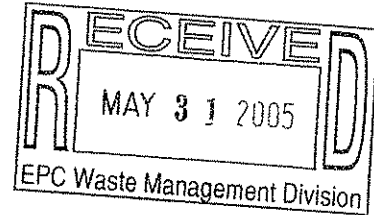


SCS ENGINEERS

May 31, 2005
File No. 09200020.15

Ms. Susan J. Pelz, P.E.
Florida Department of Environmental Protection
Southwest District
3804 Coconut Palm Drive
Tampa, Florida 33619



Subject: Southeast County Landfill Phosphatic Clay Annual Evaluation
Hillsborough County, Florida - Permit No. 35435-006-SO

Dear Mr. Ford:

On behalf of the Hillsborough County Solid Waste Department, SCS Engineers (SCS) is submitting the attached report regarding the evaluation of the performance of the waste phosphatic clays at the Southeast County Landfill (SCLF) as required by the above referenced permit, specifically Specific Condition No. 16.f. As stated in the attached report by Ardaman and Associates, Inc. (Ardaman), the piezocone soundings show that, under existing conditions, an upward gradient exists at the locations PC-1K, PC-1L, and PC-4F.

At the location of PC-3E, a slight downward potentiometric head 0.4 feet was measured (measured from the head on top of the clay versus the excess pore pressure in the middle of the clay). SCS concurs with Ardaman's assessment that this condition is not significant considering that the phosphatic clay layer has a permeability of less than 1×10^{-8} cm/sec and the pore pressures within the clays will increase upon placement of the next lift of waste in this area.

Based on the data obtained by Ardaman, the performance of the waste phosphatic clays are consolidating and strengthening in general accordance with the original design assumptions. SCS recommends that no further action be taken at this time and that the annual evaluation as required by Specific Condition No. 16.f be continued.

Please call should you have any questions or require additional information.

Sincerely,

A handwritten signature in black ink, appearing to read "Joe O'Neill".

Joseph H. O'Neill, P.E.
Project Manager
SCS ENGINEERS

A handwritten signature in black ink, appearing to read "Ray J. Dever".

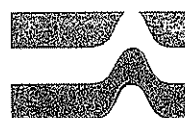
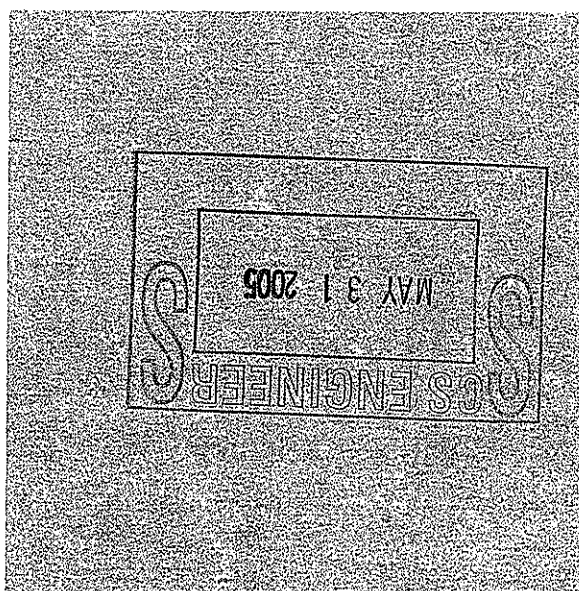
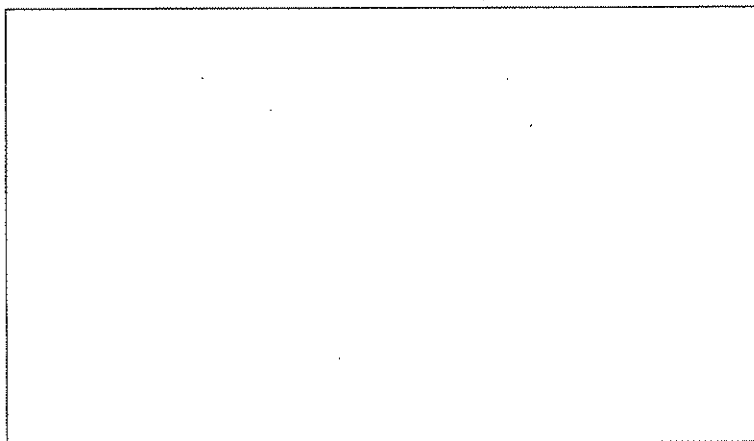
Raymond J. Dever, P.E., DEE
Vice President
SCS ENGINEERS

JHO/RJD:jho

cc: Patricia V. Berry, SWMD
Ron Cope, EPCHC



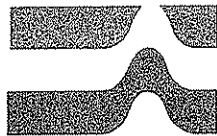
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Ardaman & Associates, Inc.

**Geotechnical Study Associated with
2005 Annual Monitoring of Phosphatic Clay Liner
Beneath the Southeast Landfill
in Hillsborough County**

May 27, 2005



Ardaman & Associates, Inc.

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MEMBERS:

A.S.F.E.

American Concrete Institute

American Society for Testing and Materials

Florida Institute of Consulting Engineers



Ardaman & Associates, Inc.

Geotechnical, Environmental and
Materials Consultants

May 27, 2005
File Number 05-010

SCS Engineers
3012 US Highway 301 North
Suite 700
Tampa, FL 33619

Attention: Mr. Joe O'Neill, P.E.

Subject: Geotechnical Study Associated with 2005 Annual Monitoring of Phosphatic Clay Liner
Beneath the Southeast Landfill in Hillsborough County

Gentlemen:

As requested by SCS Engineers (SCS), Ardaman & Associates, Inc., (Ardaman) has completed a geotechnical study associated with annual monitoring of the phosphatic clay liner beneath the Southeast Landfill in Hillsborough County. The annual monitoring program was mandated by the Florida Department of Environmental Protection (FDEP) under Specific Condition No. 16f of the Landfill Operation Permit No. 35435-006-SO issued on June 25, 2002, which requires performance of piezocone soundings and measurements of pore water pressures in the vicinity of the following four test sites where a number of piezocone soundings and pore pressure measurements had previously been performed by Ardaman and Madrid Engineering Group, Inc., (Madrid) in 2001/2002: (i) PC-1B in the Phase I area; (ii) PC-4B/PC-4C in the Phase IV area; (iii) PC-3/PC-3B in the Phase III area; and (iv) PC-1F in the Phase I area. Specifically, the permit condition required documentation and interpretation of the following data:

- Piezometric elevations in the drainage sand layer above the phosphatic clay.
- Top and bottom elevations of the phosphatic clay layer.
- Pore water pressures near the top, middle, and bottom of the phosphatic clay layer.
- Piezometric elevations in the natural soils below the phosphatic clay.

Evaluations of the phosphatic clay liner in 2003 and 2004 were documented in two Ardaman reports dated April 12, 2003 and April 15, 2004. This report contains the results of the piezocone soundings and pore pressure measurements obtained by Ardaman in March and April 2005, and presents our interpretation of the test data.

For this year, in addition to the four test sites in the Phase I, III, and IV areas required by the FDEP permit, SCS requested Ardaman to perform an electric Dutch cone sounding at a test site in the Phase VI area for the sole purpose of determining the top elevation of the phosphatic clay.

Site Location

The Southeast Landfill is located within Sections 14, 15, 22, and 23 of Township 31 South, Range 21 East, in Hillsborough County, Florida. More specifically, the landfill site is located between

Picnic and Pinecrest, about 2 miles west of State Road 39 and about 0.5 miles north of County Road 672. The approximate site location, as superimposed on a reproduction of the United States Geological Survey (USGS) quadrangle map of Lithia, Florida (1955, photorevised 1987), is shown in Figure 1.

Project History

The Southeast Landfill is constructed directly above a waste clay settling area at a former phosphate mine known as Lonesome Phosphate Mine or Boyette Mine. The settling area, also known as Settling Area No. 1, was built on natural ground within a perimeter dike constructed of sand borrowed from surrounding areas. Waste phosphatic clay was deposited within the settling area for a number of years during the mining operation.

A comprehensive geotechnical study was conducted by Ardaman between 1981 and 1983 to characterize the phosphatic clay deposit and to evaluate the feasibility of constructing a landfill within the waste clay settling area. Results from that study were documented in an Ardaman report titled "Hydrogeological Investigation, Southeast County Landfill, Hillsborough County, Florida", dated February 22, 1983. Based on the data and analyses documented in that report, Ardaman concluded that a landfill could be constructed directly on top of the phosphatic clay. However, to maintain an adequate factor of safety against slope failure, the waste disposal area was divided into different phases, and each phase had to be filled in lifts such that filling above a previous lift would occur only when the underlying phosphatic clay had consolidated under the weight of the previous refuse lift and experienced sufficient increase in shear strength to support the additional load. In areas where the clay thickness was greater than 14 feet, it was recommended that the clay should be pre-loaded prior to placement of the first lift of refuse. A diagram that shows the original thickness of the phosphatic clay within the settling area, as reproduced from the 1983 Ardaman report, is shown in Figure 2. As shown, the phosphatic clay deposit had an original thickness that varied between 4 and 18 feet.

Another comprehensive geotechnical study was completed by Ardaman in 1994 in association with operation permit renewal for the Southeast Landfill. Results from that study were documented in an Ardaman report titled "Geotechnical Investigation at Southeast Landfill, Hillsborough County, Florida", dated March 7, 1994. The strength and consolidation properties of the phosphatic clay obtained from that study were in good agreement with those used in the original stability analyses and affirmed the recommended filling schedule.

In support of the last operation permit renewal application in 2002, SCS retained Madrid and Ardaman to perform supplemental studies to confirm the engineering properties of the phosphatic clay, and to determine whether the material had been consolidating and gaining strength as predicted and whether the 7-year waiting period for placements of successive refuse lifts in the landfill should be modified. Results from the latest study were presented in an Ardaman report titled "Geotechnical Study Associated with Operation Permit Renewal for Hillsborough County Southeast Landfill", dated March 4, 2002.

The original geotechnical investigation completed in 1983 and the follow-up studies completed in 1994 and 2002 recommended that each lift of refuse should have a thickness no greater than 20 feet and that a minimum waiting period of 7 years should be provided between placements of successive refuse lifts. These requirements were derived based on stability analyses using an undrained shear strength to effective vertical stress ratio of 0.21 and a coefficient of consolidation

of 1.5×10^{-4} cm²/sec for the waste phosphatic clay. The undrained shear strength to effective vertical stress ratio determines the magnitude of strength increase in the phosphatic clay, whereas the coefficient of consolidation governs the rate of strength increase.

Field Test Program

Current operation at the Southeast Landfill divides the waste disposal area into six phases designated Phases I through VI, as shown on a topographic site plan in Figure 3. The topographic site plan was generated from survey data obtained on January 18, 2005. As part of our scope of work for the annual monitoring program stipulated in Specific Condition No. 16f of the FDEP Permit No. 35435-006-SO issued on June 25, 2002, Ardaman performed piezocone soundings and pore water pressure measurements at four test sites within the Phase I, III, and IV areas where a number of field tests and measurements had previously been performed by Madrid and Ardaman in 2001/2002.

Four test sites, designated PC-1K, PC-1L, PC-3E, and PC-4F, were selected by SCS for performance of piezocone soundings and installation of piezoprobes. The approximate locations of these test sites are shown in Figure 3 along with the test site locations selected in the 2001/2002 studies performed by Madrid and Ardaman as well as the test site locations selected for the 2003 and 2004 annual monitoring programs. As shown in Figure 3, two of the four test sites were located in the Phase I area (i.e., PC-1K and PC-1L), one test site was located within the Phase III area (i.e., PC-3E), and the remaining test site was located within the Phase IV area (i.e., PC-4F). PC-1K was located in close proximity to the previous PC-1B, PC-1H, and PC-1J test site locations, and PC-1L was selected adjacent to the previous PC-1F and PC-1I test site locations. PC-3E was located near the previous PC-3, PC-3B, and PC-3D test site locations. PC-4F was selected adjacent to the PC-4E test site location used in the 2004 annual monitoring program. The current field work and testing were undertaken by Ardaman in March and April 2005.

For this year, in addition to performance of piezocone soundings and installation of piezoprobes at the above four test sites, SCS also requested the performance of an electric Dutch Cone sounding at a test site designated PC-6F in the Phase VI area for the sole purpose of determining the top elevation of the phosphatic clay layer.

