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January 14, 2004

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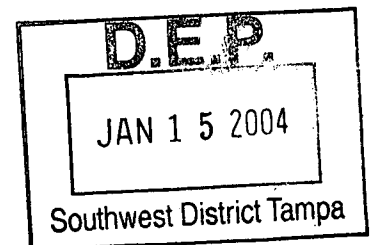
Phase 5

File 12.0

Via Hand Delivery

Ms. Susan Pelz, P.E.
Florida Department of Environmental Protection
Southwest District
3804 Coconut Palm Drive
Tampa, Florida 33619

**Subject: Cell 1 and Landfill Site Certification Addendum 2
and Responses to Department Certification Comments
Enterprise Recycling & Disposal Facility
Angelo's Aggregate Materials, Ltd.
FDEP Permit Nos. 177982-001-SC, 177982-002-SO
Pasco County, Florida**



Dear Ms. Pelz:

On behalf of Angelo's Aggregate Materials, Ltd. (Angelo's), Hartman & Associates, Inc. (HAI) is submitting responses to the Department's comments, dated December 8, 2003, regarding the certification submittals for the above referenced facility. Our responses are based on the discussion from our meeting with the Department on January 5, 2004. For your convenience, your comments are stated first with our responses, when applicable, following.

Comments from Kim Ford's Letter:

- 1) **Document entitled Cell 1 Certification dated October 8, 2003:**
- a) **The cover letter page 1 - HAI states that "the attached document indicates that Cell 1 has a continuous confining layer, at least 36-inches thick, with a permeability value no greater than 1x10⁻⁶cm/sec, and is prepared to accept waste". [The Department has not reached this same conclusion at this time as described below. This comment does not require a response.]**

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- b) **The cover letter page 2** - HAI states that "The confining unit may be at the cell base, under the cell base, or a combination of both, as long as it is at least 36-inches thick and continuous with a maximum permeability of 1×10^{-6} cm/sec". *[The Department agrees that this is the approved design criteria.]* HAI states that "All borings used to verify the confining unit are shown on Figure 1 in Appendix A." *[Figure 1 in Appendix A was resubmitted as part of the addendum. Confirmation that Figure 1 (for both documents) includes the locations for all borings and all failing test locations should be provided.]*

Response: Figure 1 has been revised to include all borings and failing test locations, and is attached in Appendix A.

- c) **The cover letter page 3** - HAI states that "Based on this correlation, a sample with at least 31% passing the #200 sieve was considered acceptable as confining material." *[Related comments (memorandum dated December 4, 2003, by Mr. John Morris) are provided as an attachment to this letter.]* HAI states that "The Confining Layer Contour Map for Cell 1 (Figure 38) is in Appendix C." *[Figure 38 in Appendix C was resubmitted as part of the addendum. Confirmation that Figure 38 (for both documents) includes the locations for all borings and all failing test locations should be provided.]*

Response: The purpose of Figure 38 is to show the continuity of confining material throughout Cell 1 and part of Cell 15 in the areas it was proven to exist. We believe that including the requested information creates unnecessary clutter on the map and is not needed since previous submittals provide evidence that these areas were over-excavated and properly patched. The revised Figure 38 is attached in Appendix B.

- d) **The cover letter page 4** - HAI states that a benchmark is installed in a cell corner post. *[A description of the benchmark should be provided with its elevation (NGVD).]*

Response: The benchmark is included on Figure 1 with its elevation (NGVD). The benchmark is a PVC pipe in the southeast corner of Cell 1. The benchmark elevation and ground elevation are provided on Figure 1.

- e) **The cover letter page 5** - *[no comments]*

- f) **The cover letter page 6** - HAI states that "As shown on the Confining Layer Contour Map for the temporary pond area, the continuous confining unit extends from the north end of Cell 1 through Cell 16. This map is provided as Figure 39 in Appendix C. Therefore, no additional quality assurance testing or construction will be required for certification of the confining unit in Cell 15 and in Cell 16." *[The Department has not*

reached this same conclusion at this time. This comment does not require a response. Related comments are provided in item 1)m) of this letter.]

- g) **The cover letter page 7** - HAI states that a stormwater permit modification is currently under review for changes, and a copy the approval for the construction will be provided. *[A stormwater permit must be issued prior to acceptance of waste. This may be resolved as a condition of approval for Cell 1. This comment does not require a response.]*

Response: A stormwater permit was issued for this facility in February 2001. A modification to this permit is pending and is expected by February 16, 2004. The stormwater management system, as modified, has been constructed. The system will be certified, based on the as-built survey provided by Foresight Surveyors, once the modification is issued. Since the system is constructed, and essentially approved by the FDEP, Angelo's requests approval for operation prior to the issuance of the modification to the permit.

- h) **The cover letter page 8** - HAI states that Pond 2 may need to be redesigned and Appendix D includes revised entrance plan details. *[Record drawings should be provided to show all changes. This may be resolved as a condition of approval for Cell 1. This comment does not require a response.]*

- i) **Certification of Construction Completion (DEP Form #62-701.900(2))** - *[This form includes the certification for all of Cells 1, 15, and 16. A new certification (on DEP Form #62-701.900(2) as a replacement) should be provided to include only those areas that have been completely excavated and confirmed to have the specified clay layer/confining unit.]*

Response: A replacement certification form (DEP Form #62-701.900(2)) is attached in Appendix C.

- j) **Summary of Deviations (attached to certification form)** - HAI states: the as-built survey for Cell 1 is provided in Appendix D and that Cell 1 was not entirely excavated *[The as-built topographic survey by Foresight shows that only parts of Cells 1, 15, 16 have been excavated. Related comments are provided in item 1)p) of this letter.]*, swales were not constructed to drain Cell 1 or the west slope to the temporary pond, a berm was constructed with an open channel *[Related comments are provided in item 2)d) of this letter.]*; the side slopes of Cell 1 were not constructed as designed and the north-central boundary of Cell 1 was over-excavated *[The "north-central boundary of Cell 1" appears to be adjacent to Cell 15 and would require no redesign. Clarification should be provided.]*; Pond 1 was modified, the location of the entrance, scalehouse, scales, and

maintenance area have changed. *[Record drawings should be provided to show all changes from the approved plans.]*

Response: HAI acknowledges that the stormwater conveyance located in the west of Cell 1 is a berm and open channel, rather than a swale as previously stated. This change will not affect the operation of the conveyance. The above statement regarding over-excavation of the Cell 1 side slope should reference the north and central portions of the eastern cell boundary. Therefore, Pond 2 will be redesigned after operation begins, as agreed between HAI and the Department. Record drawings have been prepared and are attached along with the as-built survey information in Appendix D.

- k) **Appendix A, Figure 1 (map of borings)** - *[There are very few borings shown on this figure that include the boundaries of the cells, and there are no borings located on any the part of the selected cross-sections shown on this figure that include the boundaries of any of the cells. Only those areas that have been completely excavated and confirmed to have the specified clay layer/confining unit should be certified. All cross-sections must include borings shown for the certified area in order to include the entire certified waste disposal area for approval. All borings that represent the lithology in the vicinity of each cross-section, and the lithology for each of the detection well locations in the vicinity of the cross-section, should be included on the selected cross-section to show the continuity of the clay layer/confining unit. The degree of accuracy (for example: +/- 20 feet) must be provided for the boring locations. Related comments are provided in items 1)l), 1)m), and 1)n) of this letter.]*

Response: Figure 1 has been revised to include a line connecting the cell markers that indicate the areas to be certified at this time. Any changes to the cross-sections that were necessary to intersect this line have been made. The revised cross-sections are attached in Appendix A. A note referencing the signed and sealed survey sheets, provided by Foresight Surveyors, for the boring locations has been added to the Figure 1.

- l) **Appendix B (soil test results, tables of values, and correlation)** - *[All borings must have an elevation (referred to the NGVD, rather than "MSL" referred to for the "AS" borings) at surface to be useful, especially for the shallower borings at surface. All other elevations below surface should be calculated from the surface elevation minus the depth. ("The elevations that are normally used in topographic mapping, geodetic surveys, engineering studies, and engineering construction surveys are referred to the NGVD. The NGVD should not be confused with local mean sea level datums." - from textbook entitled Surveying, by Davis, Foote, Anderson, and Mikhail.) The tables of values for the borings should provide the date of each boring, elevation (NGVD) at surface, elevation (NGVD) at the top of the clay layer/confining unit (that may be at the cell base/floor,*

under the cell base/floor, or a combination of both, as long as it is at least 36-inches thick and continuous with a maximum permeability of 1×10^{-6} cm/sec), clay layer/confining unit thickness, elevation (NGVD) for each sieve sample, % passing the #200 sieve, elevation (NGVD) for each permeability sample, and permeability test results in cm/sec. A separate table for each type of boring (such as for "B", "AS", "SSA", "ST", etc.), with each boring listed sequentially by the boring #, would be very helpful. The actual elevations (NGVD) at surface at the time of each boring must be provided. Related comments are provided in items 1)k), 1)m), and 1)n) of this letter.]

Response: The boring logs have been revised to reference the surface elevations as NGVD, rather than MSL, and to include the surface elevations as provided by Foresight Surveyors. The detailed tables requested for the borings have been developed and include the name of each boring location, date, elevation of the boring, elevation of the top of the confining material, thickness of the confining material, elevations for any sieve samples and corresponding results, elevations for any permeability samples and corresponding results. Separate tables are provided for: standard penetration test (SPT) borings, bucket auger (AS) borings, and solid stem auger (SSA) borings. The logs and tables are attached in Appendix E.

m) **Appendix C, clay layer/confining unit contour maps:** *A topographic survey provided or referenced for the at surface elevations for each boring (at the time of each boring) would be very helpful. The actual elevations (NGVD) at surface at the time for each boring are essential to the success of this certification and should be provided either on the topographic survey or listed separately. In either case, the list and the survey must be signed, dated, and sealed by a registered professional surveyor and mapper (F.S. 472.003(2)). For the clay layer/confining unit contour maps, a registered professional geologist may interpret the topographic survey/data and prepare the Figures 38 and 39, however, the professional surveyor must either cosign, date, and seal the figures or the referenced topographic survey/data (FAC 61G16-2.005, FS 492.102(7), F.S. 472.003(2)). Each figure should reference the topographic survey/data. With the topographic survey/data, the professional surveyor should provide the horizontal degree of accuracy (for example: +/- 20 feet) for the boring locations and the degree of accuracy (for example: +/- 6 inches) vertically for the elevations (NGVD) at surface at the time of each boring. The clay layer/confining unit contour maps show the clay layer/confining unit beyond the last boring along the cell boundaries and must be revised to limit the contours to the areas where supporting data is available. Related comments are provided in items 1)k), 1)l), and 1)n) of this letter.]*

Response: As discussed via e-mail with the Department and during the January 5 meeting, it is not appropriate for the surveyor to co-sign HAI's maps. In order to resolve this comment, it was agreed that HAI would add any necessary notes to the figures referencing the signed and

sealed survey documents. In addition, HAI is requesting that Foresight Surveyors add the boring elevations and degree of accuracy to the as-built survey. The confining unit contour maps are generated by a computer model using site-specific geologic data obtained from the site. This results in extrapolation of some of the contours beyond the limits of the area to be certified. It was agreed in the January 5 meeting that the extrapolated contours would be appropriately indicated, and that a line connecting the cell markers would be added to confirm the certified area has the required confining material.

At the request of the Department, HAI is providing copies of the input parameters for the computer model used to develop the confining unit maps. This data is attached in Appendix B.

Figure 38 (Cell 1 clay layer/confining unit contour map) - *[This figure/map is based on a topographic survey (with both horizontal and vertical control) and is intended to show both the Cell 1 clay layer/confining unit contours and the locations of specific features (borings, the limits of the excavated cells, and test pits/excavated areas). This figure/map shows that portions of the top of the clay layer/confining unit is at elevation +85 (3 feet above the surface) and higher, the cells (sideslopes) are not completely excavated, and no corner posts are shown for the limits of waste disposal. Boring B-18 is missing from this drawing. All borings must be shown on all top of clay layer/confining unit contour maps, or a specific reason provided for each excluded boring. This figure shows the excavated area for "AS-10" to be extremely small in comparison to the size of other excavated areas, and the location of boring "AS-42" is not shown within the excavated area. Additional borings should be provided in the vicinity of these two areas to confirm the presence of the clay layer/confining unit. The corner posts appear to be located at the cell boundaries and in areas along the toe of slope that have not been confirmed to have the specified clay layer/confining unit. The certified area should be provided on one drawing (drawn to scale) and limited to the area completely excavated and prepared for waste disposal, and the limits of waste disposal/certified area must be marked by corner posts. The scale of 1"=100' appears to be incorrect and must be corrected. This drawing must be corrected prior to the approval for Cell 1. A scale of 1"=60' is suggested, the same as for the cross-sections. Related comments are provided in items 1)k), 1)l), and 1)n) of this letter.]*

Response: Figure 38 has been revised as discussed during the January 5 meeting to demonstrate the presence of the required confining material within the designated area to be certified. The limits of certification are included in this figure as a dashed line in Cells 1 and 15. The excavated area for "AS-10" was excavated to the extent that confining unit material was identified on each wall and suitable as tie-in material. The location for "AS-42" was confirmed to be within the excavation area. The contractor surveyed many of the excavation areas as simple geometric shapes to facilitate the excavation volume calculations. The scale of 1"=100'

has been replaced with a scale of 1"=80'. The scale used for this figure was chosen to display the area of interest in a standard 11" x 17" format

Figure 39 (Cells 15 and 16 clay layer/confining unit contour map) - *[This figure/map is based on a topographic survey (with both horizontal and vertical control) and is intended to show both the Cells 15 and 16 clay layer/confining unit contours and the locations of specific features (borings and the limits of the excavated cells). This figure/map shows that Cell 15 and Cell 16 are not completely excavated and there are no corner posts shown at the cell boundaries. The top of clay layer/confining unit contours dramatically change near the center of Cell 16, and the clay layer/confining unit thickness of at least 3 feet is not confirmed by an adequate number of borings. A top of the clay layer/confining unit drop of five feet between two borings spaced 100 feet apart is significant but acceptable when adequate thickness and permeability are confirmed. However, the top of the clay layer/confining unit drop of 30 feet in 100 feet (from B-33 to B-22) is dramatic and must be confirmed by additional borings and permeability tests. Therefore, the clay layer/confining unit thickness of 3 feet at the bottom of B-22 is of concern, and reasonable assurance of at least 3 feet of thickness must be provided by more closely spaced borings and/or greater than 3 feet of thickness. More continuity between borings allows for more widely spaced borings. Less continuity and dramatic changes requires more closely spaced borings. For areas with dramatic changes (such as those shown for Cell 16), additional borings on a grid of 50 feet are suggested. The certified area should be provided on one drawing (drawn to scale) and limited to the area completely excavated and prepared for waste disposal, and the limits of waste disposal/certified area must be marked by corner posts. All borings must be shown on all top of clay layer/confining unit contour maps. The scale of 1"=100' appears to be incorrect and must be corrected. A scale of 1"=60' is suggested, same as for the cross-sections. Related comments are provided in items 1)k), 1)l), and 1)n) of this letter.]*

Response: Figure 39 has been revised as discussed during the January 5 meeting to demonstrate the presence of the required confining material within the designated area to be certified at this time (Cell 15) and is attached in Appendix B. Also as discussed in the meeting, a temporary two-foot berm has been constructed along the cell boundary of Cell 15/Cell 16. The limits of certification are included in this figure as a dashed line in Cell 15. This will allow for the appropriate confining unit patching to be completed for later certification of Cell 16. The scale of 1"=100' has been changed to 1"=80. The scale used for this figure was chosen to display the area of interest in a standard 11" x 17" format.

- n) **Appendix C, cross-sections (Figures 1 through 7):** *[The cross-sections show clay below borings with no supporting data and must be revised to show all the lithologies on cross-sections end at the lower tip of the deepest representative boring, and at the last*

included boring, unless other deeper borings are shown to provide supporting data. All scales on each cross-section should be the same (1"=60'H, 1"=20'V suggested). More E-W cross-sections would be very helpful. More specific comments are provided for each cross-section. Related comments are provided in item 1)k) of this letter.]

Response: It is common in geologic practice to continue cross-sections for a limited depth beyond the extent of borings to represent the likely soil conditions of a site. However, the cross-sections have been revised to include notation (question marks) where appropriate to demonstrate that no deeper borings were completed to confirm the presence of the indicated soil types. The cross-sections do not end at the last included borings since soil types and extents are generally extrapolated in both directions from each boring to the midpoint of the distance to the next. This is done based on knowledge of the site characteristics and available site data. Additional boring locations have been included as discussed during the January 5 meeting, to extend the cross-sections and maximize the areas to be certified. Some AS and SSA borings were added to the cross-sections as deemed necessary by HAI. Each cross-section is shown to depict the geology through a horizontal distance and a vertical elevation. The horizontal distances vary and therefore different scales are required to provide magnification and clarity for ease of evaluation. It was agreed between HAI and the Department through e-mail correspondence that the scales would not be revised. This response is intended to address the entire 1)n) comment, and therefore, each individual component will not have an individual response.

Figure 1, cross-section A-A' (Scale: 1"=100'H, 1"=10'V) - *[This cross-section shows only one permeability test for 6 deeper borings. This cross-section shows silty clay on the southeast slope with no borings and no borings on the cell base/floor along the west side for 350 feet from ST-2. This cross-section should show many of the "AS" borings and others (B-17 and SSA-11), and should be extended to the east to include SSA-20 and SSA-37.]*

Figure 2, cross-section B-B' (Scale: 1"=100'H, 1"=10'V) - *[This cross-section shows no boring for the south edge of Cell 1, or for the north edge. This cross-section provides more information, permeability tests, more borings, and is more helpful, and shows more clay layer/confining unit continuity. This cross-section should show many more of the "AS" borings and others (such as SSA-24).]*

Figure 3, cross-section C-C' (Scale: 1"=60'H, 1"=30'V) - *[Shows no borings for the east and west edges of Cell 15, and only one permeability test. This cross-section should be extended to the east to include MW-7A and MW-7B, and to the west to include DCL01-8.]*

Figure 4, cross-section D-D' (Scale: 1"=50'H, 1"=25'V) - *[This cross-section shows no boring for the north edge of Cell 16, and no permeability test for Cells 15 and 16. This cross-section should be extended to the north to include more borings (ST-7, B-22, B-26, B-31, DCL01-14, B-32, B-20, SSA-26, SSA-28, SSA-29). All of Cell 16 could be included.]*

Figure 5, cross-section E-E' (Scale: 1"=60'H, 1"=30'V) - *[This cross-section shows no borings for west edge of Cell 15 and the east edge of Cell 16. The top of the clay layer/confining unit drop of 30 feet in 100 feet (from B-33 to B-22) is dramatic and should be confirmed by additional borings and permeability tests, therefore the clay layer/confining unit thickness of 3 feet at B-22 is of concern. This cross-section should be extended to the northeast to include B-6, MW-5A and MW-5B, and to the southwest to include DCL01-8.]*

Figure 6, cross-section F-F' (Scale: 1"=40'H, 1"=20'V) - *[This cross-section shows no borings for the east and west edges of Cell 15. This cross-section should be extended to the east to include B-8, and on the west to include DCL01-8.]*

Figure 7, cross-section G-G' (Scale: 1"=60'H, 1"=30'V) - *[This cross-section shows no borings for the east and west edges of Cell 16. This cross-section should include more borings (B-5, SSA-27), and should be extended to the northwest to include MW-4 and to the southeast to include MW-6.]*

- o) **Appendix C, boring logs (B-15 -B-34, SSA-1 -SSA-37, AS-1 -AS-47):** *[Related comments are provided in item 1)l) of this letter.]*

Response: Please see the response to comment 1)l).

- p) **Appendix D (as-built surveys)** - *[All as-built topographic surveys by Foresight must signed, dated, and sealed by a professional surveyor. The elevations on Sheet 1 of 2 are difficult to read due to the scale. A scale of 1"=60' is suggested, same as for the cross-sections, and for the top of clay layer/confining unit contour maps. The cell corner posts, and the posts for the certification limits/disposal limits should be shown on each drawing, with the benchmarks described and shown with elevations (NGVD). The Plan & Profile Entrance Road drawing PP-1 (Sheet 1 of 4) appears to be part of another set of drawings that were not reviewed or approved by the Department. A complete set of these drawings should be provided. The As-Built Drainage Plan is not legible and should be provided as a record drawing. All drawings provided as record drawings must be signed, dated, and sealed by the professional engineer of record.]*

Response: The as-built surveys were revised to include the cell corner posts and cell certification markers, and the descriptions of the benchmarks. Signed and sealed surveys are attached in Appendix D. The Plan & Profile Entrance Road drawing (PP-1, Sheet 1 of 4) was included to show the entrance plan changes approved through Pasco County. Sheets 2, 3, and 4 of the set provide details and general notes, and are attached in Appendix F. The As-Built Drainage Plan has been reprinted and has been renamed as a Record Drawing, and is signed and sealed by a registered civil engineer for stormwater, and a registered environmental engineer for solid waste. Notes regarding the source of the as-built survey data are included.

- q) **Appendix E (soil stockpile test results)** - *[no comments]*
- r) **Appendix F (photos of tie-ins)** - *[no comments]*
- s) **Appendix G (field test results for tie-ins)** - *[no comments]*
- t) **Appendix H (test results)** - *[The note "For information purposes only" appears to indicate that these test reports are not the official final reports. The official final reports should be provided.]*

Response: The reports provided were copies of the official final reports provided to HAI by Universal Engineering Sciences.

- u) **Appendix I (water table elevations)** - *[An evaluation of the water elevations should be provided to demonstrate that there is not a direct connection to the deeper Floridan LS aquifer and that the wells are adequately placed to monitor the groundwater (both the surficial and the deeper Floridan LS aquifer), and that the base/floor of each cell will remain at least 5 feet above the SHWT. Additionally, the location for a piezometer along the west side of the temporary pond to measure the groundwater fluctuations should be provided.]*

Response: Florida Administrative Code Chapter 40C-42 states that the "Seasonal high groundwater table elevation" means "the highest level of the saturated zone in the soil in a year with normal rainfall." HAI's estimate of the seasonal high groundwater elevation was based on historical groundwater elevations for the area during the wet and dry seasons. Rainfall in 2003 was above the normal level and therefore caused the groundwater elevations to rise above the estimated high levels. Additionally, while drilling in the temporary pond, stormwater was pumped from the temporary pond, causing standing water around several of the monitor wells. As a result of the standing water, groundwater elevations in these wells appeared unusually high. The maximum elevation of water in nearby wells was lower than the base of Cell 1, except for water in monitor well MW-8. MW-8 is located in an area that conveys stormwater collected east

of Cell 1 to lower areas east of Cell 16. During high rainfall periods, that area is subjected to additional infiltration from overland flow. Under normal operations a stormwater pond (Pond 2) will intercept stormwater and hold it in a lower elevation. Therefore, groundwater levels should not exceed the elevation of the base of Cell 1. We believe that the hydrographs of those wells demonstrate that under high rainfall the water table will be below the base of the adjacent cells. It is only when additional stormwater is routed to surface depressions, rather than correctly designed stormwater ponds, that the water table exceeded the elevation of the base of Cell 1.

Hydrographs for all current wells and piezometers have been generated and are attached in Appendix G. During the January 5 meeting, the Department acknowledged that 2003 rainfall was greater than normal. Based on discussion during the meeting, it appears the landfill base grades are adequate. Monitor well MW-1B is to the west of the temporary pond and is adequate to monitor water levels. An additional piezometer is not intended to be installed in Cell 5 due to likely construction of the landfill cell and operational necessities in the near future.

2) **Document entitled Cell 1 and Landfill Site Certification Addendum dated November 13, 2003:**

- a) **The cover letter page 1** - HAI states that "The boring logs for AS-1 through AS-47 describe the depth from surface of each boring in NGVD." *[Related comments are provided in item 1)l) of this letter.]*

Response: Please see the response to comment 1)l).

