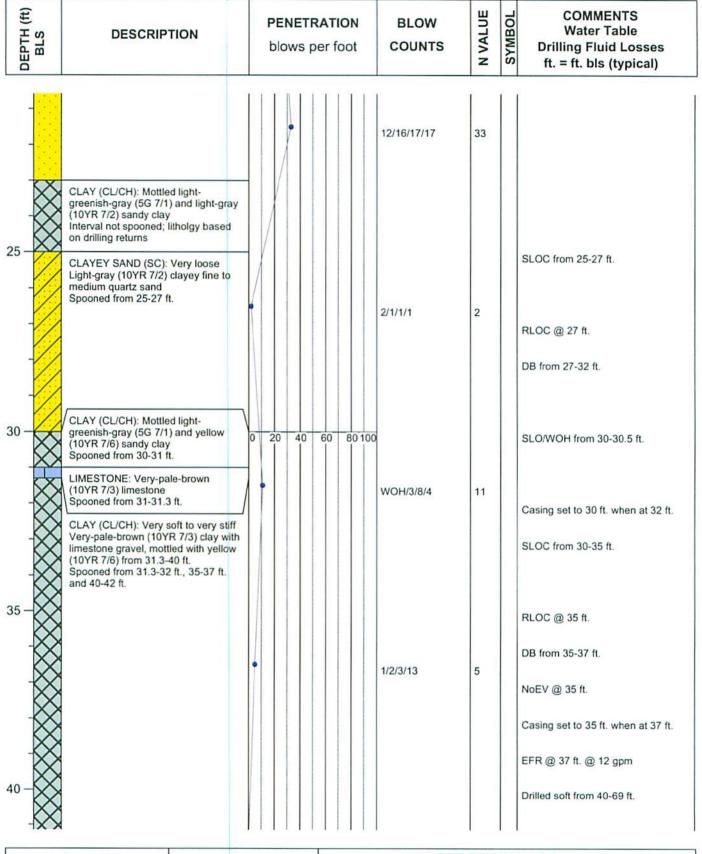
SPT BORING LOG

SITE NAME: PCRRF - South Stormwater Pond

SITE LOCATION: Spring Hill, Florida

DESIGNED BY: CHECKED BY: **JMW** DRAWN BY: MVK

PROJECT NO: 8053622 BORING DATE: 11/14/12 PAGE NO: 1 of 5





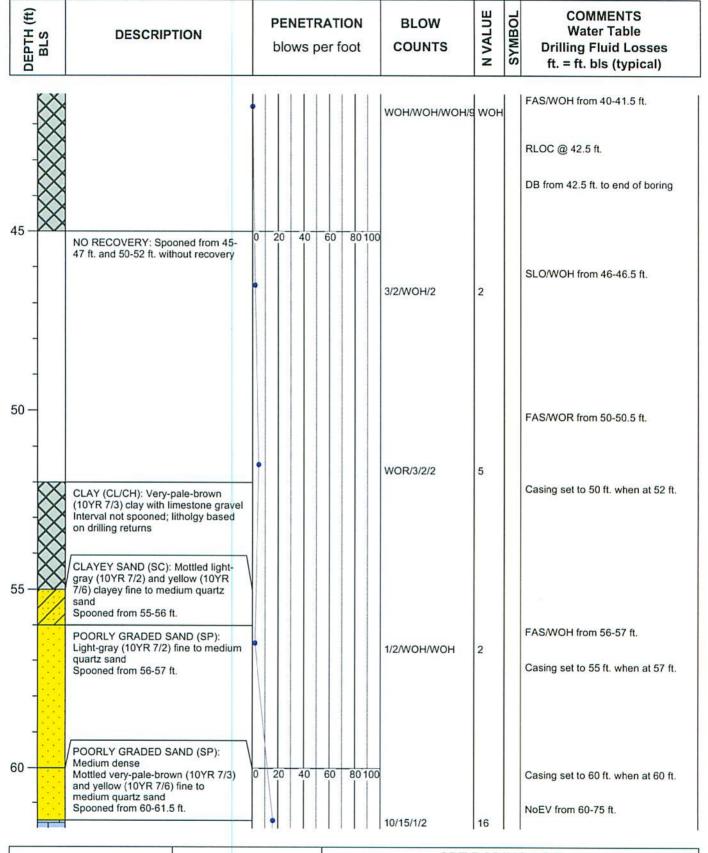
SPT BORING LOG

SITE NAME: PCRRF - South Stormwater Pond

SITE LOCATION: Spring Hill, Florida

DESIGNED BY: DR PR
CHECKED BY: JMW BC
DRAWN BY: MVK

PROJECT NO: 8053622
BORING DATE: 11/14/12
PAGE NO: 2 of 5





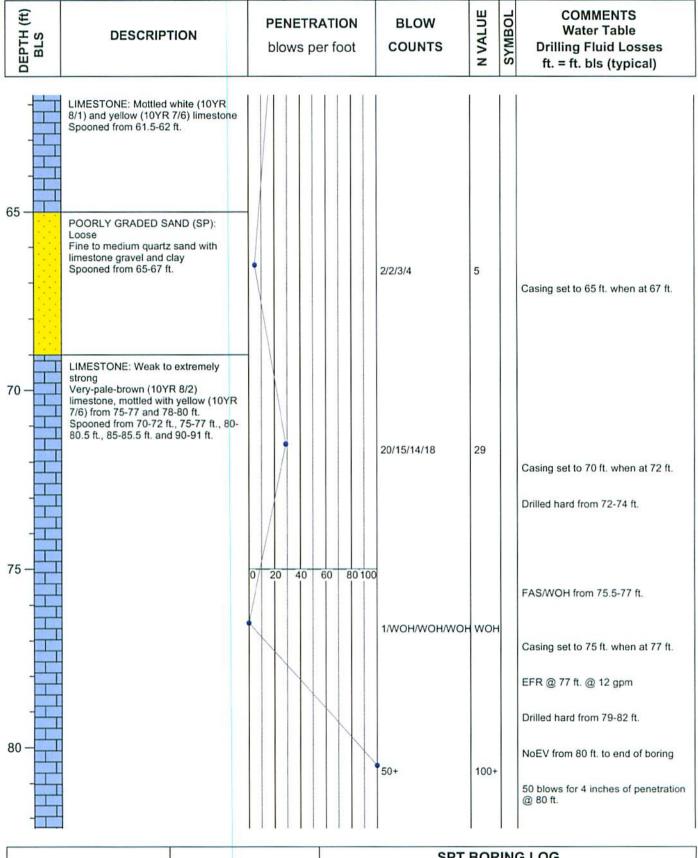
SPT BORING LOG

SITE LOCATION: Spring Hill Florida

SITE LOCATION: Spring Hill, Florida

DESIGNED BY: DR
CHECKED BY: JMW
DRAWN BY: MVK

PROJECT NO: 8053622 BORING DATE: 11/14/12 PAGE NO: 3 of 5



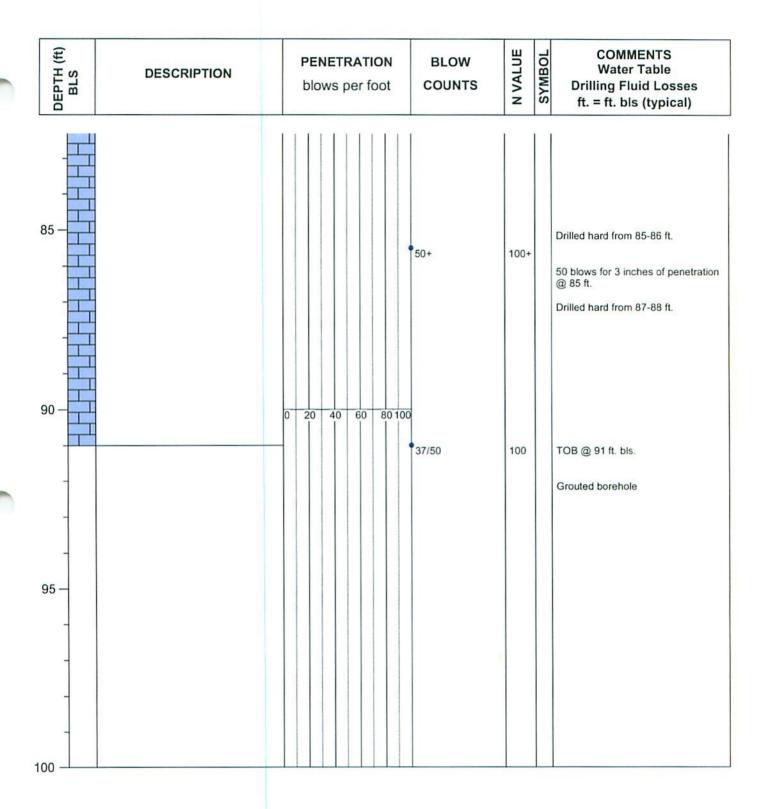


SPT BORING LOG

SITE NAME: **PCRRF** - South Stormwater Pond SITE LOCATION: Spring Hill, Florida

DESIGNED BY: CHECKED BY: **JMW** DRAWN BY: MVK

PROJECT NO: 8053622 **BORING DATE: 11/14/12** PAGE NO: 4 of 5





SPT BORING LOG

SITE NAME: PCRRF - South Stormwater Pond

SITE LOCATION: Spring Hill, Florida

DESIGNED BY: CHECKED BY: **JMW** MVK DRAWN BY:

PROJECT NO: 8053622 BORING DATE: 11/14/12 PAGE NO: 5 of 5

DEPTH (ft) BLS	DESCRIPTION	PENETRATION blows per foot	BLOW	N VALUE	SYMBOL	COMMENTS Water Table Drilling Fluid Losses ft. = ft. bls (typical)
0-				•	_	
	POORLY GRADED SAND (SP): Loose					NoEV throughout boring
	Mottled light-brownish-gray (10YR 6/2) and brown (10YR 5/3) fine to medium quartz sand Spooned from 0-2 ft.		2/2/3/5	5		EFR @ 2 ft. @ 12 gpm
5—	SILTY SAND (SM): Loose Dark-grayish-brown (10YR 4/2) silty fine to medium quartz sand Spooned from 2-6 ft.		4/4/5/5	9		ETTY (@ 2 II. (@ 12 gpm
	POORLY GRADED SAND (SP): Medium dense Mottled grayish-brown (10YR 5/2) and very-pale-brown (10YR 7/4) fine to medium quartz sand		3/3/4/5	7		
7	Spooned from 6-8 ft.		4/4/8/10	12		
10	CLAY (CL/CH): Stiff Mottled light-greenish-gray (5G 7/1) and light-gray (10YR 7/1) clay with sand Spooned from 8-10 ft. CLAY (CL/CH): Light-gray (10YR		4/7/7/8	14		
	7/1) sand clay Spooned from 10-11 ft. CLAY (CL/CH): Mottled yellow (10YR 7/6) and greenish-gray (5G 5/1) clay Spooned from 11-12 ft.		6/4/5/11	9		Casing set to 10 ft. when at 12 ft.
15 - 1	LIMESTONE: Strong to extremely strong Very-pale-brown (10YR 8/2) limestone, mottled with light-brownish-gray (10YR 6/2) from 15-15.9 ft. Spooned from 15-16 ft., 20-22 ft. and 25-27 ft.	0 20 40 60 80 100	27/50+	100+		Drilled hard from 14 ft. to end of boring 50 blows for 5 inches of penetration @ 15.5 ft. Casing set to 15 ft. when at 16 ft.
20 —						SLOC from 20-24 ft.