The surveyed coordinates and ground surface elevations at the current test site locations, as provided by Pickett & Associates, Inc., are summarized in Table 1. The elevations were surveyed using both the NGVD29 and NGVD88 datums. Although Hillsborough County mostly uses the NGVD88 datum for elevation data, the elevation data used in the previous geotechnical studies were based on the NGVD29 datum. For ease of data comparisons, the elevations based on the NGVD29 datum are used in this report.

Piezocone Penetration Tests

The piezocone consists of a conical point attached to a steel rod and a friction sleeve. The test is performed by pushing the assembly into the soil at a constant rate of penetration. Resistance to penetration at the cone tip and on the friction sleeve are measured by load cells placed within the assembly, and the pore pressure in the soil is measured using a pressure transducer connected to the porous element placed near the cone tip. Prior to pushing of the piezocone through the waste phosphatic clay, a hollow stem auger was used to create a borehole through the refuse.

Results of the four piezocone penetration tests (i.e., PC-1K, PC-1L, PC-3E, and PC-4F) performed at the four test site locations are presented in Figures 4 through 7, respectively. As shown in the figures, results are presented in the form of tip resistance (i.e., the resistance to penetration at the cone tip), sleeve resistance (i.e., the resistance to penetration of the friction sleeve), pore pressure (i.e., the total pore water pressure including the pore pressure generated due to penetration of the cone), friction ratio (i.e., the ratio of sleeve resistance to tip resistance), soil type (i.e., soil classification), and approximate standard penetration test blow count values (i.e., SPT N values) versus depth.

Since sandy soils typically exist above and below the phosphatic clay, the depth and thickness of the phosphatic clay layer could be inferred by examining the variations of tip resistance and pore pressure with depth. The tip resistance and the pore pressure in a clayey soil are expected to be lower and higher, respectively, than those in a sandy soil. Higher friction ratios are generally indicative of clayey soil types, whereas lower ratios generally indicate the presence of silty and sandy soils. Sudden changes in tip resistance, pore pressure, and friction ratio are expected to occur at the interface between the drainage sand layer and the underlying phosphatic clay as well as the interface between the phosphatic clay and the underlying natural sandy soils.

Thickness of Refuse

Based on results of the auger borings and piezocone soundings, the refuse thicknesses at the four test sites (i.e., PC-1K, PC-1L, PC-3E, and PC-4F) in March 2005 ranged from approximately 35 feet at PC-3E to 65 feet at PC-1L.

PC-1K was located in the western portion of the Phase I area. The refuse thickness at PC-1K in March 2005 was estimated to be 50 feet, compared to a refuse thickness of 65 feet at PC-1J in March 2004 and a refuse thickness of 50 to 55 feet at PC-1H in March 2003. The reason for the variation in refuse thickness was because the test sites were not at identical locations. PC-1K was located approximately 110 feet northwest of PC-1J and at the toe of slope between the Phase I and IV areas. The land surface elevation at PC-1K was approximately 14 feet lower than the land surface elevation at PC-1J, but was very close to the land surface elevation at PC-1H.

PC-1L was located in the eastern portion of the Phase I area. The refuse thickness at PC-1L in March 2005 was estimated to be 65 feet, compared to a refuse thickness of 50 feet at the nearby PC-1I in March 2004 and a refuse thickness of 50 to 55 feet at the nearby PC-1G in March 2003.

The refuse thickness at PC-3E in March 2005 was approximately 35 feet. We understand that the refuse thickness in the Phase III area has remained approximately the same since 2001/2002.

The refuse thickness at PC-4F in March 2005 was slightly greater than 60 feet, compared to a refuse thickness of slightly greater than 50 feet at PC-4E in March 2004 and at PC-4D in March 2003. We understand that refuse was last placed near these test site locations in the Phase IV area in March/April 2003.

Filling Schedule

In March 2005, refuse was being placed in the Phase II area. The filling originated in the Phase I area that began in March 2003 and progressed from west to east towards the Phase II area. Filling occurred in the vicinity of PC-1K around June 2003 and in the vicinity of PC-1L around July

2004. A comparison of the surveyed data in the Phase I area indicated that the thickness of the refuse lift was approximately 16 to 17 feet. The previous refuse lift in the Phase I area was completed in August 1997.

Filling of the Phase III area began in December 1990 and ended in June 1994. In the Phase IV area, approximately 13 feet of refuse was placed in the vicinity of PC-4F in March/April 2003 (i.e., just prior to filling of the Phase I area).

Elevations and Thicknesses of Phosphatic Clay

Prior to landfill construction, the surface of the waste phosphatic clay within the former settling area was documented to have typical elevations in the range of +121 to +123 feet (NGVD), and the bottom of the phosphatic clay reportedly occurred at typical elevations ranging from +103 to +117 feet (NGVD). As indicated previously, the original thickness of the phosphatic clay ranged from 4 to 18 feet.

Based on results of the piezocone soundings, the top and bottom elevations of the phosphatic clay as well as the phosphatic clay thickness encountered at the four test sites are summarized in Table 2. As shown, the top elevations of the phosphatic clay ranged from a low of +110.7 feet (NGVD) at PC-1K to a high of +118.7 feet (NGVD) at PC-3E. The bottom elevations of the phosphatic clay layer ranged from a low of +103.3 feet (NGVD) at PC-1K to a high of +112.9 feet (NGVD) at PC-1L.

At the location of PC-1K, the top and bottom of the phosphatic clay layer were encountered at approximately +110.7 and +103.3 feet (NGVD), respectively, for a clay thickness of about 7.4 feet. This thickness was approximately 1.2 feet greater than the clay thickness (6.2 feet) documented at PC-1J in March 2004. As indicated previously, approximately 17 feet of refuse was placed in the vicinity of PC-1K in June 2003. Because the PC-1K test site in 2005 was located about 110 feet from the PC-1J test site in 2004, the apparent difference could be a result of variation of phosphate clay thickness between the two test sites.

Based on the piezocone sounding performed at the location of PC-1L, the phosphatic clay had top and bottom elevations of +116.9 and +112.9 feet (NGVD), respectively, for a phosphatic clay thickness of 4.0 feet, which is close to the thickness (3.6 feet) documented at the nearby PC-1I in 2004. As indicated previously, approximately 17 feet of refuse was placed in the vicinity of PC-1L in July 2004.

The piezocone sounding performed at the location of PC-3E revealed the top elevation of the phosphatic clay at +118.7 feet (NGVD) and the bottom elevation at +110.3 feet (NGVD), for a clay thickness of 8.3 feet, which is very close to the thickness (8.0 feet) documented at the nearby PC-3D test site in 2004. As indicated previously, no refuse has been placed in the Phase III area since June 1994.

Based on the piezocone sounding performed at the location of PC-4F, the top and bottom elevations of the phosphatic clay were documented at +113.7 and +106.8 feet (NGVD), for a clay thickness of 6.9 feet, which is 2.9 feet less than the thickness (9.8 feet) documented at the nearby PC-4E test site in 2004. As noted previously, approximately 13 feet of refuse was added at this location in April 2003.

In summary, the phosphatic clay elevations and thicknesses documented at the four test sites in 2005 (i.e., PC-1K, PC-1L, PC-3E, and PC-4F) indicated that there were minimal changes in clay thickness from March 2004 to March 2005 at the locations of PC-1L and PC-3E. The apparent increase in clay thickness at PC-1K from 2004 to 2005 could be attributed to variation of clay thickness between the 2004 and 2005 test sites. The observed decrease in phosphatic clay layer thickness at PC-4F from March 2004 to March 2005 (about 3 feet) could be attributed to compression of the phosphatic clay layer by refuse loading. However, it could also be a result of variation of phosphatic clay thickness between the test sites in 2004 and 2005.

Piezometric Elevations on Top of Phosphatic Clay

The piezometric heads in the drainage sand layer on top of the phosphatic clay could be inferred from the piezocone penetration test results. As the piezocone was pushed through the drainage sand layer on top of the phosphatic clay, it was held stationary at selected depths to allow the excess pore water pressure generated as a result of pushing of the piezocone to stabilize. Because of the relatively high permeability of the sand, any excess pore pressure should dissipate in a short duration. The pore pressures were monitored for several minutes to make sure that the final readings represented the stabilized pore pressures at the selected depths.

Based on the piezocone soundings performed at the four test sites, the piezometric heads in the drainage sand layer on top of the phosphatic clay are summarized in Table 3 and are further displayed in Figure 8. Piezometric heads documented from previous studies are also shown on the same figure for comparisons.

As shown in Table 3 and Figure 8, results of the piezocone soundings indicated that the piezometric heads on top of the phosphatic clay at the four test site locations ranged from 6.6 to 8.8 feet. The highest piezometric elevation of +125.3 feet (NGVD) was observed at PC-3E (i.e., 6.6 feet above the top of the phosphatic clay). The piezometric heads in the drainage sand on top of the phosphatic clay at PC-1K, PC-1L, and PC-4F were measured to be 8.8, 7.2 and 6.9 feet above the top of the phosphatic clay, respectively. On the average, the piezometric heads in the drainage sand layer in March 2005 were approximately 3 feet higher than the previous readings obtained in March 2004 and March 2003.

Piezometric Elevations Below Phosphatic Clay

Based on the piezocone sounding results, the piezometric elevations in the natural soils below the phosphatic clay are summarized in Table 4. As shown, the piezometric elevations at the test site locations were documented at approximately +118 to +122 feet (NGVD). These piezometric elevations were generally in good agreement with those obtained in March 2004 and March 2003.

Piezoprobe Tests Within Phosphatic Clay

The piezoprobe tests were performed by installing piezoprobes to pre-selected depths and holding them stationary until the excess pore pressure generated from probe penetration completely dissipated, and the measured pore pressure reached the actual pore pressure before probe penetration.

The dissipation of excess pore pressures generated due to probe penetrations are presented in Figures 9 to 12 in the form of normalized excess pore pressure (i.e., the ratio of excess pore water

pressure at any time to the initial excess pore water pressure immediately after piezoprobe penetration) versus time. As shown in the figures, all pore pressures reached equilibrium conditions at the end of the monitoring periods. The rate of dissipation of excess pore pressure generated by probe penetration can be used to estimate the *in situ* coefficient of consolidation of the phosphatic clay.

At the locations of PC-1K, PC-3E, and PC-4F with approximately 7 to 8 feet of phosphatic clay, piezoprobe tests were performed at three different depths to measure the pore pressures near the top, middle, and bottom of the phosphatic clay layer. Because of the limited phosphatic clay thickness at PC-1L (4 feet), only one piezoprobe was installed. Results from the piezoprobe tests are summarized in Table 5.

At the location of PC-1K, the equilibrium piezometric elevations for the three piezoprobes installed with tip elevations at +109.3, +107.2 and +105.2 feet (NGVD) were documented to be approximately +130.4, +135.4 and +132.0 feet (NGVD), respectively. The excess pore water pressures within the phosphatic clay layer at the tip elevations were estimated to be 10.4, 14.9 and 10.8 feet of water, respectively. As indicated previously, approximately 17 feet of refuse was placed in the vicinity of PC-1K in June 2003.

At the location of PC-1L, one piezoprobe test was performed with the piezoprobe tip elevation at +115.7 feet (NGVD). The piezometric elevation at that depth was documented to be +129.5 feet (NGVD), corresponding to an excess pore water pressure that is equivalent to approximately 7.1 feet of water. As indicated previously, approximately 17 feet of refuse was placed in the vicinity of PC-1L in July 2004. Because the phosphatic clay thickness at PC-1L (4.0 feet) was less than that at PC-1K (7.4 feet), the excess pore water pressure from refuse loading would have dissipated at a faster rate.

At the location of PC-3E, three piezoprobe tests were performed within the phosphatic clay layer with the piezoprobe tip elevations at +116.7, +114.7 and +112.7 feet (NGVD). The excess pore water pressures documented by the piezoprobes were equivalent to only 0.2 to 1.8 feet of water. This is not unexpected since there has been no refuse placed in the Phase III area since July 1994.

At the location of PC-4F, three piezoprobe tests were performed within the phosphatic clay layer with the piezoprobe tip elevations at +111.9, +110.2 and +108.4 feet (NGVD) with the equilibrium piezometric elevations at +124.7, +124.9 and +124.5 feet (NGVD), respectively. The excess pore water pressures near the top, middle, and bottom of the phosphatic clay were equivalent to 19.4, 29.0 and 31.9 feet of water, respectively. High excess pore water pressures were also observed in the Phase IV area in previous piezoprobe measurements in 2003 and 2004. The high excess pore water pressures could be attributed to refuse loading in the Phase IV area. As noted previously, approximately 13 feet of refuse was added in the vicinity of PC-4F in April 2003.