- b) **The cover letter page 2** - HAI states that "borings confirmed areas as limestone surface lenses underlain by clay and not connected to the limestone aquifer. *[The Department has not reached this same conclusion at this time. This comment does not require a response. Related comments are provided in items 1)k), 1)l), 1)m), and 1)n) of this letter.]* HAI states that FDEP did not stipulate additional borings in Cell 16 *[Related comments are provided in item 1)m) of this letter.]*. HAI states that "The data collected from Cells 15 and 16 provide reasonable assurance of a confining layer below the temporary pond" *[The Department has not reached this same conclusion at this time. This comment does not require a response. Related comments are provided in items 1)k), 1)l), 1)m), and 1)n) of this letter.]*, and "water level observations in the temporary pond show very little percolation, a direct test of confinement" *[The basis for this conclusion should be provided. Related comments are provided in item 1)u) of this letter.]*, and the dilution calculation predicts that only iron will exceed DEP groundwater quality criteria *[Related comments are provided in item 4)b) of this letter.]* HAI states that "AS-10 and AS-42 were located in sandy areas that were excavated and replaced with confining unit material." *[Related comments are provided in item 1)m) of this letter.]*

Response: The statement that "water level observations in the temporary pond show very little percolation, a direct test of confinement", applies to the situation where water stands in the pond without significant change in level in the absence of additional inflow. Because of the uncertainty of inflows and outflows of water to the pond, that statement was not quantified. Please see the responses for the other related comments as stated in the body of this comment.

- c) **The cover letter page 3** - HAI states that "Any intervals that did not meet the minimum of 31% fines passing, were either identified as non-conforming material in the boring logs, or located in areas not specified for certification, or had boring intervals below the failed interval." *[Related comments (memorandum dated December 4, 2003, by Mr. John Morris) are provided as an attachment to this letter.]* HAI states that "the locations of the corner posts that define the area of waste disposal on the floor of Cell 1 were included in Appendix D, Figures 1 and 38." *[Related comments are provided in items 1)k), 1)m), and 1)p) of this letter]*

Response: A revised correlation was discussed during the January 5 meeting that indicated a requirement of 30% fines passing the No. 200 sieve for appropriate permeability. The same data were used to develop the previously submitted correlation, however, on the revised graph permeability was plotted as the dependent variable (on the y-axis), and this changed the resulting fines content requirement since the standard deviation was based on a different variable. HAI explained that R^2 values for these types of trend lines are generally low due to the use of a log scale (dependent variable varies by orders of magnitude), and that the best visible fit line should be used to determine the correlation. HAI faxed reference information to the Department prior to submittal of this addendum for review; however, an additional copy is attached in Appendix H.

- d) **The cover letter page 4** - HAI states that approval of the ERP is expected. *[The ERP must be issued prior to acceptance of waste. This may be resolved as a condition of approval for Cell 1.]* HAI states that a swale was constructed in the west portion of Cell 1, and a survey of the swale is included in Appendix C. *[This survey shows a berm (not a swale) that appears to divert the stormwater (from beyond the excavated portion of Cell 1) into the temporary pond away from the waste. This design does not appear to be compatible with the intended function of the temporary pond. The temporary pond was/is designed to contain the contact water that drains from the partially filled waste disposal areas only (not to collect stormwater from beyond the excavated disposal areas). Therefore, capacity calculations should be provided to demonstrate that the temporary pond will have the capacity to contain both the stormwater from beyond the excavated disposal areas and the contact water that drains from the partially filled waste disposal areas. Drainage from Cell 1 must be allowed to flow freely from the cell. Clarification should be provided.]*

Response: The intent of the temporary pond was discussed during the January 5 meeting. The purpose of the pond is to collect runoff from the excavated areas, and also from other unexcavated portions of the property. Angelo's has been informed that the modification to the stormwater permit will be issued on February 16, 2004. Capacity calculations for the temporary pond are on file with the Department.

- e) **The cover letter pages 4 and 5** - HAI describes several items not yet completed: the fence, signs, special non-conforming waste containers for batteries and paint, video camera, perimeter road, containers for Class I waste, and financial assurance. *[These items must be completed prior to acceptance of waste. This may be resolved as a condition of approval for Cell 1.]*

Response: All of the items listed in the above comment have been completed since submittal of the Cell 1 Certification package. Approval of the financial assurance mechanism has been received from the FDEP Tallahassee office.

- f) **Figures 1 and 38** - *[Related comments are provided in items 1)k) and 1)m) of this letter.]*

Response: Please see the referenced related responses.

- g) **Appendix A (estimated effluent concentration)** - *[Related comments are provided in item 4)b) of this letter.]*

Response: Please see the referenced related responses.

- h) **Appendix B, "ST" borings (table of values)** - *[Related comments are provided in item 1)l) of this letter.]*

Response: Please see the referenced related responses.

- i) **Appendix C (topographic survey for west side of Cell 1)** - *[This survey shows a berm (not a swale). Related comments are provided in item 2)d) of this letter.]*

Response: Please see the referenced related responses.

- j) **Appendix D (water elevations)** - *[Related comments are provided in item 1)u) of this letter.]*

Response: Please see the referenced related responses.

To gain a better understanding of the project, the Department has reviewed the original permit application and Department files, and offers the following comments *[comments provided in italics with cross-references underlined]*.

3) **Document entitled Enterprise Recycling and Disposal Facility Class III Landfill Permit Application, November 2000 (with revisions included):**

Response: HAI confirmed that this comment and associated subparts are informational and do not require a response.

- a) **Section 1 (application form, part B.21.)** - HAI requested a liner and LCRS exemption due to acceptance of only Class III waste and the presence of a natural confining layer. *[This has been typical for Class III landfill designs.]*
- b) **Section 3.7 (Engineering Report)** - HAI states that the landfill base will be "at least 5 feet above SHWT". *[This was/is part of the original design and part of the Department's basis for approval. Confirmation that the SHWT has been and will be at least 5 feet below the Cell 1 base/floor should be provided. Related comments are provided in item 1)u) of this letter.]*
- c) **Section 3.7 (Engineering Report)** - HAI states that each cell will be overcut by 50 feet for truck traffic and stormwater transport from the cell to the temporary pond, and a 6 feet high berm will prevent stormwater from entering the working face, as shown on Sheet G-1. *[Cell 1 was not overcut for stormwater on the south or the west, and the berm to divert stormwater and the conveyance for runoff from Cell 1 to the temporary pond do not appear to be constructed as designed. Related comments are provided in item 2)d) of this letter.]*
- d) **Section 3.8.3 (Engineering Report)** - HAI states that each 6-acre cell is expected to last 2 years with two ten-foot lifts per year. *[no comments]*
- e) **Section 3.10.1 (Engineering Report)** - HAI states that "Surface water and groundwater contact with the Class III wastes will be prevented by the proposed facility design." *[Related comments are provided in items 1)u), 2)d) and 3)b) of this letter.]*
- f) **Section 3.10.1 (Engineering Report)** - HAI states that "Since the facility proposes to accept only those wastes described in 62-701.340(3)(c), FAC, *[now described in 62-701.200(14)]* it is not expected to produce a leachate that would pose a threat to public health or the environment" *[The Department has not reached this same conclusion at*

this time. This comment does not require a response.], and "the strict method of controlling type of wastes disposed of also supports the liner exemption". [Related comments are provided in item 4)b) of this letter.]

- g) **Section 3.15 (Engineering Report)** - HAI states that "if the test data from the cell floor section does not meet the requirements, additional random samples may be tested, and if the additional testing demonstrates that the hydraulic conductivity meets the requirements, the cell will be considered acceptable". *[The Department agrees that retests are appropriate in some instances. An explanation should be provided for all retests (such as an area was reworked and retested, or due to a specifically described laboratory error).]*
- h) **Figures 3-6 - 3-12 (Sheets C-1 - C-6, and G-1)** - *[These drawings should be provided as record drawings for the initial construction, and should show the SHWT based on more data, and to show the initial berms and conveyances, and other new features (such as the locations of gas probes, groundwater monitoring wells, and stormwater ponds).]*
- 4) **Review of correspondence in Department files - #177982-001-SC:**

Response: HAI confirmed that this comment and associated subparts are informational and do not require a response.

- a) **5/31/01, HAI response** - HAI explains that after dilution from rainfall and not including dispersion, diffusion, sorption, and biodegradation, only iron will exceed at the ZOD. *[Related comments are provided in item 4)b) of this letter.]*
- b) **6/28/01, Department memorandum** - *[As indicated during the permit review, the Department has not considered the dilution equation and associated assumptions as adequate to describe the potential impacts to groundwater quality, however, other assurances were provided by the "control of unauthorized wastes, site hydrogeology, stormwater control, groundwater monitoring, and cell certification." Due to the variable geology of the site uncovered during construction that appears to be different than what was intended as part of permitting, additional assurances should be provided to demonstrate and confirm adequate environmental protection. Related comments are provided in item 1)u) of this letter.]*
- c) **7/25/03 report by HAI** - Borings and LS areas are shown on Figure 1 (dated 7/14/03). *[This figure/map is the first topographic survey (with both horizontal and vertical control) for the Cell 1 base/floor and shows the LS areas, with elevations to the nearest tenth of a foot. Related comments are provided in items 1)k), and 1)m) of this letter.]*

- d) **8/05/03 report by HAI** - HAI describes CQA for patching the LS areas, and states that "a soil liner is not being constructed at this site. The tie-ins are being constructed to ensure a continuous confining unit at the base of Cell 1." The report includes Figure 3 (not dated). *[This figure/map is the second topographic survey (with both horizontal and vertical control) for the Cell 1 base/floor and shows the locations for the "AS" (at surface) borings, with elevations to the nearest hundredth of a foot. Related comments are provided in items 1)k), and 1)m) of this letter.]*
- e) **8/12/03, report by HAI** - This report includes Figure 6 (not dated). *[This figure/map appears to be incomplete and does not appear to qualify as a topographic survey (with both horizontal and vertical control) for the Cell 1 base/floor and shows the locations for some of the "AS" and "SSA" borings, (without elevations) with a note that states that the "locations are approximate". Both horizontal and vertical control are essential to the success of this certification. The intended use of this map should be described and clarification should be provided for the lack of both horizontal and vertical control. Related comments are provided in items 1)k), and 1)m) of this letter.]*
- f) **8/19/03, Department meeting with HAI and Angelo's (notes in files)** - Discussed the Plan of Action required by specific condition #5 for LS, and that the intended design was/is that the clay layer/confining unit is either at the base/floor and/or at depth below the base/floor. Agreed that the top of clay layer/confining unit contour map must show the clay layer/confining unit to be continuous with at least 3 feet at 1x10⁻⁶cm/sec, with no averaging. Discussed the north half of Cell #1 and the concern for demonstrating that the clay layer/confining unit is at the base/floor due to the west side sandy area that may have the clay layer/confining unit at depth. *[Related comments are provided in item 1)m) of this letter.]* Discussed Cell #15 and the concern that the bottom cannot be observed due to water. Discussed Cell #16 and the concern with the sandy area at north end, and the need for acceptable permeability test results in the target clay layer/confining unit. *[Related comments are provided in item 1)m) of this letter.]*
- g) **9/08/03 Fax from SWFWMD** - SWFWMD approves an on-site potable drinking water supply well (converted from an existing irrigation well). *[The operations plan should be revised to describe the use of this on-site potable drinking water supply well rather than bottled drinking water, and to describe the plans for its future use or abandonment as waste disposal progresses to within 500 feet.]*

Comments from John Morris' Memo (memo text is in italics with responses inserted as necessary):

*The section of the HAI transmittal letter dated October 8, 2003 entitled "**TEMPORARY POND AREA**" includes a sub-heading entitled "Cell 16" (page 6) that indicates the following: "Due to a conflict of a preliminary correlation between permeability and percent fines with the actual permeability values obtained from test locations ST-13, ST-14, ST-16, and ST-17 in Cell 16, and ST-21 in Cell 14, tested by Ardaman (considered as outliers), some of the remaining intact Shelby tube samples were re-evaluated by UES." The initial and retest results for the referenced locations are summarized below:*

<u>Boring #</u>	<u>Cell #</u>	<u>Sample #</u>	<u>Sample Depth (ft BLS/elevation)</u>	<u>% Fines</u>	<u>Perm. (cm/sec)</u>
B-20 (initial test)	16	ST-13	8 - 10 / 65 - 67	38.1	5.3×10^{-6}
B-20 (retest)	16	ST-13	8 - 10 / 65 - 67	47.6	4.9×10^{-7}
B-21 (initial test)	16	ST-14	4 - 6 / 69 - 71	64.7	2.6×10^{-6}
B-21 (retest)	16	ST-14	4 - 6 / 69 - 71	52.3	6.7×10^{-8}
B-22 (initial test)	16	ST-16	44 - 46 / 29 - 31	13.3	9.2×10^{-6}
B-22 (retest)	16	ST-16	44 - 46 / 29 - 31	57.2	1.9×10^{-7}
B-23 (initial test)	16	ST-17	2 - 4 / 71 - 73	25.4	1.3×10^{-5}
B-23 (retest)	16	ST-17	2 - 4 / 71 - 73	38.8	6.9×10^{-8}
B-28 (initial test)	14	ST-21	32 - 34 / 62 - 64	33.8	2.1×10^{-6}
B-28 (retest)	14	ST-21	32 - 34 / 62 - 64	33.1	7.5×10^{-7}

The basis for determining that the initial results from these five locations were outliers was not presented in the certification submittals. The procedure for handling the Shelby tubes to prepare the samples that were submitted for retesting and the intervals within the Shelby tube submitted for retesting were not described in the certification submittals. The reasons for the different results were not presented in the certification submittals. Descriptions of the lithology of the samples submitted for retesting were not presented in the certification submittals.

Response: It was agreed during the January 5 meeting that Cell 16 would not be certified at this time. A berm is to be constructed between Cells 15 and 16 until the sandy areas of Cell 16 can be properly "patched" with appropriate confining material.

Samples from ST-13, ST-14, ST-16, ST-17, and ST-21 were re-evaluated by Universal Engineering Sciences due to the questionable results originally received from Ardaman & Associates, Inc.

The section of the HAI transmittal letter dated October 8, 2003 entitled "CELL 1 CQA TESTING" indicates that the correlation between permeability and percent fines results in acceptable confining materials containing at least 31% fines, as presented in Appendix B of the submittal. It appears that the initial testing results from these five locations ("outliers") were omitted from the correlation between sample permeability and percent fines. It also appears that a power function was selected by HAI for this correlation for all permeability tests conducted at the facility excluding the outliers, with an R^2 value of about 0.54. Alternate correlation #1 was prepared with the same data points using an exponential function to better fit the data and resulted in an R^2 value of about 0.66. Using alternate correlation #1, it is estimated that acceptable confining materials would be required to contain about 42% fines (see attached plot entitled "Perms. vs % Fines (Alternate Correlation #1) and attached summary table).

The section of the HAI transmittal letter dated October 8, 2003 entitled "CELL 1 CQA TESTING" describes seven "field units" in the soils encountered at the facility, four of which were indicated to be acceptable confining materials (sandy clay, silty clay, clay and clayey sand) and three of which were indicated to be unacceptable confining materials (silty sand, limestone marl and limestone). It appears that the clayey sand field unit exhibits a range of percent fine and permeability values that require further evaluation regarding its suitability as confining material. Alternate correlation #2 was prepared with only the samples that were described to be clayey sands to obtain a better solution ("better fit") for the data points. To be consistent with the approach taken by HAI, the "outliers" were excluded from alternate correlation #2, and an exponential function was selected, with an R^2 value of about 0.77. Using alternate correlation #2, it is estimated that acceptable confining materials would be required to contain about 37% fines (see attached plot entitled "Perms. vs % Fines (Alternate Correlation #2) and attached summary table).

The differences in the correlation between percent fines and permeability provided by HAI and the alternate correlations described above appear to be significant when depicting the soil types that are included as part of the confining unit. It appears appropriate to exclude the clayey sand field unit from the confining materials based solely on physical description unless the clayey sand sediments in individual borings have testing data that demonstrates an acceptable % fines content is present.

Instances where the thickness of the confining unit within Cell 1 as shown on Figure 38 in the HAI submittal dated October 8, 2003 is subject to revision if acceptable confining materials are determined by using at least 37% fines and clayey sands are excluded unless supported by a sieve test are summarized below:

- AS-6: a sieve test conducted on the clayey sands encountered at 2 feet BLS indicated the sample was 31.7% fines; it appears that no acceptable confining materials were demonstrated at this location rather than the 3 feet shown on Figure 38.
- AS-11: it does not appear that a sieve test was conducted on the clayey sands that were encountered from 2.5-3 feet BLS; it appears that 2.5 feet of acceptable confining materials were demonstrated at this location rather than the 3 feet shown on Figure 38.
- AS-16: it does not appear that a sieve test was conducted on the clayey sands that were encountered from 0-2 feet BLS; it appears that 1 foot of acceptable confining materials was demonstrated at this location rather than the 3 feet shown on Figure 38; it is unclear if this boring location was included in the area excavated as part of Test Pit No. 4.
- AS-18: the silty sand sediments encountered from 0-2 feet BLS were indicated to be unacceptable materials for the confining layer; it appears that the boring was too shallow to demonstrate the occurrence of 3 feet of acceptable confining materials at this location.
- AS-19: it does not appear that a sieve test was conducted on the clayey sands that were encountered from 0-2 feet BLS; a sieve test conducted on the sandy clay encountered at 2 feet BLS indicated the sample was 34.9% fines; it appears that no acceptable confining materials were demonstrated at this location rather than the 3 feet shown on Figure 38.
- AS-34: it does not appear that a sieve test was conducted on the clayey sands that were encountered from 0-1 foot below land surface (ft BLS); it appears that 2 feet of acceptable confining materials were demonstrated at this location rather than the 3 feet shown on Figure 38; it is unclear if this boring location is outside the portion of Cell 1 that was intended to be included in the certification.
- AS-37: it does not appear that a sieve test was conducted on the clayey sands that were encountered from 1.5-1.75 feet BLS; it appears that 1.25 feet of acceptable confining materials were demonstrated at this location rather than the 3 feet shown on Figure 38.
- AS-38: it does not appear that a sieve test was conducted on the clayey sands that were encountered from 1-1.75 feet BLS; it appears that 1.25 feet of acceptable confining materials were demonstrated at this location rather than the 3 feet shown on Figure 38.
- AS-42: a sieve test conducted on the clayey sands encountered at 2 feet BLS indicated the sample was 15.8% fines; it appears that no acceptable confining materials were demonstrated at this location rather than the 3 feet shown on Figure 38.

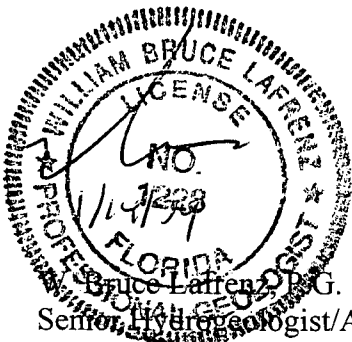
- AS-44: a sieve test conducted on the clayey sands encountered at 2 feet BLS indicated the sample was 33.5% fines; it appears that no acceptable confining materials were demonstrated at this location rather than the 3 feet shown on Figure 38.
- SSA-19: it does not appear that sieve tests were conducted on the clayey sands that were encountered from 0-4 ft BLS and 9-13 ft BLS; it appears that 5 feet of acceptable confining materials were demonstrated at this location rather than the 13 feet shown on Figure 38; it is unclear if this boring location is outside the portion of Cell 1 that was intended to be included in the certification.
- SSA-20: a sieve test conducted on the clayey sands encountered from 5-10 feet BLS indicated the sample was 29% fines; it appears that 4 feet of acceptable confining materials were demonstrated at this location rather than the 18 feet shown on Figure 38; it is unclear if this boring location is outside the portion of Cell 1 that was intended to be included in the certification.

Response: Please see the response to comment 2)c).

Given the importance of determining the occurrence of acceptable confining materials at the facility, it does not seem appropriate to further review the boring logs, contour maps and cross sections until the questions about the correlation evaluation have been resolved.

Response: The boring logs, contour maps, and cross-sections have already been reviewed by Kim Ford, with responses provided to the Department.

We trust this submittal will allow the Department's approval for operation of the landfill in Cell 1. Please call us if you have any questions.

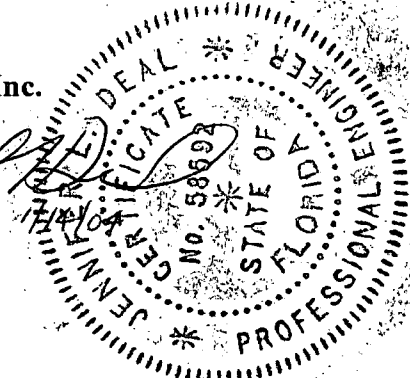


Bruce Lafrenz, P.E.
Senior Hydrogeologist/Associate

Very truly yours,

Hartman & Associates, Inc.

Jennifer L. Deal, P.E.
Project Manager

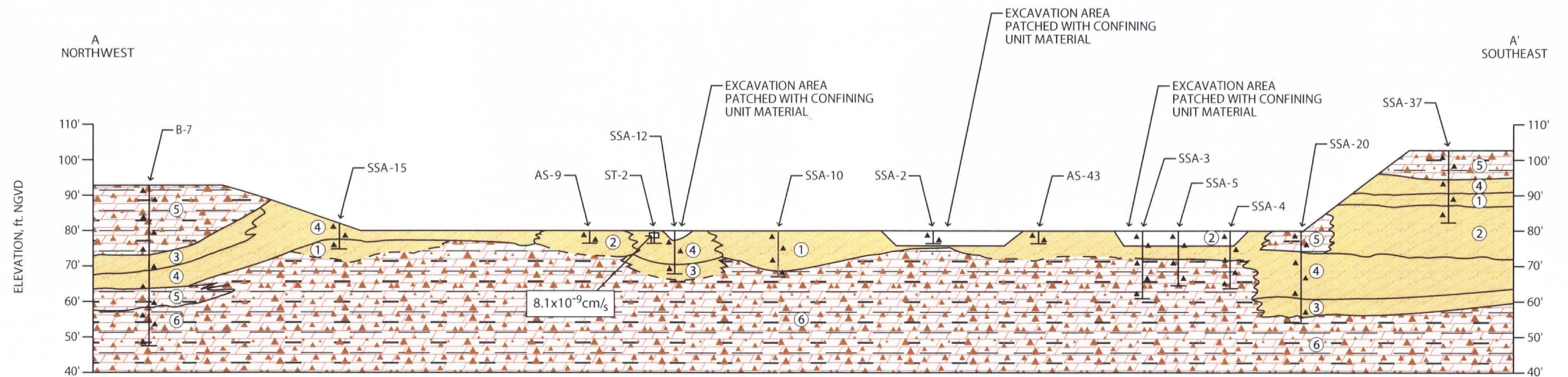


Ms. Susan Pelz, P.E.
January 14, 2004
Page 21

cc: Dominic Iafrate, Angelo's
Craig Bryan, Angelo's
Kim Ford, P.E., FDEP
John Morris, P.G., FDEP
James E. Golden, P.G., HSA Golden

APPENDICES

APPENDIX A



HORIZONTAL SCALE 1" = 60'
VERTICAL SCALE 1" = 30'



LEGEND

- | | |
|---|---|
| CONFINING UNITS 1-4:
(1) SANDY CLAY
(2) SILTY CLAY
(3) CLAY
(4) CLAYEY SAND | UNITS 5-7: (5) SILTY SAND
(6) LS MARL
(7) LIMESTONE |
| 5.1 x 10 ⁻⁸ CM/S - PERMEABILITY TEST RESULTS | LIMESTONE FRAGMENTS |



FIGURE
1



HARTMAN & ASSOCIATES, INC.
engineers, hydrogeologists, surveyors, & management consultants
201 EAST PINE STREET - SUITE 1000 - ORLANDO, FL 32801
TELEPHONE (407) 839-3955 - FAX (407) 839-3790