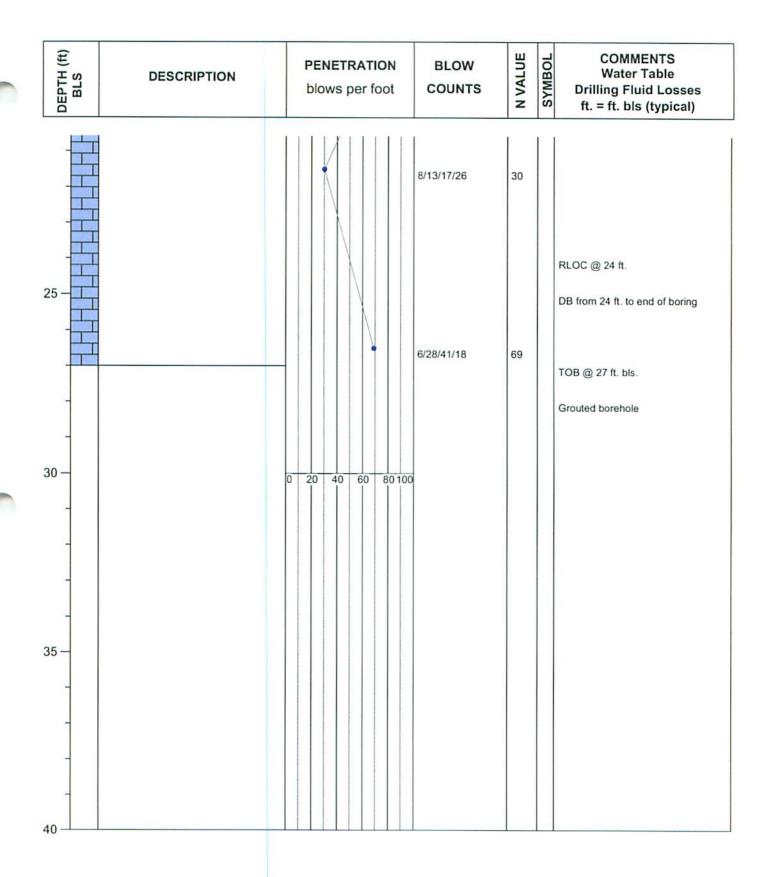
SPT BORING LOG

SITE NAME: PCRRF - South Stormwater Pond

SITE LOCATION: Spring Hill, Florida

DESIGNED BY: CHECKED BY: JMW DRAWN BY: MVK

PROJECT NO: 8053622 BORING DATE: 11/30/12 PAGE NO: 1 of 2





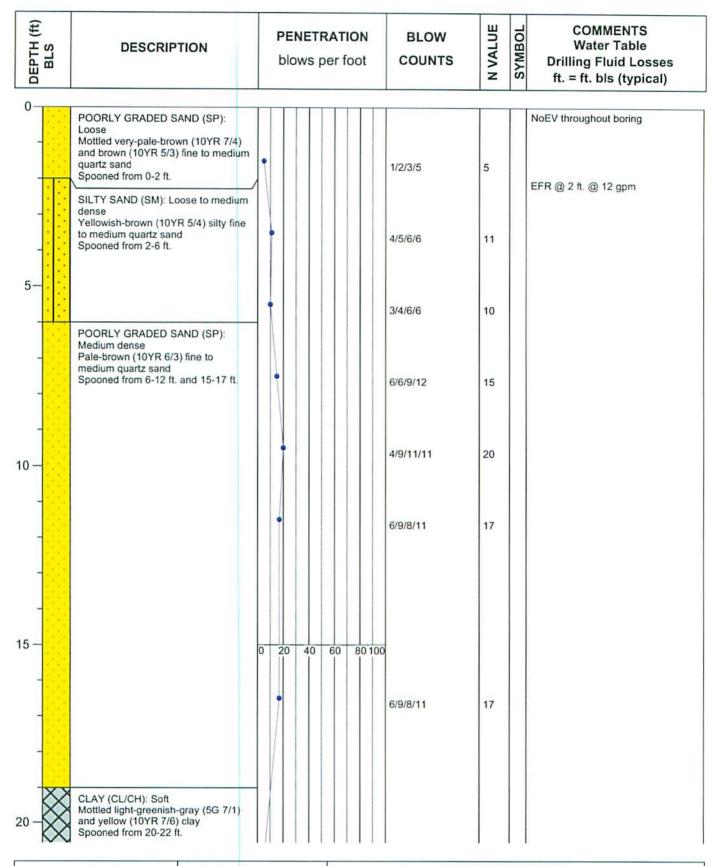
SPT BORING LOG

SITE NAME: PCRRF - South Stormwater Pond

SITE LOCATION: Spring Hill, Florida

DESIGNED BY: DR
CHECKED BY: JMW
DRAWN BY: MVK

PROJECT NO: 8053622
BORING DATE: 11/30/12
PAGE NO: 2 of 2



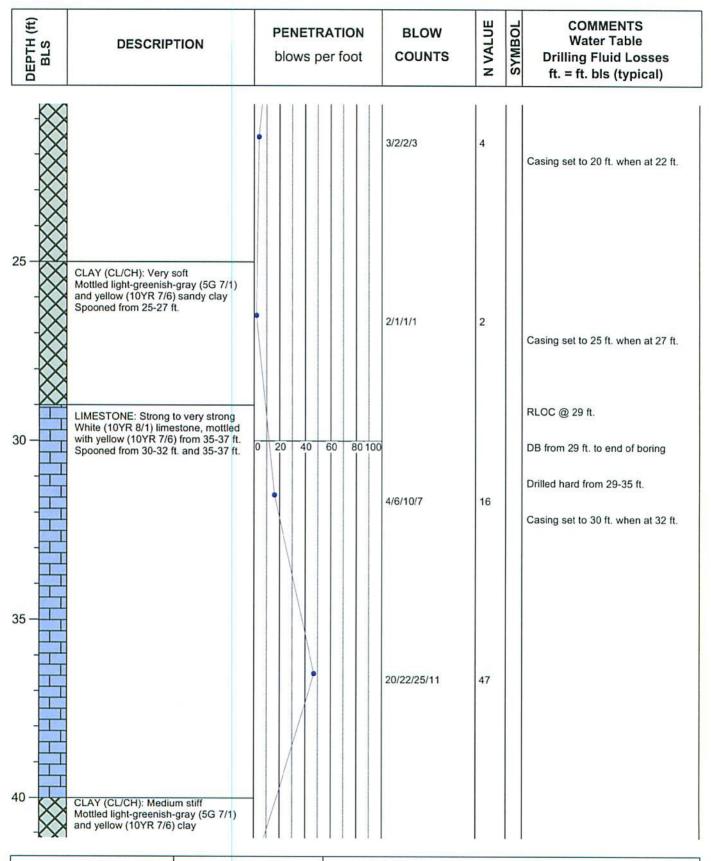


SPT BORING LOG

SITE NAME: PCRRF - South Stormwater Pond

SITE LOCATION: Spring Hill, Florida

DESIGNED BY: DR PROJECT NO: 8053622
CHECKED BY: JMW BORING DATE: 11/26/12
DRAWN BY: MVK PAGE NO: 1 of 3





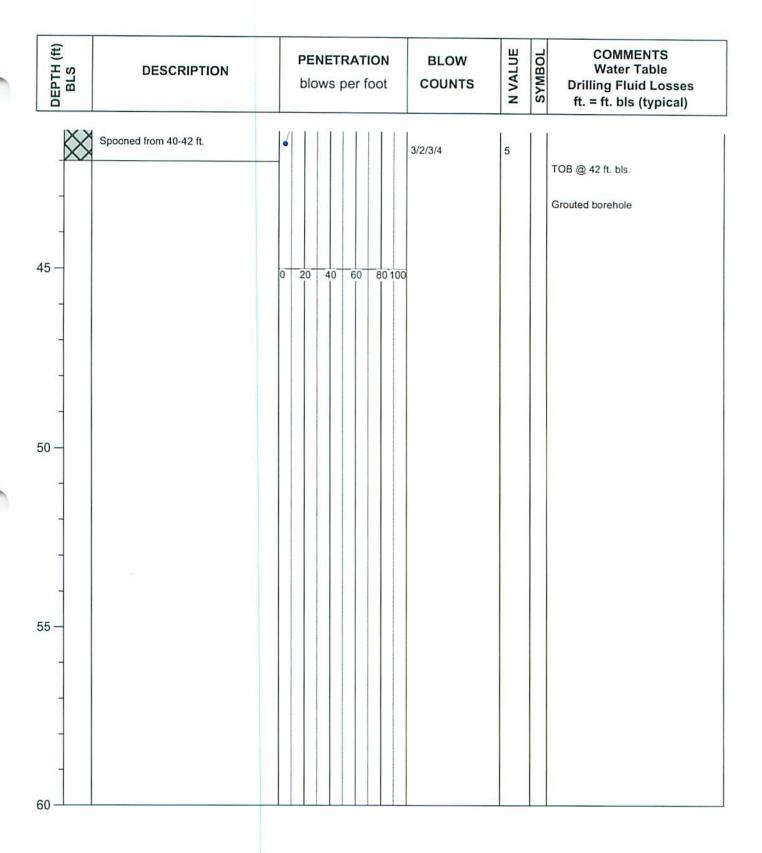
SPT BORING LOG

SITE NAME: PCRRF - South Stormwater Pond

SITE LOCATION: Spring Hill, Florida

DESIGNED BY: CHECKED BY: **JMW** DRAWN BY: MVK

PROJECT NO: 8053622 **BORING DATE: 11/26/12** PAGE NO: 2 of 3





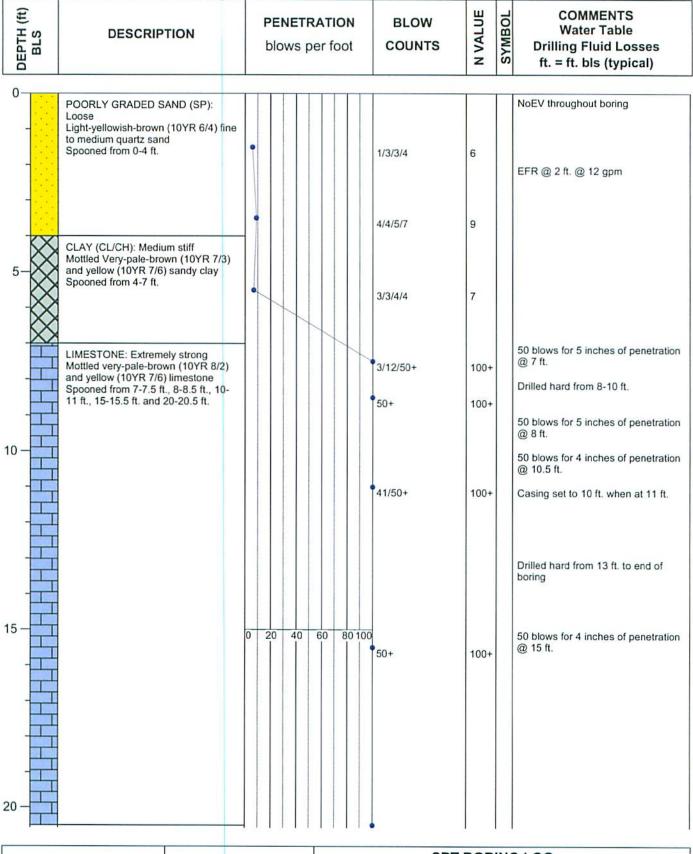
SPT BORING LOG

SITE NAME: PCRRF - South Stormwater Pond

SITE LOCATION: Spring Hill, Florida

DESIGNED BY: PROJECT NO: 8053622 CHECKED BY: **JMW** DRAWN BY: MVK

BORING DATE: 11/26/12 PAGE NO: 3 of 3



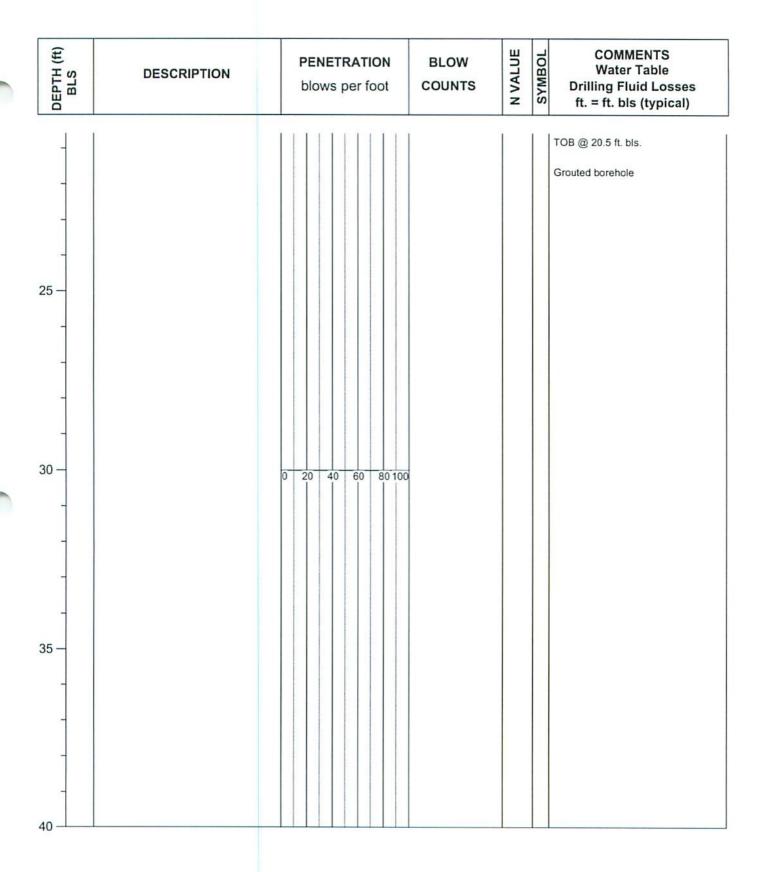


SPT BORING LOG

SITE NAME: PCRRF - South Stormwater Pond SITE LOCATION: Spring Hill, Florida

DESIGNED BY: DR
CHECKED BY: JMW
DRAWN BY: MVK

PROJECT NO: 8053622 BORING DATE: 11/29/12 PAGE NO: 1 of 2





SPT BORING LOG

SITE NAME: PCRRF - South Stormwater Pond

SITE LOCATION: Spring Hill, Florida

DESIGNED BY: DR PROJECT NO: 8053622
CHECKED BY: JMW BORING DATE: 11/29/12
DRAWN BY: MVK PAGE NO: 2 of 2

BORING

DEPTH (ft) BLS	DESCRIPTION	PENETRATION blows per foot	BLOW	N VALUE	SYMBOL	COMMENTS Water Table Drilling Fluid Losses ft. = ft. bls (typical)
0	POORLY GRADED SAND (SP): Loose Light-yellowish-brown (10YR 6/4) fine					NoEV from 0-35 ft.
	to medium quartz sand, mottled with light-brownish-gray (10YR 6/2) from 0-1 ft. Spooned from 0-2 ft.		1/2/3/5	5		EFR @ 2 ft. @ 12 gpm
-:::	SILTY SAND (SM): Loose Yellowish-brown (10YR 5/4) silty fine to medium quartz sand Spooned from 2-5 ft.		4/4/3/4	7		
5	SILTY SAND (SM): Reddish-brown (5YR 5/3) silty fine to medium quartz sand Spooned from 5-6 ft.		2/3/4/4	7		
-	POORLY GRADED SAND (SP): Loose Yellowish-brown (10YR 5/4) fine to medium quartz sand Spooned from 6-8 ft.		3/4/6/8	10		
10	CLAYEY SAND (SC): Medium dense Mottled very-pale-brown (10YR 7/3) and light-brownish-gray (10YR 6/2) clayey fine to medium quartz sand Spooned from 8-10 ft.		4/10/6/9	16		
	CLAY (CL/CH): Medium stiff to stiff Light-greenish-gray (5G 7/1) sandy clay, mottled with yellow (10YR 7/6) from 15-19.5 ft. Spooned from 10-12 ft. and 15-17 ft.		2/3/5/7	8		
15 —		0 20 40 60 80 100	0			Casing set to 15 ft. when at 17 ft.
			3/5/5/6	10		
20	LIMESTONE: White (10YR 8/1) limestone Interval not spooned; lithology based on drilling returns CLAY (CL/CH): Stiff					Drilled hard from 19.5-20 ft.
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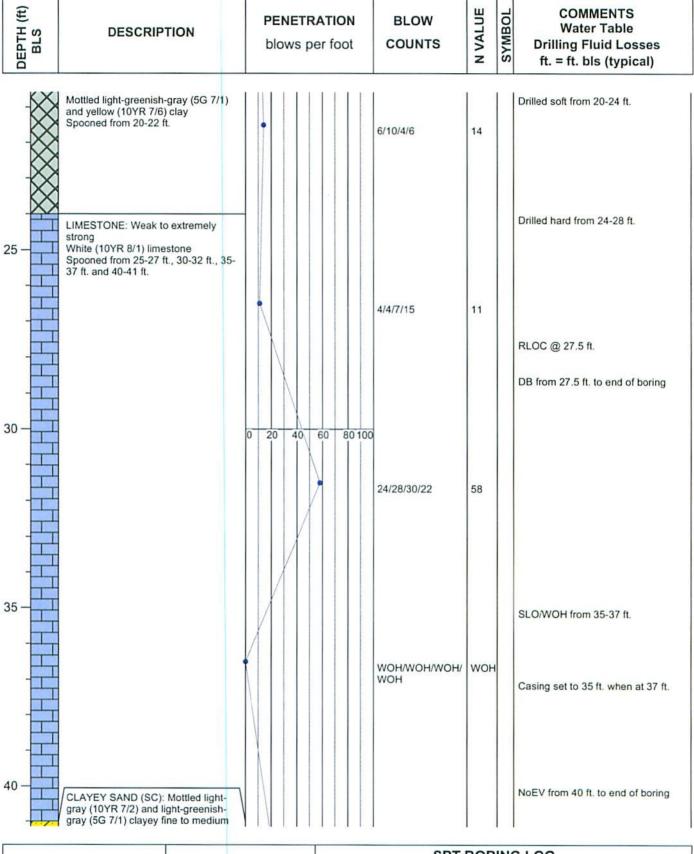


SPT BORING LOG

SITE NAME: PCRRF - South Stormwater Pond

SITE LOCATION: Spring Hill, Florida

DESIGNED BY: DR PROJECT NO: 8053622 CHECKED BY: JMW BORING DATE: 11/20/12 DRAWN BY: MVK PAGE NO: 1 of 3





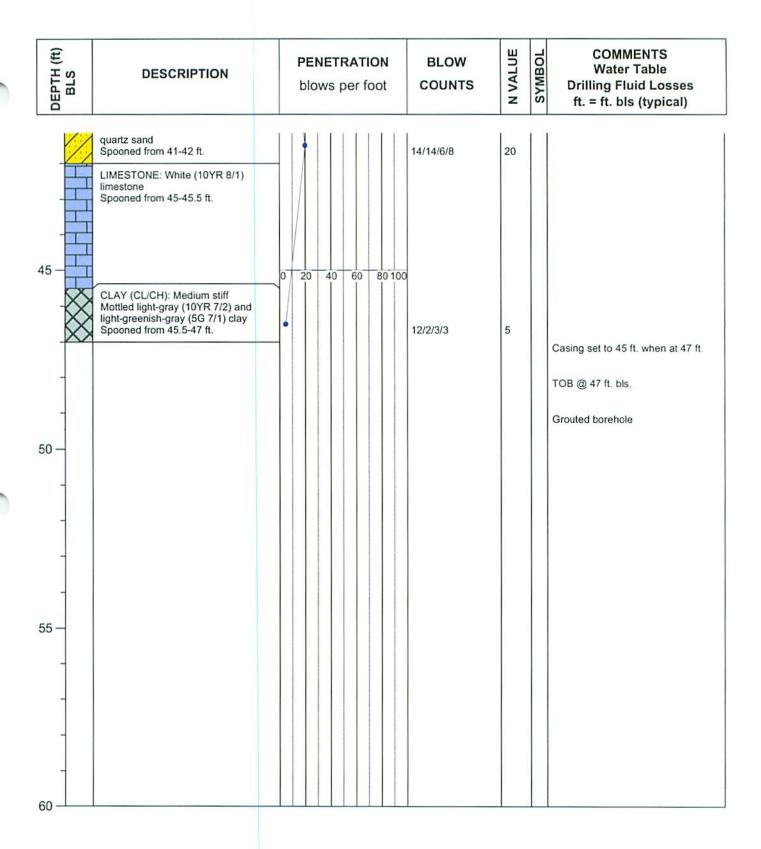
SPT BORING LOG

SITE NAME: PCRRF - South Stormwater Pond

SITE LOCATION: Spring Hill, Florida

DESIGNED BY: CHECKED BY: **JMW** DRAWN BY: MVK

PROJECT NO: 8053622 **BORING DATE: 11/20/12** PAGE NO: 2 of 3





SPT BORING LOG

SITE NAME: PCRRF - South Stormwater Pond

SITE LOCATION: Spring Hill, Florida

DESIGNED BY: DR
CHECKED BY: JMW
DRAWN BY: MVK

PROJECT NO: 8053622 BORING DATE: 11/20/12 PAGE NO: 3 of 3

DEPTH (ft) BLS	DESCRIPTION	PENETRATION blows per foot	BLOW	N VALUE	SYMBOL	COMMENTS Water Table Drilling Fluid Losses ft. = ft. bls (typical)
0						
	POORLY GRADED SAND (SP): Pale-brown (10YR 6/3) fine to medium quartz sand Spooned from 0-1 ft. SILTY SAND (SM): Medium dense Light-brownish-gray (10YR 6/2) silty fine to medium quartz sand, mottled with yellow (10YR 7/6) from 2-4 ft. Spooned from 1-4 ft.		1/3/5/8	8		NoEV throughout boring EFR @ 2 ft. @ 12 gpm
			5/7/7/10	14		
5-	POORLY GRADED SAND (SP): Loose to medium dense Mottled white (10YR 8/1) and light- brownish-gray (10YR 6/2) fine to medium quartz sand Spooned from 4-12.5 ft.	•	4/4/6/6	10		
-			4/5/5/7	10		
10 —			4/7/8/8	15		
-	CLAYEY SAND (SC): Mottled light- brownish-gray (10YR 6/2) and yellow (10YR 7/6) clayey fine to medium quartz sand Spooned from 12.5-13 ft.		5/8/10/12	18		
	CLAY (CL/CH): Stiff Mottled light-greenish-gray (5G 7/1) and yellow (10YR 7/6) clay Spooned from 13-19 ft.	4	4/5/4/6	9		
15 —		0 20 40 60 80 100	7/10/6/8	16		
		•	3/4/6/5	10		Casing set to 15 ft. when at 18 ft.
20	LIMESTONE: White (10YR 8/1) limestone Spooned from 19-20 ft. POORLY GRADED SAND (SP):		4/14/13/8	27		
.,			97			

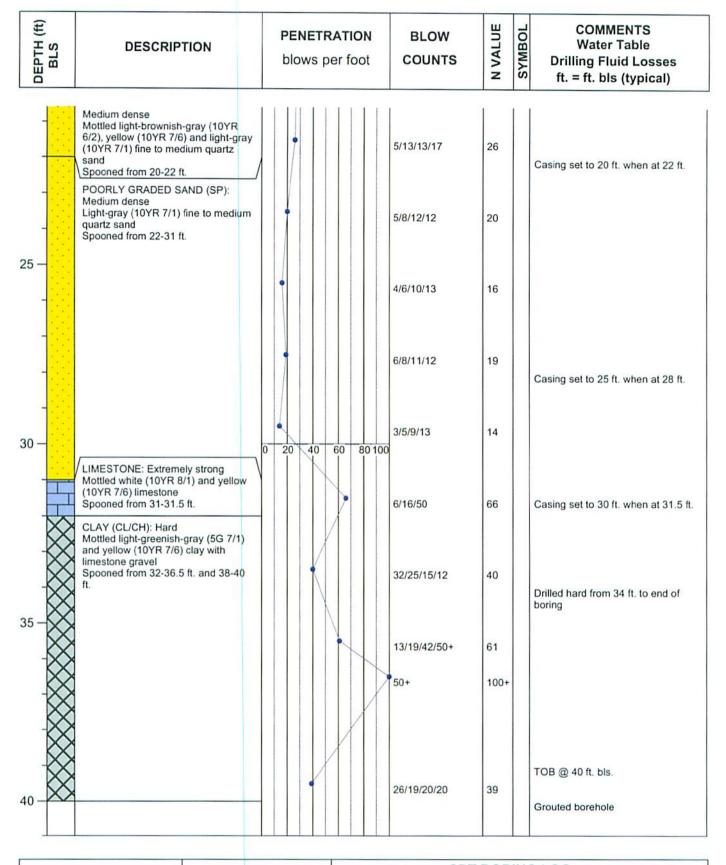


SPT BORING LOG

SITE NAME: PCRRF - South Stormwater Pond SITE LOCATION: Spring Hill, Florida

DESIGNED BY: DR CHECKED BY: JMW DRAWN BY: MVK

PROJECT NO: 8053622 **BORING DATE: 11/28/12** PAGE NO: 1 of 2





SPT BORING LOG

SITE NAME: **PCRRF** - South Stormwater Pond SITE LOCATION: Spring Hill, Florida

DESIGNED BY: CHECKED BY: JMW DRAWN BY: MVK

PROJECT NO: 8053622 **BORING DATE: 11/28/12** PAGE NO: 2 of 2

DEPTH (ft) BLS	DESCRIPTION	PENETRATION blows per foot	BLOW	N VALUE	COMMENTS Water Table Drilling Fluid Losses ft. = ft. bls (typical)	
0	POORLY GRADED SAND (SP):				NoEV throughout boring	
-	Loose Mottled light-yellowish-brown (10YR 6/4), grayish-brown (10YR 5/2) and brown (10YR 5/3) fine to medium quartz sand Spooned from 0-2 ft.		1/2/5/6	7	EFR @ 2 ft. @ 12 gpm	
	CLAYEY SAND (SC): Loose Mottled brown (10YR 5/3), yellow (10YR 7/6) and light-gray (10YR 7/1) clayey fine to medium quartz sand Spooned from 2-3.5 ft.		6/4/4/4	8		
5-	CLAY (CL/CH): Medium dense Mottled light-gray (10YR 7/1), light- yellowish-brown (10YR 6/4) and dark-brown (10YR 3/3) sandy clay Spooned from 3.5-8 ft.		3/5/6/6	11		
	LIMESTONE, Edwards and		3/4/7/9	11	Drilled hard from 8-13 ft.	
10 —	LIMESTONE: Extremely strong Very-pale-brown (10YR 8/2) limestone, mottled with light- greenish-gray (5G 7/1) from 12-14 ft., yellow (10YR 7/6) from 14-15.3 ft. and light-brownish-gray (10YR 6/2) from 20-22 ft.		50+	100+	50 blows for 4 inches of penetration @ 8 ft.	
-	Spooned from 8-8.5 ft., 10-10.5 ft., 15-15.5 ft., 20-22 ft. and 25-25.5 ft.		50+	100+	50 blows for 3 inches of penetration @ 10.5 ft. Casing set to 10 ft. when at 10.5 ft.	
					Drilled hard from 13-15 ft.	
15		0 20 40 60 80	50+	100+	50 blows for 3 inches of penetration @ 15 ft.	
20 —					RLOC @ 19 ft. DB from 19 ft. to end of boring	
		111111/11	The state of the s			
SPT BORING LOG						

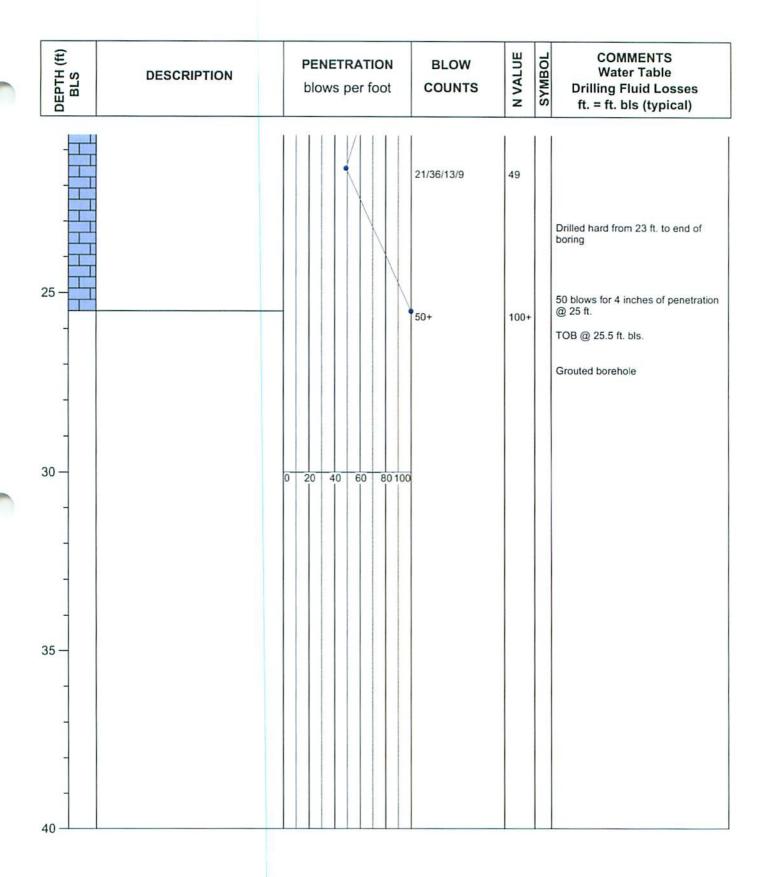




SITE NAME: PCRRF - South Stormwater Pond SITE LOCATION: Spring Hill, Florida

DESIGNED BY: DR
CHECKED BY: JMW
DRAWN BY: MVK

PROJECT NO: 8053622 BORING DATE: 11/29/12 PAGE NO: 1 of 2





SPT BORING LOG

SITE NAME: PCRRF - South Stormwater Pond

SITE LOCATION: Spring Hill, Florida

DESIGNED BY: DR
CHECKED BY: JMW
DRAWN BY: MVK

PROJECT NO: 8053622
BORING DATE: 11/29/12
PAGE NO: 2 of 2

DEPTH (ft) BLS	DESCRIPTION	PENETRATION blows per foot	BLOW	N VALUE	SYMBOL	COMMENTS Water Table Drilling Fluid Losses ft. = ft. bls (typical)
0	POORLY GRADED SAND (SP): Loose Mottled very-pale-brown (10YR 7/4) and light-brownish-gray (10YR 6/2)fine to medium quartz sand Spooned from 0-4 ft.		1/3/5/5	8		NoEV throughout boring EFR @ 2 ft. @ 12 gpm
5—	SILTY SAND (SM): Loose Brown (10YR 4/3) silty fine to medium quartz sand, mottled with Light-yellowish-brown (10YR 6/4) from 7-8 ft. Spooned from 4-8 ft.		3/3/4/4	7 10		
10	POORLY GRADED SAND (SP): Medium dense Light-yellowish-brown (10YR 6/4) fine to medium quartz sand Spooned from 8-10 ft.		4/5/5/9	10		
10	POORLY GRADED SAND (SP): Medium dense Mottled light-gray (10YR 7/1) and grayish-brown (10YR 5/2) fine to medium quartz sand Spooned from 10-12 ft. and 15-17 ft.		5/6/8/8	14		
15 —		0 20 40 60 80 10	7/8/9/12	17		
20	CLAY (CL/CH): Stiff					