Coefficient of Consolidation

The coefficient of consolidation governs the rate of strength increase in the phosphatic clay upon loading from a refuse lift and thus the waiting period between placement of successive refuse lifts in an area. The filling schedule at the Southeast Landfill was originally based on a design vertical coefficient of consolidation, c_v , of 1.5×10^{-4} cm²/sec.

Based on the rates of excess pore pressure dissipation shown in Figures 9 through 12, the horizontal coefficients of consolidation, c_h , of the phosphatic clay obtained from the piezoprobe tests were calculated and are presented in Table 6. The *in situ* vertical coefficients of consolidation, c_v , can be estimated from the horizontal coefficients of consolidation, c_h , based on the following empirical relationship proposed by Baligh, M. et al (ASCE Journal of Geotechnical Engineering, Vol. 112, No. 7, July, 1986): $c_v = 0.05 \times c_h$, and are also presented in Table 6.

The *in situ* vertical coefficients of consolidation from the piezoprobe measurement at PC-1L and the piezoprobe measurements at the top of the phosphatic clay layer at PC-1K and PC-3E indicated an average c_v value of 1.7×10^{-4} cm²/sec, which is close to the design vertical coefficient of consolidation. However, the *in situ* vertical coefficients of consolidation from the piezoprobe measurements at mid-depth and bottom of the phosphatic clay layer at PC-1K and PC-3E had an average c_v value of 3.1×10^{-5} cm²/sec. The c_v values of the phosphatic clay layer at PC-4F averaged 2.8×10^{-5} cm²/sec for the three measurements at different depths.

Although the vertical coefficients of consolidation documented at the middle and bottom of PC-1K and PC-3E were lower than the original design value of 1.5×10^{-4} cm/sec, the piezoprobe measurements in 2003 and 2004 at nearby test sites had vertical coefficients of consolidation that were consistent with the design value. Because the Phase I area has just been recently loaded and no significant excess pore water pressure was observed in the phosphatic clay within the Phase III area, the lower vertical coefficients of consolidation documented in 2005 should not be an issue at this time. At PC-4F, because low vertical coefficients of consolidation were also documented at the nearby test site in 2004, Ardaman recommends performance of additional field tests to measure the undrained shear strength of the clay or any excess pore water pressure in the phosphatic clay prior to placement of the next lift of refuse in the Phase IV area.

Settlement plate data at the location of Pump Station B in the Phase VI area, as provided by Hillsborough County, are plotted in Figure 13. At this location, the initial thickness of the phosphatic clay was approximately 13 feet. As shown in Figure 13, the phosphatic clay has undergone a settlement of about 4 feet over a period of 5 years. Assuming that an average degree of consolidation of 95% has been reached under double drainage condition with an average consolidating thickness of 11 feet, the backcalculated c_v from the settlement observations is 2.0×10^{-4} cm²/sec, which is consistent with the design value.

Comparisons of Piezometric Heads

The piezometric elevations in the materials directly above and below the phosphatic clay layer, as documented from the piezocone soundings, are summarized in Table 7. The piezometric heads within the phosphatic clay layer are also shown on the same table for comparison.

The piezometric head within the phosphatic clay will be highest after loading of a new refuse lift and will decrease gradually as excess pore water pressure dissipates. If the piezometric head within the phosphatic clay is higher than the piezometric head on top of the phosphatic clay, there will be no downward migration of leachate. Once the excess pore water pressure from landfill loading dissipates, the flow direction through the phosphatic clay will be a function of the piezometric head difference across the phosphatic clay. If the piezometric elevation in the natural soils below the phosphatic clay is higher than the piezometric elevation on top of the phosphatic clay, upward flow will occur. Conversely, if the piezometric elevation in the natural soils is lower than the piezometric elevation on top of the phosphatic clay, leachate will migrate downward, at a rate governed by the

piezometric head difference, and the hydraulic conductivity and thickness of the phosphatic clay deposit.

As shown in Table 7, the existing maximum piezometric heads within the phosphatic clay layer are higher than the piezometric heads in the materials above and below the phosphatic clay at the PC-1K, PC-1L, and PC-4F test site locations. Accordingly, under existing condition, there should be no downward leachate migration or upward groundwater flow through the phosphatic clay layer at these locations. At the PC-3E test site location, the piezometric elevation on top of the phosphatic clay layer is slightly higher than the maximum piezometric head within the phosphatic clay and the piezometric head in the material below the phosphatic clay. This condition will result in downward leachate flow. However, the flow is not expected to be significant considering the thickness of the phosphatic clay layer (8 feet), the relatively small hydraulic head (< 4 feet), and the low hydraulic conductivity of the phosphatic clay material ($< 1 \times 10^{-8}$ cm/sec).

Electric Dutch Cone Sounding in Phase VI Area

For this year, SCS requested Ardaman to perform an electric Dutch cone sounding at a test site designated PC-6F in the Phase VI area for the sole purpose of determining the elevation at the top of the phosphatic clay. Results of the Dutch cone sounding test are presented in Figure 14. As shown in the figure, results are presented in the form of tip resistance (i.e., the resistance to penetration at the cone tip), sleeve resistance (i.e., the resistance to penetration of the friction sleeve), friction ratio (i.e., the ratio of sleeve resistance to tip resistance), soil type (i.e., soil classification), and approximate standard penetration test blow count values (i.e., SPT N values) versus depth. As shown in Table 2, the top of the phosphatic clay was encountered at a depth of 52.0 feet. The elevation of ground surface at the test location was at +167.31 feet (NGVD), and the top of the phosphatic clay was at elevation +115.3 feet (NGVD).

Summary of Observations

The following key observations were made from the 2005 annual monitoring data based on the field work completed:

- The refuse surface elevations increased by 17 and 13 feet compared to March 2004 at test locations PC-1L and PC-4F, respectively. At test location PC-3E, the existing grade remained unchanged. At test location PC-1K, the refuse elevation was about the same as test location PC-1H in 2003.
- The top elevations of the phosphatic clay were as expected and generally consistent with the data obtained in 2004.
- The thicknesses of the phosphatic clay were also generally consistent with the data obtained in 2004. The phosphatic clay thickness at PC-1K, PC-1L, and PC-3E were found to be higher by 1.2, 0.4 and 0.3 feet, respectively, compared to the thicknesses estimated in 2004. At PC-4F, the clay thickness was 2.9 feet less than that observed in PC-4E in 2004. The observed decrease in phosphatic clay layer thickness at PC-4F from March 2004 to March 2005 could be attributed to compression of the phosphatic clay layer by refuse loading. However, it could also be a result of variation of phosphatic clay thickness between the test sites in 2004 and 2005.

- A considerable amount of excess pore pressure was documented within the phosphatic clay layer at PC-1L, PC-1K, and especially PC-4F. Because these areas have been loaded recently, the excess pore pressure was not unexpected. There were minimal excess pore pressure documented within the phosphatic clay at PC-3E.
- The piezometric heads above the phosphatic clay at the PC-1K, PC-1L, PC-3E, and PC-4F test site locations were measured to be 8.8, 7.2, 6.6 and 6.9 feet, respectively. On the average, these measurements were about 3 feet higher than the readings obtained in 2004.
- The piezometric elevations below the phosphatic clay were observed to be at approximately +118 to +122 feet (NGVD). These elevations are similar to those observed in 2003 and 2004.

Recommendation


High excess pore water pressures at PC-4F were recorded but are not unexpected due to recent refuse placement in this area. However, because of the lower than expected coefficients of consolidation documented at different depths of the phosphatic clay layer at PC-4F, additional field tests should be performed to measure the undrained shear strength of the clay or any excess pore water pressure in the clay prior to placement of the next lift of refuse. For all other areas, the 7-year waiting period between placement of successive refuse lifts remain valid.


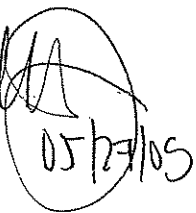
Closure

This report has been prepared for the exclusive use of SCS and the Hillsborough County Solid Waste Department for specific application to annual monitoring of the phosphatic clay liner at the Southeast Landfill in accordance with generally accepted geotechnical engineering practice. No other warranty, expressed or implied, is made.

Ardaman appreciates the opportunity to assist you on this project. Please contact us if you have any questions concerning this report or need additional information.

Very truly yours,
ARDAMAN & ASSOCIATES, INC.


Shawkat Ali, Ph.D., P.E.
Project Engineer


Francis K. Cheung, P.E.
Senior Project Manager
Florida License No. 36382


Enclosures

Table 1

Test Site Locations and Elevations

Test Site	Area	State Plan Coordinates		Approx Ground Surface Elevations (feet)	
		Northing	Easting	NGVD29	NGVD88
PC-1K	Phase I	1,250,443.55	595,886.47	+163.53	+162.62
PC-1L	Phase I	1,249,916.34	597,283.83	+188.66	+187.75
PC-3E	Phase III	1,251,089.80	597,409.63	+154.67	+153.76
PC-4F	Phase IV	1,250,656.68	596,423.09	+178.15	+177.24
PC-6F	Phase VI	1,250,983.97	596,262.97	+167.31	+166.40

Table 2

Top and Bottom Elevations of Phosphatic Clay

Area	Test Site	Date	Approx Ground Surface Elevation (ft, NGVD)	Top of Clay		Bottom of Clay		Clay Thickness (ft)
				Depth (ft, BLS)	Elevation (ft, NGVD)	Depth (ft, BLS)	Elevation (ft, NGVD)	
Phase I	PC-1K	03/14/05	+163.53	52.8	+110.7	60.2	+103.3	7.4
	PC-1L	03/16/05	+188.66	71.8	+116.9	75.8	+112.9	4.0
Phase III	PC-3E	03/11/05	+154.67	36.0	+118.7	44.3	+110.3	8.3
Phase IV	PC-4F	03/09/05	+178.15	64.5	+113.7	71.4	+106.8	6.9
Phase VI	PC-6F	03/03/05	+167.31	52.0	+115.3	--	--	--

Table 3

Piezometric Levels on Top of Phosphatic Clay

Area	Test Site	Date	Ground Surface Elevation (ft, NGVD)	Piezometric Elevation on Top of Phosphatic Clay (ft, NGVD)	Top of Clay Elevation (ft, NGVD)	Piezometric Head on Top of Phosphatic Clay (ft)
Phase I	PC-1K	04/14/05	+163.53	+119.5	+110.73	8.8
	PC-1L	04/12/05	+188.66	+124.2	+116.9	7.2
Phase III	PC-3E	04/14/05	+154.67	+125.3	+118.7	6.6
Phase IV	PC-4F	04/14/05	+178.15	+119.5	+113.7	6.9

Table 4

Piezometric Elevations Below Phosphatic Clay

Area	Test Site	Date	Ground Surface Elevation (ft, NGVD)	Piezometric Elevation Below Phosphatic Clay (ft, NGVD)
Phase I	PC-1K	03/14/05	+163.53	+121.7
	PC-1L	04/18/05	+188.66	+118.3
Phase III	PC-3E	04/25/05	+154.67	+121.8
Phase IV	PC-4F	03/09/05	+178.15	+118.4

Table 5

Pore Pressures Within Phosphatic Clay

Area	Test Site	Ground Surface Elevation (ft, NGVD)	Top of Clay Elevation (ft, NGVD)	Bottom of Clay Elevation (ft, NGVD)	Piezoprobe Designation	Piezoprobe Tip Below Ground Surface (ft)	Elevation of Piezoprobe Tip (ft, NGVD)	Stabilized Pore Pressure at Piezoprobe Tip After Dissipation (ft of H ₂ O)	Piezometric Elevation at Piezoprobe Tip Level (ft, NGVD)	Piezometric Elevation on Top of Clay (ft, NGVD)	Piezometric Elevation Below Clay (ft, NGVD)	Excess Pore Pressure at Piezoprobe Tip Level (ft of H ₂ O)
Phase I	PC-1K	+163.53	+110.7	+103.3	PC-1K	54.2 56.3 58.3	+109.3 +107.2 +105.2	21.1 28.2 26.7	+130.4 +135.4 +132.0	+119.5	+121.7	10.4 14.9 10.8
	PC-1L	+188.66	+116.9	+112.9	PC-1L	73.0	+115.7	13.8	+129.5	+124.2*	+118.2	7.1
Phase III	PC-3E	+154.67	+118.7	+121.2	PC-3E	38.0 40.0 42.0	+116.7 +114.7 +112.7	8.0 10.2 11.87	+124.7 +124.9 +124.5	+125.3	+121.8	0.2 1.3 1.8
	PC-4F	+1178.15	+113.7	+106.8	PC-4E	66.3 68.0 69.8	+111.9 +110.2 +108.4	26.7 37.8 42.2	+138.6 +148.0 +150.6	+119.5	+118.5	19.4 29.0 31.9