CELL No. 1- CROSS-SECTION A-A' - CERTIFICATION
ENTERPRISE RECYCLING AND DISPOSAL FACILITY
DADE CITY, FLORIDA

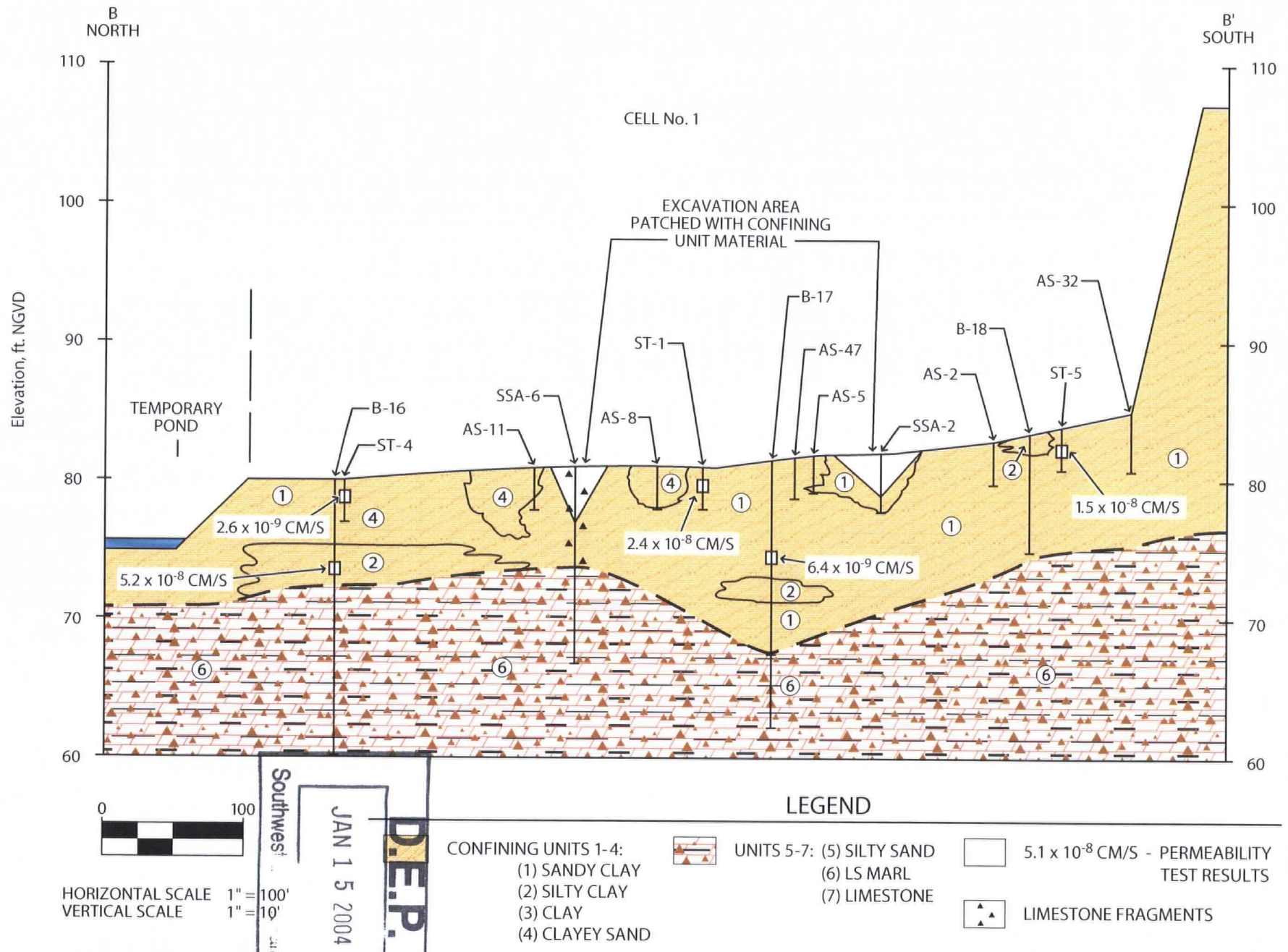
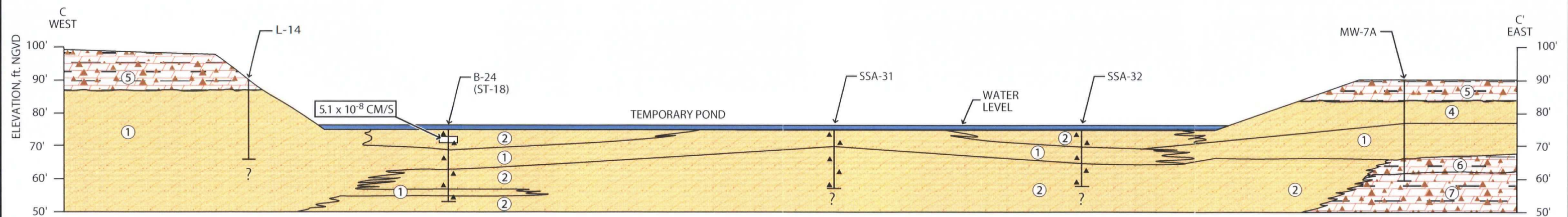


FIGURE 2



HARTMAN & ASSOCIATES, INC.
engineers, hydrogeologists, surveyors, & management consultants
201 EAST PINE STREET - SUITE 1000 - ORLANDO, FL 32801
TELEPHONE (407) 839-3955 - FAX (407) 839-3790

**CELL No. 1 - CROSS-SECTION B-B' - CERTIFICATION
ENTERPRISE RECYCLING AND DISPOSAL FACILITY
DADE CITY, FLORIDA**



HORIZONTAL SCALE 1" = 60'
 VERTICAL SCALE 1" = 30'

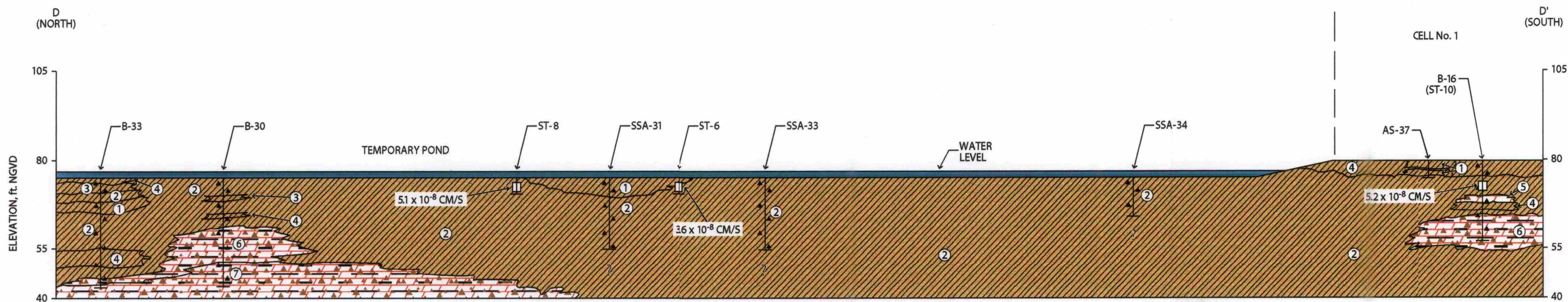


LEGEND

- | | |
|---|---------------------------|
| CONFINING UNITS 1-4: | UNITS 5-7: (5) SILTY SAND |
| (1) SANDY CLAY | (6) LS MARL |
| (2) SILTY CLAY | (7) LIMESTONE |
| (3) CLAY | |
| (4) CLAYEY SAND | |
| 5.1×10^{-8} CM/S - PERMEABILITY TEST RESULTS | LIMESTONE FRAGMENTS |



1572197 ①



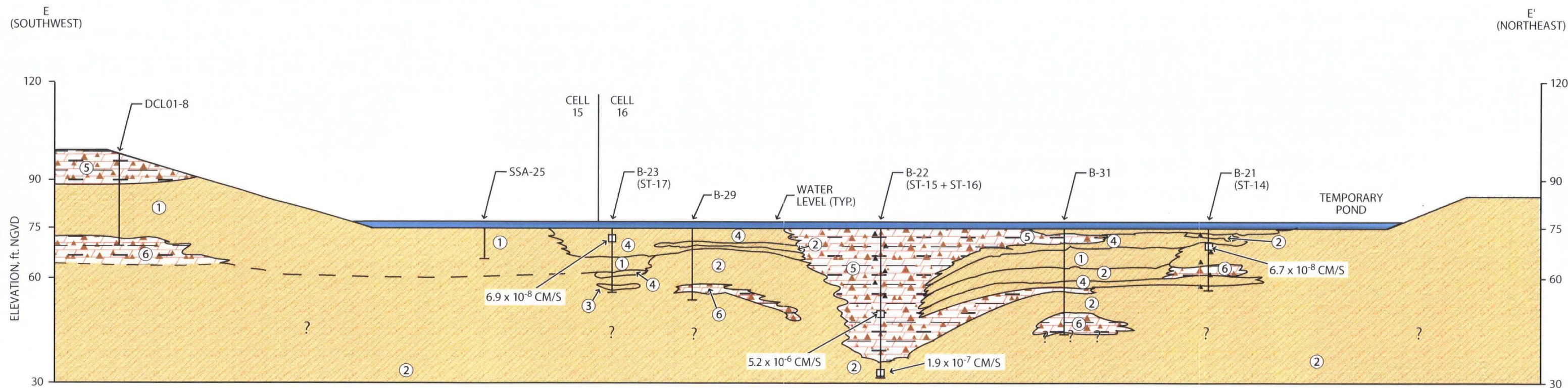
HORIZONTAL SCALE 1" = 50'
VERTICAL SCALE 1" = 25'



LEGEND

- | | |
|--|--|
| <p>CONFINING UNITS 1-4:</p> <p>(1) SANDY CLAY</p> <p>(2) SILTY CLAY</p> <p>(3) CLAY</p> <p>(4) CLAYEY SAND</p> | <p>UNITS 5-7: (5) SILTY SAND</p> <p>(6) LS MARL</p> <p>(7) LIMESTONE</p> |
| <p>5.1 x 10⁻⁸ CM/S - PERMEABILITY TEST RESULTS</p> | <p>LIMESTONE FRAGMENTS</p> |

99.0331.007.15.dbf Fig. 4, 19.25.11, 10/07/03

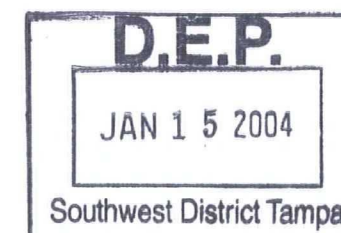


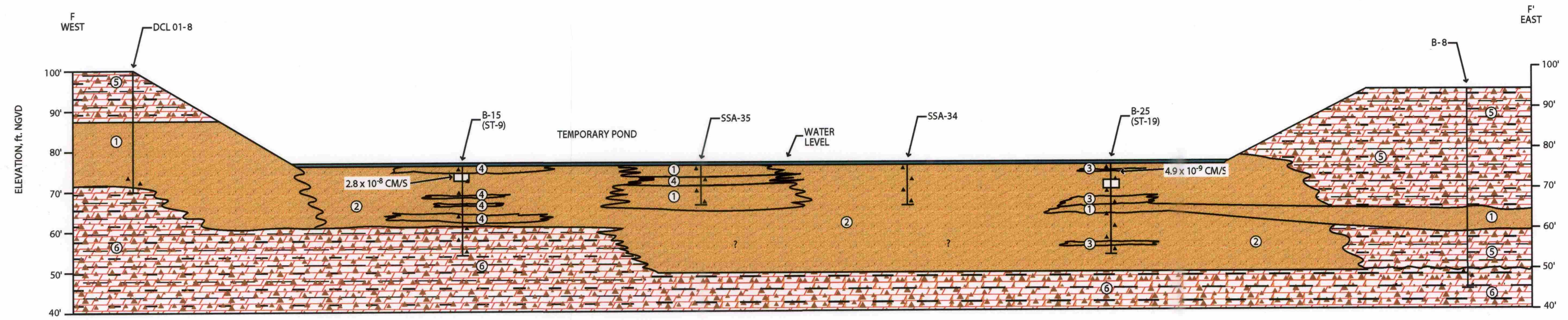
HORIZONTAL SCALE 1" = 60'
 VERTICAL SCALE 1" = 30'



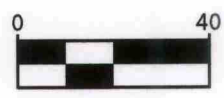
LEGEND

- | | |
|--|--|
| <p> CONFINING UNITS 1-4:
 (1) SANDY CLAY
 (2) SILTY CLAY
 (3) CLAY
 (4) CLAYEY SAND</p> <p> 5.1 x 10⁻⁸ CM/S - PERMEABILITY TEST RESULTS</p> | <p> UNITS 5-7: (5) SILTY SAND
 (6) LS MARL
 (7) LIMESTONE</p> <p> LIMESTONE FRAGMENTS</p> |
|--|--|





HORIZONTAL SCALE 1" = 40'
VERTICAL SCALE 1" = 20'



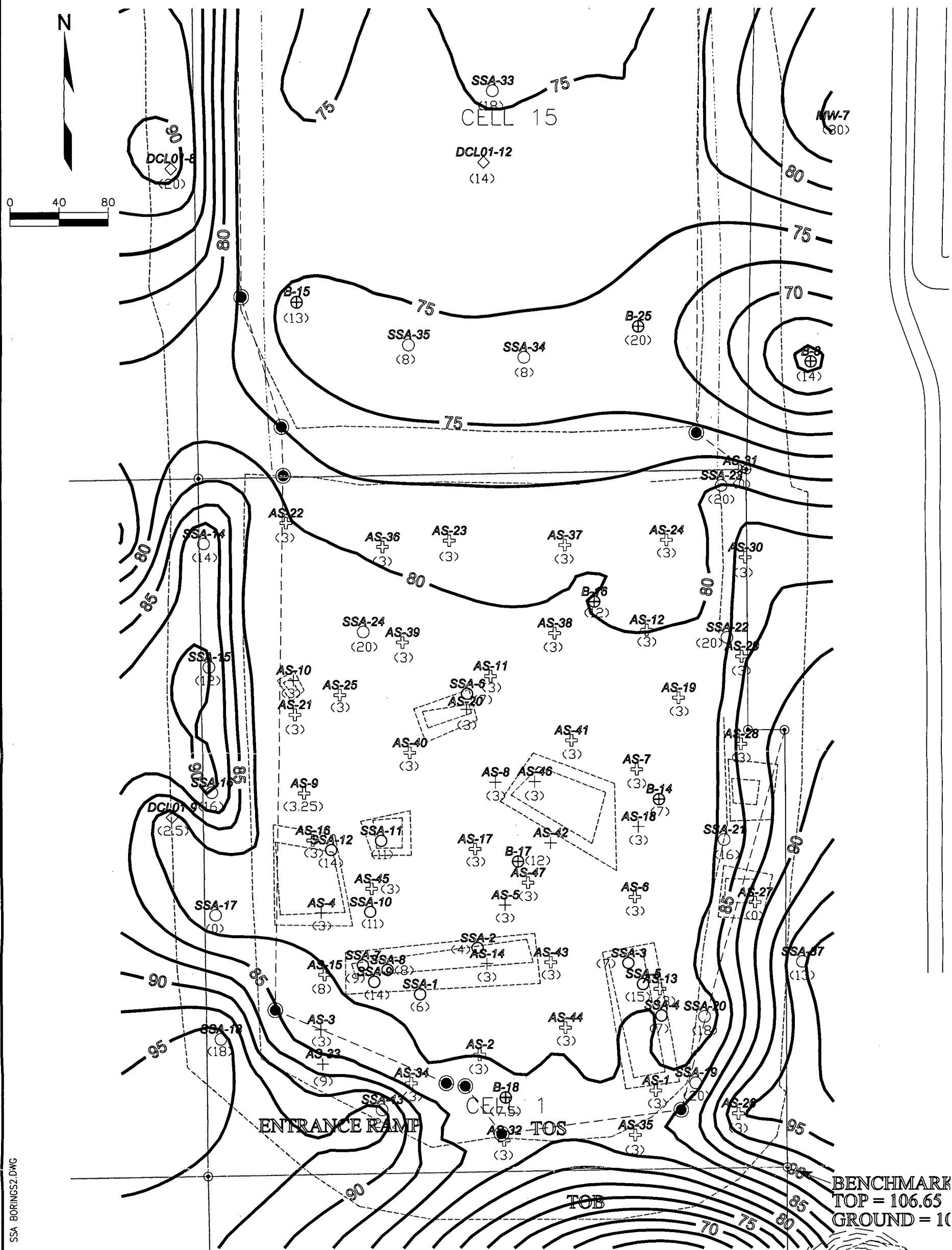
LEGEND

- | | |
|---|---|
| CONFINING UNITS 1-4:
(1) SANDY CLAY
(2) SILTY CLAY
(3) CLAY
(4) CLAYEY SAND | UNITS 5-7: (5) SILTY SAND
(6) LS MARL
(7) LIMESTONE |
| 5.1×10^{-8} CM/S - PERMEABILITY TEST RESULTS | LIMESTONE FRAGMENTS |

D.E.P.
JAN 15 2004
Southwest District Tampa

990331.007 - 6 - 01/15/04 - 11x17 - 1/10/04/02/04

APPENDIX B



LEGEND:

7.5
B-18

BORING W/ THICKNESS OF
CONFINING LAYER, FT.



TEST PIT, TYPICAL / LIMESTONE OVER
EXCAVATION AREA W/ PIT NUMBER

80

CONFINING LAYER CONTOUR,
FT., NGVD

○

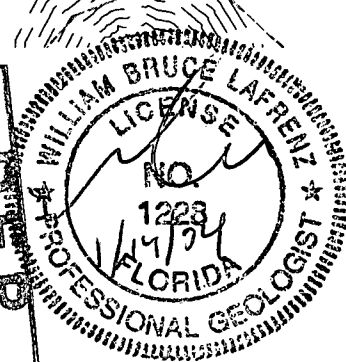
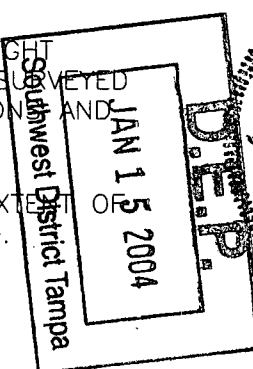
CELL SURVEYED CORNER POSTS

●

SURVEYED EXTENT OF CERTIFICATION

*NOTE: PLEASE REFER TO FORESIGHT
SURVEYORS SHEET 3 FOR ALL SURVEYED
BORING LOCATIONS WITH ELEVATION AND
DEGREE OF ACCURACY.

*CONTOUR LINES OUTSIDE THE EXTENT OF
CERTIFICATION ARE EXTRAPOLATED.



N:\CAD\DWG\1999\99-331.01\MODELS\CELL 1 SSA BORINGS2.DWG

Plotted: Jan 14, 2004 - 1:19pm by smf



LEGEND:

7.5
B-18

BORING W/ THICKNESS OF
CONFINING LAYER, FT.



TEST PIT, TYPICAL / LIMESTONE OVER
EXCAVATION AREA W/ PIT NUMBER

80

CONFINING LAYER CONTOUR,
FT., NGVD



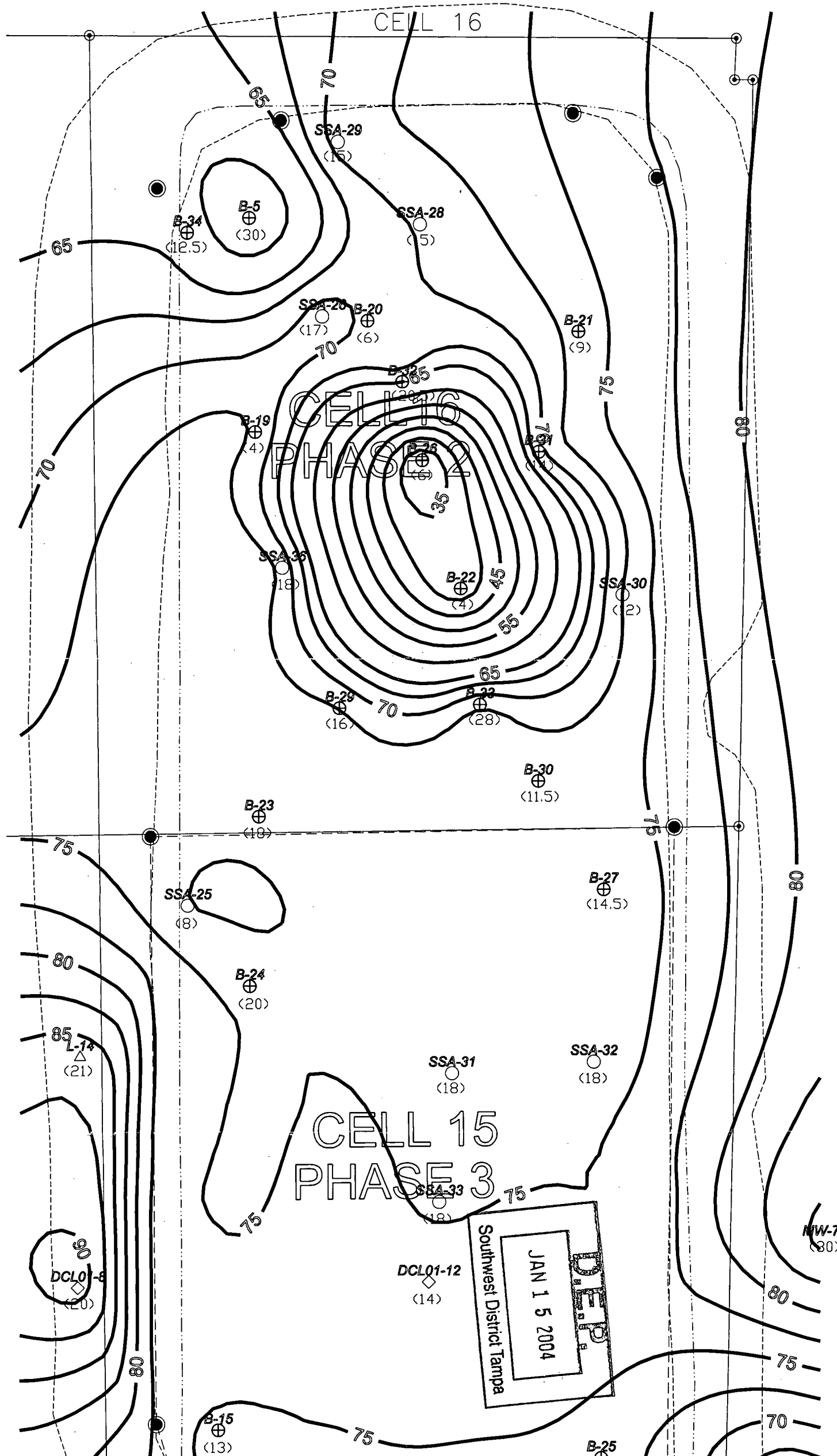
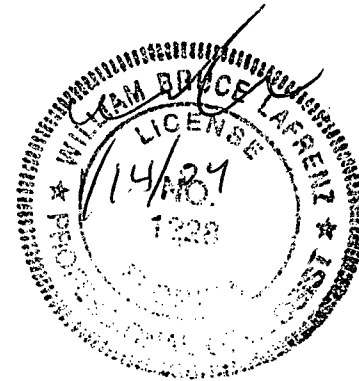
CELL SURVEYED CORNER POSTS



SURVEYED EXTENT OF
CERTIFICATION

*NOTE: PLEASE REFER TO FORESIGHT
SURVEYORS SHEET 3 FOR ALL SURVEYED
BORING LOCATIONS WITH ELEVATIONS AND
DEGREE OF ACCURACY.

*CONTOUR LINES OUTSIDE THE EXTENT OF
CERTIFICATION ARE EXTRAPOLATED.