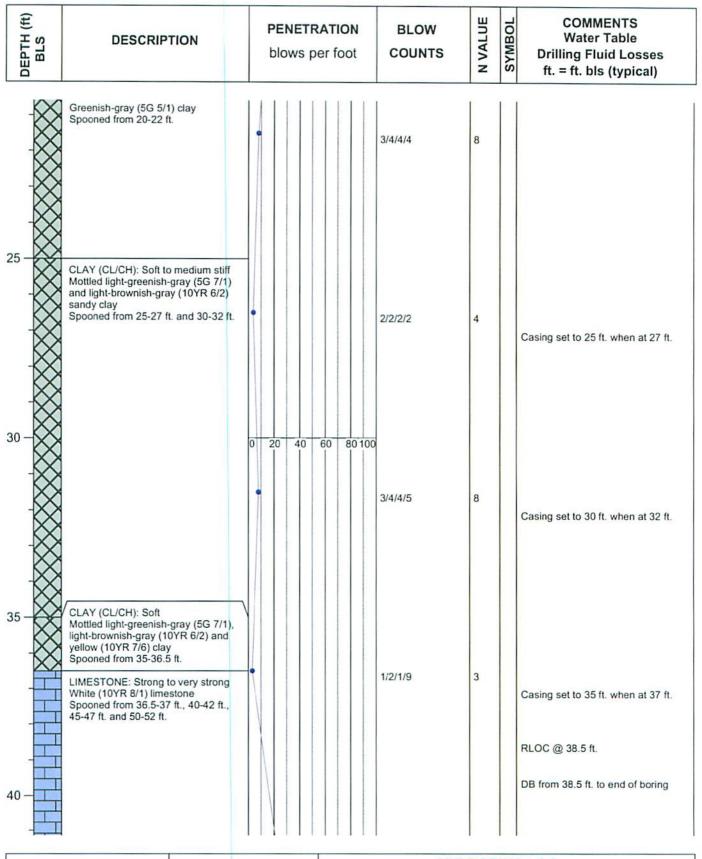
SPT BORING LOG

SITE NAME: PCRRF - South Stormwater Pond

SITE LOCATION: Spring Hill, Florida

DESIGNED BY: DR CHECKED BY: **JMW** DRAWN BY: MVK

PROJECT NO: 8053622 BORING DATE: 11/29/12 PAGE NO: 1 of 3





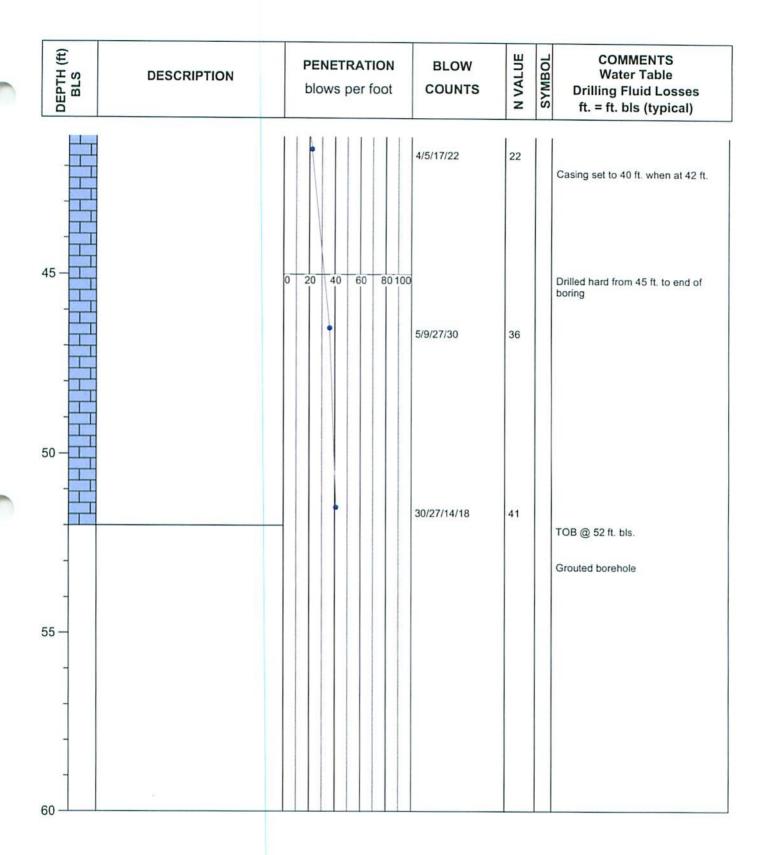
SPT BORING LOG

SITE NAME: PCRRF - South Stormwater Pond

SITE LOCATION: Spring Hill, Florida

DESIGNED BY: DR
CHECKED BY: JMW
DRAWN BY: MVK

PROJECT NO: 8053622 BORING DATE: 11/29/12 PAGE NO: 2 of 3





SPT BORING LOG

SITE NAME: PCRRF - South Stormwater Pond

SITE LOCATION: Spring Hill, Florida

DESIGNED BY: DR PROJECT NO: 8053622
CHECKED BY: JMW BORING DATE: 11/29/12
DRAWN BY: MVK PAGE NO: 3 of 3

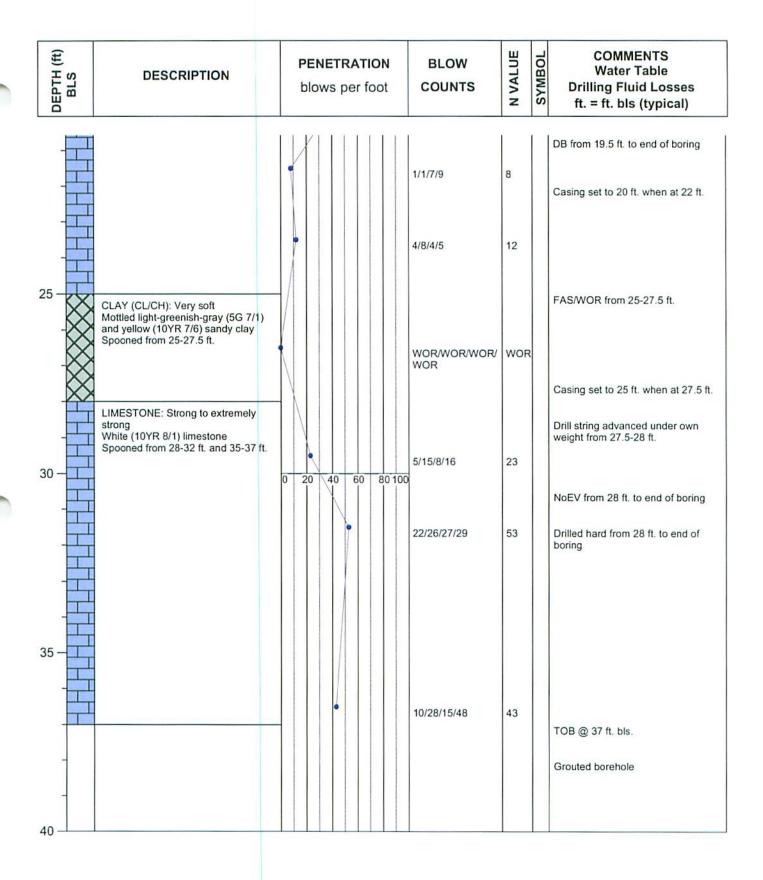
DEPTH (ft) BLS	DESCRIPTION	PENETRATION blows per foot	BLOW	N VALUE	SYMBOL	COMMENTS Water Table Drilling Fluid Losses ft. = ft. bls (typical)
0						
	POORLY GRADED SAND (SP): Loose to medium dense Mottled light-yellowish-brown (10YR 6/4) and yellow (10YR 7/6) fine to medium quartz sand Spooned from 0-4 ft.		1/2/3/5	5		NoEV from 0-25 ft. EFR @ 2 ft. @ 12 gpm
			4/7/8/8	15		
5-	POORLY GRADED SAND (SP): Medium dense Mottled light-brownish-gray (10YR 6/2) and light-yellowish-brown (10YR 6/4) fine to medium quartz sand Spooned from 4-6 ft. SILTY SAND (SM): Medium dense		4/8/9/11	17		
	Mottled brown (10YR 4/3) and yellowish-brown (10YR 5/4) silty fine to medium quartz sand Spooned from 6-10 ft.		6/6/7/10	13		
10	POORLY GRADED SAND (SP): Dense Mottled white (10YR 8/1) and light- yellowish-brown (10YR 6/4) fine to medium quartz sand Spooned from 10-12 ft.		4/8/10/15 10/15/16/17	18		
Z/	CLAYEY SAND (SC): Grayish-brown (10YR 5/2) clayey fine to medium quartz sand Spooned from 12-13 ft.		10/10/17	31		
15 —	CLAY (CL/CH): Stiff Mottled light-greenish-gray (5G 7/1) and grayish-brown (10YR 5/2) sandy clay	0 20 40 60 80100	5/8/8/9	16		
	CLAY (CL/CH): Medium stiff Mottled light-greenish-gray (5G 7/1)		4/5/7/8	12		
	and yellow (10YR 7/6) clay Spooned from 16-18 ft. CLAY (CL/CH): Mottled light- greenish-gray (5G 7/1) and yellow (10YR 7/6) sandy clay		3/4/4/4	8		Casing set to 15 ft. when at 18 ft.
20	Spooned from 18-19 ft. LIMESTONE: Moderately strong White (10YR 8/1) limestone, mottled with yellow (10YR 7/6) from 19-20 ft. Spooned from 19-24 ft.		2/10/36/18	46		RLOC @ 19.5 ft.
						2100



SPT BORING LOG

SITE NAME: PCRRF - South Stormwater Pond SITE LOCATION: Spring Hill, Florida

DESIGNED BY: DR PROJECT NO: 8053622 CHECKED BY: JMW BORING DATE: 11/30/12 DRAWN BY: MVK PAGE NO: 1 of 2





SPT BORING LOG

SITE NAME: PCRRF - South Stormwater Pond

SITE LOCATION: Spring Hill, Florida

 DESIGNED BY:
 DR
 PROJECT NO:
 8053622

 CHECKED BY:
 JMW
 BORING DATE:
 11/30/12

 DRAWN BY:
 MVK
 PAGE NO:
 2 of 2

DEPTH (ft) BLS	DESCRIPTION	PENETR blows p		BLOW	N VALUE	SYMBOL	COMMENTS Water Table Drilling Fluid Losses ft. = ft. bls (typical)
0	POORLY GRADED SAND (SP): Loose Mottled light-yellowish-brown (10YR 6/4) and yellow (10YR 7/6) fine to medium quartz sand Spooned from 0-4 ft.			1/4/4/5	8		NoEV throughout boring EFR @ 2 ft. @ 12 gpm
	POORLY GRADED SAND (SP):	_		5/5/5/5	10		
5—	Loose Grayish-brown (10YR 5/2) fine to medium quartz sand Spooned from 4-8 ft.	•		3/3/4/6	7		
	SILTY SAND (SM): Medium dense	_		3/4/6/5	10		
10	Mottled brown (10YR 4/3) and very- dark-grayish-brown (10YR 3/2) silty fine to medium quartz sand Spooned from 8-10 ft.			5/9/12/13	21		
	SILTY SAND (SM): Medium dense Mottled yellowish-brown (10YR 5/4) and very-dark-grayish-brown (10YR 3/2) silty fine to medium quartz sand Spooned from 10-12 ft.			7/9/15/14	24		
15	POORLY GRADED SAND (SP): Dense Mottled gray (10YR 6/1), gray (10YR 5/1) and light-brownish-gray (10YR 6/2) fine to medium quartz sand Spooned from 15-17 ft.	0 20 40	60 8010	9/16/17/20	33		
20 -	CLAY (CL/CH): Medium stiff Mottled light-greenish-gray (5G 7/1) and light-gray (10YR 7/1) sandy cla Spooned from 20-22 ft.						Drilled hard from 18 ft. to end of boring

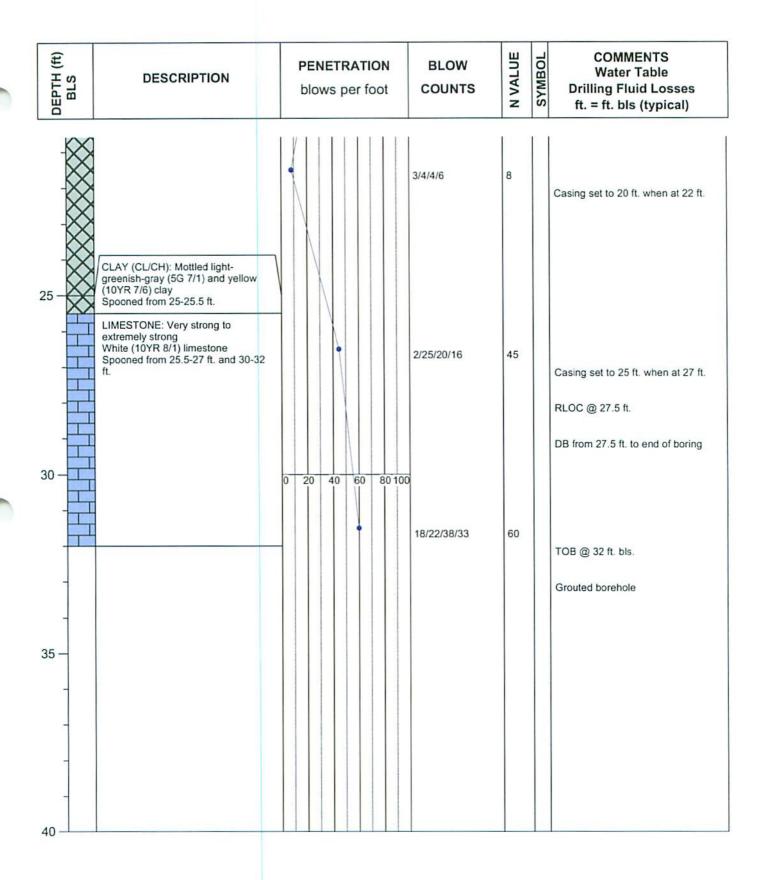


SPT BORING LOG

SITE NAME: PCRRF - South Stormwater Pond SITE LOCATION: Spring Hill, Florida

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DESIGNED BY: DR PROJECT NO: 8053622
CHECKED BY: JMW BORING DATE: 11/30/12
DRAWN BY: MVK PAGE NO: 1 of 2





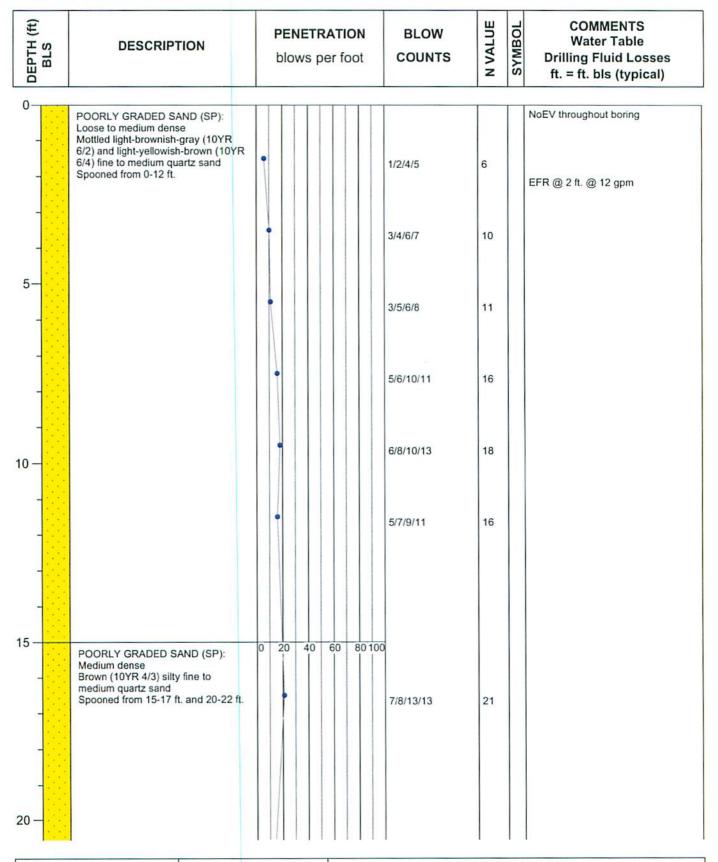
SPT BORING LOG

SITE NAME: PCRRF - South Stormwater Pond

SITE LOCATION: Spring Hill, Florida

DESIGNED BY: DR
CHECKED BY: JMW
DRAWN BY: MVK

PROJECT NO: 8053622 BORING DATE: 11/30/12 PAGE NO: 2 of 2





SPT BORING LOG

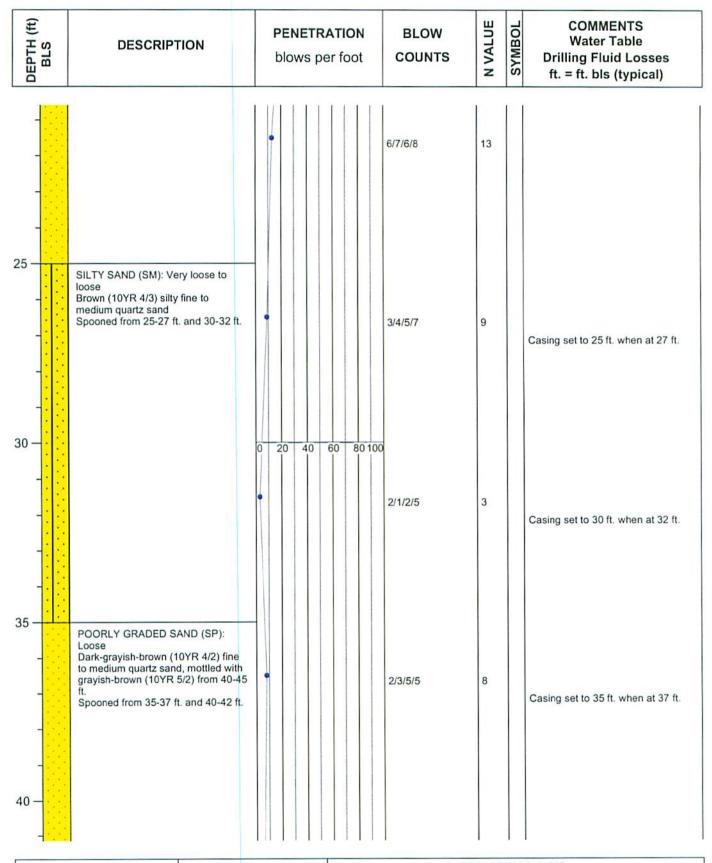
SITE NAME: PCRRF - South Stormwater Pond

SITE LOCATION: Spring Hill, Florida

 DESIGNED BY:
 DR
 PROJECT NO:
 8053622

 CHECKED BY:
 JMW
 BORING DATE:
 12/03/12

 DRAWN BY:
 MVK
 PAGE NO:
 1 of 4





SPT BORING LOG

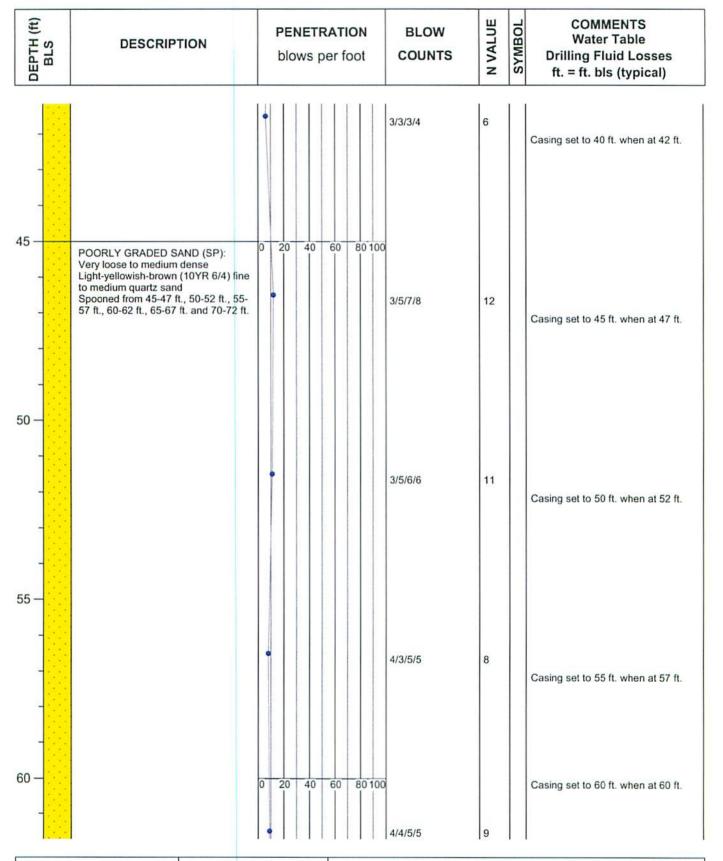
SITE NAME: PCRRF - South Stormwater Pond SITE LOCATION: Spring Hill, Florida

SITE LOCATION. Spring Hill, Florida

 DESIGNED BY:
 DR
 PROJECT NO:
 8053622

 CHECKED BY:
 JMW
 BORING DATE:
 12/03/12

 DRAWN BY:
 MVK
 PAGE NO:
 2 of 4



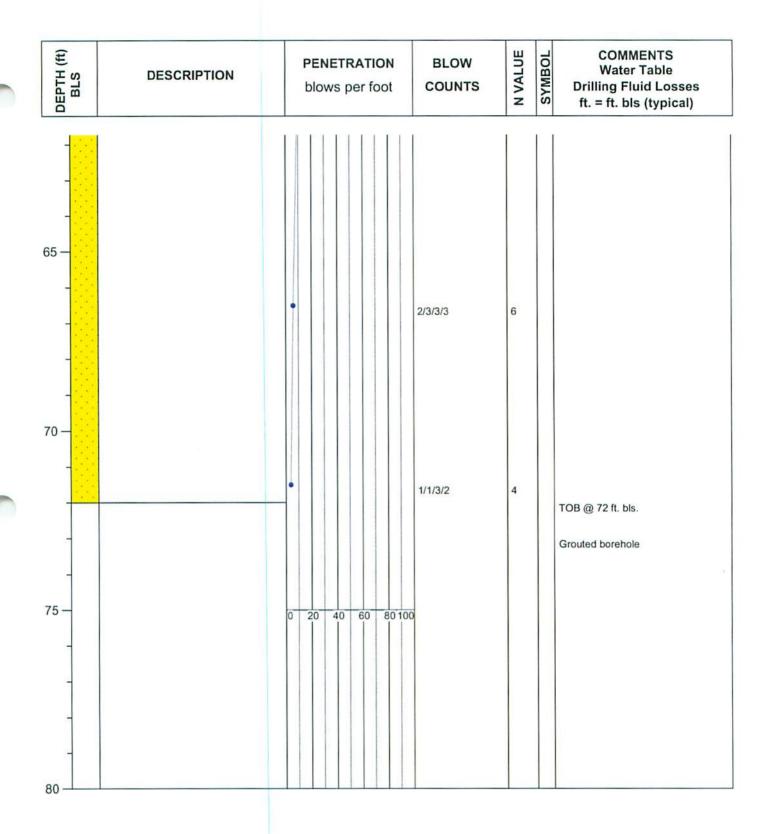


SPT BORING LOG

SITE NAME: PCRRF - South Stormwater Pond

SITE LOCATION: Spring Hill, Florida

DESIGNED BY: DR PROJECT NO: 8053622
CHECKED BY: JMW BORING DATE: 12/03/12
DRAWN BY: MVK PAGE NO: 3 of 4





SPT BORING LOG

SITE NAME: PCRRF - South Stormwater Pond

SITE LOCATION: Spring Hill, Florida

DESIGNED BY: DR CHECKED BY: **JMW** DRAWN BY: MVK

PROJECT NO: 8053622 **BORING DATE: 12/03/12** PAGE NO: 4 of 4

DEPTH (ft) BLS	DESCRIPTION	PENETRATION blows per foot	BLOW	N VALUE	SYMBOL	COMMENTS Water Table Drilling Fluid Losses ft. = ft. bls (typical)
0	CLAY (CL/CH): Medium stiff Mottled very-pale-brown (10YR 7/3), reddish-brown (5YR 5/3) and yellow (10YR 7/6)sandy clay Spooned from 0-2 ft.		2/4/4/4	8		NoEV throughout boring
	CLAY (CL/CH): Medium stiff to stiff Mottled very-pale-brown (10YR 7/3), reddish-brown (5YR 5/3) and yellow (10YR 7/6) clay Spooned from 2-6 ft.		3/3/3/4	6		EFR @ 2 ft. @ 12 gpm
5	LIMESTONE: Mottled very-pale- brown (10YR 7/3) and grayish-brown (10YR 5/2) limestone Spooned from 6-7 ft.		3/5/6/12	11		
	CLAY (CL/CH): Medium stiff to very stiff Mottled very-pale-brown (10YR 7/3) and yellow (10YR 7/6) clay with limestone gravel from 10-15 ft. Spooned from 7-12 ft.		20/14/10/8	24		
10 -			6/14/16/14	30		
			3/2/4/2	6		SLOC from 11-12 ft. RLOC @ 12 ft.
15						DB from 12 ft. to end of boring Casing set to 10 ft. when at 12 ft.
	LIMESTONE: Strong to extremely strong White (10YR 8/1) limestone, mottled with Yellow (10YR 7/6) from 15-17 ft. and 30-30.5 ft. Spooned from 15-17 ft., 20-22 ft., 25-27 ft. and 30-30.5 ft.	0 20 40 60 80 100	1/2/2/15	4		Drilled soft from 14-15 ft. Casing set to 15 ft. when at 17 ft.
20 -						