Computed Coefficients of Consolidation from Piezoprobe Tests

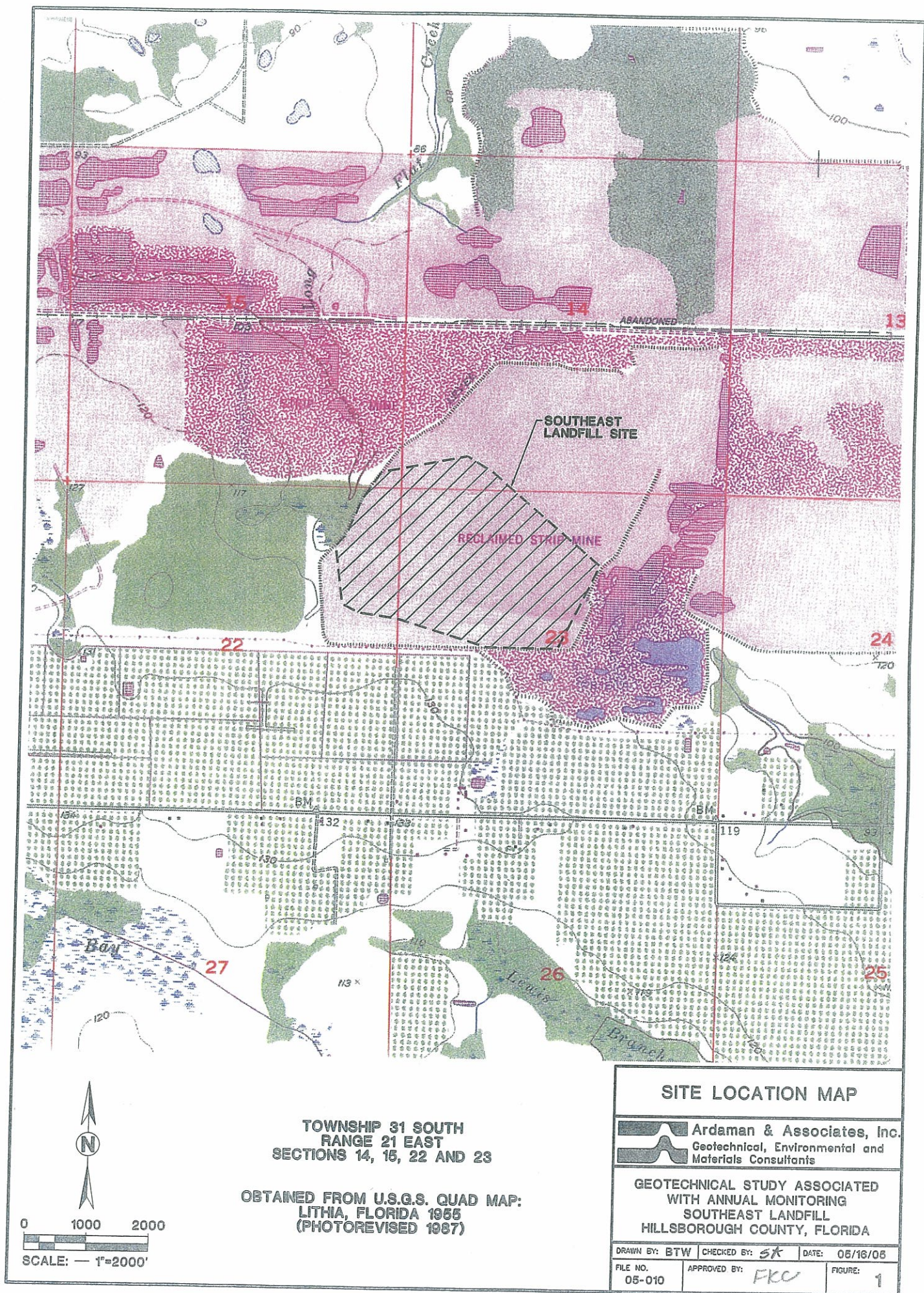
* c_v using empirical relationship recommended by Baligh et al., 1986.

Table 7

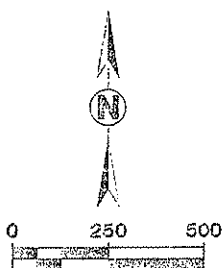
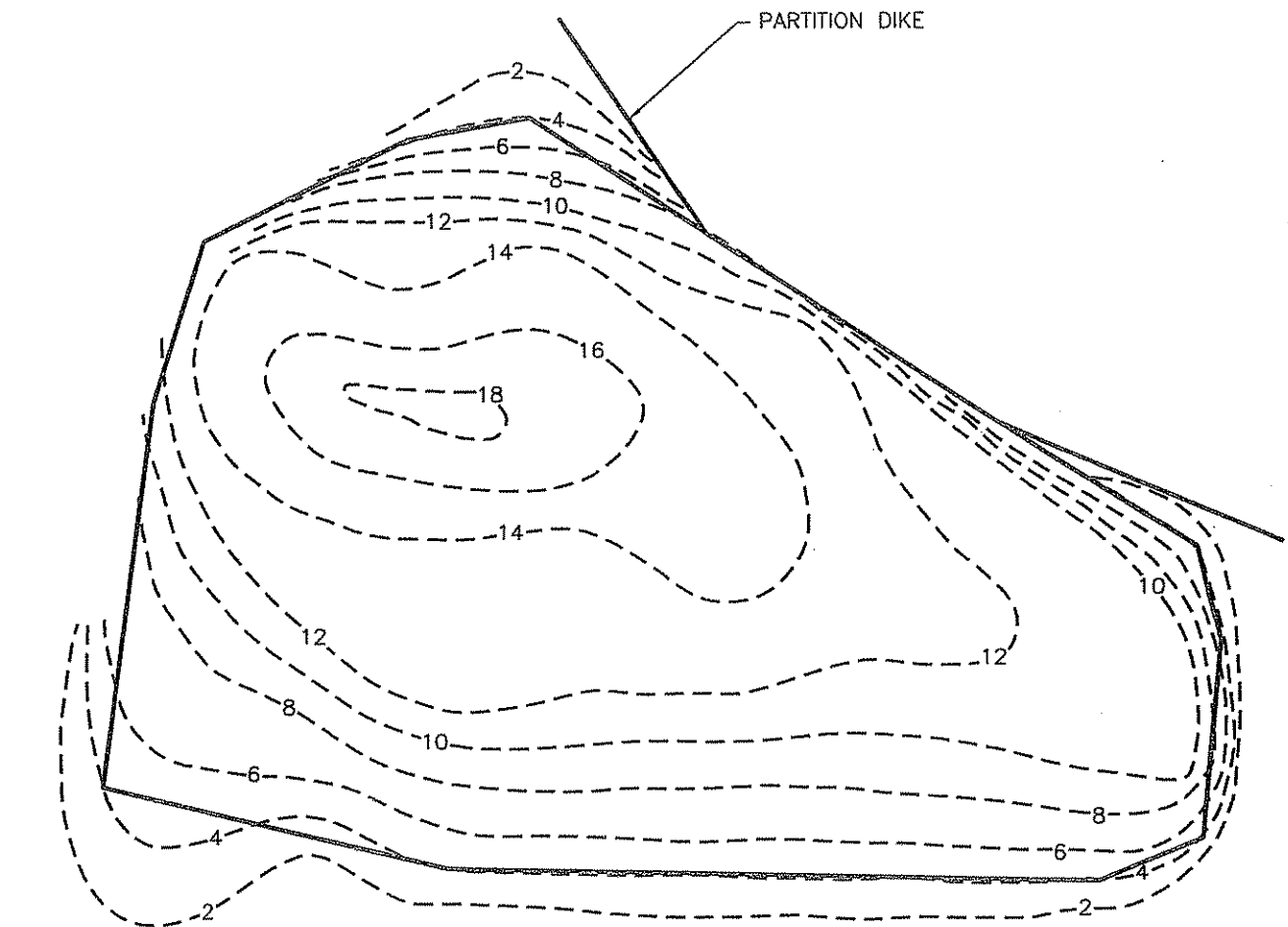
Comparisons of Piezometric Elevations

Area	Test Site	Date	Ground Surface Elevation (ft, NGVD)	Piezometric Elevation on Top of Phosphatic Clay (ft, NGVD)	Existing Maximum Piezometric Elevation Within Clay (ft, NGVD)	Piezometric Elevation Below Phosphatic Clay (ft, NGVD)	Location of Existing Maximum Head
Phase I	PC-1K	04/14/05	+163.53	+119.5	+135.5	+121.7	Within Clay
	PC-1L	04/12/05	+188.66	+124.2	+129.5	+118.3	Within Clay
Phase III	PC-3E	04/14/05	+154.67	+125.3	+124.9	+121.8	Above Clay
Phase IV	PC-4F	04/14/05	+178.15	+119.5	+150.6	+118.4	Within Clay

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- - - 10 - - -
CONTOURS OF CLAY
THICKNESS IN FEET

THICKNESS OF PHOSPHATIC CLAY BEFORE LANDFILL CONSTRUCTION

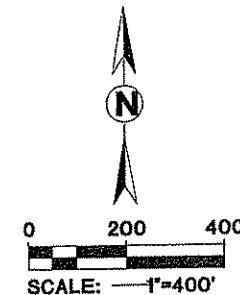
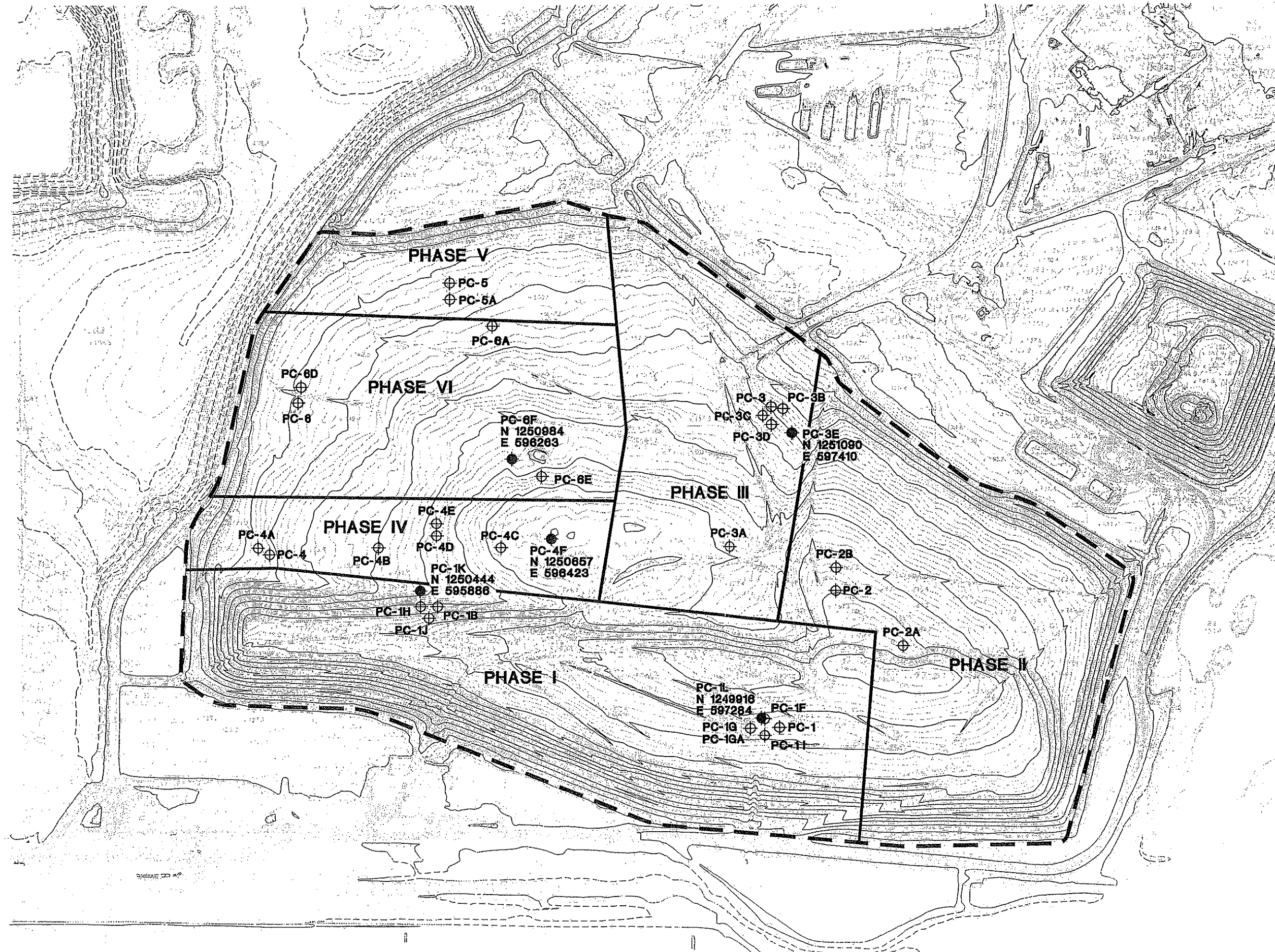
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Geotechnical, Environmental and
Materials Consultants