N:\CAD\DWG\1999\99-331.01\MODELS\CELL 1 SSA BORINGS2.DWG

Plotted: Jan 14, 2004 - 1:03pm by smf

FIGURE
39



HARTMAN & ASSOCIATES, INC.
engineers, hydrogeologists, surveyors & management consultants
201 EAST PINE STREET - SUITE 1000 - ORLANDO, FL 32801
TELEPHONE (407) 839-3955 - FAX (407) 839-3790

**CONFINING LAYER CONTOUR MAP TEMPORARY POND
ENTERPRISE RECYCLING & DISPOSAL FACILITY
DADE CITY, FLORIDA**

Cell 1													
X	Y	Top of Clay (ft, NGVD)	Ground Elev (ft, NGVD)	Clay thickness (ft, BTOG)	Boring No.	Symbol) for plotting)							
613666.21	1454553.35	52.5	75.00	30	B-5	44							
613499.52	1453482.00	73	93.00	10	B-7	44							
614062.48	1453624.70	63	93.00	14	B-8	44							
613483.64	1453255.17	90	110.00	19	L-13	18							
613543.36	1453943.14	87	100.00	21	L-14	18							
613937.96	1453268.36	81.5	110.00	7	B-14	44							
613991.27	1452779.82	12.5	98.00	5	B-16P	44							
613542.01	1453775.63	91.5	100.00	20	DCL01-8	6							
613541.98	1453253.67	79.5	104.00	2.5	DCL01-9	6							
613549.11	1452667.25	92.5	106.00	16.5	DCL01-11	6							
613796.49	1453780.55	86.5	95.00	14	DCL01-12	6							
614080.63	1453809.83	86	91.00	30	MW-7	103							
614115.80	1453252.03	94.5	98.00	26.5	MW-8	103							
614117.78	1452752.03	107.5	111.00	21.5	MW-9	103							
613643.87	1453672.59	73.27	75.27	13	B-15	44							
613885.70	1453429.42	80.38	80.38	12	B-16	44							
613823.27	1453217.37	81.02	81.02	12	B-17	44							
613812.84	1453023.70	83.43	83.43	7.5	B-18	44							
613670.48	1454397.54	72.89	74.89	4	B-19	44							
613751.91	1454480.13	69.23	75.23	6	B-20	44							
613904.87	1454472.28	73.27	75.27	9	B-21	44							
613819.30	1454283.82	34.18	75.18	4	B-22	44							
613672.92	1454117.22	73	75.00	18	B-23	44							
613666.42	1453994.73	73.19	75.19	20	B-24	44							
613921.87	1453653.02	73.62	75.62	20	B-25	44							
613791.55	1454377.08	27	75.00	6	B-26	44							
613923.38	1454065.01	72.89	74.89	14.5	B-27	44							
613371.99	1454257.30	64	96.00	8	B-28	44							
613731.41	1454196.56	72.97	74.97	16	B-29	44							
613875.70	1454143.76	73.12	75.12	11.5	B-30	44							
613876.14	1454383.18	71.02	75.02	14	B-31	44							
613777.05	1454434.64	67.55	75.05	26.5	B-32	44							
613833.12	1454199.21	73.51	75.51	28	B-33	44							
613621.17	1454542.79	63.97	75.43	12.5	B-34	44							
613712.21	1453013.87	95	95.00	24	SSA-13	12							
613568.19	1453478.04	92	92.00	14	SSA-14	12							
613572.19	1453376.67	91.5	91.50	12	SSA-15	12							

613574.22	1453273.94	93	93.00	16	SSA-16	12								
613577.06	1453172.90	80	94.00	0	SSA-17	12	clay at less than 80 feet							
613581.01	1453070.33	97	97.00	18	SSA-18	12								
613967.84	1453035.54	84.89	84.89	20	SSA-19	12								
613975.02	1453088.72	79.43	84.43	18	SSA-20	12								
613991.18	1453235.26	82.77	82.77	16	SSA-21	12								
613993.89	1453400.13	82.26	82.26	20	SSA-22	12								
613989.67	1453525.38	80.7	80.70	20	SSA-23	12								
613697.65	1453405.08	80.11	80.11	20	SSA-24	12								
613621.30	1454052.73	72.25	74.25	8	SSA-25	12								
613719.12	1454483.28	72.01	75.01	17	SSA-26	12								
							613666.10	1454553.46	no clay	74.79	0	SSA-27	12	
613789.86	1454548.77	70.07	75.07	15	SSA-28	12								
613730.34	1454607.37	71.77	76.77	15	SSA-29	12								
613936.84	1454279.58	73.12	75.12	12	SSA-30	12								
613813.08	1453931.81	73.53	75.53	18	SSA-31	12								
613916.19	1453940.40	73.03	75.03	18	SSA-32	12								
613803.86	1453838.52	73.26	75.26	18	SSA-33	12								
613828.80	1453628.21	72.93	74.93	8	SSA-34	12								
613734.96	1453638.36	73.27	75.27	8	SSA-35	12								
613690.11	1454299.11	72.89	74.89	18	SSA-36	12								
614054.76	1453133.21	98	103.00	13	SSA-37	12								
613935.21	1453029.25	83.29	83.29	3	AS-1	2								
613791.82	1453057.84	82.49	82.49	3	AS-2	2								
613918.35	1453187.88	81.1	81.1	3	AS-6	2								
613920.00	1453292.65	81.03	81.03	3	AS-7	2								
613649.25	1453273.24	80.3	80.3	3.25	AS-9	2								
613801.24	1453368.97	81.04	81.04	3	AS-11	2								
613928.37	1453406.26	80.29	80.29	3	AS-12	2								
613938.63	1453110.74	82.74	82.74	3	AS-13	2								
613665.68	1453122.47	82.6	82.6	3	AS-15	2								
613656.18	1453234.63	80.55	80.55	3	AS-16	2								
613788.38	1453227.58	80.89	80.89	3	AS-17	2								
613954.09	1453351.20	81.04	81.04	3	AS-19	2								
613641.90	1453337.49	80.43	80.43	3	AS-21	2								
613634.96	1453495.09	79.98	79.98	3	AS-22	2								
613767.99	1453480.20	79.93	79.93	3	AS-23	2								
613944.84	1453480.30	79.88	79.88	3	AS-24	2								
613678.58	1453353.70	80.31	80.31	3	AS-25	2								
614002.73	1453010.81	96.25	96.25	3	AS-26	2								
614016.46	1453183.52	no clay	92.18	0	AS-27	2								
614005.07	1453313.59	86.57	86.57	3	AS-28	2								

614005.78	1453384.78	84.98	84.98	3	AS-29	2							
614008.96	1453465.79	85.03	85.03	3	AS-30	2							
614004.88	1453537.17	no clay	84.87	0	AS-31	2							
613811.59	1452988.89	85.14	85.14	3	AS-32	2							
613736.03	1453035.22	82.81	82.81	3	AS-34	2							
613918.46	1452993.19	85.62	85.62	3	AS-35	2							
613713.63	1453475.07	79.98	79.98	3	AS-36	2							
613861.94	1453476.25	79.96	79.96	3	AS-37	2							
613853.35	1453403.70	80.2	80.2	3	AS-38	2							
613729.59	1453396.38	79.96	79.96	3	AS-39	2							
613735.31	1453306.43	81.07	81.07	3	AS-40	2							
613866.79	1453316.45	81.13	81.13	3	AS-41	2							
613849.70	1453132.69	81.73	81.73	3	AS-43	2							
613861.53	1453079.57	82.25	82.25	3	AS-44	2							
613704.05	1453194.87	81.26	81.26	3	AS-45	2							
613831.31	1453199.87	81.29	81.29	3	AS-47	2							
							excavated/patched borings						
613662.47	1453078.03	83.23	83.23	3	AS-3	0							
613662.93	1453174.84	81.49	81.49	3	AS-4	0							
613812.76	1453181.39	80.9	80.9	3	AS-5	0							
613804.95	1453282.22	80.64	80.64	3	AS-8	0							
613640.78	1453365.78	80.33	80.33	3	AS-10	0							
613797.95	1453131.64	81.77	81.77	3	AS-14	0							
613921.27	1453245.96	81.02	81.02	3	AS-18	0							
613781.62	1453341.84	81.05	81.05	3	AS-20	0							
613663.88	1453050.36	85.67	85.67	3	AS-33	0							
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613743.35	1453106.69	82.2	82.2	6	SSA-1	11							
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613939.92	1453089.47	83.0	83.0	7	SSA-4	11							
613924.84	1453114.76	82.5	82.5	15	SSA-5	11							
613782.12	1453354.61	80.8	80.8	7	SSA-6	11							
613696.87	1453130.51	82.0	82.0	9	SSA-7	11							
613716.93	1453126.64	81.7	81.7	8	SSA-8	11							
613706.02	1453116.80	82.1	82.1	14	SSA-9	11							
613702.82	1453175.65	82.1	82.1	11	SSA-10	11							
613711.86	1453234.41	80.9	80.9	11	SSA-11	11							
613671.16	1453226.89	80.7	80.7	14	SSA-12	11							

APPENDIX C



Florida Department of Environmental Protection
Twin Towers Office Bldg. • 2600 Blair Stone Road • Tallahassee, FL 32399-2400

DEP Form # 62-701.900(2)
Form Title Certification of Construction Completion
Effective Date May 19, 1994

DEP Application No. _____
(Filled by DEP)

Certification of Construction Completion of a Solid Waste Management Facility

DEP Construction Permit No: 177982-001-SC County: Pasco

Name of Project: Enterprise Recycling & Disposal Facility

Name of Owner: Angelo's Aggregate Materials, Ltd.

Name of Engineer: Hartman & Associates, Inc.

Type of Project: Confining layer certification for Cells 1 and 15, as indicated by survey markers on
Figure 1 of Appendix A of this report.

Cost: Estimate \$ 400,000 Actual \$ 450,000

Site Design: Quantity: 1500 cy/day ton/day Site Acreage: 9.2 Acres

Deviations from Plans and Application Approved by DEP: Please see the attached summary.

Address and Telephone No. of Site: Enterprise Road, west of Auton Road, Dade City
813-781-6177

Name(s) of Site Supervisor: Jeff Rogers

Date Site inspection is requested: Pre-arranged inspection on January 21, 2004

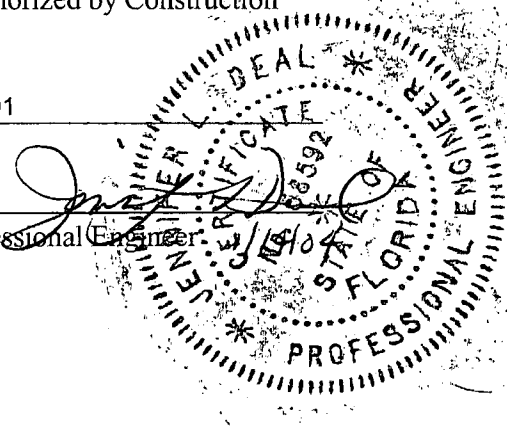
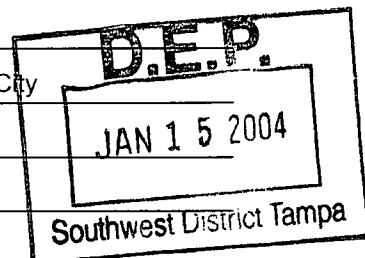
This is to certify that, with the exception of any deviation noted above, the construction of the project has been completed in substantial accordance with the plans authorized by Construction

Permit No. 177982-001-SC :Dated: October 5, 2001

Date: 1/14/04

Signature of Professional Engineer: _____

Page 1 of 1



**SUMMARY OF DEVIATIONS FROM FDEP APPROVED PLANS
ENTERPRISE RECYCLING & DISPOSAL FACILITY
DADE CITY, FLORIDA**

As-built survey data to confirm the grades in Cell 1 is provided in Appendix D. The survey shows that the cell grades are in substantial compliance with the requirements of the approved excavation plan, as required by the specific conditions of the facility construction permit.

Deviations to the original approved plans for Cell 1 are indicated in the as-built survey and illustrated on the record drawing in Appendix D. The 50-foot overcut and associated berm and transport swale was not constructed as planned. A berm has been constructed near the western slope of Cell 1, and an open channel slopes towards the temporary pond. HAI has confirmed that the open channel is adequate to transport the stormwater runoff from west of the slope to the temporary pond. A modification to the stormwater permit is pending and is expected to be issued by February 16, 2004.

The interior side slopes of Cell 1 were constructed steeper than shown on the approved excavation plan. All side slopes were to be constructed to 6H:1V until immediately prior to waste placement against the slopes. At this point, the slopes were to be cut back to 2H:1V. The south and west side slopes of the cell are approximately 3H:1V, and the east slope is approximately 4H:1V. These steeper slopes will have a higher potential for erosion, so Angelo's must implement the erosion controls in the approved plans on an as needed basis, rather than when the 2H:1V slopes are excavated.

The north and central portions of the eastern boundary of Cell 1 were excavated beyond the boundary indicated on the excavation plans, into a portion of the area designated for construction of Pond 2. Clean soil will need to be placed and compacted into the eastern slope prior to pond construction. However, the newly placed soil may be inadequate for construction and the pond may need to be redesigned in the remaining setback area to accommodate the required stormwater volume. This determination will require a geotechnical evaluation prior to construction. Pond 2 must be designed, constructed, and certified prior to any waste disposal at or above the pre-construction grade elevation of Cell 1.

Ditches have been recently constructed north to south near the center of the site, and west to east across the northeastern portion of the site in order to divert stormwater to an existing borrow pit in the north-central portion of the site. These ditches have reduced the quantity of stormwater flowing to the temporary pond, and therefore, will allow for construction of the western leg of the temporary pond (Cell 14) to be postponed until additional stormwater retention is required. A stormwater permit modification is currently under review by the Department's stormwater section.

Pond 1 was modified to accept some drainage from Enterprise Road. This modification was included in the above referenced stormwater permit modification.

The location of the entrance, scalehouse, scales, and maintenance area have changed, as shown on the revised drawings.

The six (6) foot chain link fence (post and barbed wire fence in some locations), 12-foot compacted perimeter road, entrance gate, "No Trespassing" signs, entrance sign, and placement

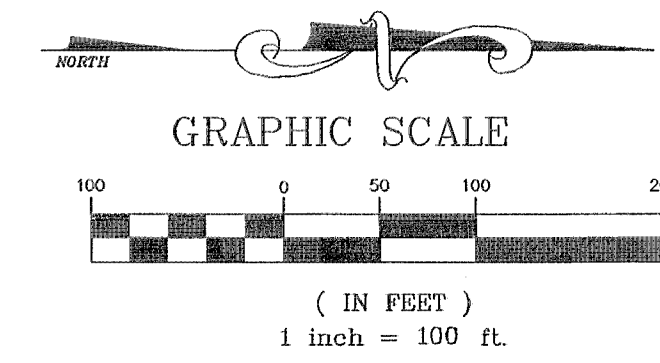
of roll-off containers have been completed. The maintenance area is intended for future use and therefore is not completed at this time.

A temporary berm has been constructed between Cells 15 and 16 and will remain in place until Cell 16 is certified.

APPENDIX D

ENTERPRISE RECYCLING AND DISPOSAL FACILITY

3



BOUNDARY LINE LANDFILL

Site Volume Table:

Site	Stratum Surf1	Surf2	yards	Cut	yards	Fill	yards	Net	Method
LIME AREAS	lime areas	LIME AREAS	TEMP POND				2410	(F)	Composite
SOUTH POND	south pond	southpond	pre southpond				44952	(F)	Composite
TEMP POND	temp pond	TEMP POND	pre contours				451895	(F)	Composite

TOTAL VOLUME REMOVED IN TEMP POND, SOUTHEAST POND, AND LIMEROCK AREAS = 499,257.00 CU YDS.

- NOTES:
- THIS DRAWING IS NOT INTENDED TO REPRESENT A BOUNDARY SURVEY.
 - ELEVATIONS SHOWN HEREON ARE BASED ON THE CONSTRUCTION PLANS FOR THE ENTERPRISE RECYCLING & DISPOSAL FACILITY AND ENTERPRISE RD. BENCH MARK USED IS A NAIL & DISK IN A POWER POLE STATION 115+74.55 58.36 LT. ELEVATION = 114.02'.
 - THIS SURVEY DRAWING WAS PREPARED FOR THE EXCLUSIVE USE OF THE PARTY OR PARTIES CERTIFIED TO BELOW FOR THE EXPRESS PURPOSE STATED HEREON AND/OR CONTAINED IN THE CONTRACT BETWEEN FORESIGHT SURVEYORS, INC. AND THE CLIENT FOR THIS PROJECT. COPYING, DISTRIBUTING, AND/OR USING THIS DRAWING, IN WHOLE OR IN PART FOR ANY PURPOSE OTHER THAN ORIGINALLY INTENDED WITHOUT WRITTEN CONSENT FROM FORESIGHT SURVEYORS, INC. IS STRICTLY PROHIBITED, AND RENDERS THE SURVEYOR'S CERTIFICATION, SIGNATURE AND SEAL HEREON NULL AND VOID. ANY QUESTIONS CONCERNING THE CONTENT OR PURPOSE OF THIS DRAWING SHOULD BE DIRECTED TO FORESIGHT SURVEYORS, INC.
 - THE DEGREE OF ACCURACY IS WITHIN 0.1 OF A FOOT HORIZONTALLY AND 0.05 OF A FOOT VERTICALLY ON ALL WORK PERFORMED.

D.E.P.
JAN 15 2004
Southwest District Tampa

TITLE: **ASBUILT SURVEY**

FOR: **GOODWIN BROS. CONSTRUCTION/HARTMAN AND ASSOCIATES**

DESC: **SECTION 8, TOWNSHIP 25 SOUTH, RANGE 22 EAST**

THIS SURVEY IS NOT VALID WITHOUT THE SIGNATURE AND ORIGINAL RAISED SEAL OF A FLORIDA LICENSED SURVEYOR AND MAPPER.

A. DANIEL MILLER
FLORIDA SURVEYORS' REGISTRATION NO. 2294

NO.	REVISION	DATE	BY
	ADD CORNER POSTS, BORINGS AND M.W.'S	01-08-04	ADM

FORESIGHT SURVEYORS, INC.
HYDROGRAPHIC, LAND & CONSTRUCTION SURVEYING
773 PROVIDENCE BOULEVARD, BROOKSVILLE, FLORIDA 34601
PH. (352) 797-6306 FAX (352) 797-6308 LB No.: 5776

CADD FILE: ASBUILT	PROJ MAN: _____	DRAWN BY: ADM	CHECKED BY: _____
PROJ. NO.: 22126	FB B64, PG 28	FIELD DATE: 11/07/03	SCALE: 1"=100'

ENTERPRISE RECYCLING AND DISPOSAL FACILITY

4

LIMEROCK AREAS



NOTES:

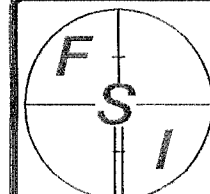
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2. ELEVATIONS SHOWN HEREON ARE BASED ON THE CONSTRUCTION PLANS FOR THE ENTERPRISE RECYCLING & DISPOSAL FACILITY AND ENTERPRISE RD. BENCH MARK USED IS A NAIL & DISK IN A POWER POLE STATION 115+74.55 58.36 LT. ELEVATION = 114.02'.
3. THIS SURVEY DRAWING WAS PREPARED FOR THE EXCLUSIVE USE OF THE PARTY OR PARTIES CERTIFIED TO BELOW FOR THE EXPRESS PURPOSE STATED HEREON AND/OR CONTAINED IN THE CONTRACT BETWEEN FORESIGHT SURVEYORS, INC. AND THE CLIENT FOR THIS PROJECT. COPYING, DISTRIBUTING, AND/OR USING THIS DRAWING, IN WHOLE OR IN PART FOR ANY PURPOSE OTHER THAN ORIGINALLY INTENDED WITHOUT WRITTEN CONSENT FROM FORESIGHT SURVEYORS, INC. IS STRICTLY PROHIBITED, AND RENDERS THE SURVEYOR'S CERTIFICATION, SIGNATURE AND SEAL HEREON NULL AND VOID. ANY QUESTIONS CONCERNING THE CONTENT OR PURPOSE OF THIS DRAWING SHOULD BE DIRECTED TO FORESIGHT SURVEYORS, INC.
4. THE DEGREE OF ACCURACY IS WITHIN 0.1 OF A FOOT HORIZONTALLY AND 0.05 OF A FOOT VERTICALLY ON ALL WORK PERFORMED.

D.E.P.
JAN 15 2004
Southwest District Tampa

TITLE: **ASBUILT SURVEY**
FOR: GOODWIN BROS. CONSTRUCTION/HARTMAN AND ASSOCIATES
DESC: SECTION 8, TOWNSHIP 25 SOUTH, RANGE 22 EAST

THIS SURVEY IS NOT VALID WITHOUT THE SIGNATURE AND ORIGINAL RAISED SEAL OF A FLORIDA LICENSED SURVEYOR AND MAPPER.

NO.	REVISION	DATE	BY



FORESIGHT SURVEYORS, INC.
HYDROGRAPHIC, LAND & CONSTRUCTION SURVEYING
773 PROVIDENCE BOULEVARD, BROOKSVILLE, FLORIDA 34601
PH (352) 797-6306 FAX (352) 797-6308 LB No.: 5776

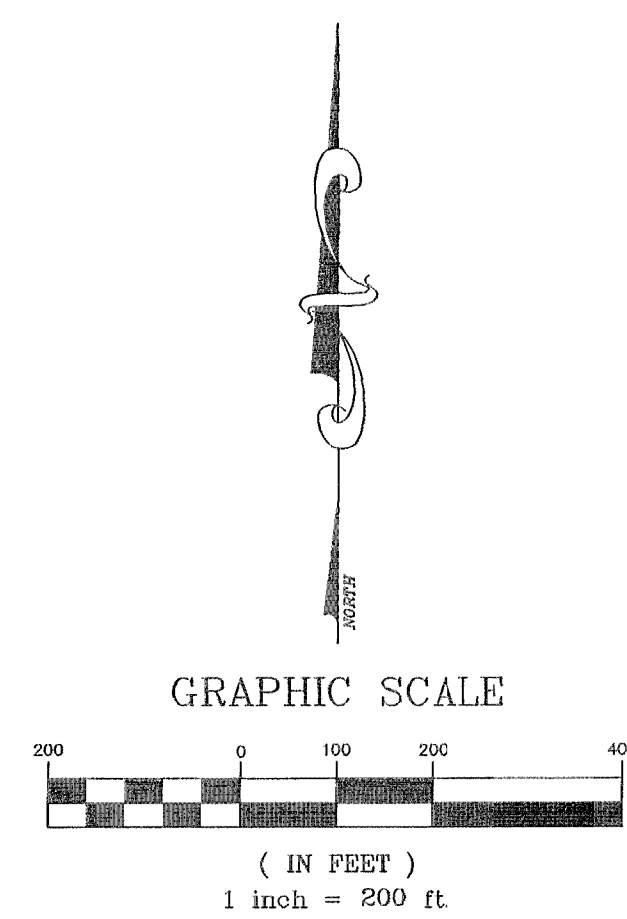
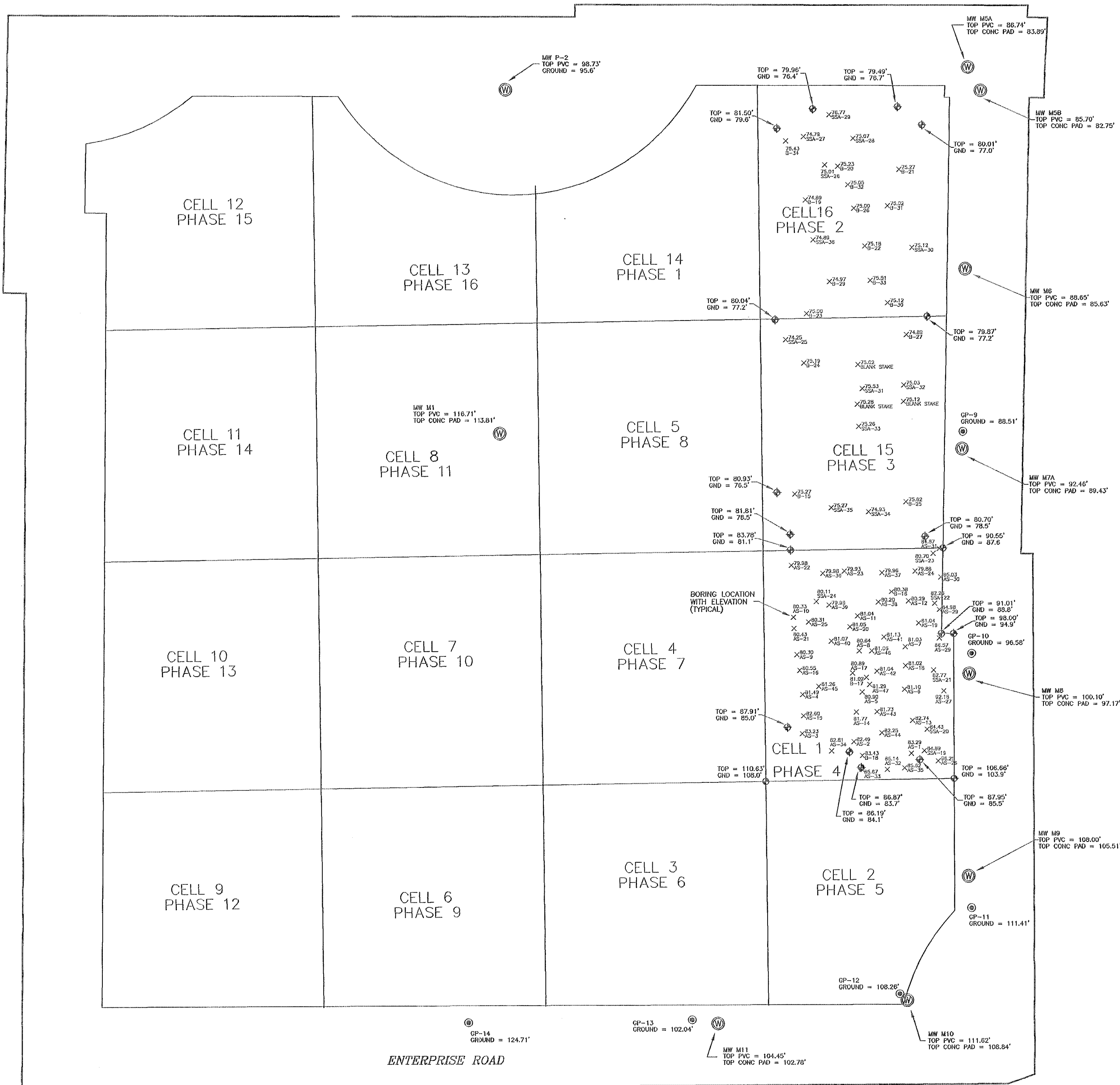
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PROJ. NO.: 22126		FIELD DATE: _____	SCALE: 1"=50'