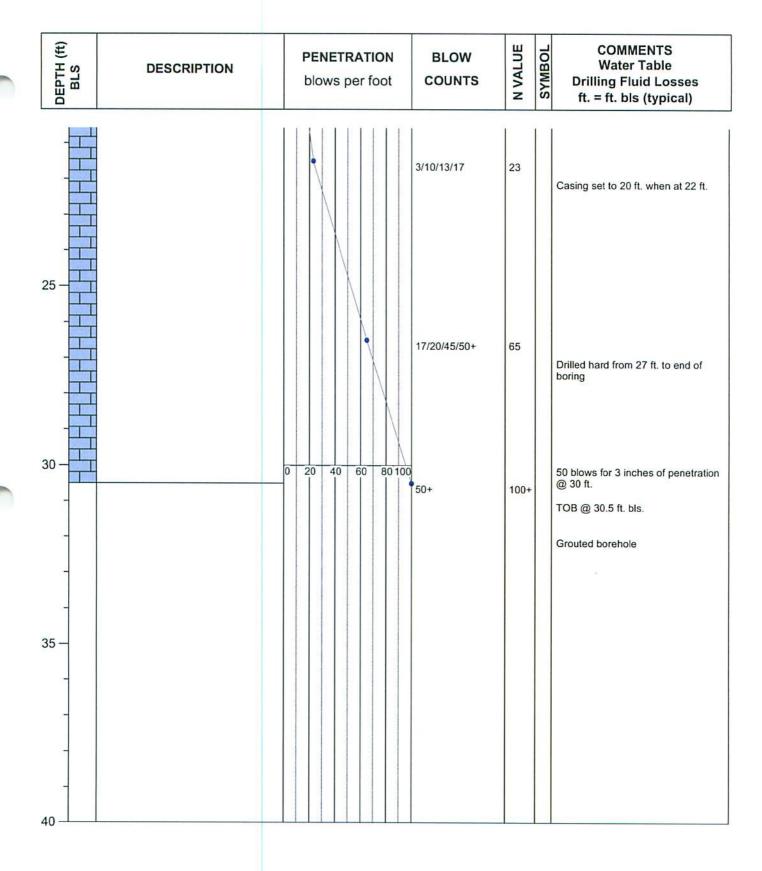


SPT BORING LOG

SITE NAME: PCRRF - South Stormwater Pond SITE LOCATION: Spring Hill, Florida

DR DESIGNED BY: CHECKED BY: JMW DRAWN BY: MVK

PROJECT NO: 8053622 BORING DATE: 11/20/12 PAGE NO: 1 of 2





SPT BORING LOG

SITE NAME: PCRRF - South Stormwater Pond

SITE LOCATION: Spring Hill, Florida

DESIGNED BY: DR
CHECKED BY: JMW
DRAWN BY: MVK

PROJECT NO: 8053622 BORING DATE: 11/20/12 PAGE NO: 2 of 2

POORLY GRADED SAND (SP): Very loose Mottled light-gray (10YR 7/2) and light-brownish-gray (10YR 6/2) fine to medium quartz sand Spooned from 0-2 ft. CLAY (CL/CH): Medium stiff Mottled light-greenish-gray (5G 7/1), yellow (10YR 7/6) and grayish-brown (10YR 7/6) and grayish-brown (10YR 7/6) and grayish-brown (10YR 7/6) and grayish-brown (10YR 7/6) and yellow (10YR 7/6) clay with limestone gravel from 10.5-11 ft. Spooned from 6-11 ft. POORLY GRADED SAND (SP): Very loose NoEV throughout boring 2/2/2/3 4 EFR @ 2 ft. @ 12 gpm CLAY (CL/CH): Medium stiff Mottled light-greenish-gray (5G 7/1) and yellow (10YR 7/6) clay with limestone gravel from 10.5-11 ft. Spooned from 6-11 ft.	DEPTH (ft) BLS	DESCRIPTION	PENETRATION blows per foot	BLOW	N VALUE	SYMBOL	COMMENTS Water Table Drilling Fluid Losses ft. = ft. bls (typical)
POORLY GRADED SAND (SP): Very loose Mottled light-gray (10YR 7/2) and light-brownish-gray (10YR 6/2) fine to medium quartz sand Spooned from 0-2 ft. CLAY (CL/CH): Medium stiff Mottled light-greenish-gray (5G 7/1), yellow (10YR 7/6) and grayish-brown (10YR 7/2) sandy clay Spooned from 2-6 ft. CLAY (CL/CH): Medium stiff Mottled light-greenish-gray (5G 7/1) and yellow (10YR 7/6) clay with limestone gravel from 10.5-11 ft. Spooned from 6-11 ft. 2/2/3/2 5 NoEV throughout boring NoEV throughout boring NoEV throughout boring 2/2/2/3 5 EFR @ 2 ft. @ 12 gpm 2/2/3/4 5	0						
yellow (10YR 7/6) and grayish-brown (10YR 5/2) sandy clay Spooned from 2-6 ft. CLAY (CL/CH): Medium stiff Mottled light-greenish-gray (5G 7/1) and yellow (10YR 7/6) clay with limestone gravel from 10.5-11 ft. Spooned from 6-11 ft. 2/2/3/2 5	<u> </u>	Very loose Mottled light-gray (10YR 7/2) and light-brownish-gray (10YR 6/2) fine to medium quartz sand Spooned from 0-2 ft. CLAY (CL/CH): Medium stiff		2/2/2/3	4		
CLAY (CL/CH): Medium stiff Mottled light-greenish-gray (5G 7/1) and yellow (10YR 7/6) clay with limestone gravel from 10.5-11 ft. Spooned from 6-11 ft. 2/2/3/2 5 2/4/3/4 7	5	yellow (10YR 7/6) and grayish-brown (10YR 5/2) sandy clay		2/2/3/4	5		
Mottled light-greenish-gray (5G 7/1) and yellow (10YR 7/6) clay with limestone gravel from 10.5-11 ft. Spooned from 6-11 ft.		CLAY (CL/CH): Medium stiff	•	2/2/3/2	5		
		Mottled light-greenish-gray (5G 7/1) and yellow (10YR 7/6) clay with limestone gravel from 10.5-11 ft.		2/2/3/2	5		
	10 -			2/4/3/4	7		
LIMESTONE: Moderately strong White (10YR 8/1) limestone Spooned from 11-16.5 ft. Casing set to 10 ft. when at 1:		White (10YR 8/1) limestone		3/3/2/4	5		Casing set to 10 ft. when at 12 ft.
3/7/6/4 13 RLOC @ 13 ft. DB from 13-22 ft.				3/7/6/4	13		
15	15	CLAY (CL/CH): Medium stiff Mottled light-gray (10YR 7/1) and yellow (10YR 7/6) clay			15		
Spooned from 16.5-17.5 ft. POORLY GRADED SAND (SP): 1/1/4/5 5	\bigotimes	Spooned from 16.5-17.5 ft. POORLY GRADED SAND (SP): Mottled white (10YR 8/1) and yellow (10YR 7/6) fine to medium quartz sand	•	1/1/4/5	5		Casing set to 15 ft. when at 18 ft.
20 — CLAY (CL/CH): Mottled light- greenish-gray (5G 7/1) and yellow (10YR 7/6) clay	20 —	CLAY (CL/CH): Mottled light- greenish-gray (5G 7/1) and yellow		3/3/8/9	11		

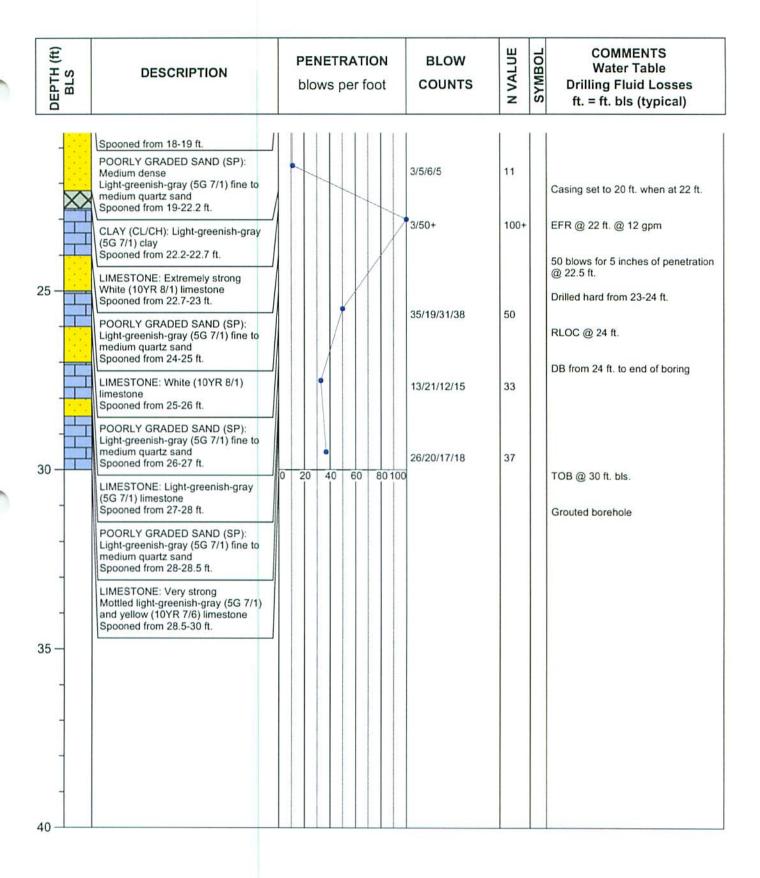


SPT BORING LOG

SITE NAME: PCRRF - South Stormwater Pond SITE LOCATION: Spring Hill, Florida

DESIGNED BY: DR CHECKED BY: JMW DRAWN BY: MVK

PROJECT NO: 8053622 BORING DATE: 12/04/12 PAGE NO: 1 of 2





SPT BORING LOG

SITE NAME: PCRRF - South Stormwater Pond

SITE LOCATION: Spring Hill, Florida

DESIGNED BY: DR
CHECKED BY: JMW
DRAWN BY: MVK

PROJECT NO: 8053622 BORING DATE: 12/04/12 PAGE NO: 2 of 2

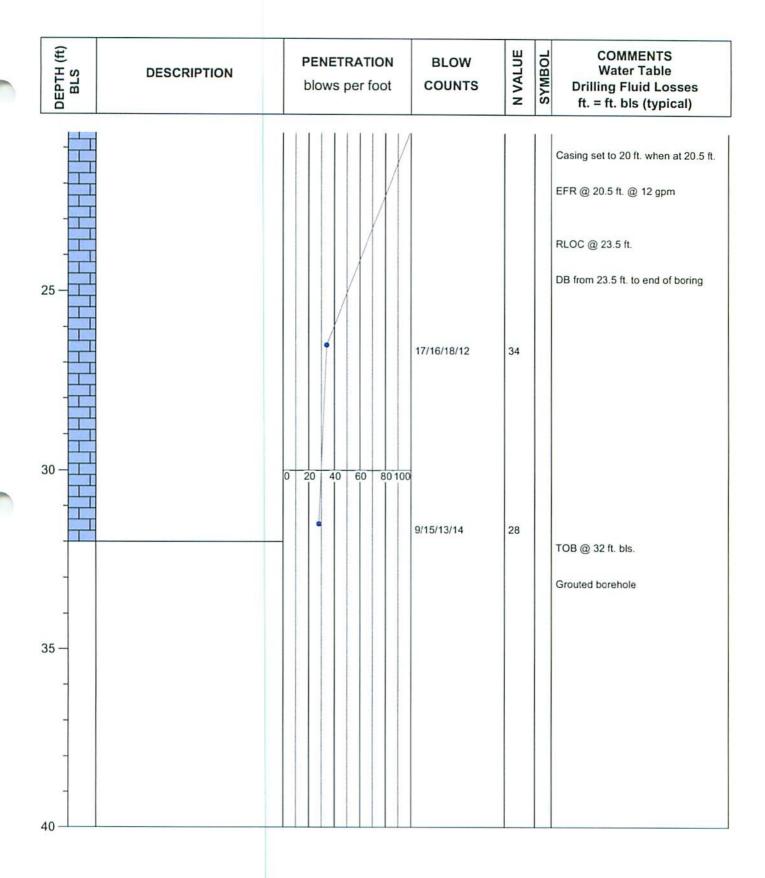
DEPTH (ft) BLS	DESCRIPTION	PENETRATION blows per foot	BLOW	N VALUE	SYMBOL	COMMENTS Water Table Drilling Fluid Losses ft. = ft. bls (typical)
0	CLAY (CL/CH): Medium stiff Mottled light-yellowish-brown (10YR 6/4), yellow (10YR 7/6) and grayish- brown (10YR 5/2) sandy clay Spooned from 0-4 ft.		5/2/3/5	5		NoEV from 0-15 ft. EFR @ 2 ft. @ 12 gpm
	CLAY (CLICH), C-4	•	2/3/2/4	5		
5-	CLAY (CL/CH): Soft Mottled brownish-yellow (10YR 6/6) and light-greenish-gray (5G 7/1) clay Spooned from 4-6 ft.		2/1/2/3	3		
	CLAY (CL/CH): Soft to medium stiff Mottled light-greenish-gray (5G 7/1) and yellow (10YR 7/6) clay Spooned from 6-10 ft.		2/3/3/4	6		
10	CLAY (CL/CH): Medium stiff Mottled light-greenish-gray (5G 7/1)		2/2/2/3	4		
	and yellow (10YR 7/6) sandy clay Spooned from 10-12 ft.	•	2/3/3/4	6		Casing set to 10 ft. when at 12 ft.
15—	LIMESTONE: Weak to extremely strong White (10YR 8/1) limestone Spooned from 15-17 ft., 20-20.5 ft., 25-27 ft. and 30-32 ft.	0 20 40 60 80100	1/WOH/1/1	1		RLOC @ 14.5 ft. DB from 14.5-20.5 ft. SLO/WOH from 15.5-16 ft.
20 -						NoEV from 20 ft. to end of boring