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WITH ANNUAL MONITORING
SOUTHEAST LANDFILL
HILLSBOROUGH COUNTY, FLORIDA

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FILE NO. 06-010	APPROVED BY: FCC	FIGURE: 2

NOTE: REPRODUCED FROM ARDAMAN & ASSOCIATES' 1981-1983 STUDY

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NOTE: THE TOPOGRAPHIC
SITE PLAN WAS BASED ON
SURVEY DATA OBTAINED
ON JAN 18, 2005.

LEGEND

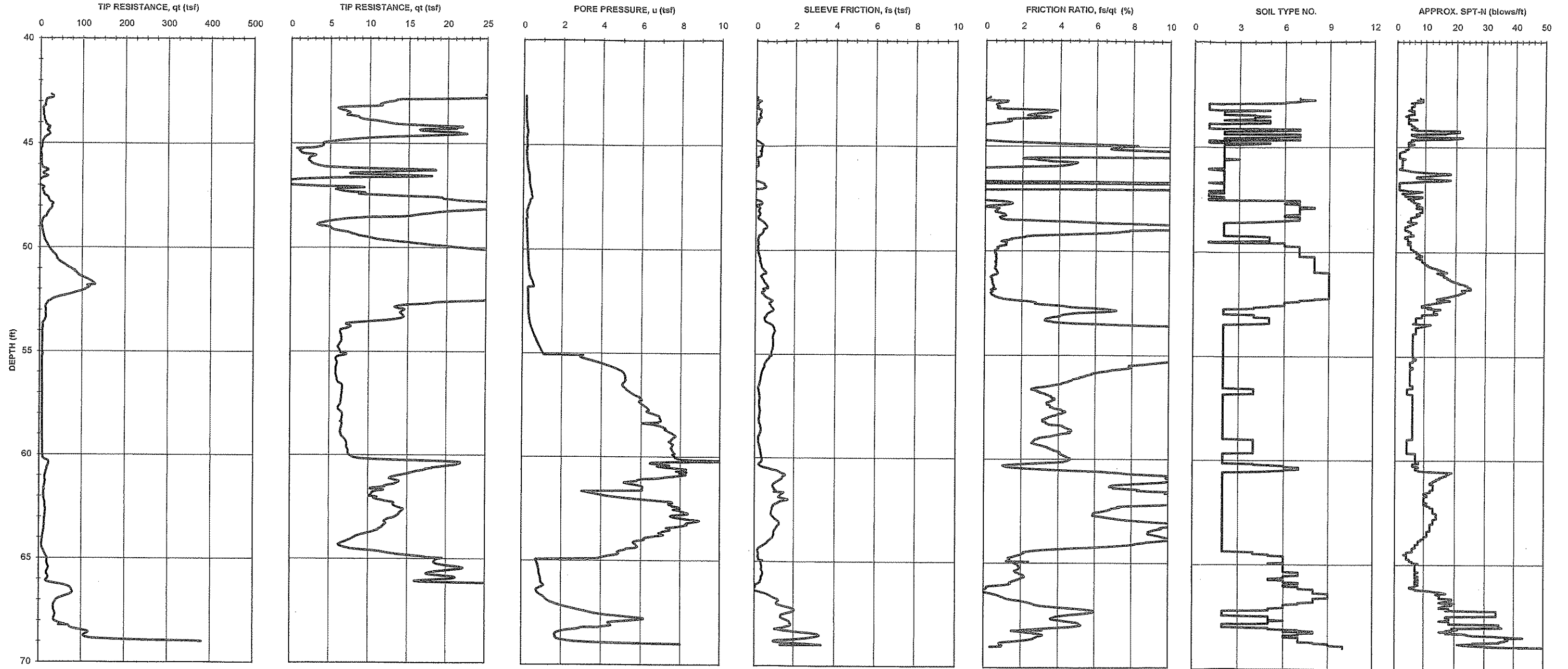
- ⊕ TEST SITES IN 2001-2004
- TEST SITES IN 2005

LANDFILL LAYOUT AND FIELD TEST LOCATION MAP

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FILE NO. 05-010	APPROVED BY: PKC	FIGURE: 3



Soil I.D. #	Soil Description	UCS
1	Sensitive Fine Grained	OH/CH
2	Organic Material	OH
3	Clay	CH
4	Silty Clay to Clay	CL/MH
5	Clayey Silt to Silty Clay	MH/CL
6	Silty Sand to Sandy Silt	SC

Soil Classification by Robertson et al, 1986


Soil I.D. #	Soil Description	UCS
7	Sand to Sandy Silt	SP/SC
8	Sand to Silty Sand	SP
9	Sand	SP/SW
10	Gravelly Sand to Sand	SP/GW
11	Very Stiff Fine Grained	OC Clay
12	Sand to Clayey Sand	Cemented

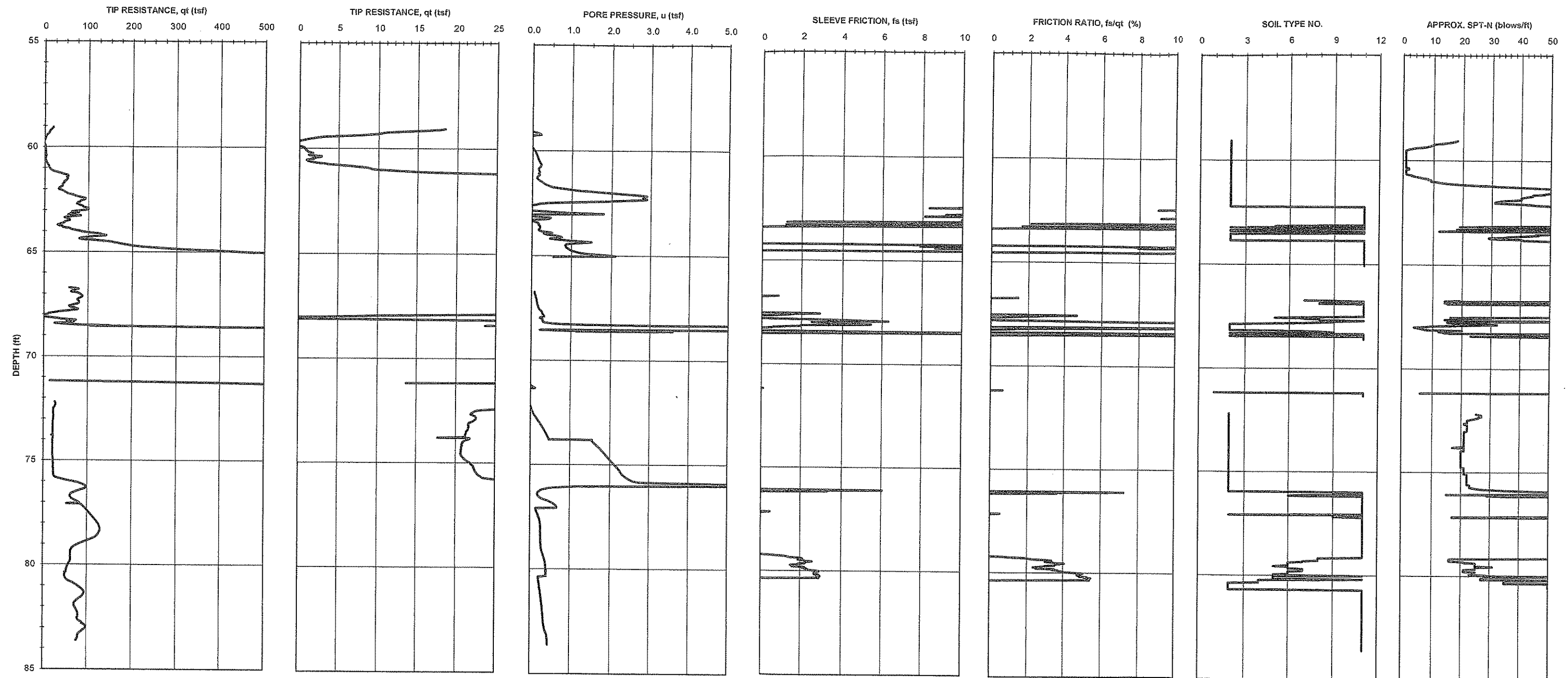
TEST SITE: PC-1K

SOUNDING DATE 3/14/05

SURFACE ELEVATION (FT, NGVD) 163.53

COORDINATE LOCATION (FT) N 1,250,443.55
E 595,886.47

 Ardaman & Associates, Inc. Piezocone Test Results		
GEOTECHNICAL STUDY ASSOCIATED WITH ANNUAL MONITORING		
SOUTHEAST LANDFILL HILLSBOROUGH COUNTY, FL		
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FILE NO.: 05-010	APPROVED BY: PKC	FIGURE: 4



Soil I.D. #	Soil Description	UCS
1	Sensitive Fine Grained	OH/CH
2	Organic Material	OH
3	Clay	CH
4	Silty Clay to Clay	CL/MH
5	Clayey Silt to Silty Clay	MH/CL
6	Silty Sand to Sandy Silt	SC

Soil Classification by Robertson et al., 1986


Soil I.D. #	Soil Description	UCS
7	Sand to Sandy Silt	SP/SC
8	Sand to Silty Sand	SP
9	Sand	SP/SW
10	Gravelly Sand to Sand	SP/GW
11	Very Stiff Fine Grained	OC Clay
12	Sand to Clayey Sand	Cemented