SHEET
2
OF 3

ENTERPRISE RECYCLING AND DISPOSAL FACILITY

5

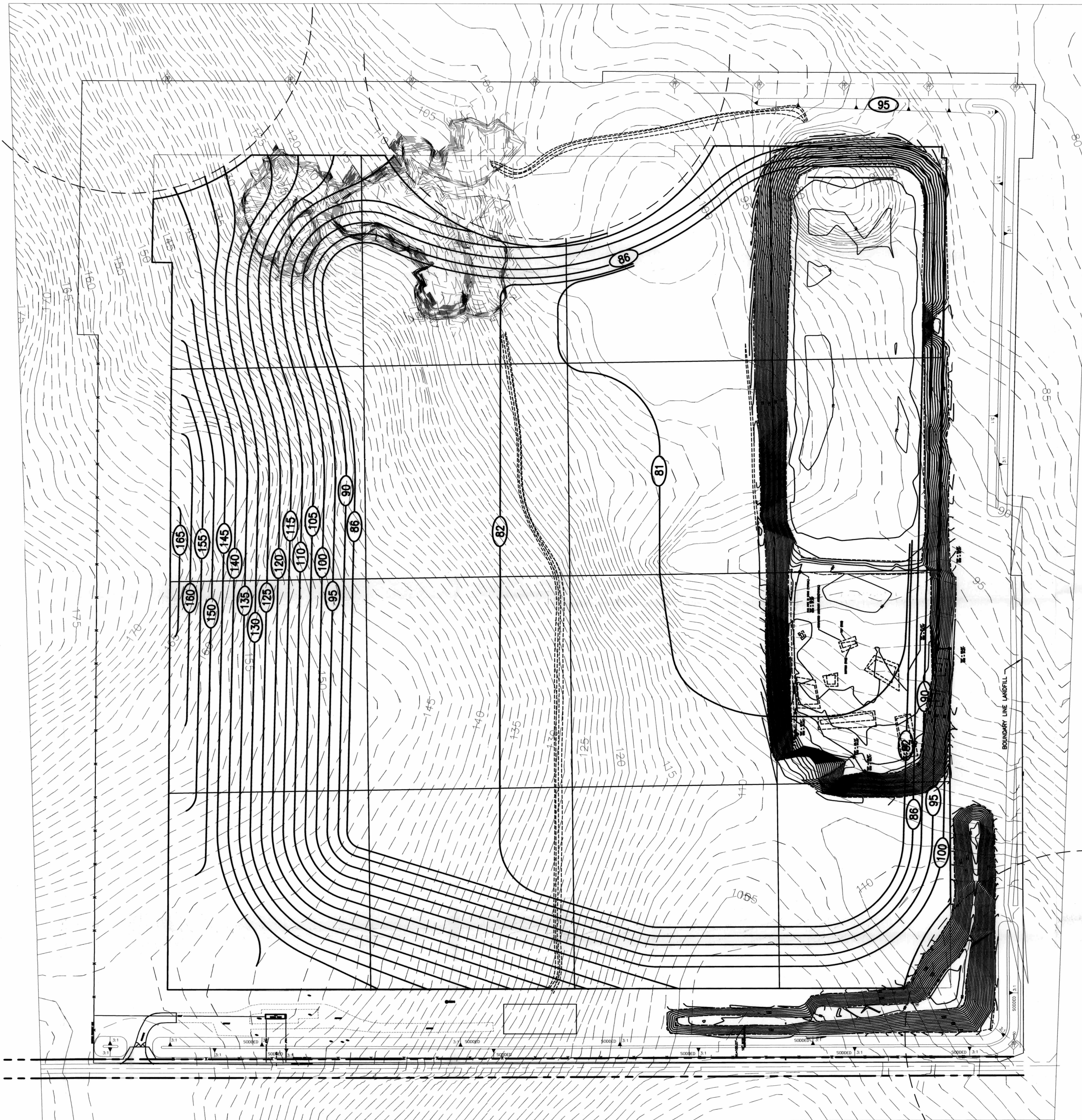
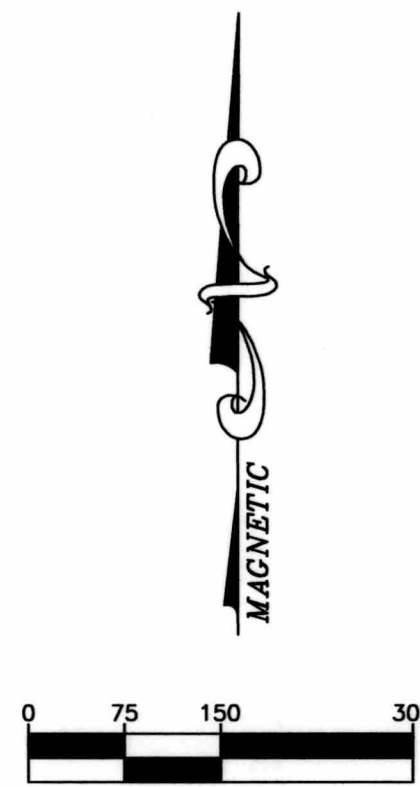
LOCATIONS OF MONITORING WELLS, GAS PROBES, AND BORING SITES



- NOTES:
- THIS DRAWING IS NOT INTENDED TO REPRESENT A BOUNDARY SURVEY.
 - ELEVATIONS SHOWN HEREON ARE BASED ON THE CONSTRUCTION PLANS FOR THE ENTERPRISE RECYCLING & DISPOSAL FACILITY AND ENTERPRISE RD. BENCH MARK USED IS A NAIL & DISK IN A POWER POLE STATION 115+74.55 58.36 LT. ELEVATION = 114.02'.
 - THIS SURVEY DRAWING WAS PREPARED FOR THE EXCLUSIVE USE OF THE PARTY OR PARTIES CERTIFIED TO BELOW FOR THE EXPRESS PURPOSE STATED HEREON AND/OR CONTAINED IN THE CONTRACT BETWEEN FORESIGHT SURVEYORS, INC. AND THE CLIENT FOR THIS PROJECT. COPYING, DISTRIBUTING, AND/OR USING THIS DRAWING, IN WHOLE OR IN PART FOR ANY PURPOSE OTHER THAN ORIGINALLY INTENDED WITHOUT WRITTEN CONSENT FROM FORESIGHT SURVEYORS, INC. IS STRICTLY PROHIBITED, AND RENDERS THE SURVEYOR'S CERTIFICATION, SIGNATURE AND SEAL HEREON NULL AND VOID. ANY QUESTIONS CONCERNING THE CONTENT OR PURPOSE OF THIS DRAWING SHOULD BE DIRECTED TO FORESIGHT SURVEYORS, INC.
 - THE DEGREE OF ACCURACY IS WITHIN 0.1 OF A FOOT HORIZONTALLY AND 0.05 OF A FOOT VERTICALLY ON ALL WORK PERFORMED.

- ⊕ = PVC PIPE IN CONCRETE (CORNER POST LOCATION)
- ⊙ = MONITORING WELL LOCATION
- ⊙ = GAS PROBE LOCATION

D.E.P.
JAN 15 2004
Southwest District Tampa



ELEV. PROPOSED EXCAVATION CONTOURS

Hartman & Associates, Inc.
201 E. Pine St. Suite 1000
Orlando, FL 32801
Engineering Business No. 5814
Jeniffer L. Deal, P.E.
Florida Registration No. 0000058592
Solid Waste

Hartman & Associates, Inc.
201 E. Pine St. Suite 1000
Orlando, FL 32801
Engineering Business No. 5814
Roderick E. Rodriguez, P.E.
Florida Registration No. 0000058592
1/15/04
Paving/Grading/Drainage

D.E.P.
JAN 15 2004
Southwest District Tampa

"RECORD DRAWING"

INCORPORATING THOSE CHANGES MADE DURING THE CONSTRUCTION PHASE OF THE PROJECT, BASED ON DATA FURNISHED BY THE CONTRACTOR TO THE ENGINEER.

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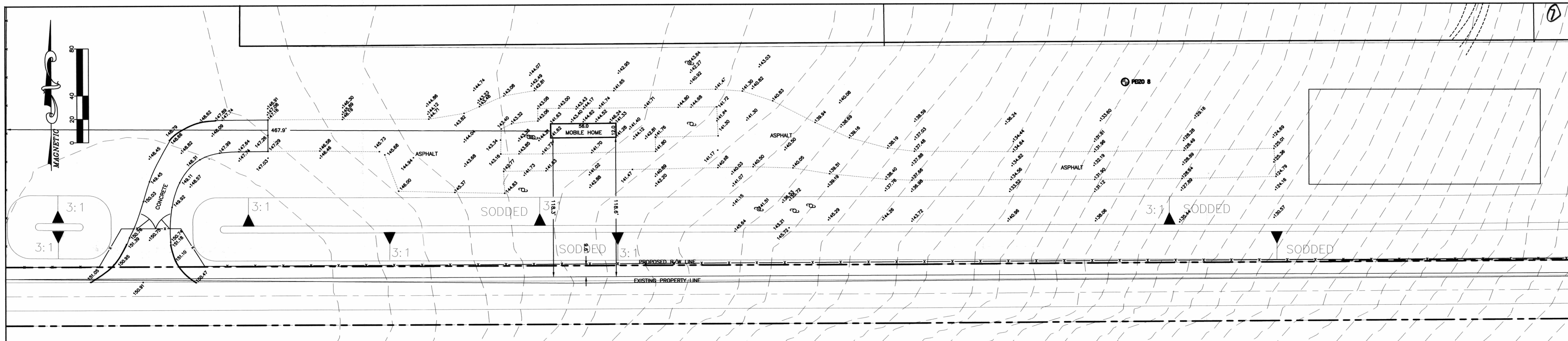
HARTMAN & ASSOCIATES, INC.
engineers, hydrogeologists, surveyors & management consultants
201 EAST PINE STREET - SUITE 1000 - ORLANDO, FL 32801
TELEPHONE (407) 839-3955 - FAX (407) 839-3790
ENGINEERING BUSINESS No. 5814

**ENTERPRISE LANDFILL
PASCO COUNTY
FLORIDA**

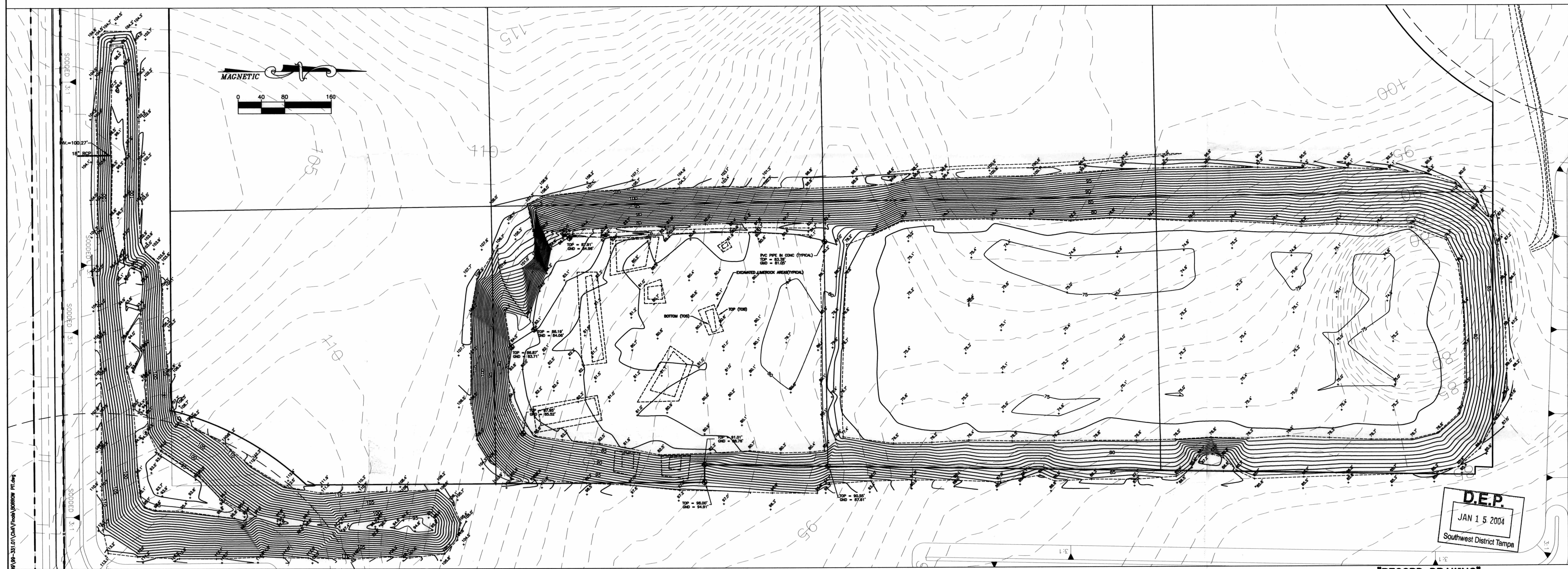
RECORD DRAWING

DATE	REVISIONS	REVISED	CHECKED

DESIGNED	BY	DATE	PROJECT NO.
	RCC	6/03	99.0331.001
DRAWN	RCC	6/03	SCALE
	RKC	1/04	1" = 150'
CHECKED	RKC	1/04	
	RKC	1/04	
QC APPROVAL	RKC	1/04	
	RKC	1/04	
FILE: BORROW PIT.dwg			SHEET OF



SCALEHOUSE ENTRANCE



POND 1

TEMP POND

"RECORD DRAWING"

INCORPORATING THOSE CHANGES MADE DURING THE CONSTRUCTION PHASE OF THE PROJECT, BASED ON DATA FURNISHED BY THE CONTRACTOR TO THE ENGINEER.

Plotted: Jan 14, 2004 - 4:05pm by ROC

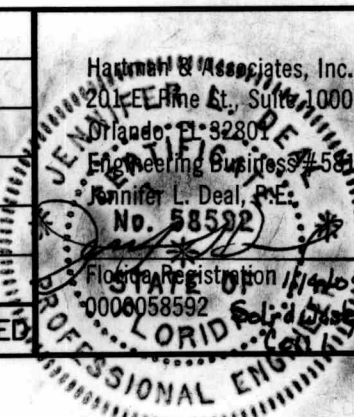
HARTMAN & ASSOCIATES, INC.
engineers, hydrogeologists, surveyors & management consultants
201 EAST PINE STREET - SUITE 1000 - ORLANDO, FL 32801
TELEPHONE (407) 839-3955 - FAX (407) 839-3790
ENGINEERING BUSINESS No. 5814

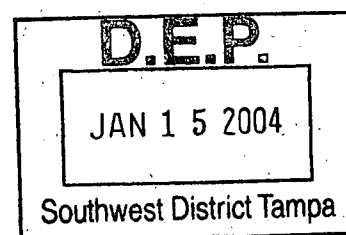
**ENTERPRISE LANDFILL
PASCO COUNTY
FLORIDA**

RECORD DRAWING



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APPENDIX E

SAMPLE DATE:	July 16, 2003
COLLECTED BY:	Miguel Garcia
WATER DEPTH:	NA

n:\hydro\larkin\boring logs\Lithas-1.xls

SAMPLE DATE:	July 16, 2003
COLLECTED BY:	Miguel Garcia
WATER DEPTH:	NA

n:\hydro\larkin\boring logs\Lithas-2.xls

SAMPLE DATE:	July 16, 2003
COLLECTED BY:	Miguel Garcia
WATER DEPTH:	NA

n:\hydro\larkin\boring logs\Lithas-3.xls

SAMPLE DATE:	July 16, 2003
COLLECTED BY:	Miguel Garcia
WATER DEPTH:	NA

n:\hydro\larkin\boring logs\Lithas-4.xls

SAMPLE DATE:	July 16, 2003
COLLECTED BY:	Miguel Garcia
WATER DEPTH:	NA

n:\hydro\larkin\boring logs\Lithas-5.xls

SAMPLE DATE:	July 16, 2003
COLLECTED BY:	Miguel Garcia
WATER DEPTH:	NA

n:\hydro\larkin\boring logs\Lithas-6.xls

SAMPLE DATE:	July 17, 2003
COLLECTED BY:	Miguel Garcia
WATER DEPTH:	NA

n:\hydro\larkin\boring logs\Lithas-7.xls

SAMPLE DATE:	July 17, 2003
COLLECTED BY:	Miguel Garcia
WATER DEPTH:	NA

n:\hydro\larkin\boring logs\Lithas-8.xls

SAMPLE DATE:	July 17, 2003
COLLECTED BY:	Miguel Garcia
WATER DEPTH:	NA

[illegible]

SAMPLE DATE:	July 17, 2003
COLLECTED BY:	Miguel Garcia
WATER DEPTH:	NA

n:\hydro\larkin\boring logs\LITHAS-10.XLS

SAMPLE DATE:	July 17, 2003
COLLECTED BY:	Miguel Garcia
WATER DEPTH:	NA

n:\hydro\larkin\boring logs\LITHAS-11.XLS

SAMPLE DATE:	July 17, 2003
COLLECTED BY:	Miguel Garcia
WATER DEPTH:	NA

[illegible]

SAMPLE DATE:	July 23, 2003
COLLECTED BY:	Miguel Garcia
WATER DEPTH:	NA

n:\hydro\larkin\boring logs\LITHAS-13.XLS

SAMPLE DATE:	July 23, 2003
COLLECTED BY:	Miguel Garcia
WATER DEPTH:	NA

[illegible]

SAMPLE DATE:	July 23, 2003
COLLECTED BY:	Miguel Garcia
WATER DEPTH:	NA

[illegible]

SAMPLE DATE:	July 23, 2003
COLLECTED BY:	Miguel Garcia
WATER DEPTH:	NA

n:\hydro\larkin\boring logs\LITHAS-16.XLS

SAMPLE DATE:	July 23, 2003
COLLECTED BY:	Miguel Garcia
WATER DEPTH:	NA

[illegible]

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SAMPLE DATE:	July 23, 2003
COLLECTED BY:	Miguel Garcia
WATER DEPTH:	NA

[illegible]

SAMPLE DATE:	July 23, 2003
COLLECTED BY:	Miguel Garcia
WATER DEPTH:	NA

[illegible]

SAMPLE DATE:	July 23, 2003
COLLECTED BY:	Miguel Garcia
WATER DEPTH:	NA

n:\hydro\larkin\boring logs\LITHAS-20.XLS

SAMPLE DATE:	July 23, 2003
COLLECTED BY:	Miguel Garcia
WATER DEPTH:	NA

n:\hydro\larkin\boring logs\LITHAS-21.XLS

SAMPLE DATE:	July 23, 2003
COLLECTED BY:	Miguel Garcia
WATER DEPTH:	NA

[illegible]

SAMPLE DATE:	July 23, 2003
COLLECTED BY:	Miguel Garcia
WATER DEPTH:	NA

[illegible]

SAMPLE DATE:	July 23, 2003
COLLECTED BY:	Miguel Garcia
WATER DEPTH:	NA

n:\hydro\larkin\boring logs\LITHAS-24.XLS

SAMPLE DATE:	July 23, 2003
COLLECTED BY:	Miguel Garcia
WATER DEPTH:	NA

[illegible]

SAMPLE DATE:	July 25, 2003
COLLECTED BY:	Miguel Garcia
WATER DEPTH:	NA

[illegible]

SAMPLE DATE:	July 25, 2003
COLLECTED BY:	Miguel Garcia
WATER DEPTH:	NA

n:\hydro\larkin\boring logs\LITHAS-27.XLS

**SS BUCKET AUGER BORING AS-28 LITHOLOGIC DESCRIPTION
ENTERPRISE RECYCLING AND DISPOSAL FACILITY
PASCO COUNTY, FLORIDA**

SAMPLE DATE:	July 25, 2003
COLLECTED BY:	Miguel Garcia
WATER DEPTH:	NA

[illegible]

1. *Chlorophyll a* (Chl *a*)

2. *Chlorophyll b* (Chl *b*)

3. *Carotenoids* (Car)

4. *Phaeophytin a* (Phe *a*)

5. *Phaeophytin b* (Phe *b*)

6. *Phaeoerythrin* (Phe *er*)

7. *Phaeoxanthophyll* (Phe *x*)

8. *Phaeo-zeaxanthin* (Phe *z*)

9. *Phaeo-lutein* (Phe *l*)

10. *Phaeo-zeaxanthin* (Phe *z*)

11. *Phaeo-lutein* (Phe *l*)

12. *Phaeo-zeaxanthin* (Phe *z*)

13. *Phaeo-lutein* (Phe *l*)

14. *Phaeo-zeaxanthin* (Phe *z*)

15. *Phaeo-lutein* (Phe *l*)

16. *Phaeo-zeaxanthin* (Phe *z*)

17. *Phaeo-lutein* (Phe *l*)

18. *Phaeo-zeaxanthin* (Phe *z*)

19. *Phaeo-lutein* (Phe *l*)

20. *Phaeo-zeaxanthin* (Phe *z*)

21. *Phaeo-lutein* (Phe *l*)

22. *Phaeo-zeaxanthin* (Phe *z*)

23. *Phaeo-lutein* (Phe *l*)

24. *Phaeo-zeaxanthin* (Phe *z*)

25. *Phaeo-lutein* (Phe *l*)

26. *Phaeo-zeaxanthin* (Phe *z*)

27. *Phaeo-lutein* (Phe *l*)

28. *Phaeo-zeaxanthin* (Phe *z*)

29. *Phaeo-lutein* (Phe *l*)

30. *Phaeo-zeaxanthin* (Phe *z*)

31. *Phaeo-lutein* (Phe *l*)

32. *Phaeo-zeaxanthin* (Phe *z*)

33. *Phaeo-lutein* (Phe *l*)

34. *Phaeo-zeaxanthin* (Phe *z*)

35. *Phaeo-lutein* (Phe *l*)

36. *Phaeo-zeaxanthin* (Phe *z*)

37. *Phaeo-lutein* (Phe *l*)

38. *Phaeo-zeaxanthin* (Phe *z*)

39. *Phaeo-lutein* (Phe *l*)

40. *Phaeo-zeaxanthin* (Phe *z*)

41. *Phaeo-lutein* (Phe *l*)

42. *Phaeo-zeaxanthin* (Phe *z*)

43. *Phaeo-lutein* (Phe *l*)

44. *Phaeo-zeaxanthin* (Phe *z*)

45. *Phaeo-lutein* (Phe *l*)

46. *Phaeo-zeaxanthin* (Phe *z*)

47. *Phaeo-lutein* (Phe *l*)

48. *Phaeo-zeaxanthin* (Phe *z*)

49. *Phaeo-lutein* (Phe *l*)

50. *Phaeo-zeaxanthin* (Phe *z*)

51. *Phaeo-lutein* (Phe *l*)

52. *Phaeo-zeaxanthin* (Phe *z*)

53. *Phaeo-lutein* (Phe *l*)

54. *Phaeo-zeaxanthin* (Phe *z*)