SPT BORING LOG

SITE NAME: PCRRF - South Stormwater Pond SITE LOCATION: Spring Hill, Florida

DESIGNED BY: PROJECT NO: 8053622 CHECKED BY: **JMW** BORING DATE: 12/04/12 DRAWN BY: MVK PAGE NO: 1 of 2





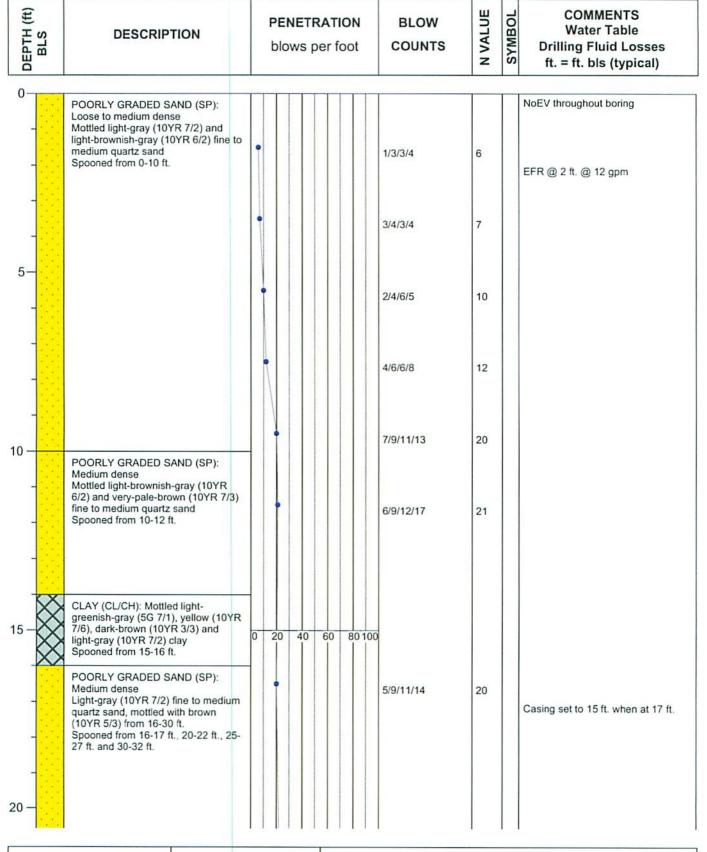
SPT BORING LOG

SITE NAME: PCRRF - South Stormwater Pond

SITE LOCATION: Spring Hill, Florida

DESIGNED BY: DR
CHECKED BY: JMW
DRAWN BY: MVK

PROJECT NO: 8053622 BORING DATE: 12/04/12 PAGE NO: 2 of 2

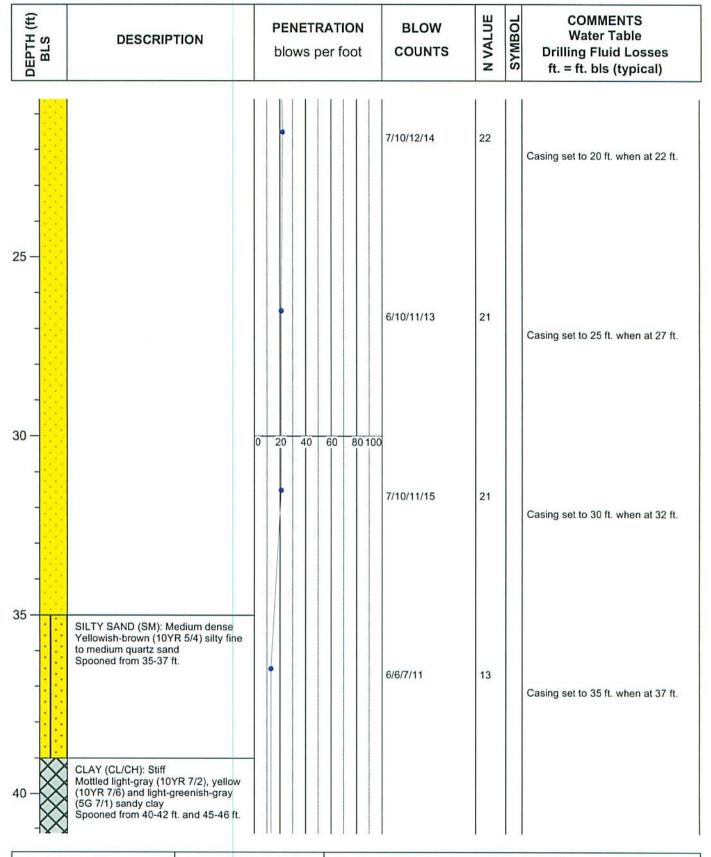




SPT BORING LOG

SITE NAME: PCRRF - South Stormwater Pond SITE LOCATION: Spring Hill, Florida

DESIGNED BY: PROJECT NO: 8053622 CHECKED BY: **JMW BORING DATE: 11/20/12** DRAWN BY: MVK PAGE NO: 1 of 4





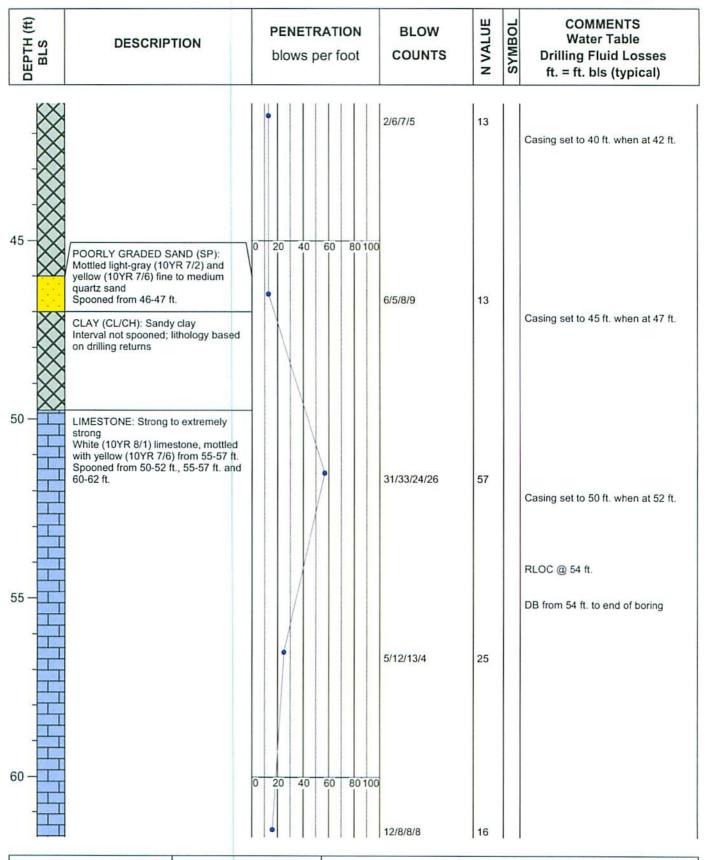
SPT BORING LOG

SITE NAME: PCRRF - South Stormwater Pond

SITE LOCATION: Spring Hill, Florida

DESIGNED BY: DR PRI
CHECKED BY: JMW BOF
DRAWN BY: MVK

PROJECT NO: 8053622 BORING DATE: 11/20/12 PAGE NO: 2 of 4





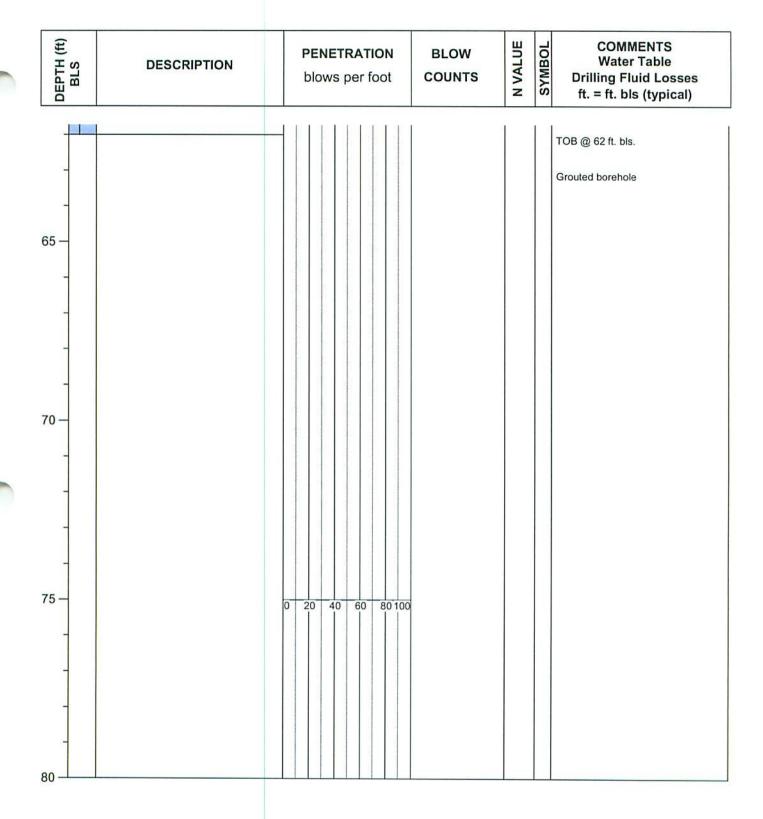
SPT BORING LOG

SITE NAME: PCRRF - South Stormwater Pond

SITE LOCATION: Spring Hill, Florida

DESIGNED BY: DR PI CHECKED BY: JMW BC DRAWN BY: MVK

PROJECT NO: 8053622 BORING DATE: 11/20/12 PAGE NO: 3 of 4





SPT BORING LOG

SITE NAME: PCRRF - South Stormwater Pond SITE LOCATION: Spring Hill, Florida

THE LOCATION: Spring Hill, Florida

DESIGNED BY: DR PROJECT
CHECKED BY: JMW BORING DRAWN BY: MVK PAGE

PROJECT NO: 8053622 BORING DATE: 11/20/12 PAGE NO: 4 of 4

DEPTH (ft) BLS	DESCRIPTION	PENETRATION blows per foot	BLOW	N VALUE	SYMBOL	COMMENTS Water Table Drilling Fluid Losses ft. = ft. bls (typical)
0	POORLY GRADED SAND (SP): Loose to medium dense Mottled light-yellowish-brown (10YR 6/4), yellow (10YR 7/6) and light- brownish-gray (10YR 6/2) fine to medium quartz sand Spooned from 0-6 ft.		1/2/5/4	7		NoEV from 0-16 ft. EFR @ 2 ft. @ 12 gpm
			6/5/5/6	10		
5-	POORLY GRADED SAND (SP): Mottled light-brownish-gray (10YR 6/2), light-yellowish-brown (10YR 6/4) and yellow (10YR 7/6) fine to medium quartz sand Spooned from 6-7 ft.		4/6/6/8	12		
	CLAY (CL/CH): Light-greenish-gray (5G 7/1) sandy clay Spooned from 7-8 ft.		5/4/4/5	8		
10 -	CLAY (CL/CH): Medium stiff to stiff Mottled light-greenish-gray (5G 7/1), yellow (10YR 7/6) and brown (10YR 5/3) clay Spooned from 8-14 ft.		3/3/4/4	7		
			3/5/6/9	11		Casing set to 10 ft. when at 12 ft.
	LIMESTONE: Weak to moderately strong	-	3/2/5/1	7		RLOC @ 14 ft.
15	White (10YR 8/1) limestone Spooned from 14-20 ft.	0 20 40 60 80 100	1/4/10/7	14		DB from 14 ft. to end of boring
-		4	1/4/WOH/4	4		SLO/WOH from 17-17.5 ft. Casing set to 15 ft. when at 18 ft.
20	CLAYEY SAND (SC): Very loose		WOR/WOR/8/1	8		FAS/WOR from 18-19 ft. Drilled soft from 20-22 ft.
			ODT			



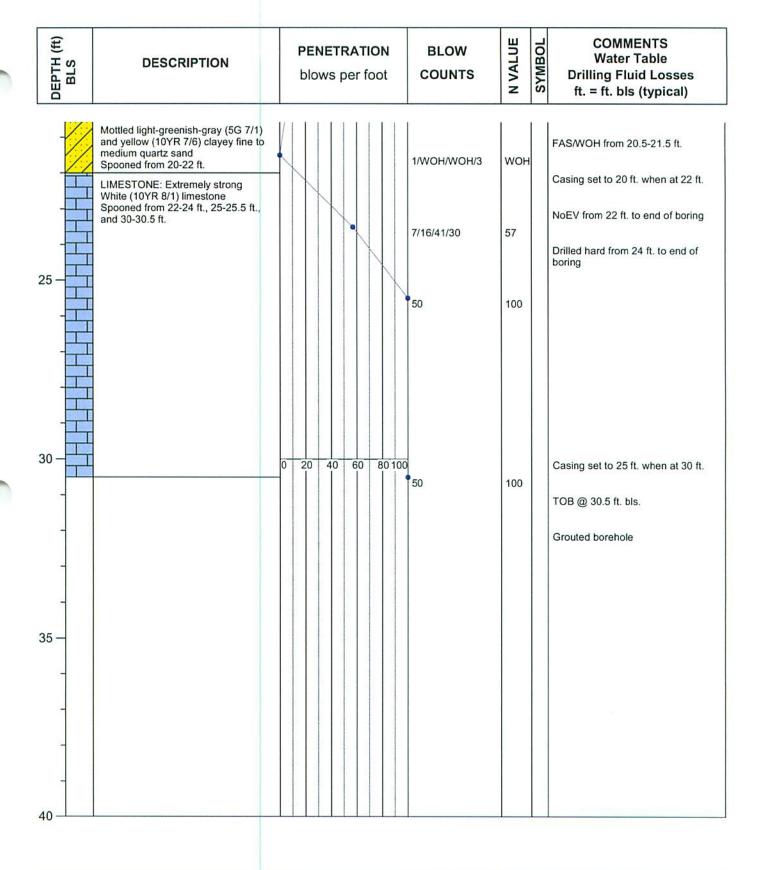
SPT BORING LOG

SITE NAME: PCRRF - South Stormwater Pond

SITE LOCATION: Spring Hill, Florida

DESIGNED BY: DR CHECKED BY: JMW DRAWN BY: MVK

PROJECT NO: 8053622 **BORING DATE: 12/03/12** PAGE NO: 1 of 2





SPT BORING LOG

SITE NAME: PCRRF - South Stormwater Pond

SITE LOCATION: Spring Hill, Florida

DESIGNED BY: CHECKED BY: **JMW** DRAWN BY: MVK

PROJECT NO: 8053622 **BORING DATE: 12/03/12** PAGE NO: 2 of 2

DEPTH (ft) BLS	DESCRIPTION	PENETRATION blows per foot	BLOW	N VALUE	SYMBOL	COMMENTS Water Table Drilling Fluid Losses ft. = ft. bls (typical)
0	CLAYEY SAND (SC): Medium dense Mottled light-yellowish-brown (10YR 6/4) and yellow (10YR 7/6) clayey fine to medium quartz sand Spooned from 0-4 ft.		5/7/7/6	14		NoEV from 0-14 ft. EFR @ 2 ft. @ 12 gpm
5-	CLAY (CL/CH): Medium stiff Mottled light-greenish-gray (5G 7/1) and yellow (10YR 7/6) clay Spooned from 4-6 ft.		4/10/11/8	21		
	CLAY (CL/CH): Stiff Light-gray (10YR 7/1) sandy clay Spooned from 6-9 ft.		4/6/5/7	11		
10	LIMESTONE: Mottled white (10YR 8/1) and yellow (10YR 7/6) limestone Spooned from 9-10 ft. CLAY (CL/CH): Medium stiff Mottled light-gray (10YR 7/1) and		3/16/19/8	35		
	yellow (10YR 7/6) sandy clay Spooned from 10-13 ft.		2/3/5/9	8		Casing set to 10 ft. when at 12 ft.
X	LIMESTONE: White (10YR 8/1) limestone Spooned from 13-14 ft. CLAY (CL/CH): Very soft Mottled light-greenish-gray (5G 7/1)		5/9/5/3	14		SLO/WOH from 14-14.5 ft.
15 -	and yellow (10YR 7/6) clay with limestone gravel Spooned from 14-20 ft.	0 20 40 60 80 100	б woн/1/woн/1	1		SLO/WOH from 15-15.5 ft. NoEV from 16 ft. to end of boring
			1/1/1/5	2		Casing set to 15 ft. when at 18 ft.
20	LIMESTONE: White (10YR 8/1)		1/1/1/7	2	vi	RLOC @ 18 ft. DB from 18 ft. to end of boring
	T					21.00