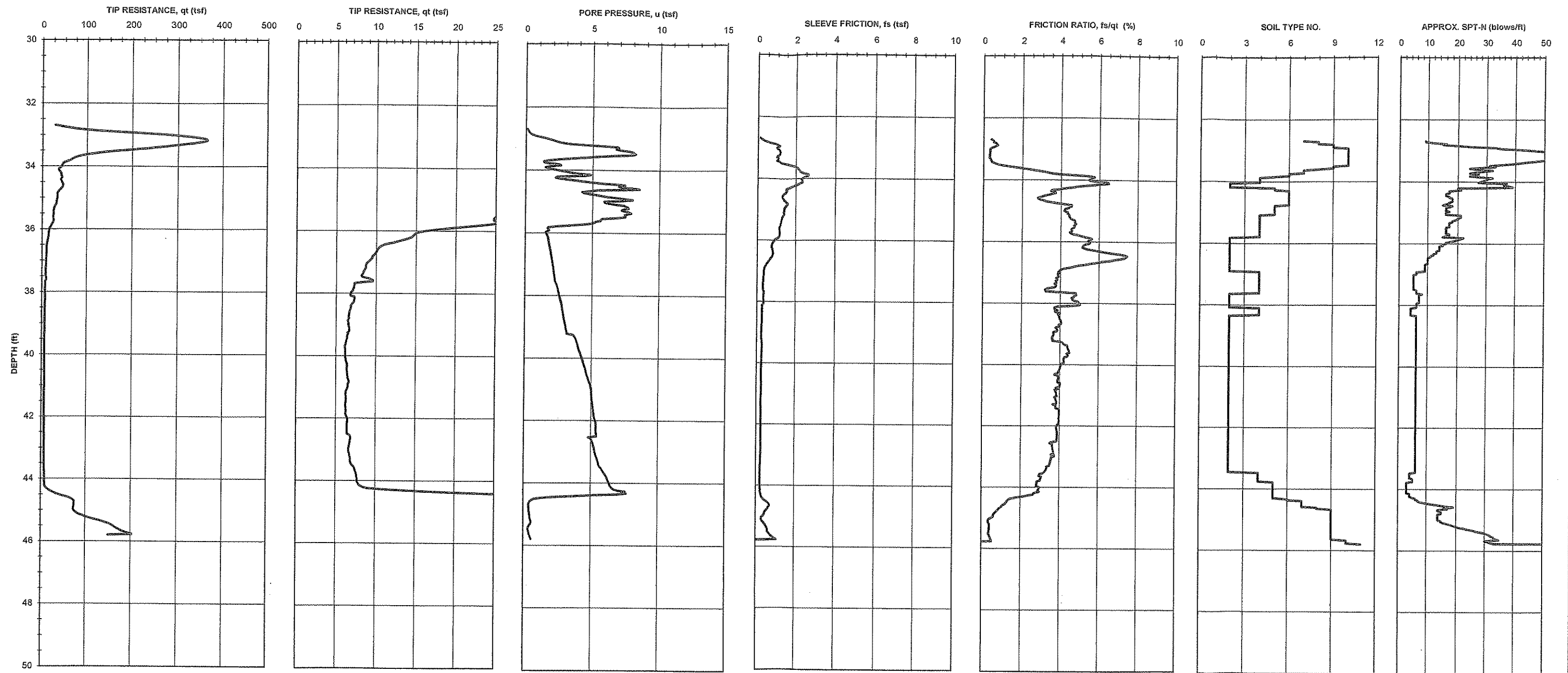
TEST SITE: PC-1L

SOUNDING DATE 3/16/05

SURFACE ELEVATION (FT, NGVD) 188.66

COORDINATE LOCATION (FT) N 1,249,916.34
E 597,283.83

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FILE NO. 05-010	APPROVED BY: <i>FCC</i>	FIGURE 5



Soil I.D. #	Soil Description	UCS
1	Sensitive Fine Grained	OH/CH
2	Organic Material	OH
3	Clay	CH
4	Silty Clay to Clay	CL/MH
5	Clayey Silt to Silty Clay	MH/CL
6	Silty Sand to Sandy Silt	SC

Soil Classification by Robertson et al., 1986


Soil I.D. #	Soil Description	UCS
7	Sand to Sandy Silt	SP/SC
8	Sand to Silty Sand	SP
9	Sand	SP/SW
10	Gravelly Sand to Sand	SP/GW
11	Very Stiff Fine Grained	OC Clay
12	Sand to Clayey Sand	Cemented

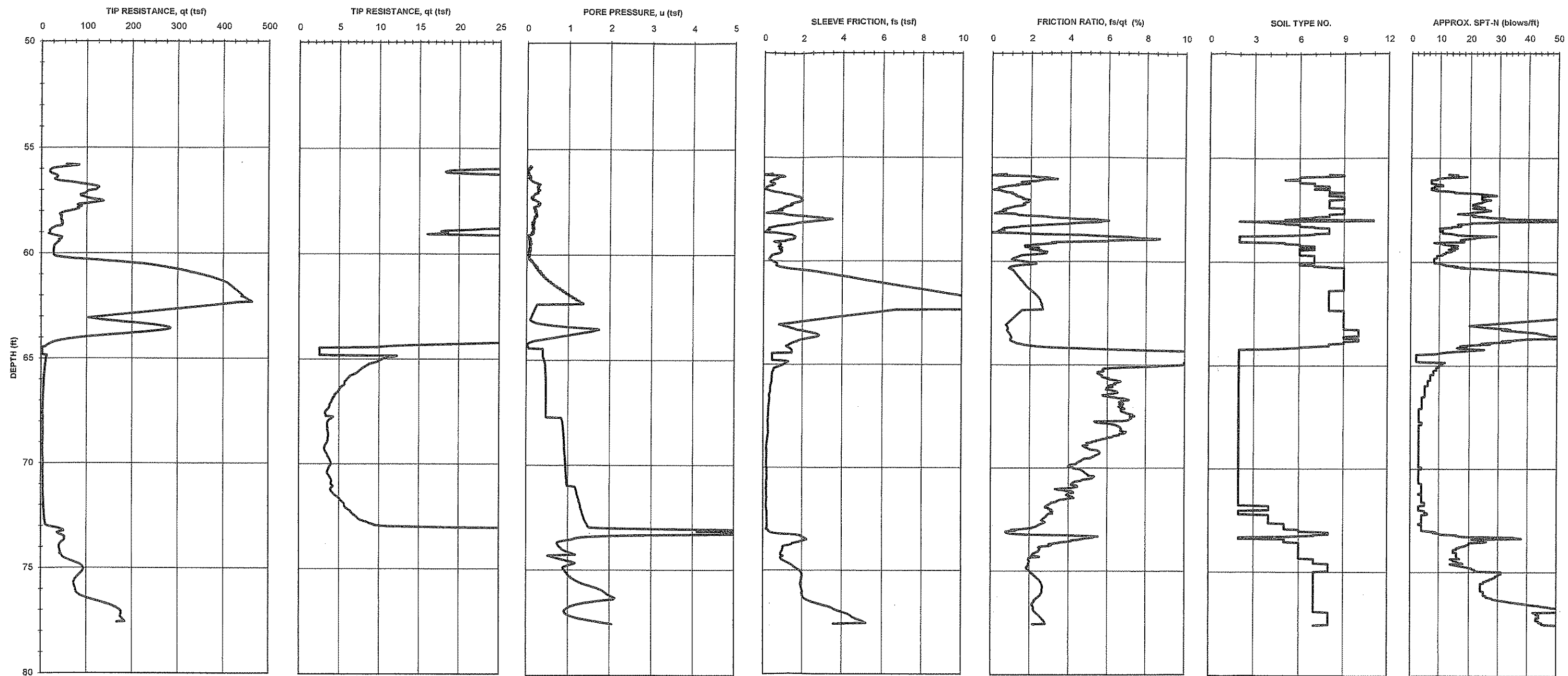
TEST SITE: PC-3E

SOUNDING DATE 3/11/05

SURFACE ELEVATION (FT, NGVD) 154.67

COORDINATE LOCATION (FT) N 1,251,089.80
E 597,409.63

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FILE NO.: 05-010	APPROVED BY: <i>[Signature]</i>	FIGURE: 6



Soil I.D. #	Soil Description	UCS
1	Sensitive Fine Grained	OH/CH
2	Organic Material	OH
3	Clay	CH
4	Silty Clay to Clay	CL/MH
5	Clayey Silt to Silty Clay	MH/CL
6	Silty Sand to Sandy Silt	SC

Soil Classification by Robertson et al., 1986


Soil I.D. #	Soil Description	UCS
7	Sand to Sandy Silt	SP/SC
8	Sand to Silty Sand	SP
9	Sand	SP/SW
10	Gravelly Sand to Sand	SP/GW
11	Very Stiff Fine Grained	OC Clay
12	Sand to Clayey Sand	Cemented

TEST SITE: PC-4F

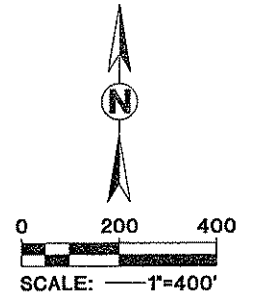
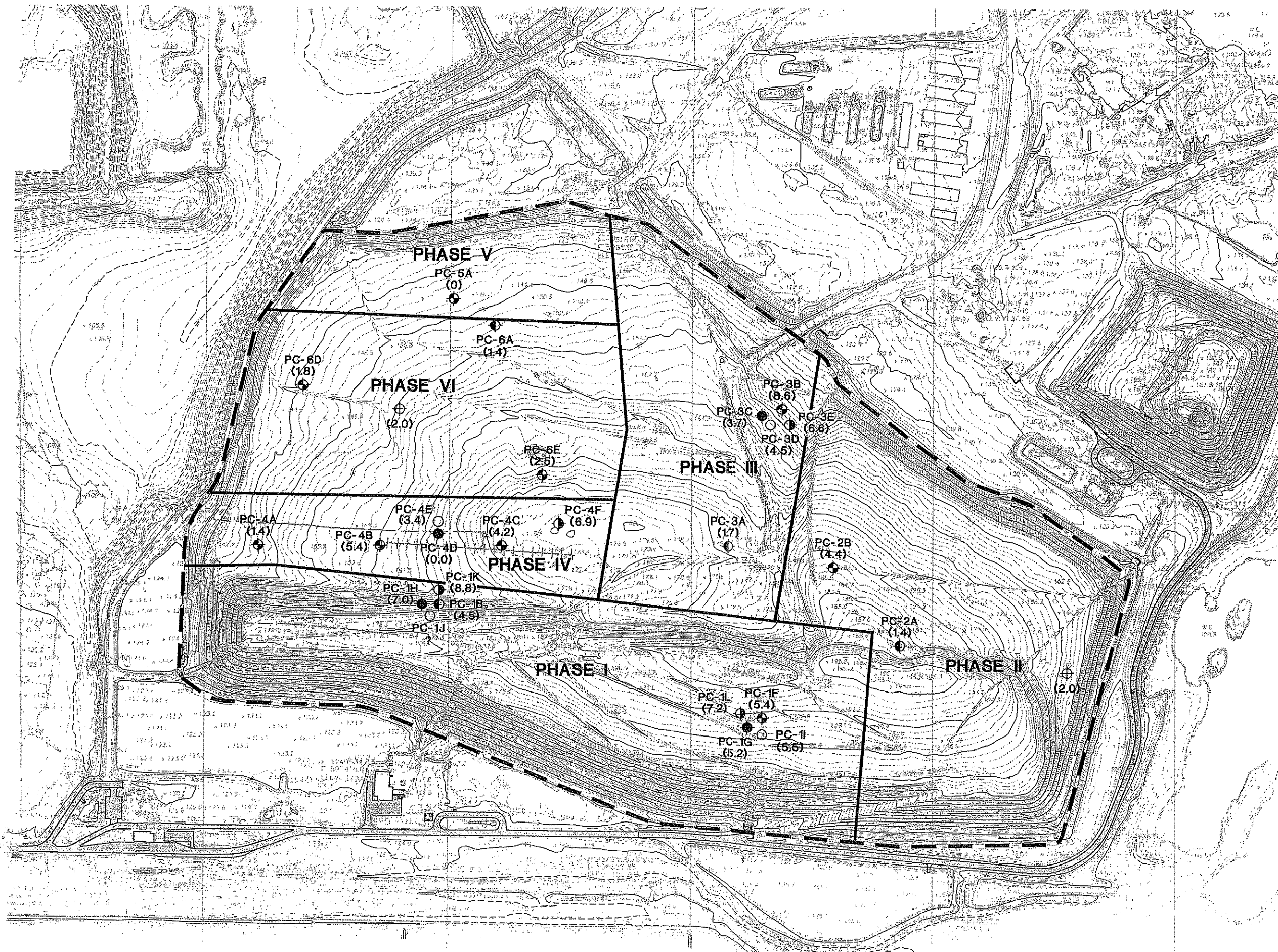
SOUNDING DATE 3/9/05

SURFACE ELEVATION (FT, NGVD) 178.15

COORDINATE LOCATION (FT) N 1,250,656.68
E 596,423.09

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GEOTECHNICAL STUDY ASSOCIATED WITH ANNUAL MONITORING		
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FILE NO.: 05-010	APPROVED BY: <i>PIC</i>	FIGURE: 7