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81. *Phaeo-lutein* (Phe *l*)

82. *Phaeo-zeaxanthin* (Phe *z*)

83. *Phaeo-lutein* (Phe *l*)

84. *Phaeo-zeaxanthin* (Phe *z*)

85. *Phaeo-lutein* (Phe *l*)

86. *Phaeo-zeaxanthin* (Phe *z*)

87. *Phaeo-lutein* (Phe *l*)

88. *Phaeo-zeaxanthin* (Phe *z*)

89. *Phaeo-lutein* (Phe *l*)

90. *Phaeo-zeaxanthin* (Phe *z*)

91. *Phaeo-lutein* (Phe *l*)

92. *Phaeo-zeaxanthin* (Phe *z*)

93. *Phaeo-lutein* (Phe *l*)

94. *Phaeo-zeaxanthin* (Phe *z*)

95. *Phaeo-lutein* (Phe *l*)

96. *Phaeo-zeaxanthin* (Phe *z*)

97. *Phaeo-lutein* (Phe *l*)

98. *Phaeo-zeaxanthin* (Phe *z*)

99. *Phaeo-lutein* (Phe *l*)

100. *Phaeo-zeaxanthin* (Phe *z*)

1. 2. 3. 4. 5. 6. 7. 8. 9. 10. 11. 12. 13. 14. 15. 16. 17. 18. 19. 20. 21. 22. 23. 24. 25. 26. 27. 28. 29. 30. 31. 32. 33. 34. 35. 36. 37. 38. 39. 40. 41. 42. 43. 44. 45. 46. 47. 48. 49. 50. 51. 52. 53. 54. 55. 56. 57. 58. 59. 60. 61. 62. 63. 64. 65. 66. 67. 68. 69. 70. 71. 72. 73. 74. 75. 76. 77. 78. 79. 80. 81. 82. 83. 84. 85. 86. 87. 88. 89. 90. 91. 92. 93. 94. 95. 96. 97. 98. 99. 100. 101. 102. 103. 104. 105. 106. 107. 108. 109. 110. 111. 112. 113. 114. 115. 116. 117. 118. 119. 120. 121. 122. 123. 124. 125. 126. 127. 128. 129. 130. 131. 132. 133. 134. 135. 136. 137. 138. 139. 140. 141. 142. 143. 144. 145. 146. 147. 148. 149. 150. 151. 152. 153. 154. 155. 156. 157. 158. 159. 160. 161. 162. 163. 164. 165. 166. 167. 168. 169. 170. 171. 172. 173. 174. 175. 176. 177. 178. 179. 180. 181. 182. 183. 184. 185. 186. 187. 188. 189. 190. 191. 192. 193. 194. 195. 196. 197. 198. 199. 200. 201. 202. 203. 204. 205. 206. 207. 208. 209. 210. 211. 212. 213. 214. 215. 216. 217. 218. 219. 220. 221. 222. 223. 224. 225. 226. 227. 228. 229. 230. 231. 232. 233. 234. 235. 236. 237. 238. 239. 240. 241. 242. 243. 244. 245. 246. 247. 248. 249. 250. 251. 252. 253. 254. 255. 256. 257. 258. 259. 260. 261. 262. 263. 264. 265. 266. 267. 268. 269. 270. 271. 272. 273. 274. 275. 276. 277. 278. 279. 280. 281. 282. 283. 284. 285. 286. 287. 288. 289. 290. 291. 292. 293. 294. 295. 296. 297. 298. 299. 300. 301. 302. 303. 304. 305. 306. 307. 308. 309. 310. 311. 312. 313. 314. 315. 316. 317. 318. 319. 320. 321. 322. 323. 324. 325. 326. 327. 328. 329. 330. 331. 332. 333. 334. 335. 336. 337. 338. 339. 340. 341. 342. 343. 344. 345. 346. 347. 348. 349. 350. 351. 352. 353. 354. 355. 356. 357. 358. 359. 360. 361. 362. 363. 364. 365. 366. 367. 368. 369. 370. 371. 372. 373. 374. 375. 376. 377. 378. 379. 380. 381. 382. 383. 384. 385. 386. 387. 388. 389. 390. 391. 392. 393. 394. 395. 396. 397. 398. 399. 400. 401. 402. 403. 404. 405. 406. 407. 408. 409. 410. 411. 412. 413. 414. 415. 416. 417. 418. 419. 420. 421. 422. 423. 424. 425. 426. 427. 428. 429. 430. 431. 432. 433. 434. 435. 436. 437. 438. 439. 440. 441. 442. 443. 444. 445. 446. 447. 448. 449. 450. 451. 452. 453. 454. 455. 456. 457. 458. 459. 460. 461. 462. 463. 464. 465. 466. 467. 468. 469. 470. 471. 472. 473. 474. 475. 476. 477. 478. 479. 480. 481. 482. 483. 484. 485. 486. 487. 488. 489. 490. 491. 492. 493. 494. 495. 496. 497. 498. 499. 500. 501. 502. 503. 504. 505. 506. 507. 508. 509. 510. 511. 512. 513. 514. 515. 516. 517. 518. 519. 520. 521. 522. 523. 524. 525. 526. 527. 528. 529. 530. 531. 532. 533. 534. 535. 536. 537. 538. 539. 540. 541. 542. 543. 544. 545. 546. 547. 548. 549. 550. 551. 552. 553. 554. 555. 556. 557. 558. 559. 560. 561. 562. 563. 564. 565. 566. 567. 568. 569. 570. 571. 572. 573. 574. 575. 576. 577. 578. 579. 580. 581. 582. 583. 584. 585. 586. 587. 588. 589. 590. 591. 592. 593. 594. 595. 596. 597. 598. 599. 600. 601. 602. 603. 604. 605. 606. 607. 608. 609. 610. 611. 612. 613. 614. 615. 616. 617. 618. 619. 620. 621. 622. 623. 624. 625. 626. 627. 628. 629. 630. 631. 632. 633. 634. 635. 636. 637. 638. 639. 640. 641. 642. 643. 644. 645. 646. 647. 648. 649. 650. 651. 652. 653. 654. 655. 656. 657. 658. 659. 660. 661. 662. 663. 664. 665. 666. 667. 668. 669. 670. 671. 672. 673. 674. 675. 676. 677. 678. 679. 680. 681. 682. 683. 684. 685. 686. 687. 688. 689. 690. 691. 692. 693. 694. 695. 696. 697. 698. 699. 700. 701. 702. 703. 704. 705. 706. 707. 708. 709. 710. 711. 712. 713. 714. 715. 716. 717. 718. 719. 720. 721. 722. 723. 724. 725. 726. 727. 728. 729. 730. 731. 732. 733. 734. 735. 736. 737. 738. 739. 740. 741. 742. 743. 744. 745. 746. 747. 748. 749. 750. 751. 752. 753. 754. 755. 756. 757. 758. 759. 760. 761. 762. 763. 764. 765. 766. 767. 768. 769. 770. 771. 772. 773. 774. 775. 776. 777. 778. 779. 780. 781. 782. 783. 784. 785. 786. 787. 788. 789. 790. 791. 792. 793. 794. 795. 796. 797. 798. 799. 800. 801. 802. 803. 804. 805. 806. 807. 808. 809. 810. 811. 812. 813. 814. 815. 816. 817. 818. 819. 820. 821. 822. 823. 824. 825. 826. 827. 828. 829. 830. 831. 832. 833. 834. 835. 836. 837. 838. 839. 840.

THE

| | |
|----------------------|---------------|
| SAMPLE DATE: | July 25, 2003 |
| COLLECTED BY: | Miguel Garcia |
| WATER DEPTH: | NA |

n:\hydro\larkin\boring logs\LITHAS-30.XLS

| | |
|----------------------|---------------|
| SAMPLE DATE: | July 25, 2003 |
| COLLECTED BY: | Miguel Garcia |
| WATER DEPTH: | NA |

n:\hydro\markin\boring logs\LITHAS-31.XLS

| | |
|----------------------|---------------|
| SAMPLE DATE: | July 25, 2003 |
| COLLECTED BY: | Miguel Garcia |
| WATER DEPTH: | NA |

n:\hydro\larkin\boring logs\LITHAS-32.XLS

| | |
|----------------------|---------------|
| SAMPLE DATE: | July 25, 2003 |
| COLLECTED BY: | Miguel Garcia |
| WATER DEPTH: | NA |

n:\hydro\larkin\boring logs\LITHAS-33.XLS

| | |
|----------------------|---------------|
| SAMPLE DATE: | July 25, 2003 |
| COLLECTED BY: | Miguel Garcia |
| WATER DEPTH: | NA |

n:\hydro\larkin\boring logs\LITHAS-34.XLS

| | |
|----------------------|---------------|
| SAMPLE DATE: | July 25, 2003 |
| COLLECTED BY: | Miguel Garcia |
| WATER DEPTH: | NA |

n:\hydro\markin\boring logs\LITHAS-35.XLS

| | |
|----------------------|----------------|
| SAMPLE DATE: | August 8, 2003 |
| COLLECTED BY: | Miguel Garcia |
| WATER DEPTH: | NA |

n:\hydro\larkin\boring logs\LITHAS-36.XLS

| | |
|----------------------|----------------|
| SAMPLE DATE: | August 8, 2003 |
| COLLECTED BY: | Miguel Garcia |
| WATER DEPTH: | NA |

n:\hydro\larkin\boring logs\LITHAS-37.XLS

| | |
|----------------------|----------------|
| SAMPLE DATE: | August 8, 2003 |
| COLLECTED BY: | Miguel Garcia |
| WATER DEPTH: | NA |

n:\hydro\larkin\boring logs\LITHAS-38.XLS

| | |
|----------------------|----------------|
| SAMPLE DATE: | August 8, 2003 |
| COLLECTED BY: | Miguel Garcia |
| WATER DEPTH: | NA |

n:\hydro\larkin\boring logs\LITHAS-39.XLS

| | |
|----------------------|----------------|
| SAMPLE DATE: | August 8, 2003 |
| COLLECTED BY: | Miguel Garcia |
| WATER DEPTH: | NA |

n:\hydro\larkin\boring logs\LITHAS-40.XLS

SAMPLE DATE: August 8, 2003
COLLECTED BY: Miguel Garcia
WATER DEPTH: NA

[illegible]

| | |
|----------------------|----------------|
| SAMPLE DATE: | August 8, 2003 |
| COLLECTED BY: | Miguel Garcia |
| WATER DEPTH: | NA |

[illegible]

SAMPLE DATE: August 8, 2003
COLLECTED BY: Miguel Garcia
WATER DEPTH: NA

[illegible]

SS BUCKET AUGER BORING AS-44 LITHOLOGIC DESCRIPTION
ENTERPRISE RECYCLING AND DISPOSAL FACILITY
PASCO COUNTY, FLORIDA

SAMPLE DATE: August 8, 2003
COLLECTED BY: Miguel Garcia
WATER DEPTH: NA

[illegible]

| | |
|----------------------|----------------|
| SAMPLE DATE: | August 8, 2003 |
| COLLECTED BY: | Miguel Garcia |
| WATER DEPTH: | NA |

n:\hydro\markin\boring logs\LITHAS-45.XLS

SAMPLE DATE: August 8, 2003
COLLECTED BY: Miguel Garcia
WATER DEPTH: NA

n:\hydro\larkin\boring logs\LITHAS-46.XLS

| | |
|----------------------|----------------|
| SAMPLE DATE: | August 8, 2003 |
| COLLECTED BY: | Miguel Garcia |
| WATER DEPTH: | NA |

n:\hydro\larkin\boring logs\LITHAS-47.XLS

B-15
(SPT Boring)

PROJECT

NUMBER: 99.0331.007

NAME: Enterprise Recycling
& Disposal Facility

LOCATION: Dade City, FL

BORING

GR. ELEV: 75.27' NGVD

DIA-TYPE: 3 3/4"-Mud Rotary

DEPTH: 22' BLS

DATE STARTED: 8-22-03

DATE ENDED: 8-22-03

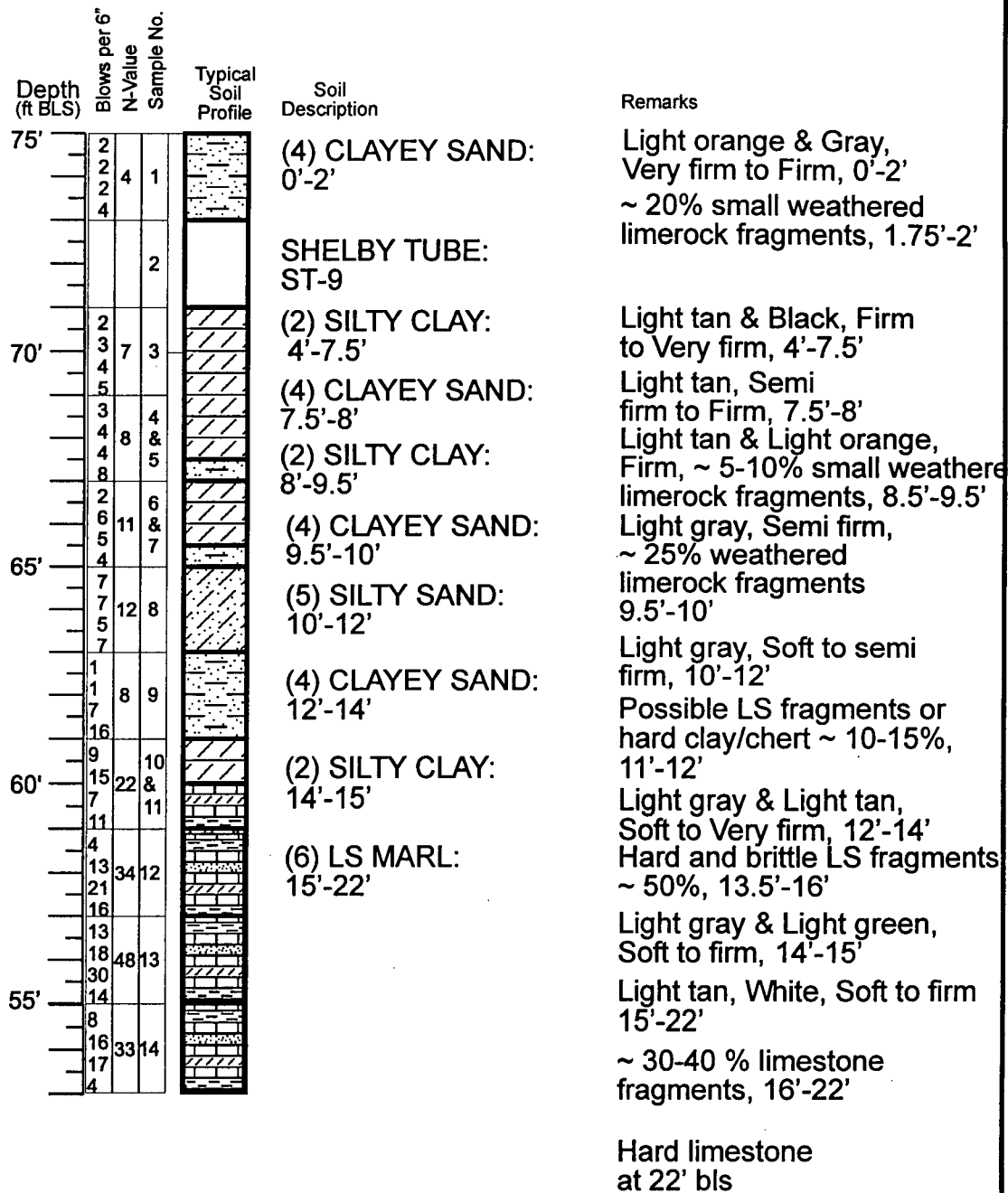
DRILLING

RIG TYPE: CME 45

CREW: Amdrill

LEGEND

- | | |
|-----------------|--|
| (1) SANDY CLAY | |
| (2) SILTY CLAY | |
| (3) CLAY | |
| (4) CLAYEY SAND | |
| (5) SILTY SAND | |
| (6) LS MARL | |
| (7) LIMESTONE | |



B-16

(SPT Boring)

PROJECT

NUMBER: 99.0331.007
NAME: Enterprise Recycling
 & Disposal Facility
LOCATION: Dade City, FL

BORING

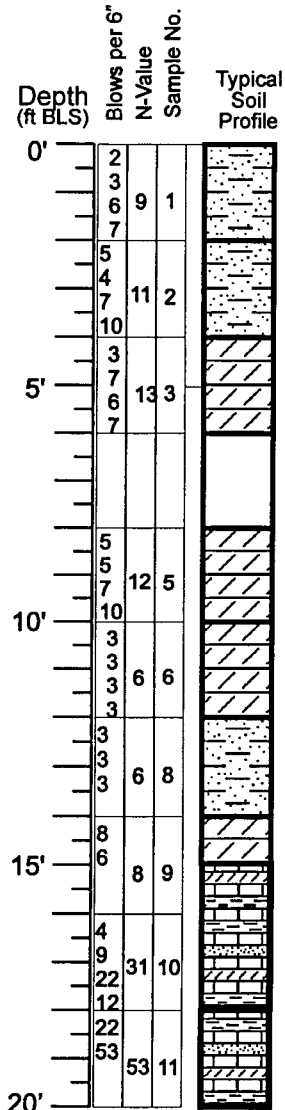
GR. ELEV: 80.38' NGVD
DIA-TYPE: 3 3/4"-Mud Rotary
DEPTH: 20' BLS
DATE STARTED: 8-22-03
DATE ENDED: 8-22-03

DRILLING

RIG TYPE: CME 45
CREW: Amdrill

LEGEND

- (1) SANDY CLAY
- (2) SILTY CLAY
- (3) CLAY
- (4) CLAYEY SAND
- (5) SILTY SAND
- (6) LS MARL
- (7) LIMESTONE



Soil Description

Remarks

(4) CLAYEY SAND:
 Light orange & gray,
 semi firm to very firm
 0'-4'

~ 5-15 % small weathered
 limerock fragments, (<1 cm)
 0'-3'

(2) SILTY CLAY:
 Light gray & Orange,
 Very firm, 4'-6'

SHELBY TUBE:
 6'-8'

ST-10

(2) SILTY CLAY:
 Light gray, Orange,
 Dark brown, Semi
 firm to Very firm,
 8'-12'

~ 5-10% small weathered
 limerock fragments, 8.5'-9.5'
 ~ 25% weathered
 limerock fragments, 9.5'-10'
 Possible LS fragments or
 hard clay/chert ~ 10-15%,
 11'-12'

(6) LS MARL:
 Soft to firm (silty clay
 matrix), 12'-16'

Hard and brittle LS
 fragments ~ 50%, 13.5'-16'

(6) LS MARL:
 light tan, Soft, 16'-18'

~ 30-40 % limestone
 fragments, 16'-20'

(6) LS MARL:
 Light tan, Mod sand,
 Soft, 18'-20'

N:\HYDROLARK\Boring logs\B-16.CDR



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STRATIGRAPHIC COLUMN
ENTERPRISE RECYCLING
& DISPOSAL FACILITY
DADE CITY, FLORIDA

FIGURE
2

B-17 (SPT Boring)

PROJECT

NUMBER: 99.0331.007
NAME: Enterprise Recycling
 & Disposal Facility
LOCATION: Dade City, FL








BORING

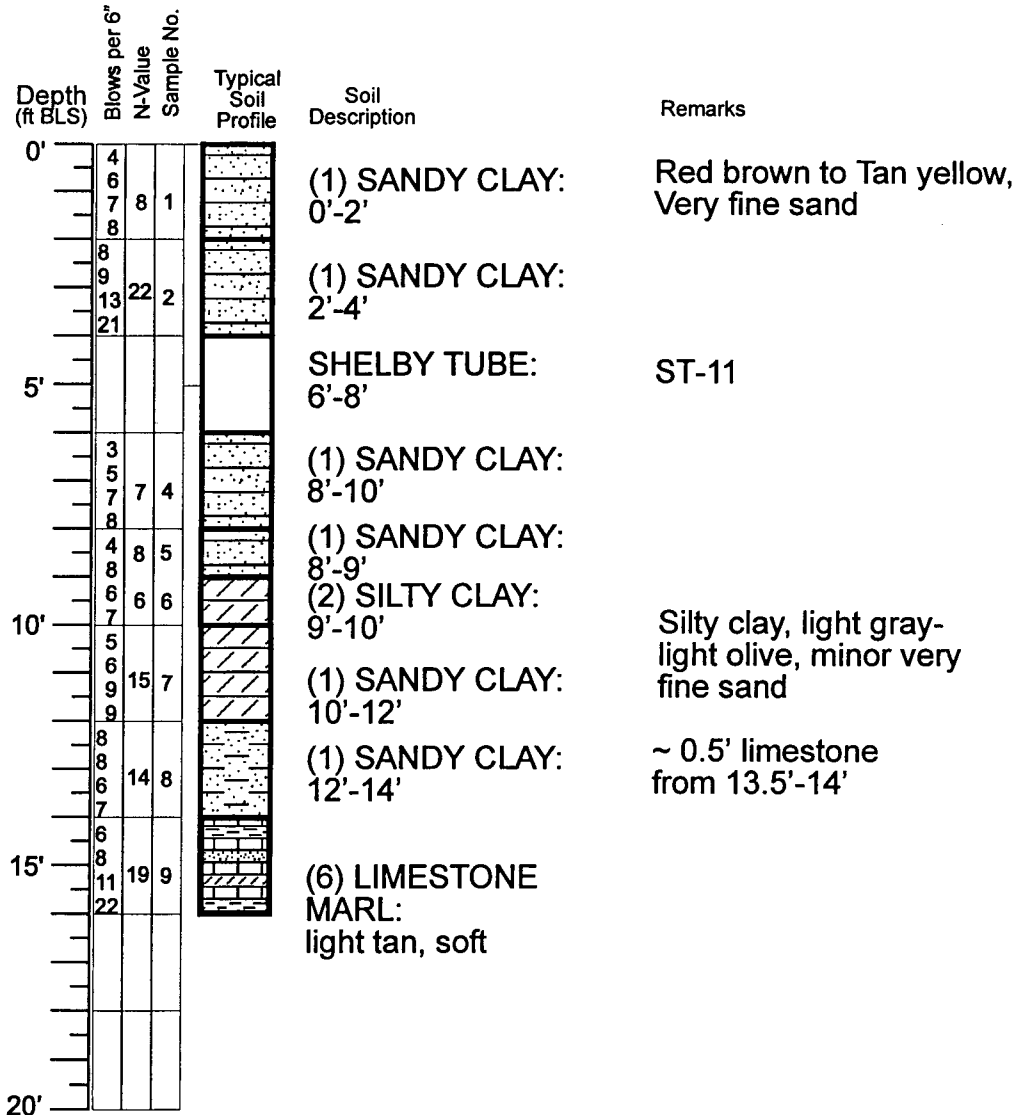
GR. ELEV: 81.02' NGVD
DIA-TYPE: 3 3/4" Mud Rotary
DEPTH: 16' BLS
DATE STARTED: 8-23-03
DATE ENDED: 8-23-03

DRILLING

RIG TYPE: CME 45
CREW: Amdrill

LEGEND

- (1) SANDY CLAY 
- (2) SILTY CLAY 
- (3) CLAY 
- (4) CLAYEY SAND 
- (5) SILTY SAND 
- (6) LS MARL 
- (7) LIMESTONE 



N:\HYDRO\ARKIN\Boring logs\B-17.CDR



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**STRATIGRAPHIC COLUMN
 ENTERPRISE RECYCLING
 & DISPOSAL FACILITY
 DADE CITY, FLORIDA**

**FIGURE
 3**

B-18
(SPT Boring)

PROJECT

NUMBER: 99.0331.007

NAME: Enterprise Recycling
& Disposal Facility

LOCATION: Dade City, FL

BORING

GR. ELEV: 83.43' NGVD

DIA-TYPE: 3 3/4"-Mud Rotary

DEPTH: 8' BLS

DATE STARTED: 8-23-03








DATE ENDED: 8-23-03





DRILLING

RIG TYPE: CME 45

CREW: Amdrill

LEGEND

- | | |
|-----------------|---|
| (1) SANDY CLAY |  |
| (2) SILTY CLAY |  |
| (3) CLAY |  |
| (4) CLAYEY SAND |  |
| (5) SILTY SAND |  |
| (6) LS MARL |  |
| (7) LIMESTONE |  |

| Depth
(ft BLS) | Blows per 6"
N-Value | Sample No. | Typical
Soil
Profile | Soil
Description |
|---------------------|-------------------------|-------------|---|--------------------------|
| 0' | 2
1
3
6 | 1
&
2 |  | (2) SILTY CLAY:
0'-1' |
| 6
18
17
20 | 6
35
4 | 3
&
4 |  | (1) SANDY CLAY:
1'-3' |
| 5'
5
9
7 | 5
14 | 5 |  | (1) SANDY CLAY:
4'-6' |
| | | |  | (1) SANDY CLAY:
6'-8' |
| 10' | | | | |