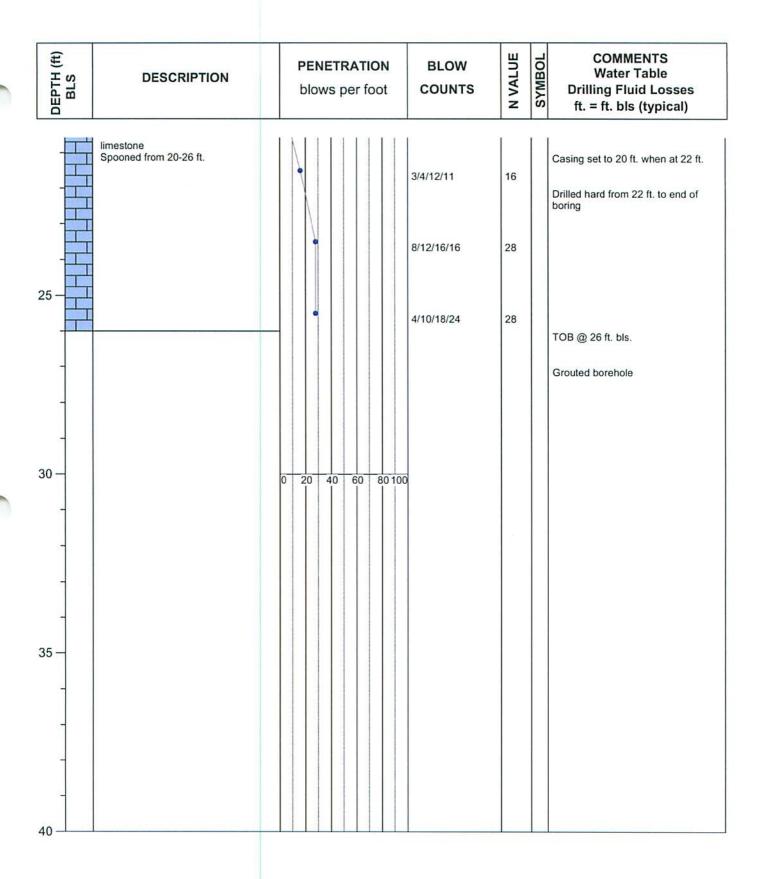


SPT BORING LOG

SITE NAME: PCRRF - South Stormwater Pond SITE LOCATION: Spring Hill, Florida

DESIGNED BY: CHECKED BY: **JMW** DRAWN BY: MVK

PROJECT NO: 8053622 BORING DATE: 12/04/12 PAGE NO: 1 of 2





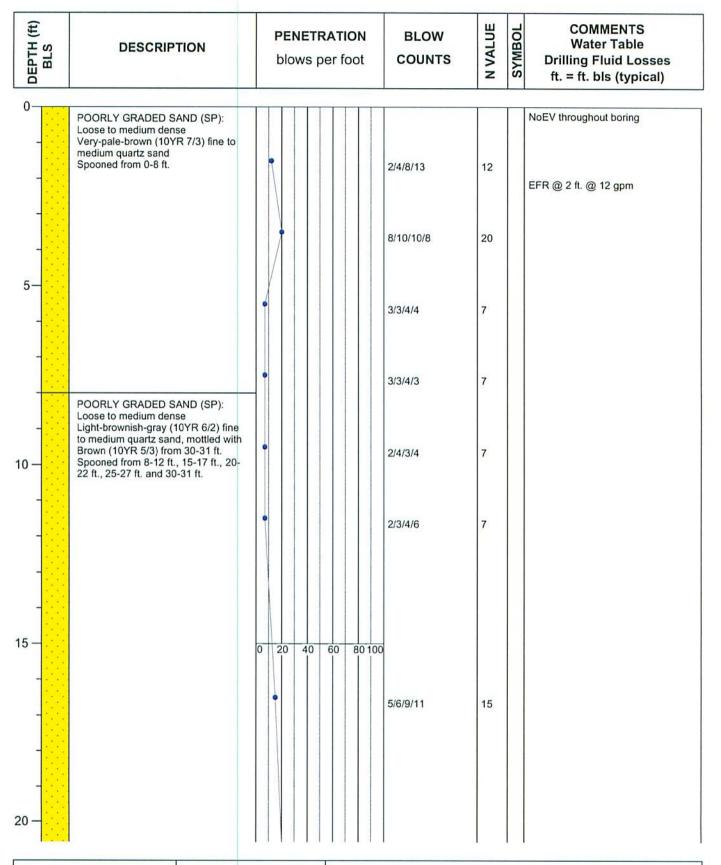
SPT BORING LOG

SITE NAME: PCRRF - South Stormwater Pond

SITE LOCATION: Spring Hill, Florida

DESIGNED BY: DR
CHECKED BY: JMW
DRAWN BY: MVK

PROJECT NO: 8053622 BORING DATE: 12/04/12 PAGE NO: 2 of 2





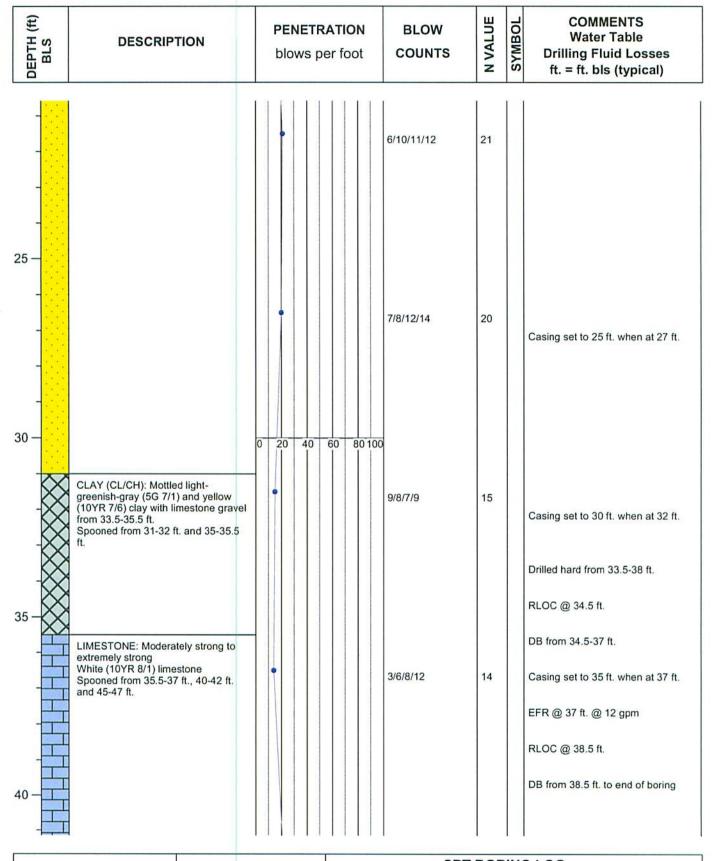
SPT BORING LOG

SITE NAME: PCRRF - South Stormwater Pond

SITE LOCATION: Spring Hill, Florida

DESIGNED BY: DR
CHECKED BY: JMW
DRAWN BY: MVK

PROJECT NO: 8053622 BORING DATE: 11/26/12 PAGE NO: 1 of 3





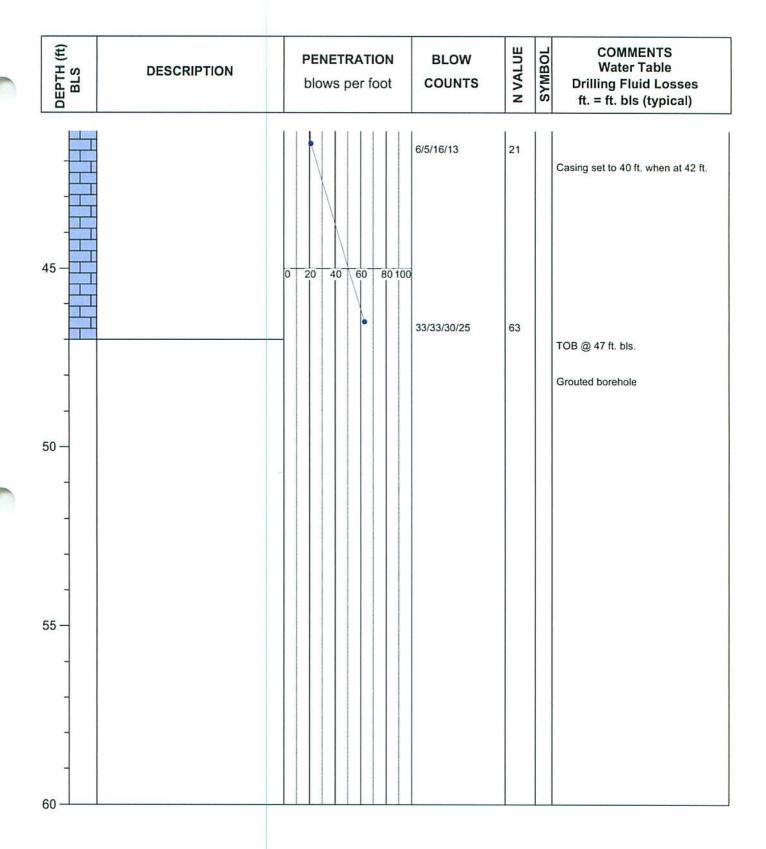
SPT BORING LOG

SITE NAME: PCRRF - South Stormwater Pond SITE LOCATION: Spring Hill, Florida

DESIGNED BY: DR PROJECT NO: 8053622

CHECKED BY: JMW BORING DATE: 11/26/12 DRAWN BY: MVK PAGE NO: 2 of 3

BORING





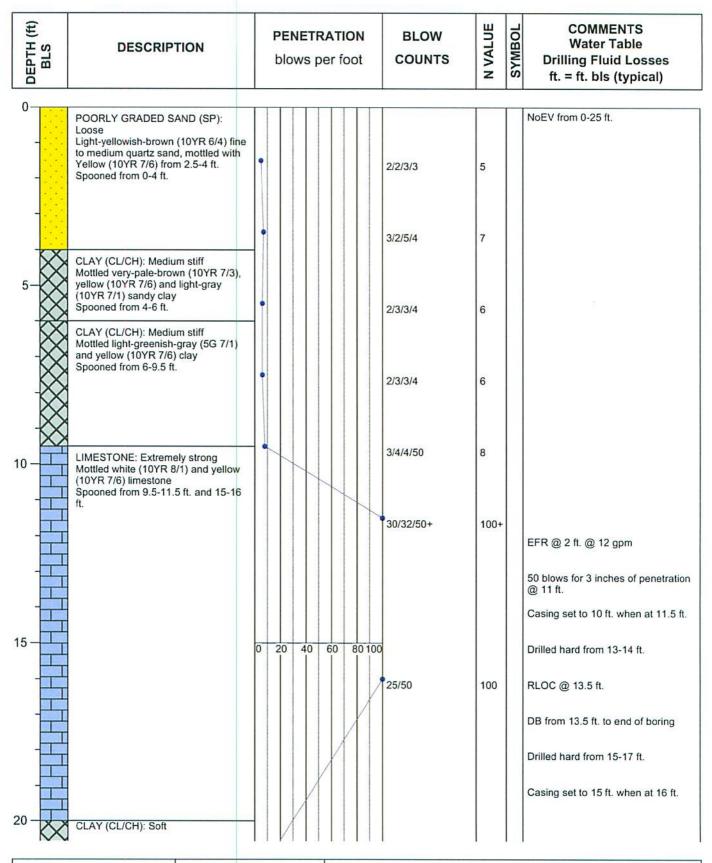
SPT BORING LOG

SITE NAME: PCRRF - South Stormwater Pond

SITE LOCATION: Spring Hill, Florida

DESIGNED BY: DR P
CHECKED BY: JMW BC
DRAWN BY: MVK

PROJECT NO: 8053622 BORING DATE: 11/26/12 PAGE NO: 3 of 3





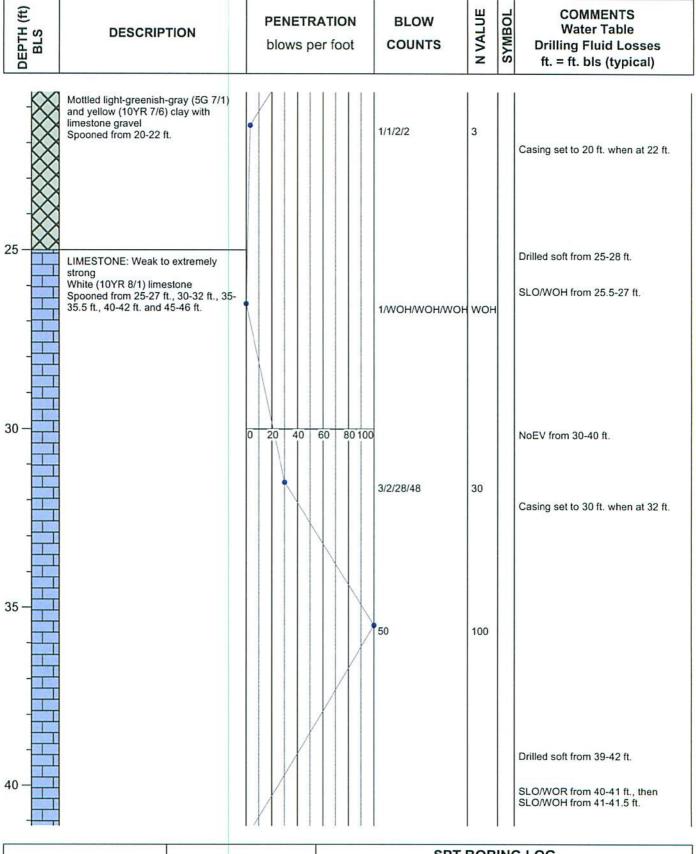
SPT BORING LOG

SITE NAME: PCRRF - South Stormwater Pond

SITE LOCATION: Spring Hill, Florida

DESIGNED BY: DR
CHECKED BY: JMW
DRAWN BY: MVK

PROJECT NO: 8053622 BORING DATE: 11/14/12 PAGE NO: 1 of 3



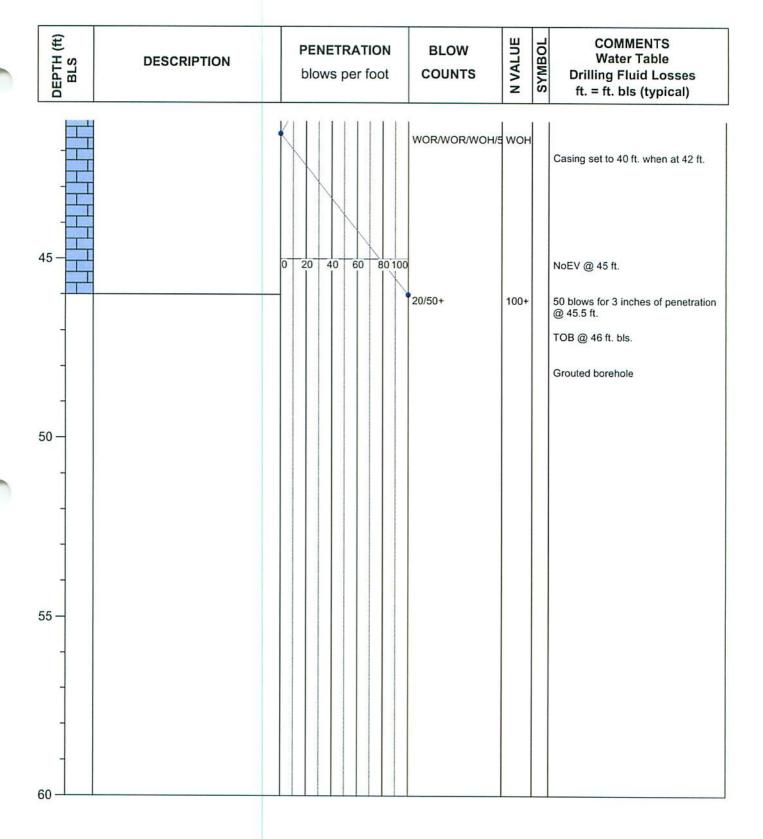


SPT BORING LOG

SITE NAME: **PCRRF** - South Stormwater Pond SITE LOCATION: Spring Hill, Florida

DESIGNED BY: CHECKED BY: **JMW** DRAWN BY: MVK

PROJECT NO: 8053622 **BORING DATE: 11/14/12** PAGE NO: 2 of 3





SPT BORING LOG

SITE NAME: PCRRF - South Stormwater Pond

SITE LOCATION: Spring Hill, Florida

DESIGNED BY: DR CHECKED BY: **JMW** DRAWN BY: MVK

PROJECT NO: 8053622 BORING DATE: 11/14/12 PAGE NO: 3 of 3

DEPTH (ft) BLS	DESCRIPTION	PENETRATION blows per foot	BLOW	N VALUE	SYMBOL	COMMENTS Water Table Drilling Fluid Losses ft. = ft. bls (typical)
0						
-	POORLY GRADED SAND (SP): Very loose to loose Mottled grayish-brown (10YR 5/2) and brown (10YR 5/3) fine to medium quartz sand Spooned from 0-8 ft.		1/3/3/3	6		NoEV from 0-8 ft. EFR @ 2 ft. @ 12 gpm
			3/3/4/4	7		
5-			3/2/3/2	5		
	CLAYEY SAND (SC): Very loose Mottled light-greenish-gray (5G 7/1),		1/1/1/1	2		
10	light-gray (10YR 7/1) and light- brownish-gray (10YR 6/2) clayey fine to medium quartz sand Spooned from 8-10 ft.		1/WOH/1/1	1		SLO/WOH from 8.5-9 ft. NoEV from 10-20 ft.
	CLAY (CL/CH): Stiff Mottled light-gray (10YR 7/1), yellow (10YR 7/6) and light-greenish-gray (5G 7/1) sandy clay Spooned from 10-12 ft.		1/4/6/4	10		Casing set to 10 ft. when at 10 ft.
-	LIMESTONE: Strong Mottled light-gray (10YR 7/1), light- brownish-gray (10YR 6/2) and light- greenish-gray (5G 7/1) limestone Spooned from 15-17 ft.					RLOC @ 13.5 ft.
15 —		0 20 40 60 80 100	5			DB from 13.5 ft. to end of boring
	CLAY (CL/CH): Soft Mottled light-greenish-gray (5G 7/1), yellow (10YR 7/6) and light-gray (10YR 7/1) sandy clay Spooned from 20-22 ft.		6/10/10/2	20		Drilled soft from 17-25 ft.
20 –						Casing set to 20 ft. when at 20 ft.

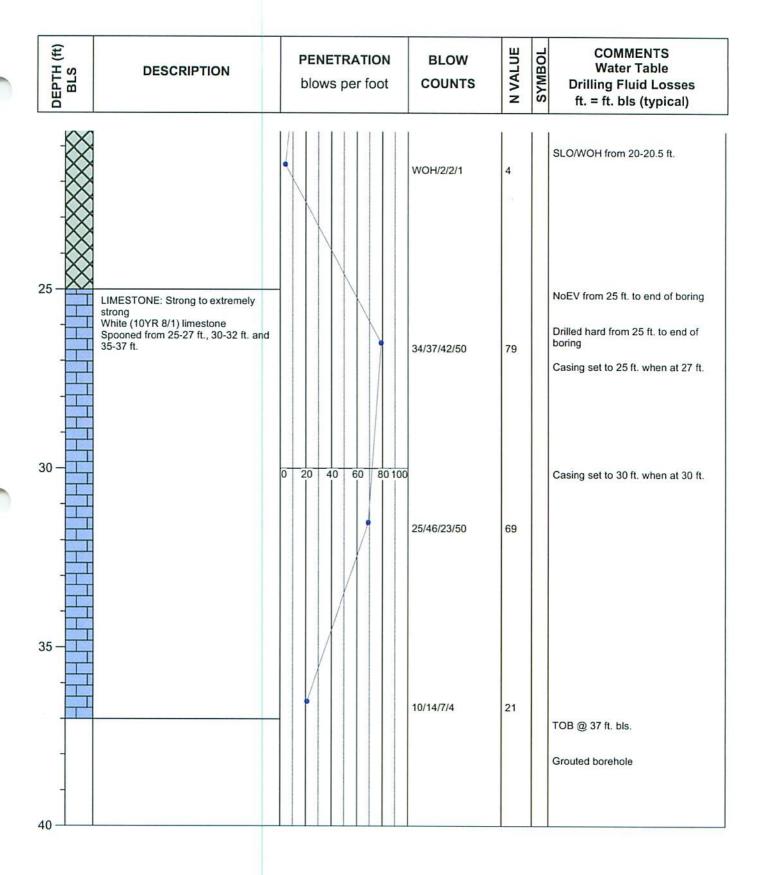


SPT BORING LOG

SITE NAME: PCRRF - South Stormwater Pond SITE LOCATION: Spring Hill, Florida

DESIGNED BY: DR CHECKED BY: **JMW** DRAWN BY: MVK

PROJECT NO: 8053622 BORING DATE: 11/15/12 PAGE NO: 1 of 2





SPT BORING LOG

SITE NAME: PCRRF - South Stormwater Pond

SITE LOCATION: Spring Hill, Florida

DESIGNED BY: DR
CHECKED BY: JMW
DRAWN BY: MVK

PROJECT NO: 8053622 BORING DATE: 11/15/12 PAGE NO: 2 of 2

DEPTH (ft) BLS	DESCRIPTION	PENETRATION blows per foot	BLOW	N VALUE	COMMENTS Water Table Drilling Fluid Losses ft. = ft. bls (typical)
0-1					
	POORLY GRADED SAND (SP): Loose Mottled light-yellowish-brown (10YR 6/4) and yellow (10YR 7/6) fine to medium quartz sand Spooned from 0-2 ft.	•	1/1/4/4	5	NoEV from 0-18 ft. EFR @ 2 ft. @ 12 gpm
-	POORLY GRADED SAND (SP): Loose to medium dense Mottled grayish-brown (10YR 5/2) and yellow (10YR 7/6) fine to medium quartz sand Spooned from 2-6 ft.		4/5/6/6	11	
5—	SILTY CAND (CM) Lease	_•	3/3/3/3	6	
- <mark></mark>	SILTY SAND (SM): Loose Mottled light-yellowish-brown (10YR 6/4) and yellow (10YR 7/6) silty fine to medium quartz sand Spooned from 6-8 ft.		3/4/5/7	9	
10	POORLY GRADED SAND (SP): Loose Mottled light-yellowish-brown (10YR 6/4) and brown (10YR 5/3) fine to medium quartz sand Spooned from 8-10 ft.	•	4/4/5/7	9	
	POORLY GRADED SAND (SP): Loose Grayish-brown (10YR 5/2) fine to medium quartz sand Spooned from 10-13 ft.		3/5/4/7	9	
	POORLY GRADED SAND (SP): White (10YR 8/1) fine to medium quartz sand Spooned from 13-14 ft. CLAY (CL/CH): Medium stiff		4/5/5/6	10	
15 —	Mottled light-greenish-gray (5G 7/1) and yellow (10YR 7/6) clay Spooned from 14-18 ft.	0 20 40 60 80100	3/3/4/6	7	
	CLAY (CL/CH): Loose		2/4/4/5	8	
20 -	Mottled light-greenish-gray (5G 7/1), light-gray (10YR 7/1) and yellow (10YR 7/6) sandy clay Spooned from 18-21 ft.	•	2/2/1/WOH	3	FAS/WOH from 19.5-20 ft.

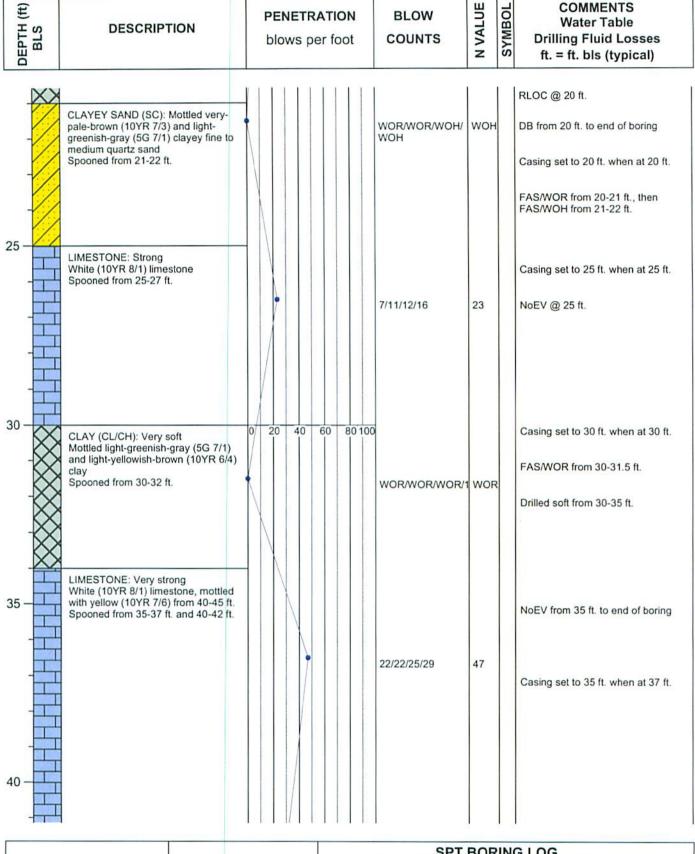


SPT BORING LOG

SITE NAME: PCRRF - South Stormwater Pond SITE LOCATION: Spring Hill, Florida

DESIGNED BY: DR
CHECKED BY: JMW
DRAWN BY: MVK

PROJECT NO: 8053622 BORING DATE: 11/16/12 PAGE NO: 1 of 3



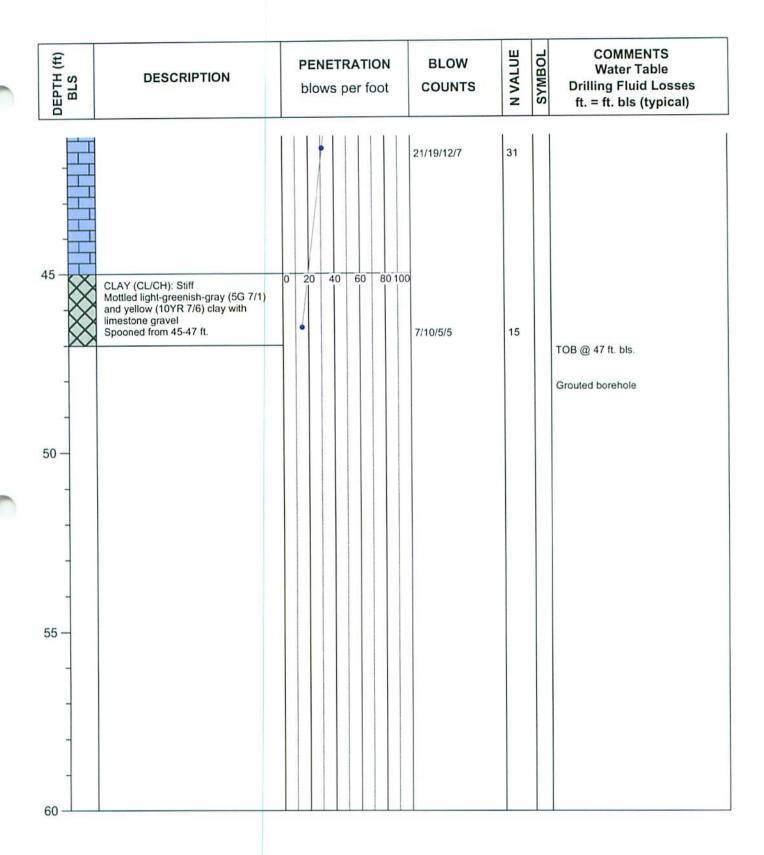


SPT BORING LOG

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DESIGNED BY: CHECKED BY: **JMW** MVK DRAWN BY:

PROJECT NO: 8053622 BORING DATE: 11/16/12 PAGE NO: 2 of 3



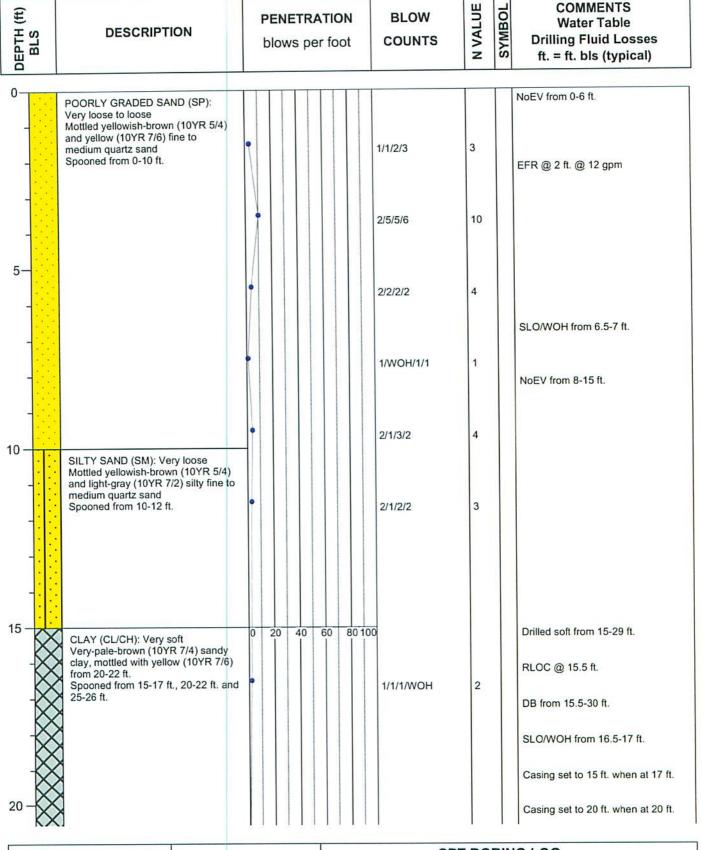


SPT BORING LOG

SITE NAME: PCRRF - South Stormwater Pond

SITE LOCATION: Spring Hill, Florida

DESIGNED BY: DR PROJECT NO: 8053622
CHECKED BY: JMW BORING DATE: 11/16/12
DRAWN BY: MVK PAGE NO: 3 of 3

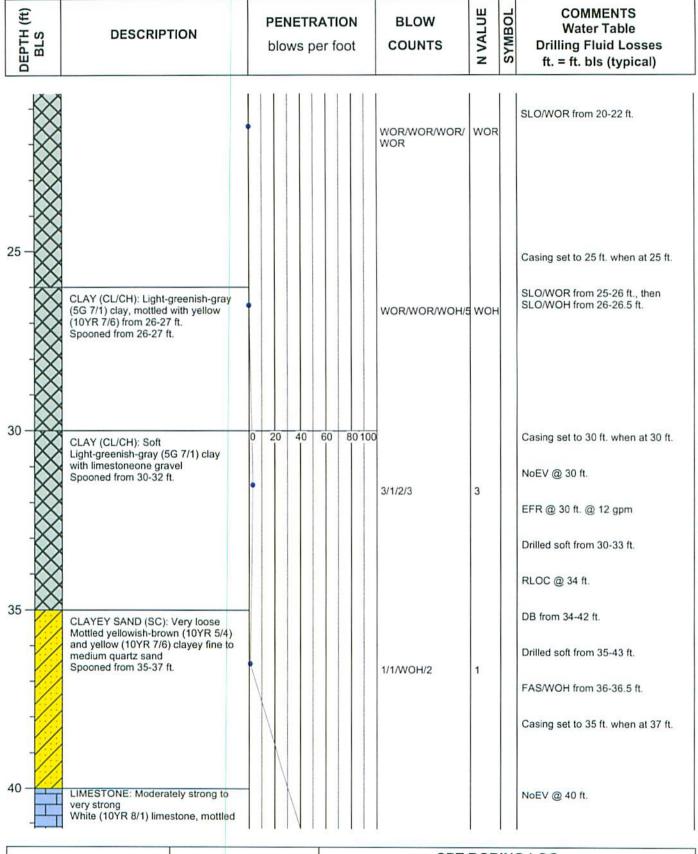




SPT BORING LOG

SITE NAME: PCRRF - South Stormwater Pond SITE LOCATION: Spring Hill, Florida

DR PROJECT NO: 8053622 **DESIGNED BY:** CHECKED BY: BORING DATE: 11/15/12 PAGE NO: 1 of 4 DRAWN BY: MVK



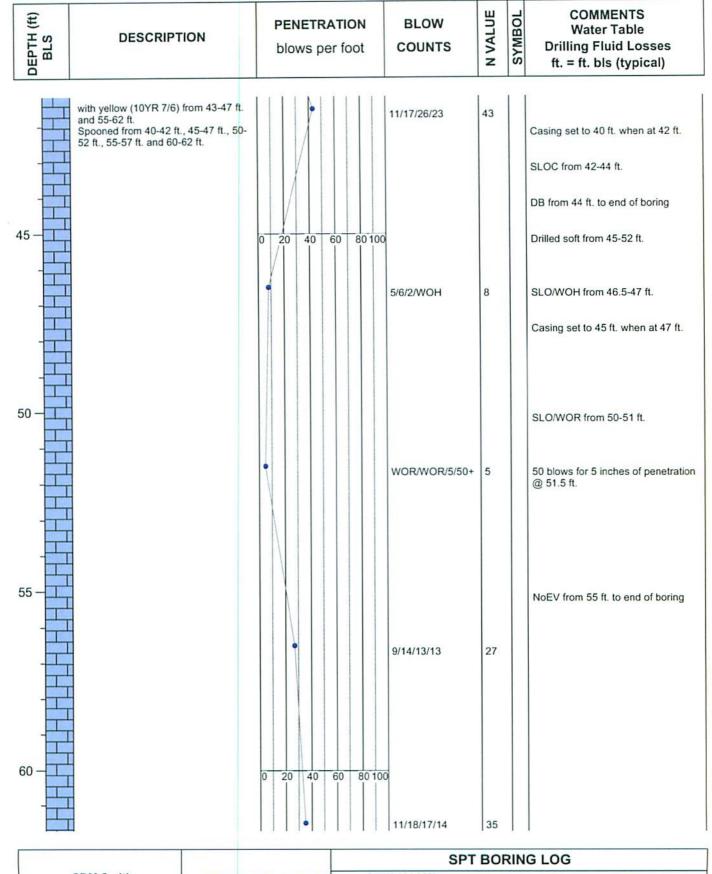


SPT BORING LOG

SITE NAME: PCRRF - South Stormwater Pond SITE LOCATION: Spring Hill, Florida

DESIGNED BY: DR
CHECKED BY: JMW
DRAWN BY: MVK

PROJECT NO: 8053622 BORING DATE: 11/15/12 PAGE NO: 2 of 4

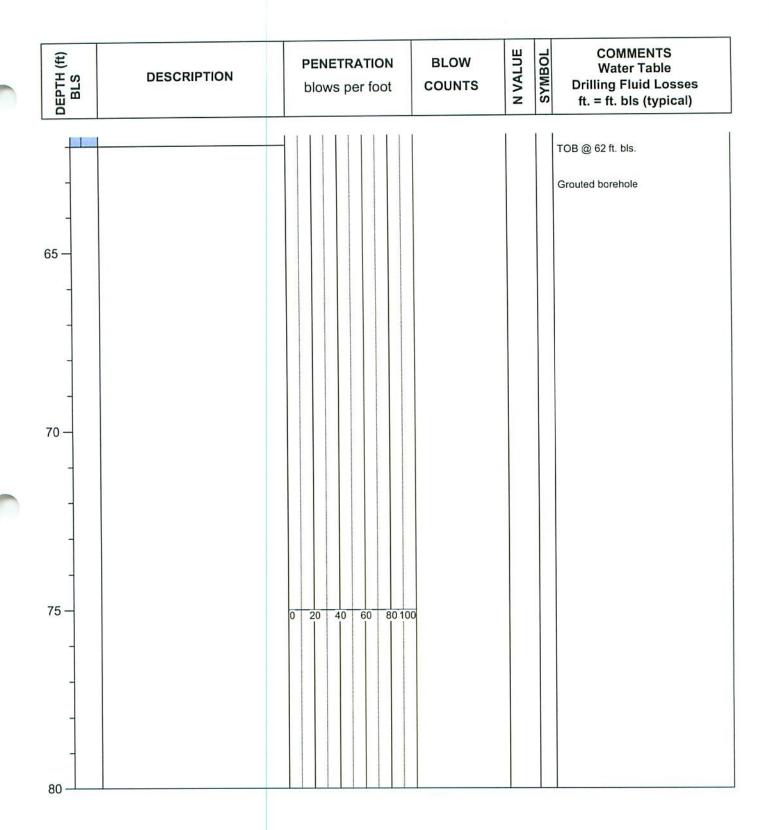




SITE NAME: PCRRF - South Stormwater Pond SITE LOCATION: Spring Hill, Florida

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DESIGNED BY: DR PROJECT NO: 8053622
CHECKED BY: JMW BORING DATE: 11/15/12
DRAWN BY: MVK PAGE NO: 3 of 4