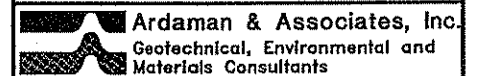
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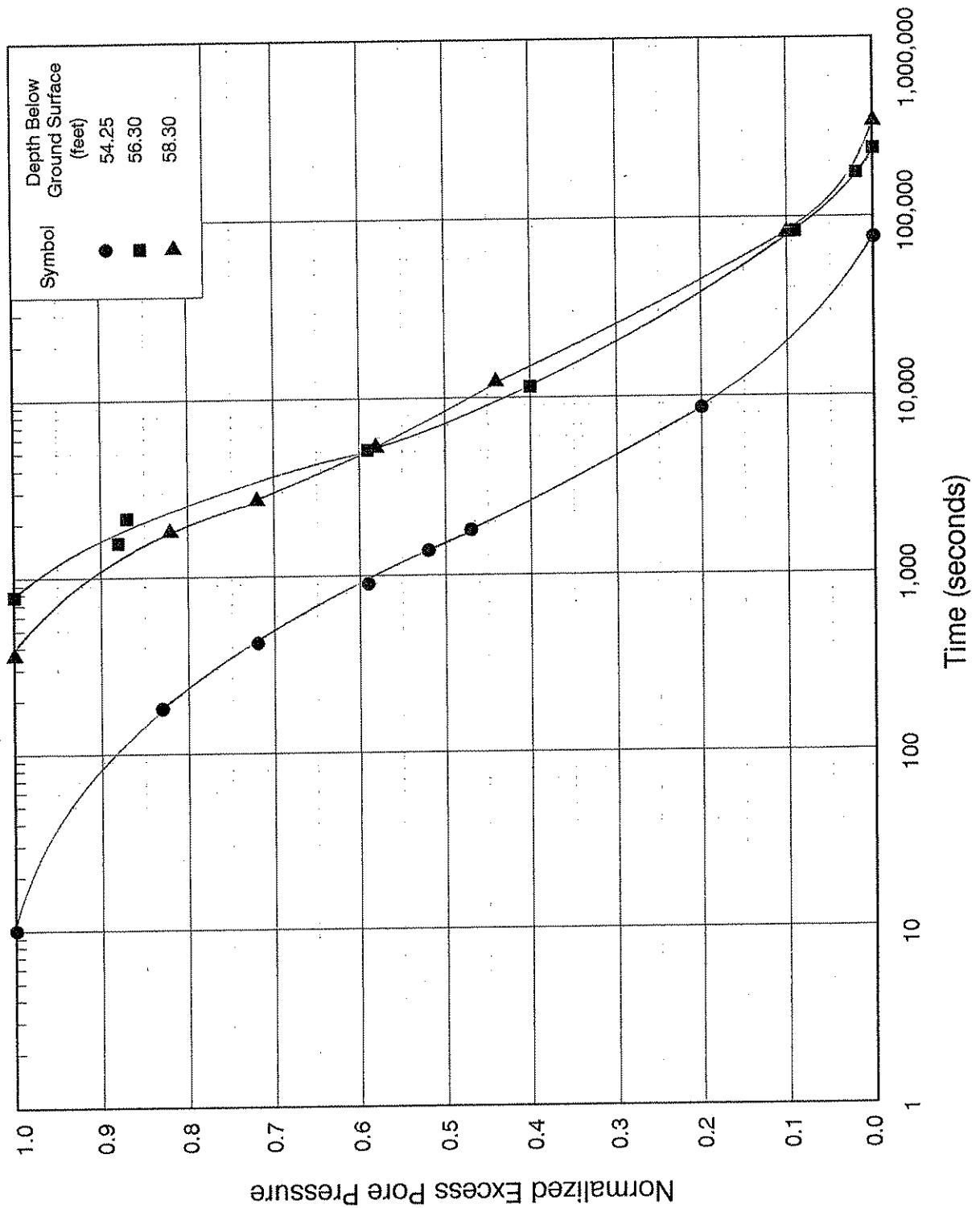
- ⊕ ARDAMAN (1994)
- ⊙ ARDAMAN (2001)
- ⊕ ARDAMAN (2002)
- ⊙ ARDAMAN (2003)
- ARDAMAN (2004)
- ⊕ ARDAMAN (2005)
- (2.0) PIEZOMETRIC LEVEL ABOVE TOP OF CLAY IN FEET

LEACHATE LEVELS ON TOP OF PHOSPHATIC CLAY



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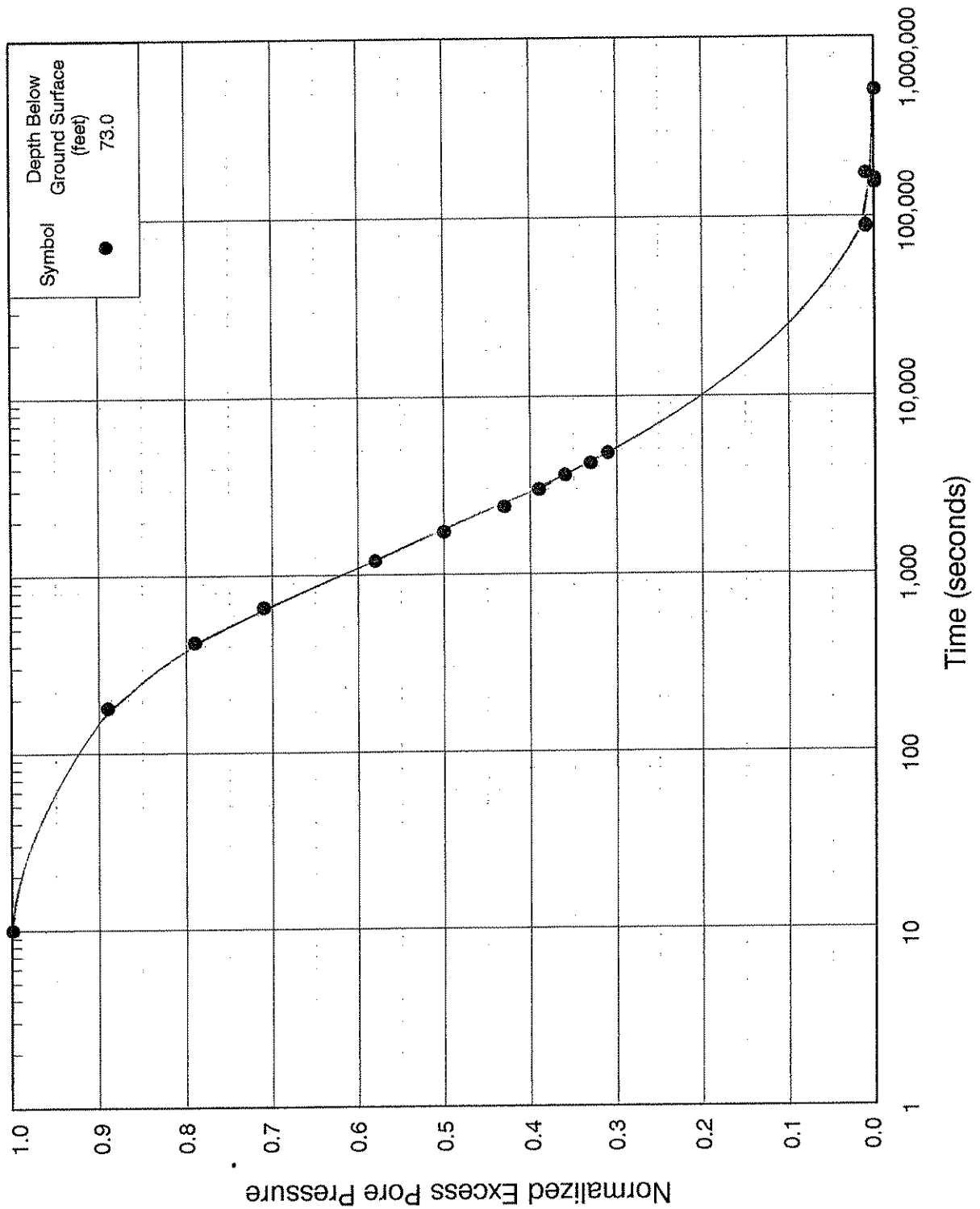


EXCESS PORE PRESSURE DISSIPATION PIEZOPROBE AT TEST LOCATION PC-1K


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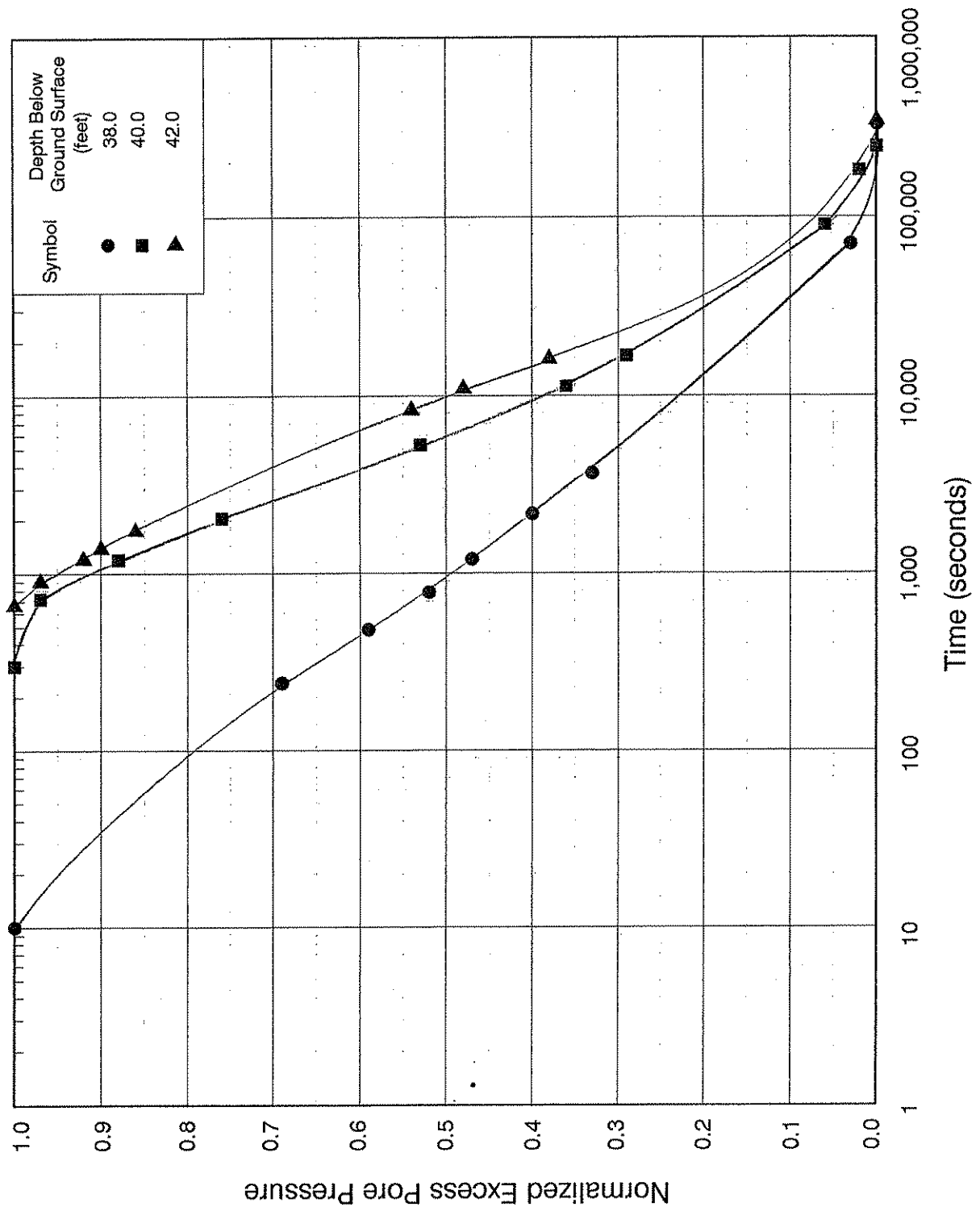
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HILLSBOROUGH COUNTY, FLORIDA

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EXCESS PORE PRESSURE DISSIPATION PIEZOPROBE AT TEST LOCATION PC-1L

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FILE NO.:	05-010	APPROVED BY:	<i>FCC</i>
DATE:	05-16-03	FIGURE:	10