Remarks

Very pale olive green

(2) SILTY CLAY:
0'-1'

(1) SANDY CLAY:
1'-3'

(1) SANDY CLAY:
4'-6'

(1) SANDY CLAY:
6'-8'

Failed shelly tube from
6'-8' (limerock @ 7.5')
rotary wash 0'-4'
in new boring, collected
ST-11 from 4'-6'

B-19 (SPT Boring)

PROJECT

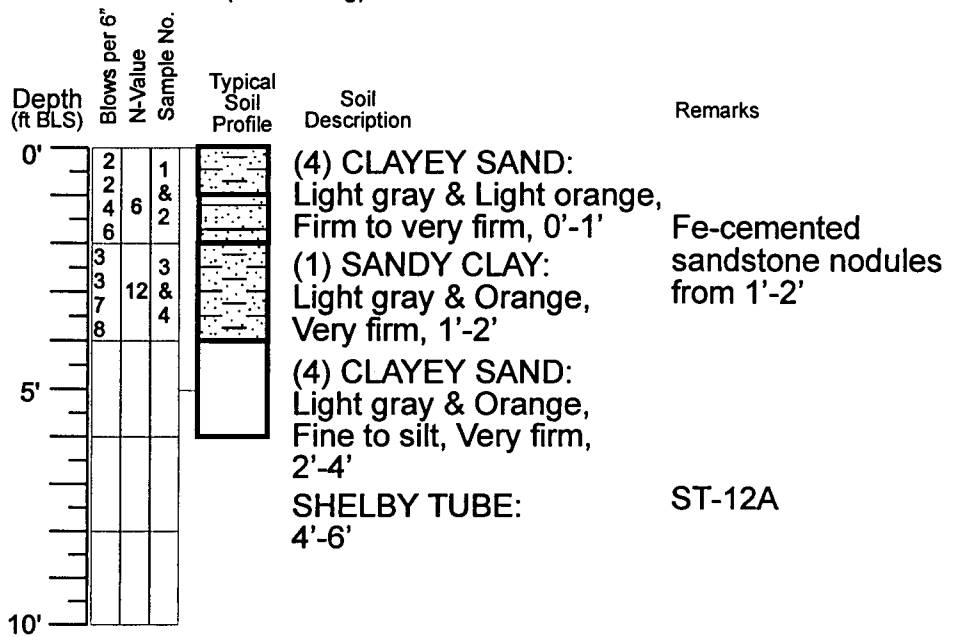
NUMBER: 99.0331.007
NAME: Enterprise Recycling
 & Disposal Facility
LOCATION: Dade City, FL

BORING








GR. ELEV: 74.89' NGVD
DIA-TYPE: 3 3/4" Mud Rotary
DEPTH: 6' BLS
DATE STARTED: 8-26-03
DATE ENDED: 8-26-03

DRILLING

RIG TYPE: CME 45
CREW: Amdrill



LEGEND

- (1) SANDY CLAY 
- (2) SILTY CLAY 
- (3) CLAY 
- (4) CLAYEY SAND 
- (5) SILTY SAND 
- (6) LS MARL 
- (7) LIMESTONE 

N:\HYDRO\ARKIN\Boring logs\B-19.CDR



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**STRATIGRAPHIC COLUMN
 ENTERPRISE RECYCLING
 & DISPOSAL FACILITY
 DADE CITY, FLORIDA**

**FIGURE
 5**

B-20 (SPT Boring)

PROJECT

NUMBER: 99.0391.007

NAME: Enterprise Recycling
& Disposal Facility

LOCATION: Dade City, FL

BORING

GR. ELEV: 75.23' NGVD

DIA-TYPE: 3 3/4" -Mud Rotary

DEPTH: 16' BLS

DATE STARTED: 8-26-03

DATE ENDED: 8-26-03

DRILLING

RIG TYPE: CME 45

CREW: Amdrill

LEGEND

| | |
|-----------------|--|
| (1) SANDY CLAY | |
| (2) SILTY CLAY | |
| (3) CLAY | |
| (4) CLAYEY SAND | |
| (5) SILTY SAND | |
| (6) LS MARL | |
| (7) LIMESTONE | |

| Depth
(ft BLS) | Blows per 6"
N-Value | Sample No. | Typical
Soil
Profile | Soil
Description | Remarks |
|-------------------|-------------------------|------------|----------------------------|-----------------------------------|--|
| 0' | 1 | 1 | | (5) SILTY SAND:
0'-2' | Fine to silt, Very small
amount of clay, Fe-cemented
nodules 0'-2' |
| 2' | 2 | 1 | | (5) SILTY SAND:
2'-4' | Gray-tan, Med to silt,
Small amount of clay,
2'-4' |
| 4' | 10 | 2 | | (5) SILTY SAND:
4'-5.5' | Gray-tan, Med to silt,
4'-5.5' |
| 5' | 2 | 3 & 4 | | (4) CLAYEY SAND:
5.5'-7.5' | Light gray & Dark orange,
firm, 5.5'-7.5' |
| 6' | 16 | 5 & 6 | | (2) SILTY CLAY:
7.5'-8' | Light gray-green,
Very firm, 7.5'-8' |
| 8' | 5 | 5 | | SHELBY TUBE:
8'-10' | ST-13 |
| 10' | 53 | 8 | | (2) SILTY CLAY:
10'-11.5' | Light gray-green,
Very firm, 10'-11.5' |
| 11' | 50 | | | | white, moderate sand,
brittle, sharp, limestone
fragments, 11'-12' |
| 12' | 6 | | | (6) LS MARL:
11.5'-12' | |
| 13' | 5 | 13 | | | lost circulation at 13.9' |
| 14' | 4 | | | (7) LIMESTONE:
hard, 12'-13.9' | |
| 15' | 12 | | | (6) LS MARL:
14'-16' | White, Moderate sand,
Large amount of hard
limestone and chert
fragments, 14'-16' |
| 16' | 16 | 11 | | | |
| 20' | | | | | |

N:\HYDROLARKIN\Boring logs\B-20.CDR



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**STRATIGRAPHIC COLUMN
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& DISPOSAL FACILITY
DADE CITY, FLORIDA**

**FIGURE
6**

B-21 (SPT Boring)

PROJECT

NUMBER: 99.0331.007
NAME: Enterprise Recycling
 & Disposal Facility
LOCATION: Dade City, FL

BORING

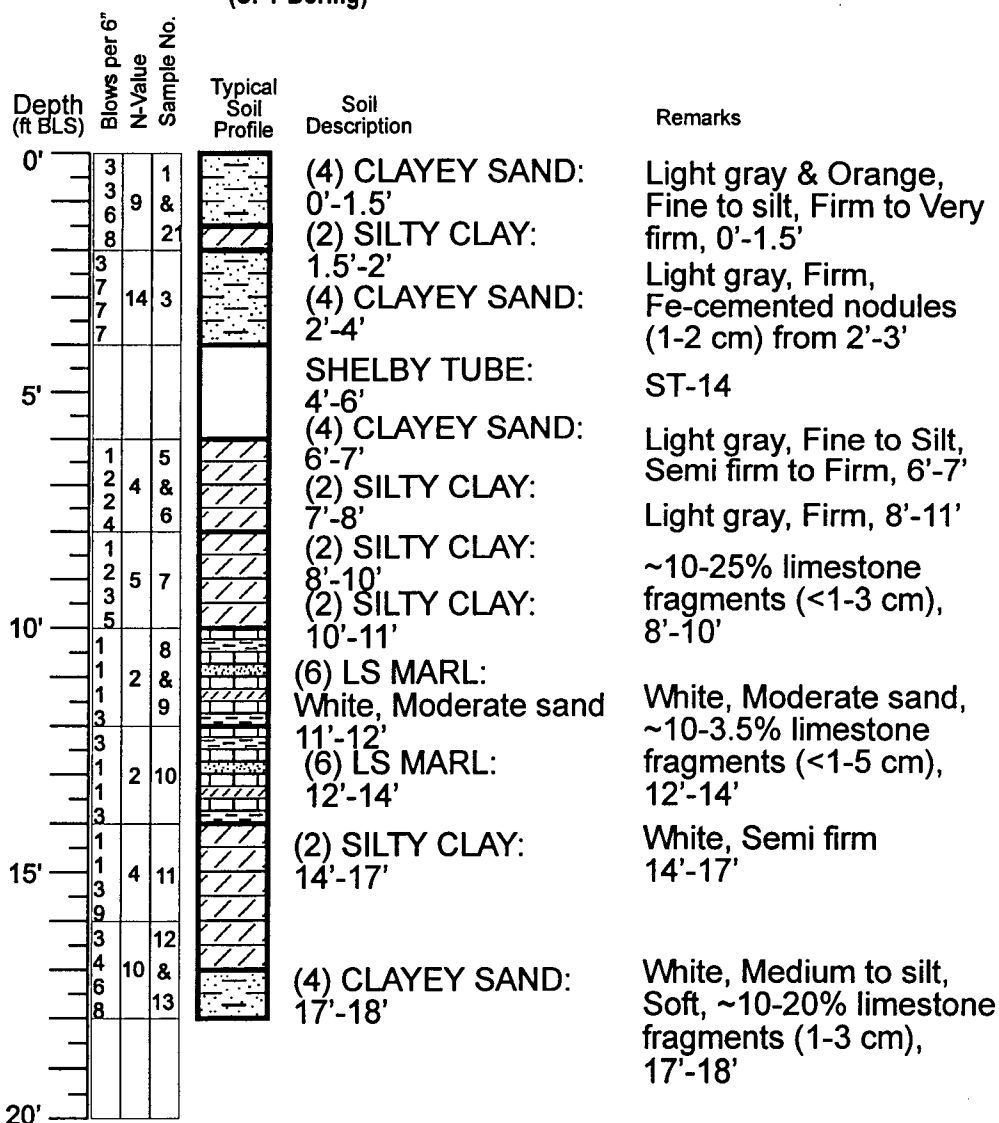
GR. ELEV: 75.27' NGVD
DIA-TYPE: 3 3/4" Mud Rotary
DEPTH: 18' BLS
DATE STARTED: 8-26-03
DATE ENDED: 8-26-03

DRILLING

RIG TYPE: CME 45
CREW: Amc drill

LEGEND

| | |
|-----------------|--|
| (1) SANDY CLAY | |
| (2) SILTY CLAY | |
| (3) CLAY | |
| (4) CLAYEY SAND | |
| (5) SILTY SAND | |
| (6) LS MARL | |
| (7) LIMESTONE | |



N:\HYDROLARKIN\Boring logs\B-21.CDR



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**STRATIGRAPHIC COLUMN
 ENTERPRISE RECYCLING
 & DISPOSAL FACILITY
 DADE CITY, FLORIDA**

**FIGURE
 7**

B-22 (SPT Boring)

PROJECT

NUMBER: 99.0331.007
NAME: Enterprise Recycling
 & Disposal Facility
LOCATION: Dade City, FL

BORING

GR. ELEV: 75.18' NGVD
DIA-TYPE: 3 3/4"-Mud Rotary
DEPTH: 46' BLS
DATE STARTED: 8-26-03
DATE ENDED: 8-26-03

DRILLING

RIG TYPE: CME 45
CREW: Amdrill

LEGEND

| | |
|-----------------|--|
| (1) SANDY CLAY | |
| (2) SILTY CLAY | |
| (3) CLAY | |
| (4) CLAYEY SAND | |
| (5) SILTY SAND | |
| (6) LS MARL | |
| (7) LIMESTONE | |

| Depth
(ft BLS) | Blows per 6"
N-Value | Sample No. | Typical
Soil
Profile |
|-------------------|--|--|----------------------------|
| 0' | 2
4
5
10 | 9 1 | |
| 5' | 2
4
18
19
17
19
17
18
18
40
30 | 22 3
36 5
58 6
40 7 | |
| 10' | 19
21
25
7
9
27
16
12
13 | 16 8
28 9 | |
| 15' | | | |
| 20' | 11
13
18
19 | 32 11 | |
| 25' | 3
6
6
9 | 12 13 | |
| 30' | 6
6
10
8
5
4
2
10
2
2
5
8
7
4
5
6
11 | 18 15
10 16
17
11 18
11 19 | |
| 35' | | | |
| 40' | 4
3
4
2
3
4
7 | 7 22
7 23 | |
| 45' | | | |
| 50' | | | |

Soil Description

(5) SILTY SAND:
 SHELBY TUBE:
 failed attempt 2'-3'
 (5) SILTY SAND:
 3'-5'
 ROTARY WASH:
 5'-6'
 (5) SILTY SAND:
 6'-16'
 ROTARY WASH:
 16'-18.5'
 (5) SILTY SAND:
 18.5'-20.5'
 ROTARY WASH:
 20.5'-23.5'
 (5) SILTY SAND:
 23.5'-25.5'
 SHELBY TUBE:
 25.5'-27.5'
 (5) SILTY SAND:
 27.5'-37.5'
 ROTARY WASH:
 37.5'-40'
 (5) SILTY SAND:
 40'-41"
 (1) SANDY CLAY:
 41'-42'
 (1) SANDY CLAY:
 42'-44'
 SHELBY TUBE:
 44'-46'

Remarks

Tan-brown, Fine to Silt,
 Semi firm to Firm, 0'-2'
 Orange-brown, Tan, Fine
 to Silt, Firm, 3'-5'
 Orange-brown, Tan, Fine
 to Silt, Firm, 6'-12'
 small limestone
 fragments (<1 cm),
 8.5'-9'
 Tan-orange, Semi firm to
 Firm, very small limestone
 fragments (<1 cm),
 12'-14.5'
 very small limestone
 fragments (<1 cm),
 19.5'-20'
 increase in clay from
 25'-25.5'
 ST-15
 Orange, Fine to Silt, Firm,
 27.5'-33.5'
 Orange rust, Fine to Silt, Firm,
 33.5'-41'
 Orange-tan, Very firm
 41'-44'
 only ~1.25' recovery
 from shelby tube
 ST-16

N:\HYDROLARK\Boring logs\B-22.CDR



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**FIGURE
8**

B-23 (SPT Boring)

PROJECT

NUMBER: 99.0331.007
NAME: Enterprise Recycling
 & Disposal Facility
LOCATION: Dade City, FL

BORING

GR. ELEV: 75.00' NGVD
DIA-TYPE: 3 3/4"-Mud Rotary
DEPTH: 20' BLS
DATE STARTED: 8-26-03
DATE ENDED: 8-26-03

DRILLING

RIG TYPE: CME 45
CREW: Amdrill

LEGEND

| | |
|-----------------|--|
| (1) SANDY CLAY | |
| (2) SILTY CLAY | |
| (3) CLAY | |
| (4) CLAYEY SAND | |
| (5) SILTY SAND | |
| (6) LS MARL | |
| (7) LIMESTONE | |

| Depth
(ft BLS) | Blows per 6"
N-Value | Sample No. | Typical
Soil
Profile | Soil
Description | Remarks |
|-------------------|-------------------------|------------|----------------------------|---------------------|---------------------------------|
| 0' | 4 | 1 | | (4) CLAYEY SAND: | White, Fine to silt, Semi |
| 1' | 5 | 2 | | 0'-2' | firm to Firm, High sand content |
| 2' | 10 | | | | 0'-1' |
| 3' | | | | SHELBY TUBE: | ST-17 |
| 4' | | | | 2'-4' | |
| 5' | 5 | 3 | | (4) CLAYEY SAND | White, Tan-orange, Medium |
| 6' | 6 | 4 | | 4'-9' | to Silt, Firm to Very firm, |
| 7' | 12 | | | | 4'-9' |
| 8' | 12 | | | | |
| 9' | 10 | 5 | | | |
| 10' | 12 | | | | |
| 11' | 26 | | | | |
| 12' | 14 | | | | |
| 13' | 15 | | | | |
| 14' | 2 | 6 | | | |
| 15' | 2 | 7 | | | |
| 16' | 3 | 8 | | (1) SANDY CLAY: | Light-tan, Orange, Extremely |
| 17' | 5 | | | 9'-14' | firm, 9'-14' |
| 18' | 3 | | | | |
| 19' | 4 | | | | |
| 20' | 10 | | | | |
| 21' | 7 | | | | |
| 22' | 6 | 1 | | | |
| 23' | 6 | 9 | | | |
| 24' | 9 | | | | |
| 25' | 9 | | | | |
| 26' | 6 | 10 | | (4) CLAYEY SAND: | Limestone fragments |
| 27' | 9 | 11 | | 14'-15.5' | (~1-2 cm), |
| 28' | 10 | | | | 11.75'-12' |
| 29' | 10 | | | | Limestone fragments |
| 30' | 4 | 12 | | (3) CLAY: 15.5'-16' | (~1-2 cm), |
| 31' | 5 | 13 | | (4) CLAYEY SAND: | 12'-13.5' |
| 32' | 6 | | | 16'-16.5' | Tan-orange, Light gray, |
| 33' | 8 | | | (2) SILTY CLAY: | Soft to Firm, 14'-15.5' |
| 34' | 6 | 14 | | 16.5'-18' | Limestone fragments |
| 35' | 6 | | | (4) CLAYEY SAND: | (~1-3 cm), |
| 36' | 8 | | | 18'-19' | 17'-18' |
| 37' | 7 | 15 | | (2) SILTY CLAY: | White, Tan-orange, Black, |
| 38' | | | | 19'-20' | Very firm, 19'-20' |

N:\HYDROLARKIN\Boring logs\B-23.CDR



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**STRATIGRAPHIC COLUMN
 ENTERPRISE RECYCLING
 & DISPOSAL FACILITY
 DADE CITY, FLORIDA**

**FIGURE
 9**

B-24

(SPT Boring)

PROJECT

NUMBER: 99.0331.007

NAME: Enterprise Recycling
& Disposal Facility

LOCATION: Dade City, FL

BORING

GR. ELEV: 75.19' NGVD

DIA-TYPE: 3 3/4"-Mud Rotary

DEPTH: 22' BLS

DATE STARTED: 8-27-03

DATE ENDED: 8-27-03

DRILLING

RIG TYPE: CME 45

CREW: Amdrill

LEGEND

| | |
|-----------------|--|
| (1) SANDY CLAY | |
| (2) SILTY CLAY | |
| (3) CLAY | |
| (4) CLAYEY SAND | |
| (5) SILTY SAND | |
| (6) LS MARL | |
| (7) LIMESTONE | |

| Depth
(ft BLS) | Blows per 6"
N-Value | Sample No. | Typical
Soil
Profile | Soil
Description | Remarks |
|-------------------|-------------------------|------------|----------------------------|---------------------|--------------------------------|
| 0' | 2 | | | (2) SILTY CLAY: | Light gray, Light orange, Firm |
| | 2 | | | 0'-2' | to Extremely firm, 0'-2' |
| | 2 | 4 | | : | |
| | 3 | | | | |
| | | | | SHELBY TUBE: | ST-18 |
| | | | | 2'-4' | |
| | 5 | | | (2) SILTY CLAY: | Light gray, Light orange, |
| 5' | 5 | | | 4'-6' | Extremely firm, |
| | 7 | 12 | | | ~10-25% limestone |
| | 8 | | | | fragments (~1-2 cm), |
| | | | | | 4'-6' |
| | 10 | | | (1) SANDY CLAY: | Light gray, Light orange, |
| | 9 | 19 | | 6'-12' | Very firm, 6'-12' |
| | 10 | | | | |
| | 3 | | | | |
| | 5 | 10 | | | |
| | 5 | | | | |
| 10' | 6 | | | | |
| | 10 | | | | |
| | 12 | | | | Dark Rust (Fe) mottling |
| | 10 | 22 | | | from 10.5'-12' |
| | 11 | | | | |
| | 2 | | | (2) SILTY CLAY: | Light gray, Light orange-rust, |
| | 4 | | | 12'-18' | Very firm, 12'-18' |
| | 4 | 8 | | | |
| | 4 | | | | |
| | 5 | | | | |
| 15' | 6 | | | | |
| | 7 | 13 | | | |
| | 7 | | | | |
| | 5 | | | | ~5-10% limestone |
| | 7 | | | | fragments (1-2 cm), |
| | 7 | 14 | | | 12.5'-16' |
| | 10 | | | | |
| | 7 | | | | |
| | 10 | | | (1) SANDY CLAY: | Light tan, Orange, Dark |
| | 12 | 17 | | 18'-20' | rust, Extremely firm, |
| 20' | 3 | | | | ~5-10% limestone |
| | 4 | | | (2) SILTY CLAY: | fragments (1-2 cm), |
| | 4 | 8 | | 20'-22' | 18'-20' |
| | 5 | | | | Tan-orange, Tan, Firm |
| | | | | | to Very firm, 20'-22' |

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STRATIGRAPHIC COLUMN
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& DISPOSAL FACILITY
DADE CITY, FLORIDA

FIGURE
10

B-25

(SPT Boring)

PROJECT

NUMBER: 99.0331.007
NAME: Enterprise Recycling
 & Disposal Facility
LOCATION: Dade City, FL

BORING

GR. ELEV: 75.62' NGVD
DIA-TYPE: 3 3/4" Mud Rotary
DEPTH: 22' BLS
DATE STARTED: 8-27-03
DATE ENDED: 8-27-03

DRILLING

RIG TYPE: CME 45
CREW: Amdrill

LEGEND

| | |
|-----------------|--|
| (1) SANDY CLAY | |
| (2) SILTY CLAY | |
| (3) CLAY | |
| (4) CLAYEY SAND | |
| (5) SILTY SAND | |
| (6) LS MARL | |
| (7) LIMESTONE | |

| Depth
(ft BLS) | Blows per 6"
N-Value | Sample No. | Typical
Soil
Profile | Soil
Description | Remarks |
|-------------------|-------------------------|------------|----------------------------|---------------------|----------------------------|
| 0' | 2 | 1 | | (2) SILTY CLAY: | Light gray, Light orange, |
| | 3 | 10 & 2 | | 0'-1.5' | Very Firm to Extremely |
| | 7 | | | (3) CLAY: | firm, Limestone fragments |
| | 8 | | | 1.5'-2' | ~(1-2cm), 0'-1.5' |
| | | | | FAILED SHELBY TUBE: | Light gray, Extremely firm |
| | | | | 2'-4' | 1.5'-2' |
| | | | | | |
| | | | | | |
| 5' | | | | SHELBY TUBE: | ST-19 |
| | | | | 4'-6' | |
| | 6 | | | (2) SILTY CLAY: | Light tan, Light orange, |
| | 10 | 21 5 | | 6'-8' | Light gray, Very Firm to |
| | 11 | | | | Extremely firm |
| | 15 | | | (3) CLAY: | 6'-8' |
| | 3 | 9 6 | | 8'-10' | Light tan, Orange, |
| | 4 | | | | Extremely firm |
| | 5 | | | | 8'-10' |
| 10' | 6 | | | (1) SANDY CLAY: | Orange, Tan, Fine sand, |
| | wh | 2 7 | | 10'-12' | Very firm, 8'-10' |
| | 1 | | | | |
| | 1 | | | | |
| | 3 | | | | |
| | 4 | 10 8 | | (2) SILTY CLAY: | Light Orange, Semi firm to |
| | 6 | | | 12'-19' | Extremely firm, 12'-19' |
| | 7 | | | | |
| 15' | 4 | | | | |
| | 5 | 16 9 | | | |
| | 11 | | | | |
| | 11 | | | | |
| | 4 | | | (3) CLAY: | ~5-10% limestone |
| | 5 | 9 10 | | 19'-20' | fragments (>2 cm), |
| | 4 | | | | 14.5'-15.5' |
| | 4 | | | | |
| | 5 | | | | |
| | 2 | 11 | | (3) CLAY: | ~5-10% limestone |
| | 4 | 4 & | | 19'-20' | fragments (>1-3 cm), |
| | 60 | 12 | | | 17'-18' |
| 20' | 7 | | | (2) SILTY CLAY: | Light Gray, Extremely firm |
| | 3 | | | 20'-22' | 19'-20' |
| | 4 | 8 13 | | | Light gray-green, Light |
| | 4 | | | | orange, Very firm |
| | 4 | | | | 20'-22' |
| | | | | | |
| | | | | | |
| 25' | | | | | |