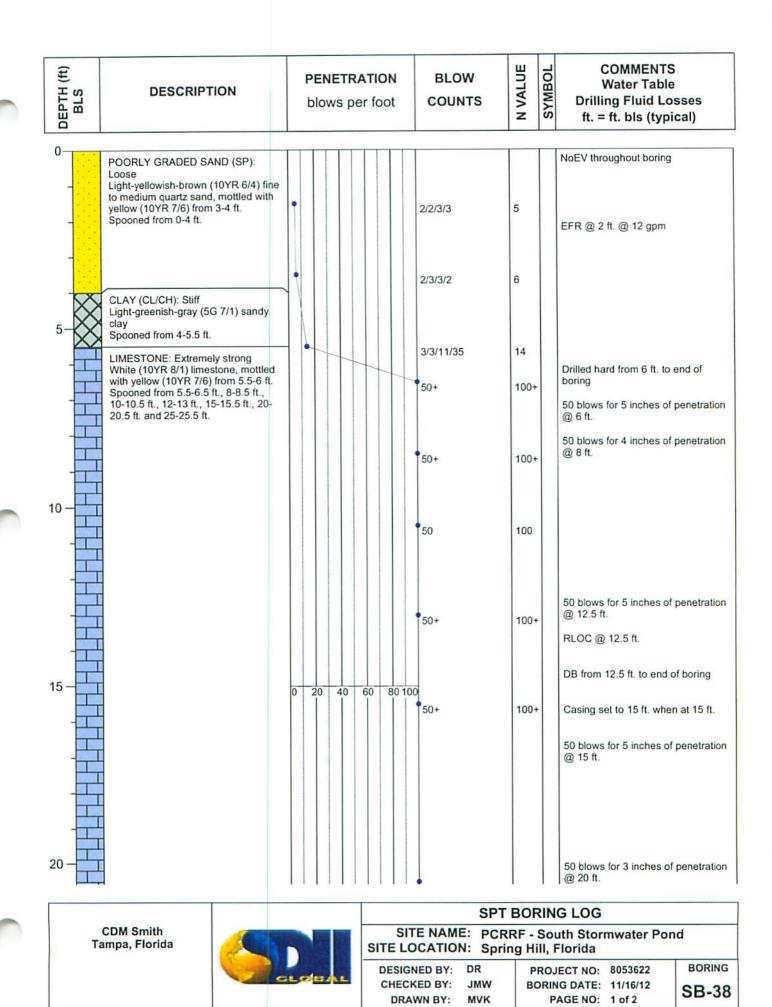
SPT BORING LOG

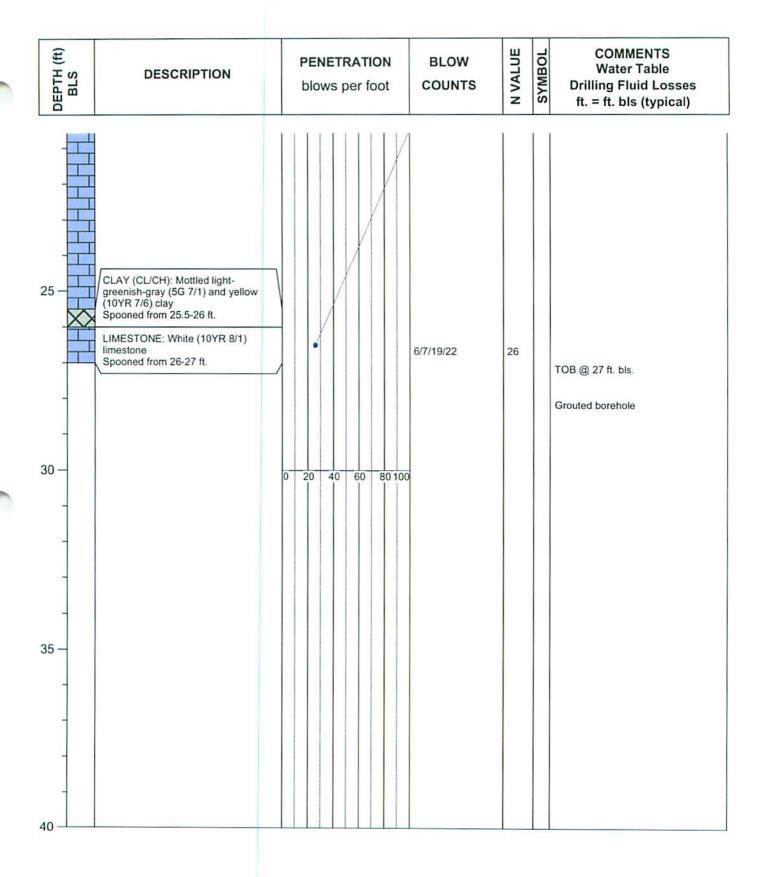
SITE NAME: PCRRF - South Stormwater Pond

SITE LOCATION: Spring Hill, Florida

DESIGNED BY: CHECKED BY: **JMW** MVK DRAWN BY:

PROJECT NO: 8053622 BORING DATE: 11/15/12 PAGE NO: 4 of 4







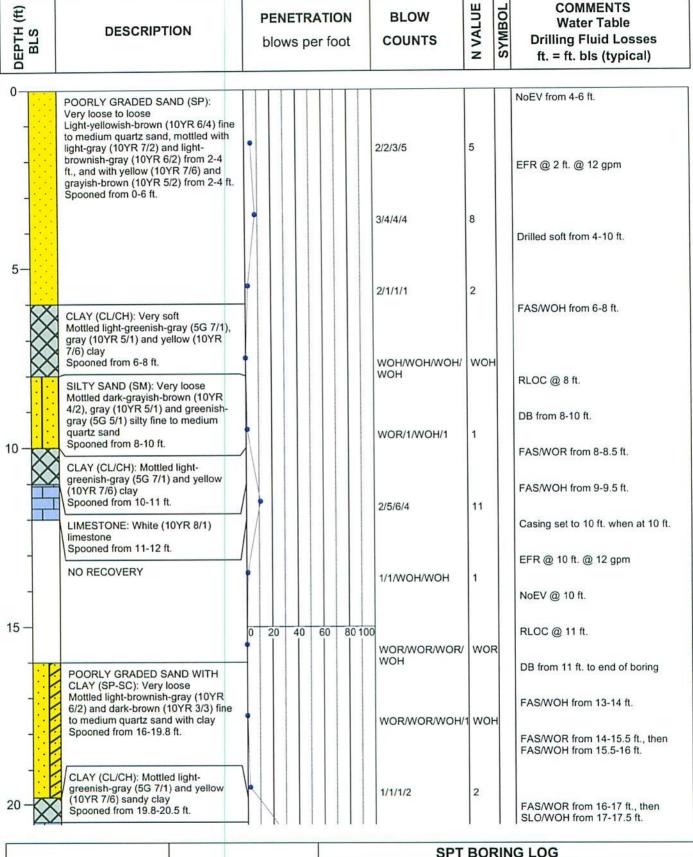
SPT BORING LOG

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SITE LOCATION: Spring Hill, Florida

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CHECKED BY: JMW
DRAWN BY: MVK

PROJECT NO: 8053622 BORING DATE: 11/16/12 PAGE NO: 2 of 2

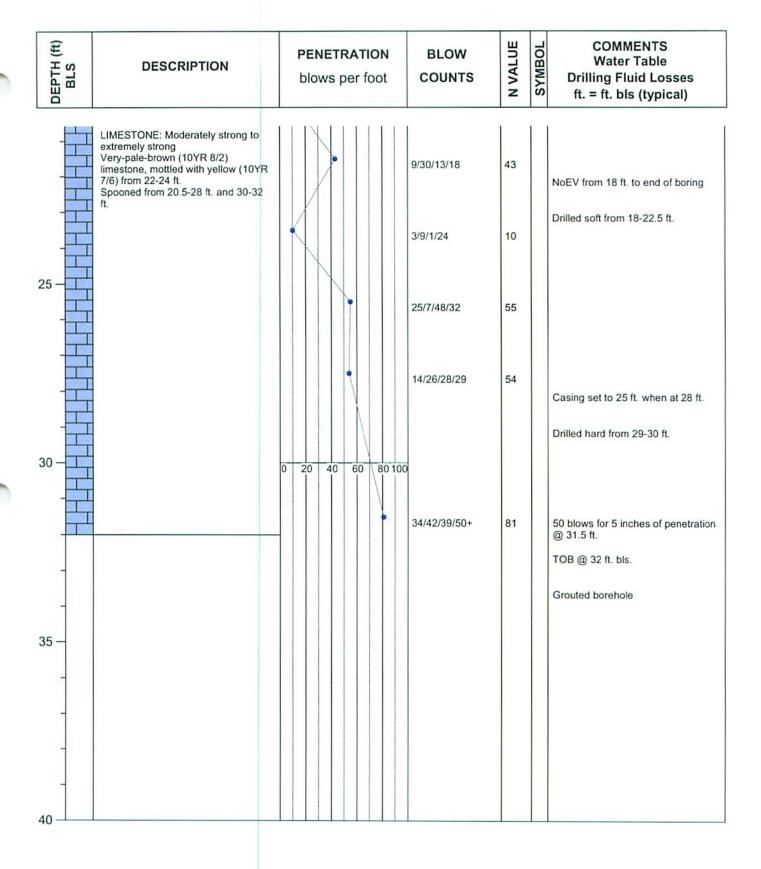




SITE NAME: PCRRF - South Stormwater Pond SITE LOCATION: Spring Hill, Florida

DESIGNED BY: CHECKED BY: JMW DRAWN BY: MVK

PROJECT NO: 8053622 **BORING DATE: 11/16/12** PAGE NO: 1 of 2





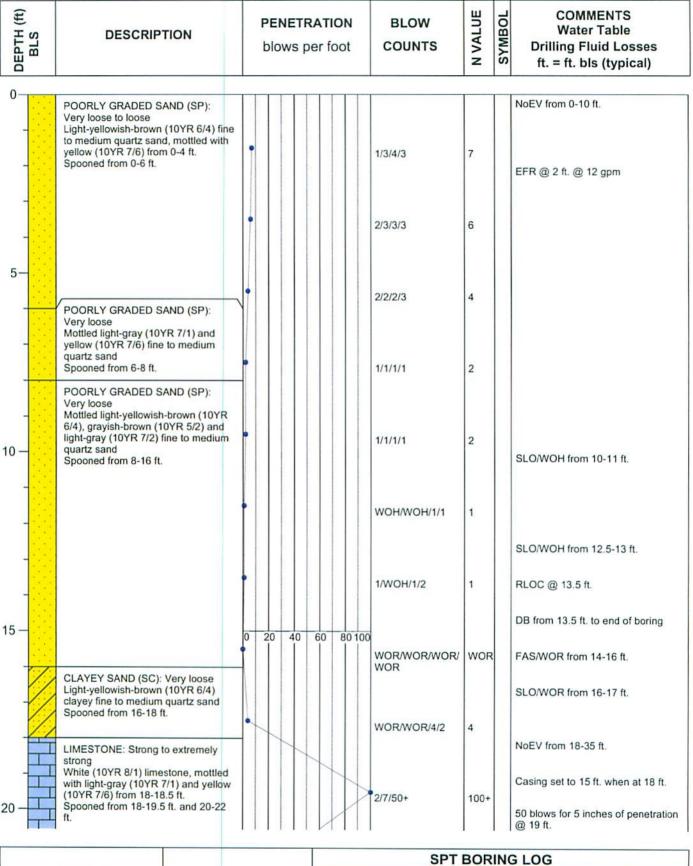
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PROJECT NO: 8053622 BORING DATE: 11/16/12 PAGE NO: 2 of 2

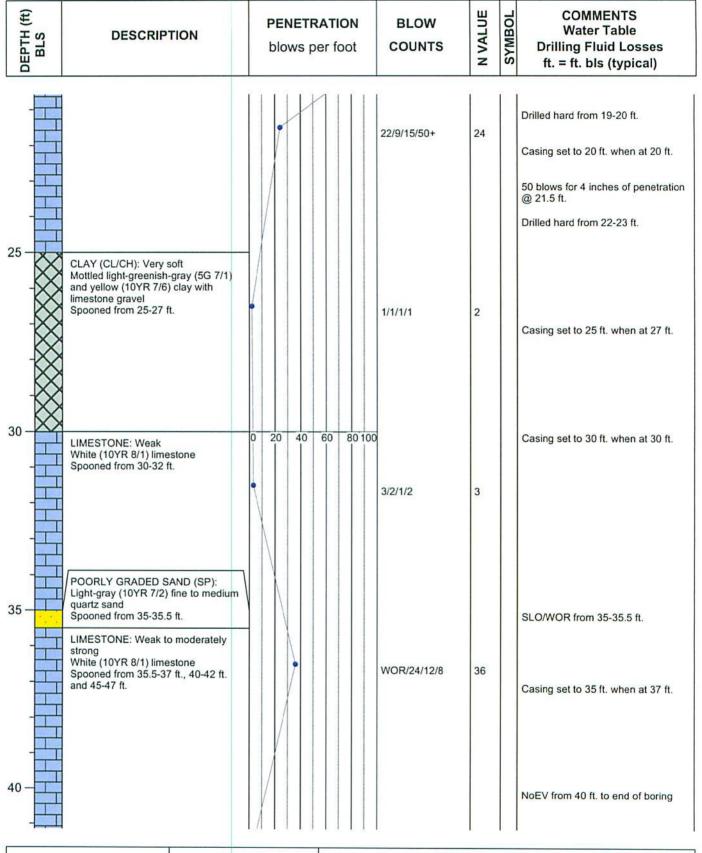




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PROJECT NO: 8053622 **BORING DATE: 11/19/12** PAGE NO: 1 of 3



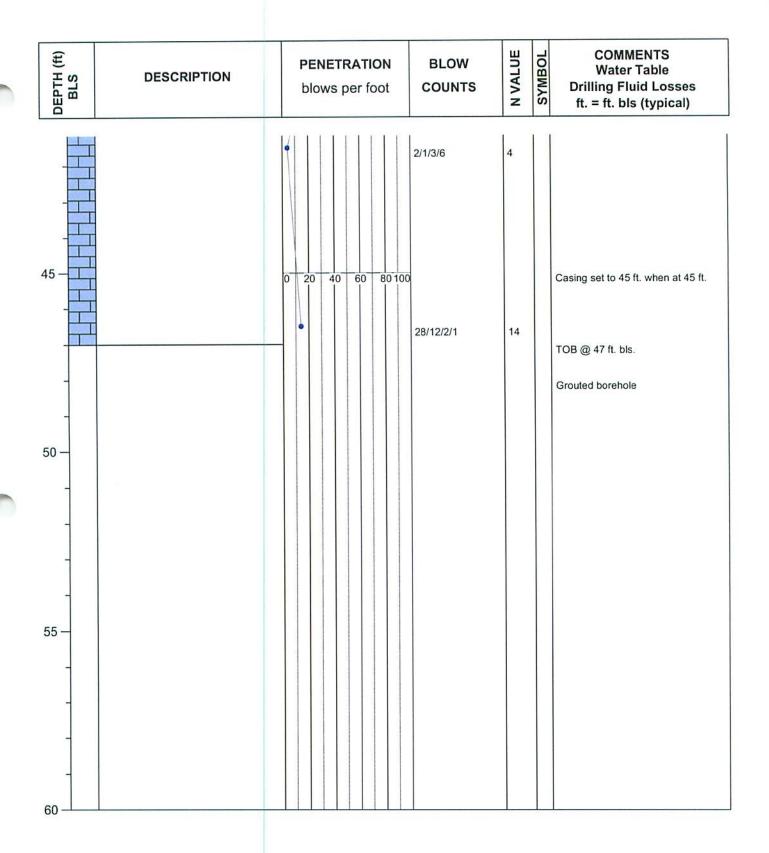


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DR **DESIGNED BY:** CHECKED BY: **JMW** DRAWN BY: MVK