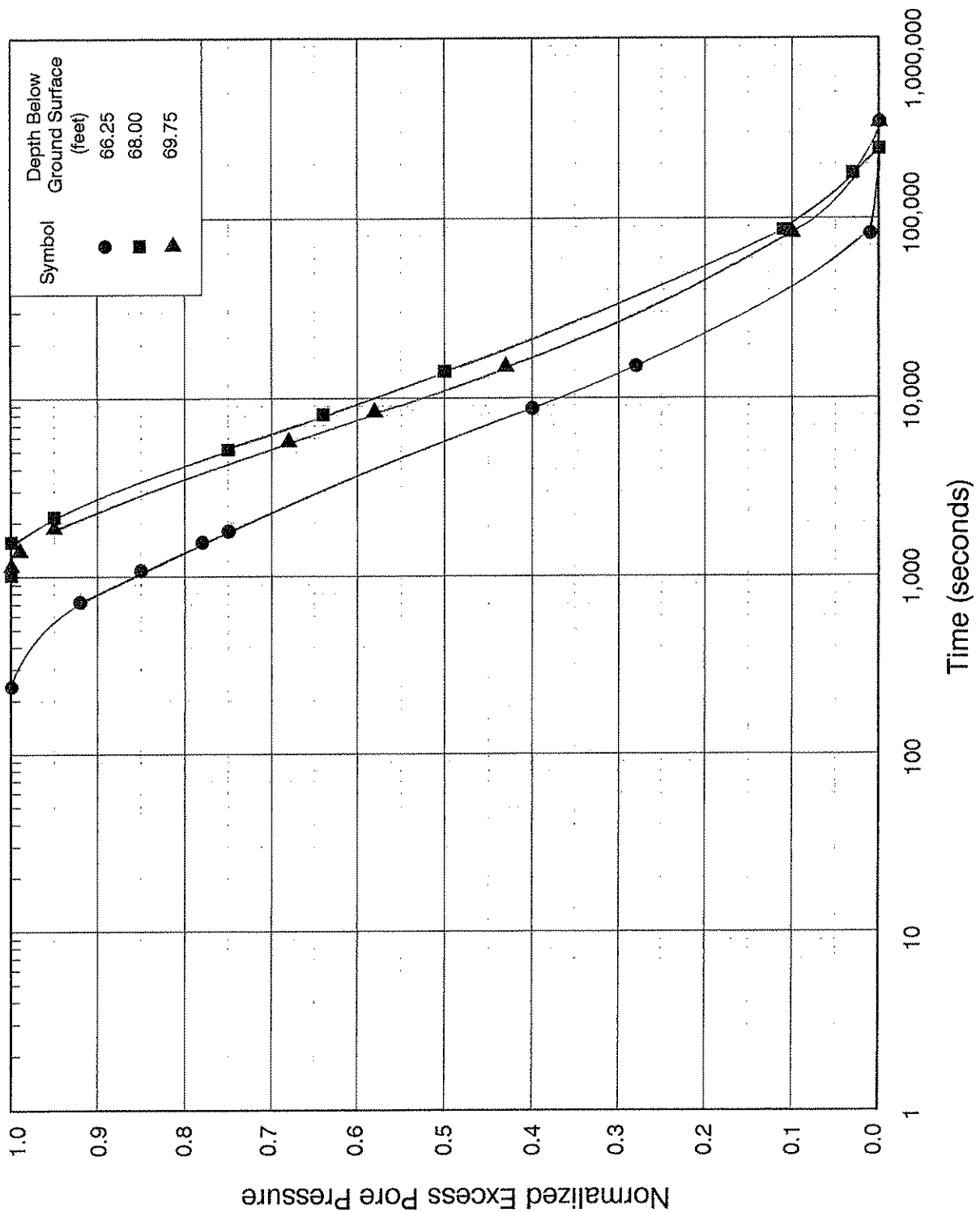
EXCESS PORE PRESSURE DISSIPATION PIEZOPROBE AT TEST LOCATION PC-3E

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HILLSBOROUGH COUNTY, FLORIDA

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FILE NO.: 05-010 APPROVED BY: *File* FIGURE: 11

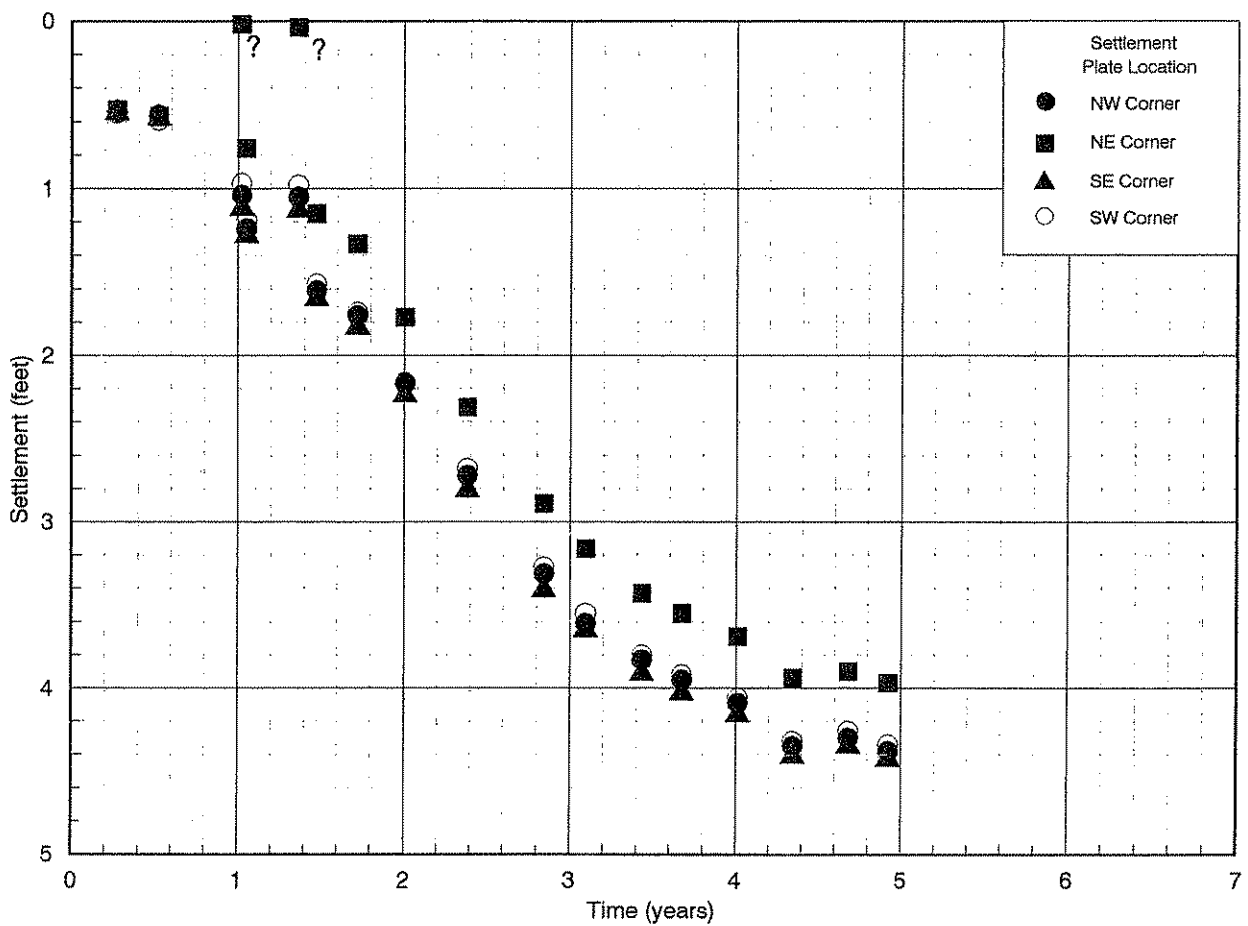


EXCESS PORE PRESSURE DISSIPATION PIEZOPROBE AT TEST LOCATION PC-4F

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Geotechnical, Environmental and
Materials Consultants


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SOUTHEAST LANDFILL
HILLSBOROUGH COUNTY, FLORIDA

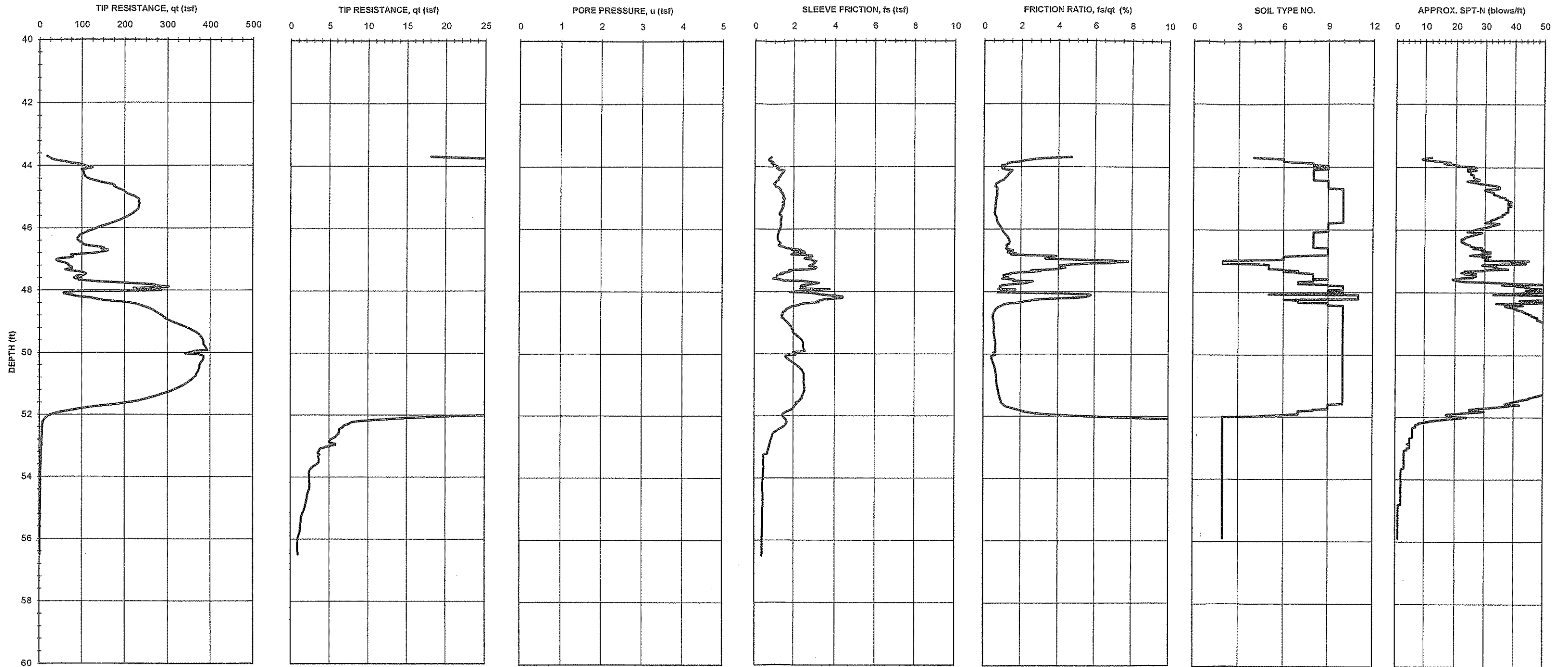
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FILE NO: 05-010	APPROVED BY: <i>FCC</i>	FIGURE: 12



SETTLEMENT VERSUS TIME

**SETTLEMENT OF TOP OF PHOSPHATIC CLAY
AT PUMP STATION B SUMP**

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DRAWN BY:	SA	CHECKED BY:	SA
FILE NO.:	05-010	APPROVED BY:	PCC
DATE:	05-16-03	FIGURE:	13



Soil I.D. #	Soil Description	UCS
1	Sensitive Fine Grained	OH/CH
2	Organic Material	OH
3	Clay	CH
4	Silty Clay to Clay	CL/MH
5	Clayey Silt to Silty Clay	MH/CL
6	Silty Sand to Sandy Silt	SC

Soil Classification by Robertson et al., 1986



Soil I.D. #	Soil Description	UCS
7	Sand to Sandy Silt	SP/SC
8	Sand to Silty Sand	SP
9	Sand	SP/SW
10	Gravelly Sand to Sand	SP/GW
11	Very Stiff Fine Grained	OC Clay
12	Sand to Clayey Sand	Cemented

TEST SITE: PC-6F

SOUNDING DATE 3/3/05

SURFACE ELEVATION (FT, NGVD) 167.31

COORDINATE LOCATION (FT) N 1,250,983.97
E 96,262.97

 Ardaman & Associates, Inc. Piezocone Test Results		
GEOTECHNICAL STUDY ASSOCIATED WITH ANNUAL MONITORING		
SOUTHEAST LANDFILL HILLSBOROUGH COUNTY, FL		
DRAWN BY: SA	CHECKED BY: SA	DATE: 05-16-05
FILE NO.: 05-010	APPROVED BY: 	FIGURE 14