PROJECT NO: 8053622 **BORING DATE: 11/19/12** PAGE NO: 2 of 3





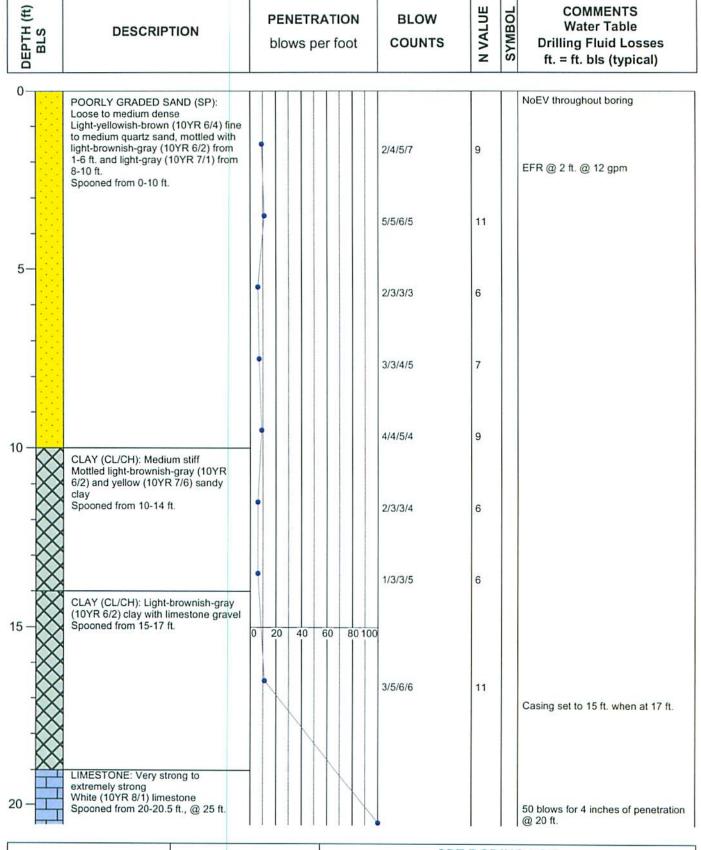
SPT BORING LOG

SITE NAME: PCRRF - South Stormwater Pond

SITE LOCATION: Spring Hill, Florida

DESIGNED BY: DR
CHECKED BY: JMW
DRAWN BY: MVK

PROJECT NO: 8053622 BORING DATE: 11/19/12 PAGE NO: 3 of 3





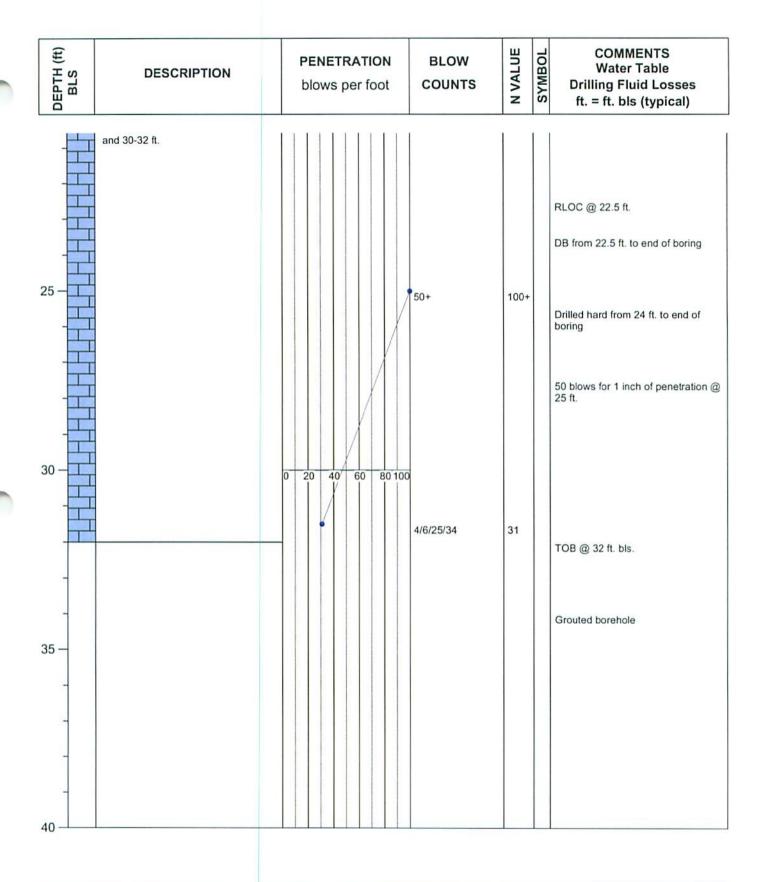
SPT BORING LOG

SITE NAME: PCRRF - South Stormwater Pond SITE LOCATION: Spring Hill, Florida

DESIGNED BY: DR PROJECT NO

CHECKED BY: JMW BOTTOM BY: MVK

PROJECT NO: 8053622 BORING DATE: 11/19/12 PAGE NO: 1 of 2





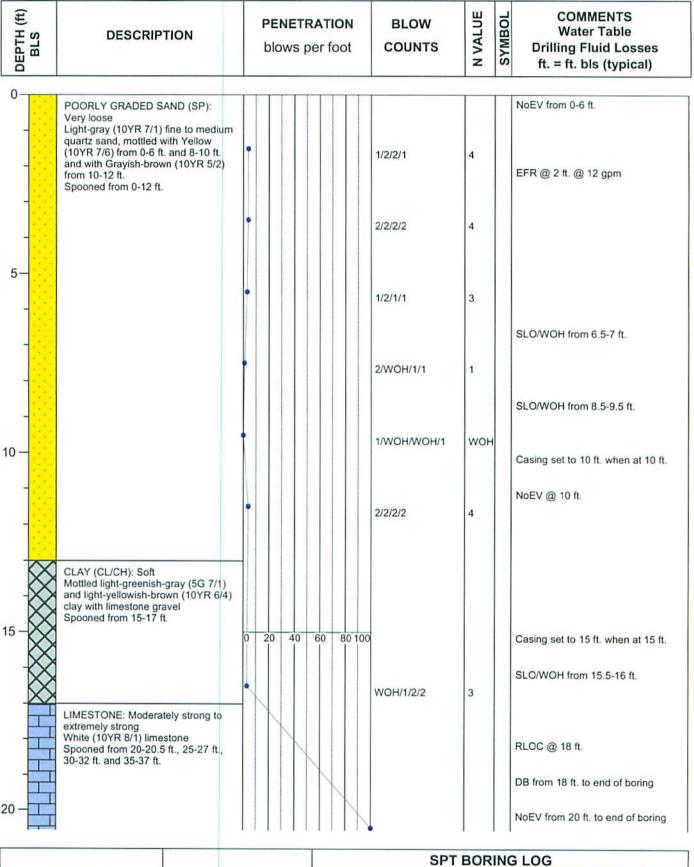
SPT BORING LOG

SITE NAME: PCRRF - South Stormwater Pond

SITE LOCATION: Spring Hill, Florida

DESIGNED BY: CHECKED BY: JMW DRAWN BY: MVK

PROJECT NO: 8053622 BORING DATE: 11/19/12 PAGE NO: 2 of 2



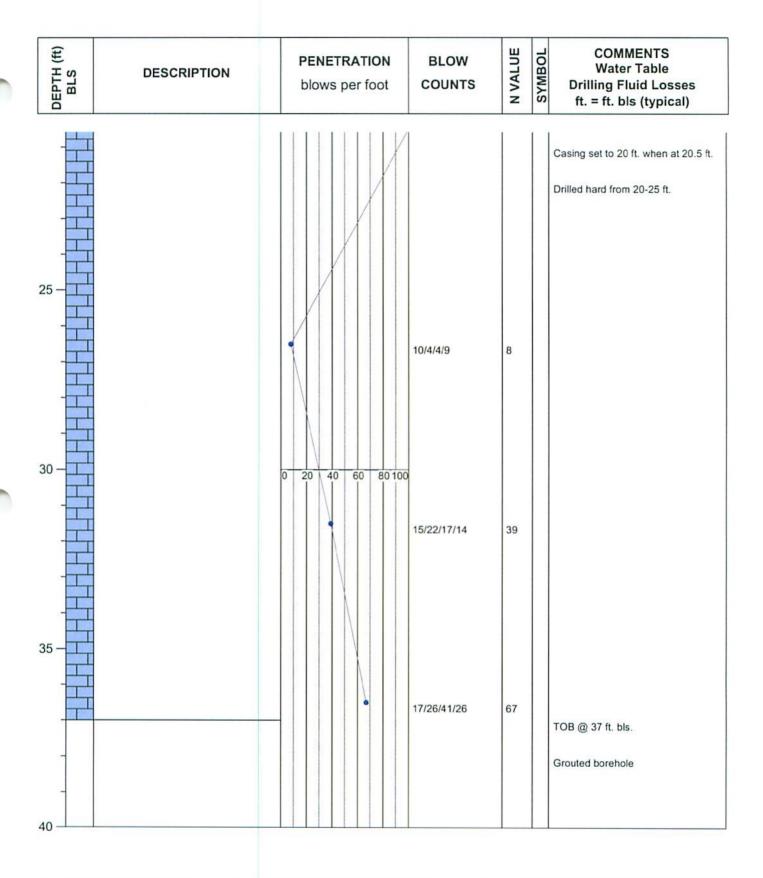


SITE NAME: PCRRF - South Stormwater Pond SITE LOCATION: Spring Hill, Florida

DESIGNED BY: DR PROJECT NO

CHECKED BY: JMW BORI DRAWN BY: MVK

PROJECT NO: 8053622 BORING DATE: 11/27/12 PAGE NO: 1 of 2





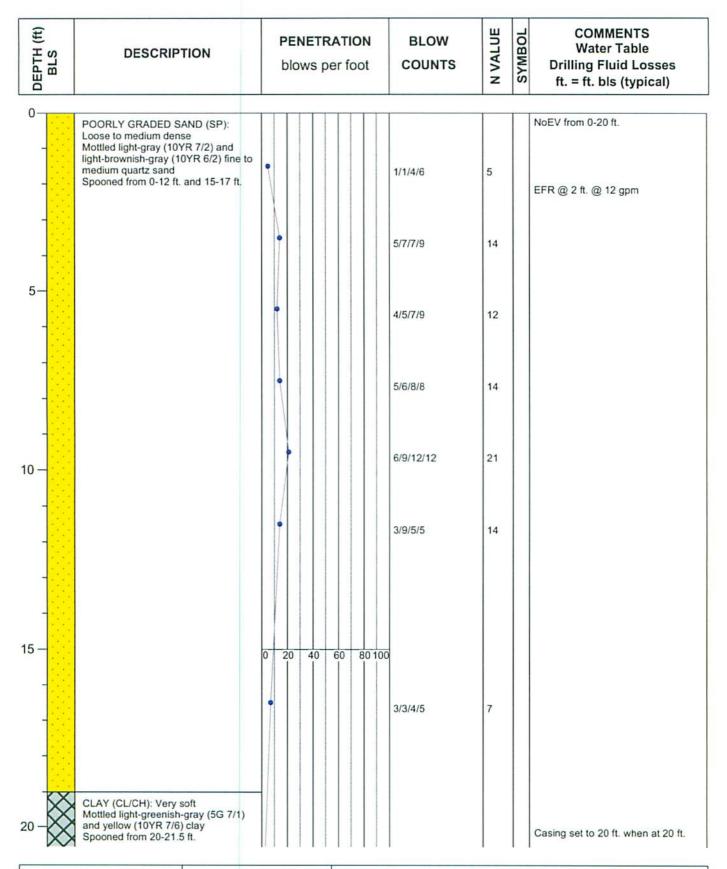
SPT BORING LOG

SITE NAME: PCRRF - South Stormwater Pond SITE LOCATION: Spring Hill, Florida

DESIGNED BY: DR PROJECT NO

CHECKED BY: JMW BO
DRAWN BY: MVK

PROJECT NO: 8053622 BORING DATE: 11/27/12 PAGE NO: 2 of 2





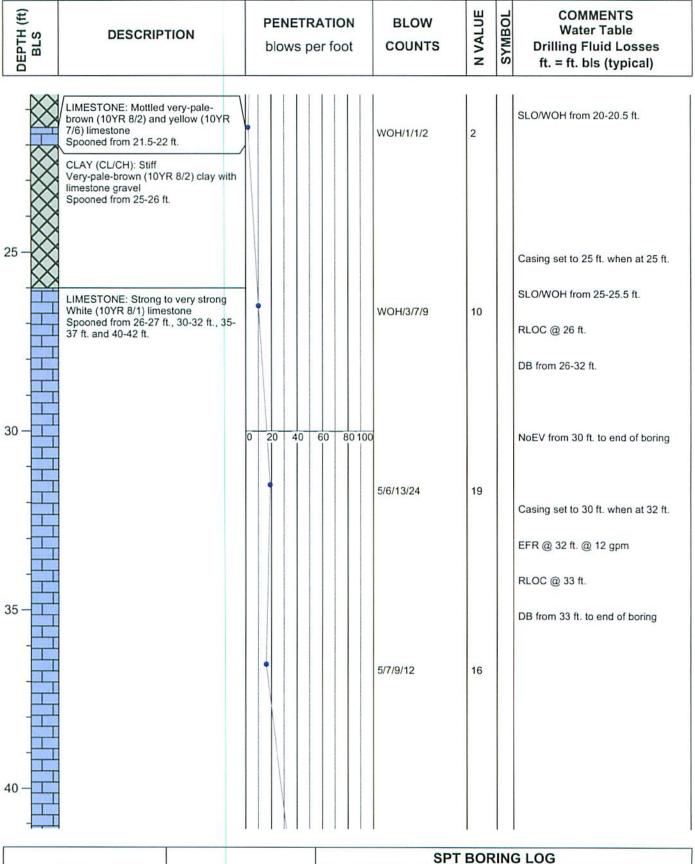
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PROJECT NO: 8053622 BORING DATE: 11/27/12 PAGE NO: 1 of 3

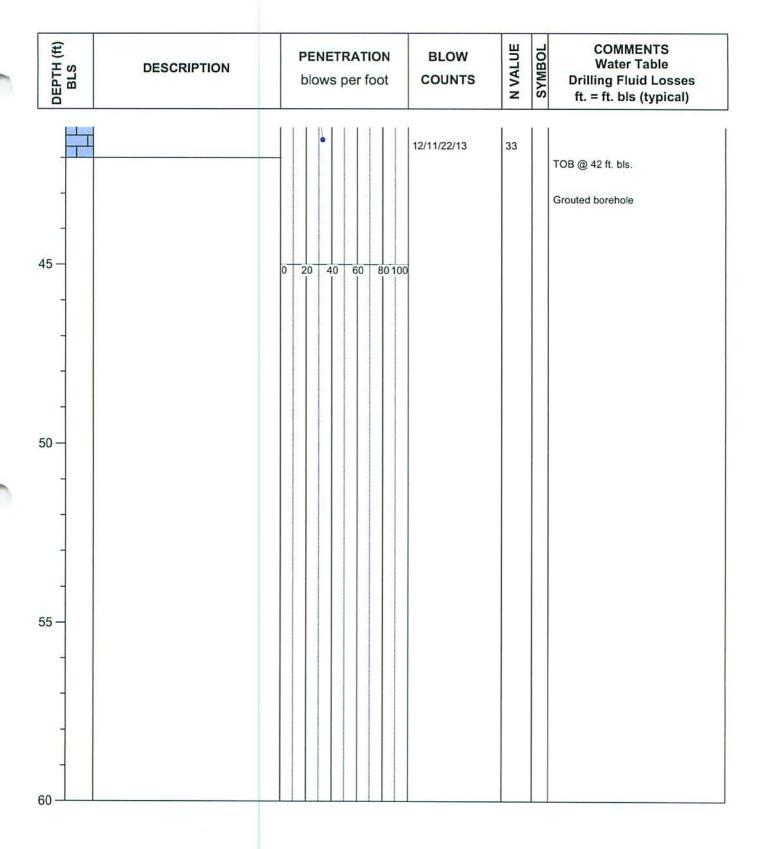




SITE NAME: PCRRF - South Stormwater Pond SITE LOCATION: Spring Hill, Florida

DESIGNED BY: CHECKED BY: **JMW** DRAWN BY: MVK

PROJECT NO: 8053622 **BORING DATE: 11/27/12** PAGE NO: 2 of 3



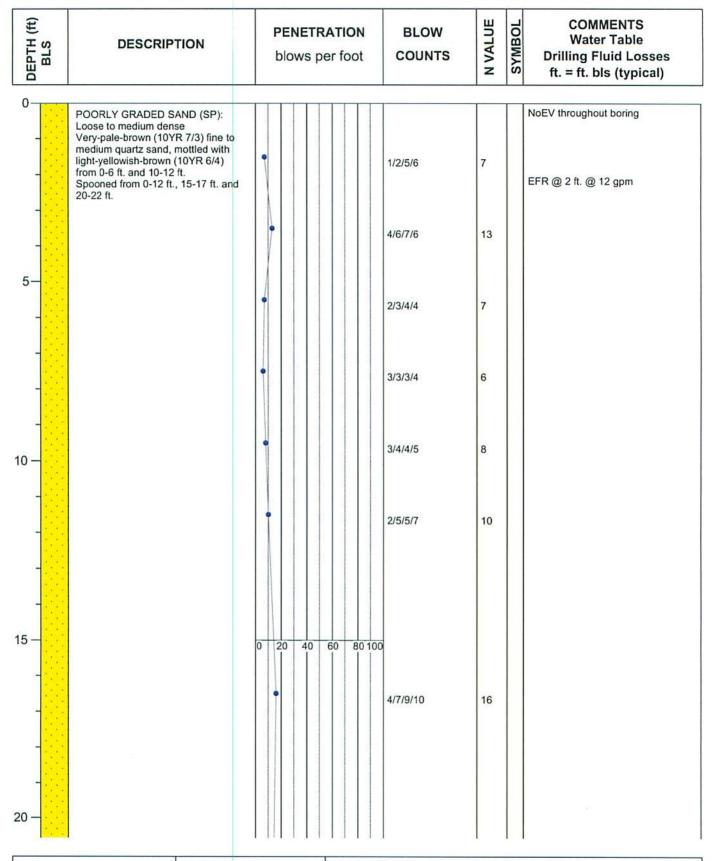


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SITE NAME: PCRRF - South Stormwater Pond SITE LOCATION: Spring Hill, Florida

DESIGNED BY: CHECKED BY: JMW DRAWN BY: MVK

PROJECT NO: 8053622 BORING DATE: 11/27/12 PAGE NO: 3 of 3





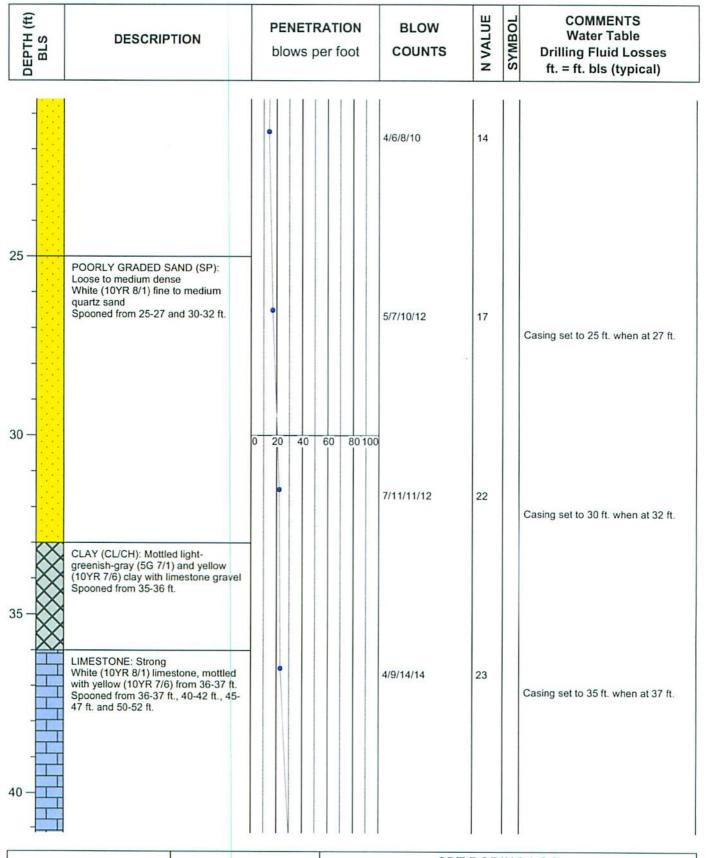
SPT BORING LOG

SITE NAME: PCRRF - South Stormwater Pond

SITE LOCATION: Spring Hill, Florida

DESIGNED BY: DR
CHECKED BY: JMW
DRAWN BY: MVK

PROJECT NO: 8053622 BORING DATE: 11/28/12 PAGE NO: 1 of 3





SPT BORING LOG

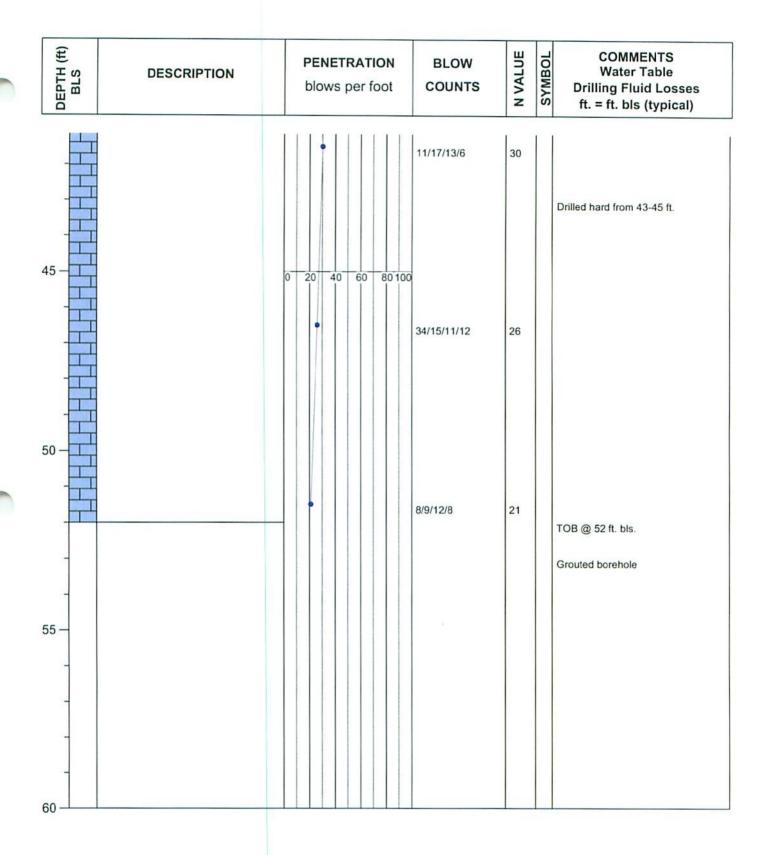
SITE NAME: PCRRF - South Stormwater Pond SITE LOCATION: Spring Hill, Florida

DESIGNED BY: DR PROJECT NO

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 DR
 PROJECT NO:
 8053622

 CHECKED BY:
 JMW
 BORING DATE:
 11/28/12

 DRAWN BY:
 MVK
 PAGE NO:
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SPT BORING LOG

SITE NAME: PCRRF - South Stormwater Pond

SITE LOCATION: Spring Hill, Florida

DESIGNED BY: DR PROJECT NO: 8053622
CHECKED BY: JMW BORING DATE: 11/28/12
DRAWN BY: MVK PAGE NO: 3 of 3

DEPTH (ft) BLS	DESCRIPTION	PENETRATION blows per foot	BLOW	N VALUE	SYMBOL	COMMENTS Water Table Drilling Fluid Losses ft. = ft. bls (typical)
0						
	POORLY GRADED SAND (SP): Loose to medium dense Light-gray (10YR 7/2) fine to medium quartz sand, mottled with light- brownish-gray (10YR 6/2) and yellow (10YR 7/6) from 0-6 ft. and with light-gray (10YR 7/1) from 6-9.5 ft. Spooned from 0-9.5 ft.	•	1/2/4/6	6		NoEV from 0-10 ft. EFR @ 2 ft. @ 12 gpm
		•	5/7/8/8	15		
5-		<u> </u>	3/4/6/4	10		
-		•	3/3/3/4	6		
10 -	CLAY (CL/CH): Very soft Mottled light-greenish-gray (5G 7/1) and yellow (10YR 7/6) sandy clay Spooned from 9.5-14 ft.		3/1/2/1	3		SLO/WOH from 10-11.5 ft., then FAS/WOH from 11.5-12 ft.
			WOH/WOH/WOH/ WOH	WOH		SLO/WOR from 12-12.5 ft., then SLO/WOH from 12.5-14 ft.
	CLAY (CL/CH): Very soft Mottled light-greenish-gray (5G 7/1),		WOR/WOH/WOH/ WOH	wон		SLO/WOR from 14-14.5 ft., then SLO/WOH from 14.5-15 ft.
15 –	yellow (10YR 7/6) and white (10YR 8/1) clay with limestone gravel Spooned from 14-17 ft.	0 20 40 60 80 100	WOR/WOH/1/2	1		NoEV from 16 ft. to end of boring
	LIMESTONE: Very strong White (10YR 8/1) limestone Spooned from 17-18 ft., 20-22 ft., 25- 27 ft. and 30-32 ft.		1/4/9/12	13		Casing set to 20 ft. when at 18 ft.
20						



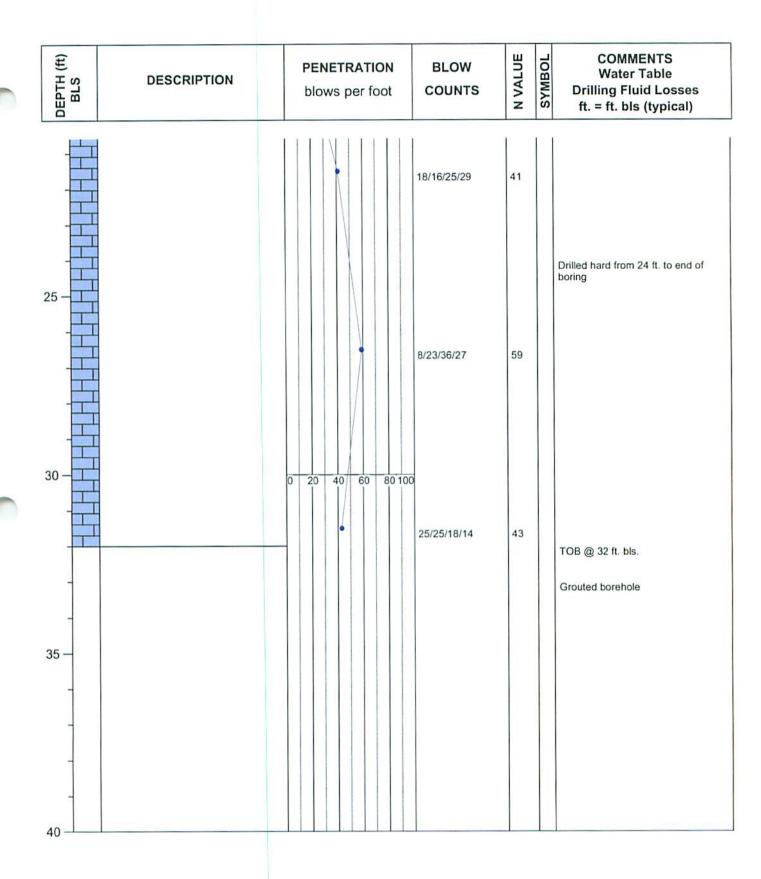
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PAGE NO: 1 of 2





SPT BORING LOG

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PROJECT NO: 8053622 BORING DATE: 11/28/12 PAGE NO: 2 of 2