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DADE CITY, FLORIDA

FIGURE
11

B-26 (SPT Boring)

PROJECT

NUMBER: 99.0331.007
NAME: Enterprise Recycling
 & Disposal Facility
LOCATION: Dade City, FL

BORING

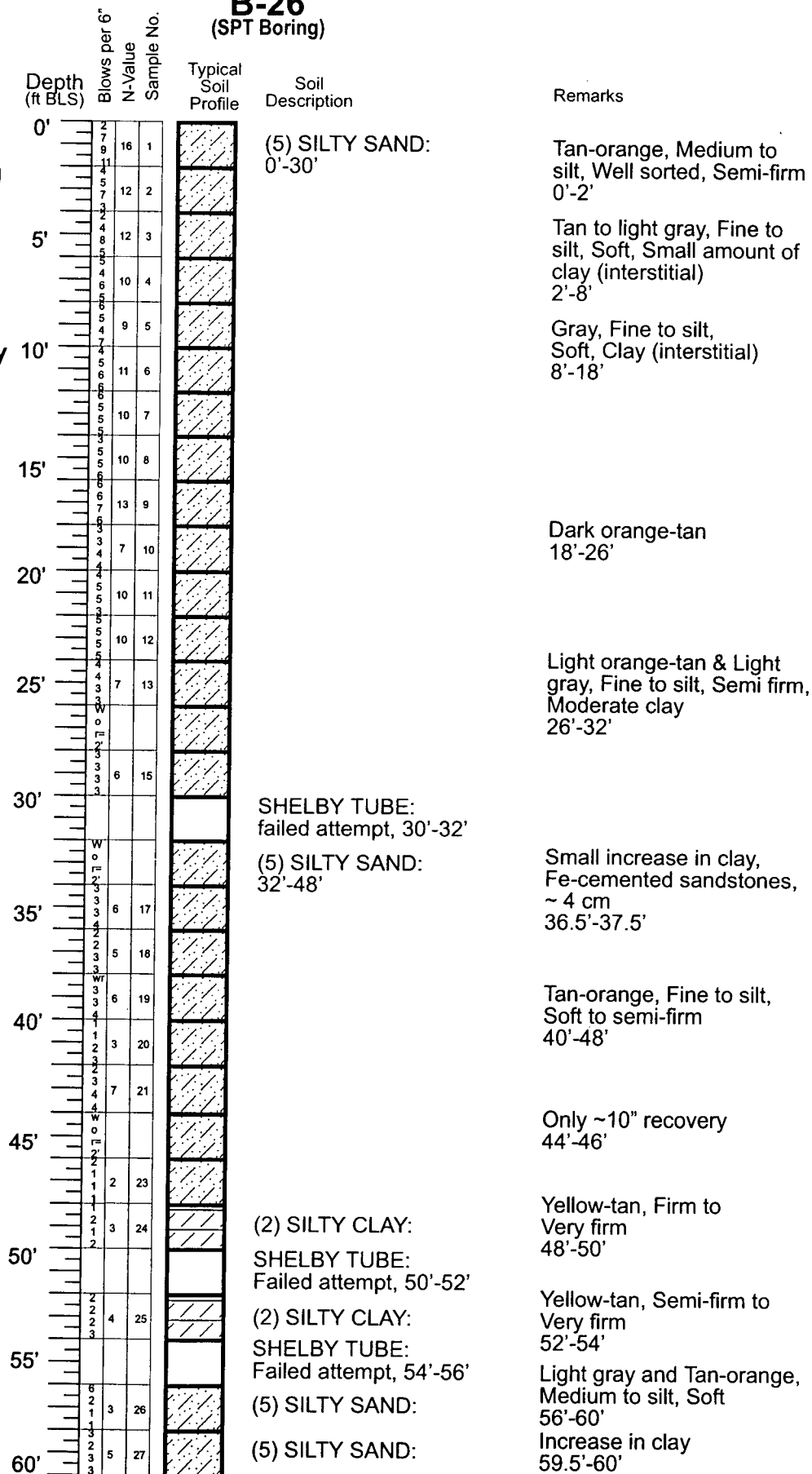
GR. ELEV: 75.00' NGVD
DIA-TYPE: 3 3/4" Mud Rotary
DEPTH: 60' BLS
DATE STARTED: 8-28-03
DATE ENDED: 8-28-03

DRILLING

RIG TYPE: CME 45
CREW: Amdrill

LEGEND

- (1) SANDY CLAY
- (2) SILTY CLAY
- (3) CLAY
- (4) CLAYEY SAND
- (5) SILTY SAND
- (6) LS MARL
- (7) LIMESTONE



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 DADE CITY, FLORIDA**

**FIGURE
 12**

B-27 (SPT Boring)

PROJECT

NUMBER: 99.0331.007
NAME: Enterprise Recycling
 & Disposal Facility
LOCATION: Dade City, FL

BORING

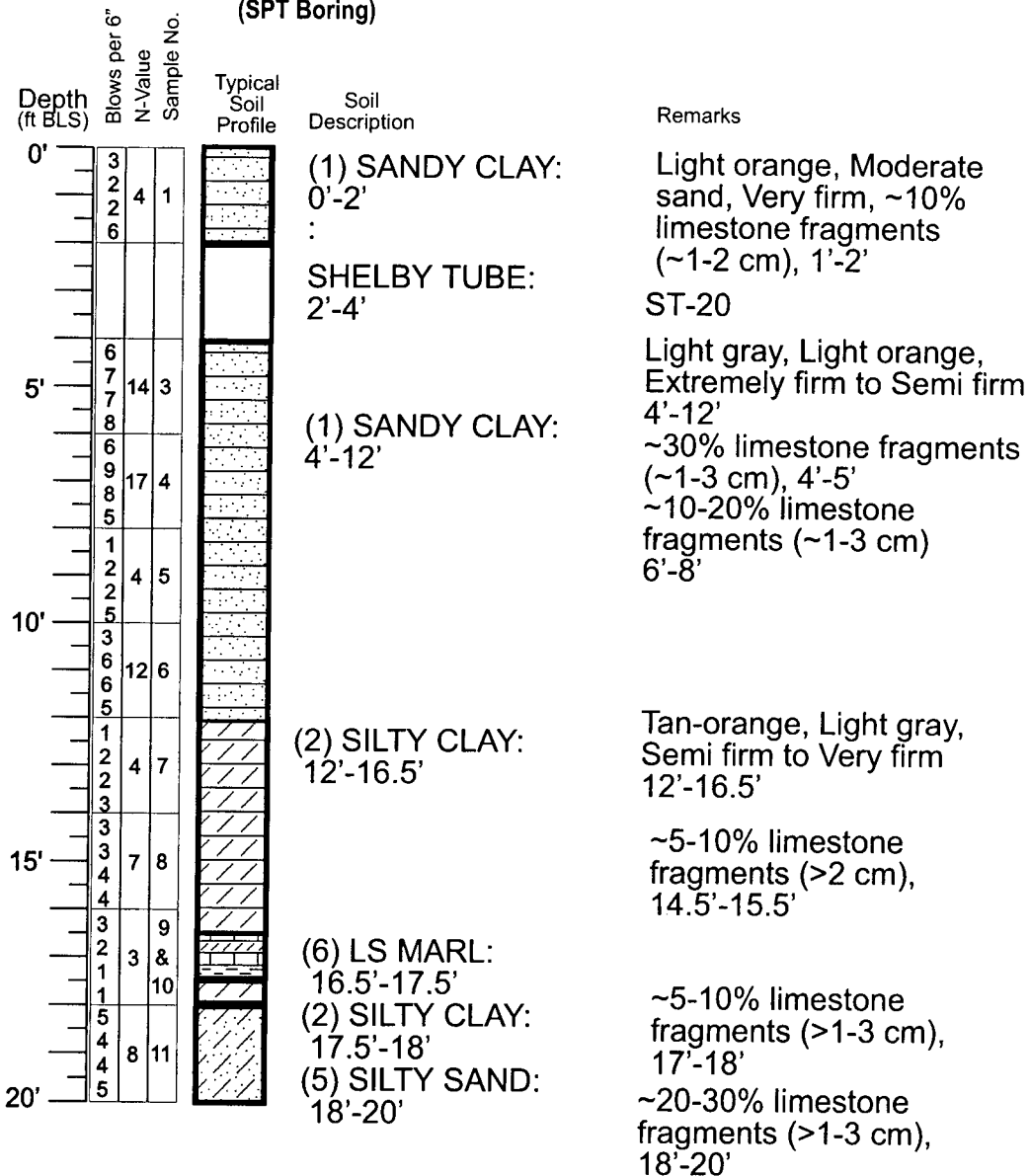
GR. ELEV: 75.89' NGVD
DIA-TYPE: 3 3/4" Mud Rotary
DEPTH: 20' BLS
DATE STARTED: 8-29-03
DATE ENDED: 8-29-03

DRILLING

RIG TYPE: CME 45
CREW: Amdrill

LEGEND

| | |
|-----------------|--|
| (1) SANDY CLAY | |
| (2) SILTY CLAY | |
| (3) CLAY | |
| (4) CLAYEY SAND | |
| (5) SILTY SAND | |
| (6) LS MARL | |
| (7) LIMESTONE | |



N:\HYDROLARKIN\Boring logs\B-27.CDR



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**STRATIGRAPHIC COLUMN
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 DADE CITY, FLORIDA**

**FIGURE
 13**

B-28

(SPT Boring)

PROJECT

NUMBER: 99.0331.007

NAME: Enterprise Recycling
& Disposal Facility

LOCATION: Dade City, FL

BORING

GR. ELEV: 96' NGVD

DIA-TYPE: 3 3/4"-Mud Rotary

DEPTH: 40' BLS

DATE STARTED: 8-29-03

DATE ENDED: 8-29-03

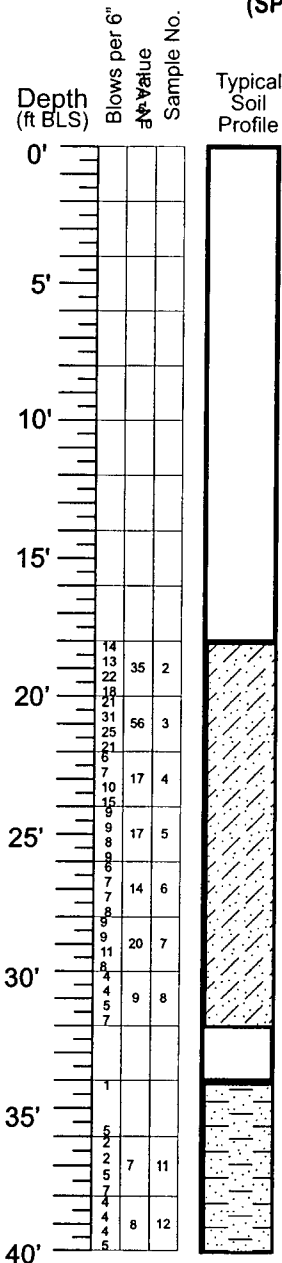
DRILLING

RIG TYPE: CME 45

CREW: Amdrill

LEGEND

- (1) SANDY CLAY
- (2) SILTY CLAY
- (3) CLAY
- (4) CLAYEY SAND
- (5) SILTY SAND
- (6) LS MARL
- (7) LIMESTONE



ROTARY WASH:
0'-18'

(5) SILTY SAND:
18'-32'

SHELBY TUBE:
32'-34'

(1) CLAYEY SAND:
34'-40'

Remarks

Dark tan-rust, Firm to very firm,
18'-22'

Light gray, Dark rust, Firm to
very firm
22'-32'

Small limestone
fragments (~1-2 cm),
18'-28'

Fe-Nodules
30'-32'

ST-21

Light gray, Light orange, Fine
to silt, Very firm
34'-40'

N:\HYDROLARKIN\Boring logs\B-28.CDR



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DADE CITY, FLORIDA**

**FIGURE
14**

B-29 (SPT Boring)

PROJECT

NUMBER: 99.0331.007
NAME: Enterprise Recycling
 & Disposal Facility
LOCATION: Dade City, FL

BORING

GR. ELEV: 74.97' NGVD
DIA-TYPE: 3 3/4" Mud Rotary
DEPTH: 22' BLS
DATE STARTED: 8-29-03
DATE ENDED: 9-2-03

DRILLING

RIG TYPE: CME 45
CREW: Amdrill

LEGEND

| | |
|-----------------|--|
| (1) SANDY CLAY | |
| (2) SILTY CLAY | |
| (3) CLAY | |
| (4) CLAYEY SAND | |
| (5) SILTY SAND | |
| (6) LS MARL | |
| (7) LIMESTONE | |

| Depth
(ft BLS) | Blows per 6"
N-Value | Sample No. | Typical
Soil
Profile | Soil
Description | Remarks |
|-------------------|-------------------------|------------|----------------------------|---------------------|------------------------------|
| 0' | 8 | 1 | | (4) CLAYEY SAND: | Light gray, Light orange, |
| 0.75' | 9 | | | 0'-3.75' | Very firm to extremely firm |
| 1.25' | 11 | | | | 0'-3.75' |
| 1.75' | 12 | | | | ~5-15% limestone |
| 2.25' | 4 | | | | fragments (<1 cm) |
| 2.75' | 4 | 2 | | | 0.5'-2' |
| 3.25' | 5 | | | | ~5-15% limestone |
| 3.75' | 6 | | | (2) SILTY CLAY: | fragments (~1-3 cm) |
| 4.25' | 1 | 3 | | 3.75'-5.5' | 2'-5.5' |
| 4.75' | 3 | 4 | | (4) CLAYEY SAND: | Light gray, Moderate sand, |
| 5.25' | 5 | | | 5.5'-6' | Very firm |
| 5.75' | 6 | | | | 5.5'-6' |
| 6.25' | 1 | | | | ~10-20% limestone |
| 6.75' | 3 | | | | fragments (~1-2 cm) |
| 7.25' | 8 | | | | 5.5'-10' |
| 7.75' | 9 | | | (2) SILTY CLAY: | Light gray-light green, Semi |
| 8.25' | 16 | 5 | | 6'-18' | to Very firm |
| 8.75' | 7 | | | | 6'-18' |
| 9.25' | 8 | | | | ~5-10% limestone |
| 9.75' | 4 | | | | fragments (>1-2 cm), |
| 10.25' | 5 | | | | 15'-15.5' |
| 10.75' | 5 | | | | White, Large amount of |
| 11.25' | 5 | | | | limestone fragments |
| 11.75' | 9 | | | | 18'-20' |
| 12.25' | 5 | | | | ~15-40% limestone |
| 12.75' | 8 | | | (6) LS MARL: | fragments (>1-4 cm), |
| 13.25' | 10 | | | 18'-20' | 15.5'-19' |
| 13.75' | 11 | | | | Lost Circulation @ 19' |
| 14.25' | 5 | | | | White, Soft to firm |
| 14.75' | 8 | | | (2) SILTY CLAY: | 20'-22' |
| 15.25' | 7 | | | 20'-22' | |
| 15.75' | 9 | | | | |
| 16.25' | 12 | 12 | | | |
| 16.75' | 7 | | | | |
| 17.25' | 9 | | | | |



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**STRATIGRAPHIC COLUMN
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 DADE CITY, FLORIDA**

**FIGURE
 15**

B-30 (SPT Boring)

PROJECT

NUMBER: 99.0331.007
NAME: Enterprise Recycling
 & Disposal Facility
LOCATION: Dade City, FL

BORING

GR. ELEV: 75.12' NGVD
DIA-TYPE: 3 3/4"-Mud Rotary
DEPTH: 28' BLS
DATE STARTED: 9-2-03
DATE ENDED: 9-2-03

DRILLING

RIG TYPE: CME 45
CREW: Amdrill

LEGEND

| | |
|-----------------|--|
| (1) SANDY CLAY | |
| (2) SILTY CLAY | |
| (3) CLAY | |
| (4) CLAYEY SAND | |
| (5) SILTY SAND | |
| (6) LS MARL | |
| (7) LIMESTONE | |

| Depth
(ft BLS) | Blows per 6"
N-Value | Sample No. | Typical
Soil
Profile | Soil
Description | Remarks |
|-------------------|-------------------------|------------|----------------------------|-------------------------------|---|
| 0' | 2 | 1 | | (2) SILTY CLAY:
0'-5' | Light gray & orange (mottled),
Firm to extremely firm,
~ 5-15% limerock fragments,
(<1 cm), 0.5'-5' |
| | 4 | | | | |
| | 4 | | | | |
| | 2 | | | | |
| | 4 | 2 | | | |
| | 4 | | | | |
| | 2 | | | | |
| | 1 | | | | |
| | 4 | 3 | | | |
| 5' | 4 | | | (3) CLAY:
5'-6' | Light gray/light green,
Very firm, Minor sand
5'-6' |
| | 5 | 4 | | | |
| | 6 | | | | |
| | 5 | | | | |
| | 6 | 5 | | (2) SILTY CLAY:
6'-10' | Light gray/light green &
orange (mottled), Very firm
6'-10' |
| | 7 | | | | ~ 10 % limestone
fragments, (<1 -2 cm)
6.25'-10' |
| | 5 | | | | |
| | 5 | 6 | | (2) SILTY CLAY:
6'-10' | |
| | 3 | | | | |
| | 2 | | | | |
| 10' | 3 | 7 | | (4) CLAYEY SAND:
10'-11' | Light tan-gray & tan- orange
(mottled), Fine sand, Firm,
Fe-nodules |
| | 3 | | | | ~1 cm, 10'-11' |
| | 4 | 8 | | (2) SILTY CLAY:
11'-13.5' | |
| | 2 | | | | |
| | 2 | 9 | | (6) LS MARL:
13.5'-16' | Light gray & light orange,
Very firm
11'-12' |
| | 6 | 10 | | | |
| | 7 | | | | |
| | 9 | | | | |
| 15' | 7 | 11 | | (7) LIMESTONE:
16'-16.5' | |
| | 8 | | | | |
| | 6 | | | | |
| | 50 | | | Rotary Wash
16.5'-18' | Light tan, Soft, ~ 30-40 %
limestone fragments
16'-22' |
| | 6" | | | | |
| | 50 | | | (7) LIMESTONE:
18'-18.25' | |
| | 3" | | | Rotary Wash
18.25'-20' | |
| 20' | 1 | | | (7) LIMESTONE:
20'-22.5' | Hard limestone
at 22' bls |
| | 2 | 16 | | | |
| | 1 | | | | |
| | 10 | | | | |
| | 10 | 17 | | (6) LS MARL:
22.5'-23.5' | Light tan, Soft, ~ 30-40 %
limestone fragments
(<1 -3 cm), 22.5'-23.5' |
| | 5 | | | | |
| | 7 | 18 | | | |
| | 9 | | | | |
| | 9 | | | | |
| 25' | 8 | 19 | | (7) LIMESTONE:
23.5'-26.5' | White, Soft, ~ 30-50 %
limestone fragments
(~1-4 cm), 26.5'-28' |
| | 5 | | | | |
| | 9 | | | | |
| | 12 | | | | |
| | 5 | | | | |
| | 13 | 20 | | (2) SILTY CLAY:
26.5'-28' | |
| | 15 | | | | |



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**STRATIGRAPHIC COLUMN
 ENTERPRISE RECYCLING &
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 DADE CITY, FLORIDA**

**FIGURE
 16**

PROJECT

NUMBER: 99.0331.007

NAME: Enterprise Recycling
& Disposal Facility

LOCATION: Dade City, FL

BORING

GR. ELEV: 75.02' NGVD

DIA-TYPE: 3 3/4"-Mud Rotary

DEPTH: 32' BLS

DATE STARTED: 9-2-03

DATE ENDED: 9-2-03

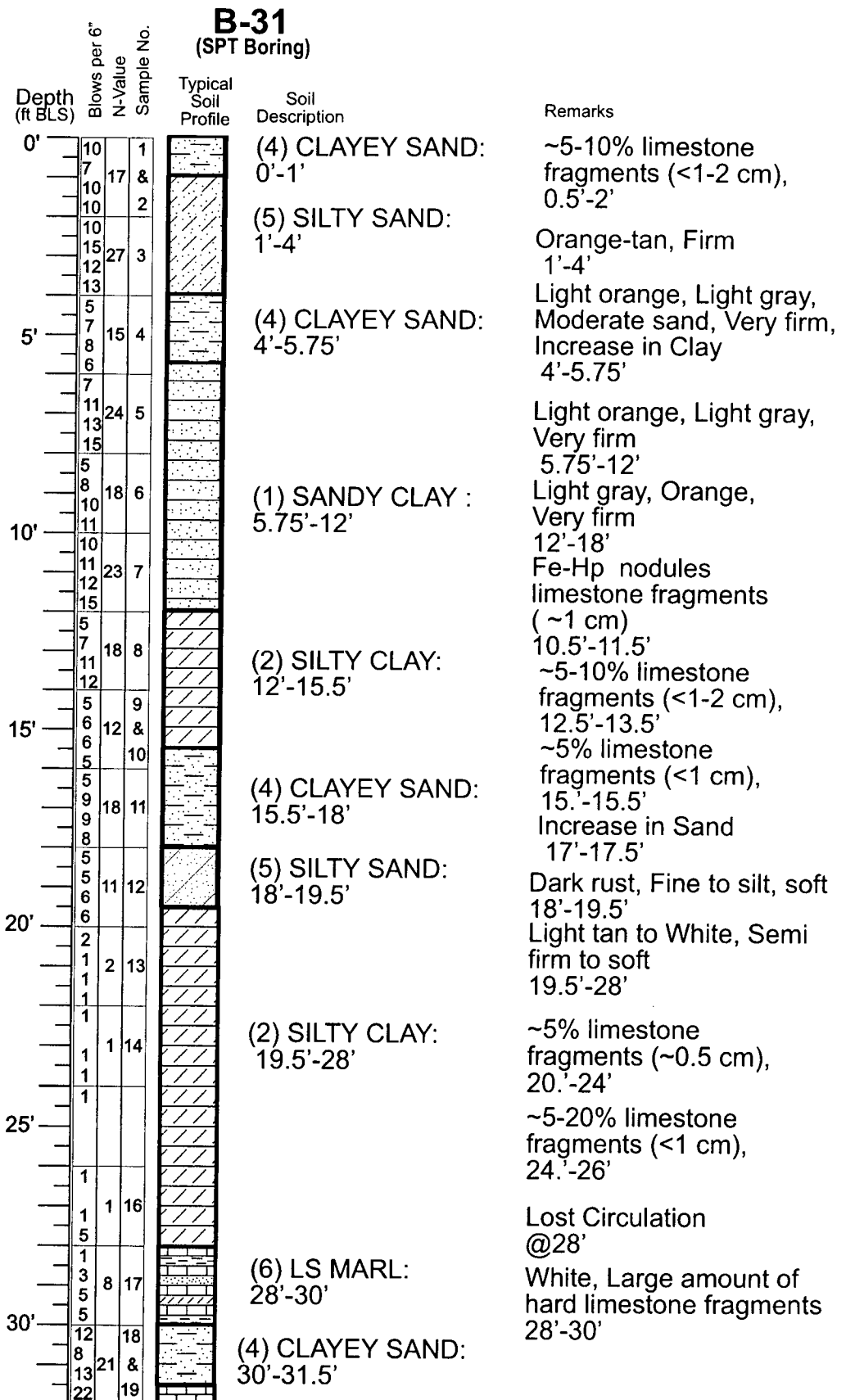
DRILLING

RIG TYPE: CME 45

CREW: Amdrill

LEGEND

| | |
|-----------------|--|
| (1) SANDY CLAY | |
| (2) SILTY CLAY | |
| (3) CLAY | |
| (4) CLAYEY SAND | |
| (5) SILTY SAND | |
| (6) LS MARL | |
| (7) LIMESTONE | |



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STRATIGRAPHIC COLUMN
ENTERPRISE RECYCLING &
DISPOSAL FACILITY
DADE CITY, FLORIDA

FIGURE
17

PROJECT

NUMBER: 99.0331.007
NAME: Enterprise Recycling
 & Disposal Facility
LOCATION: Dade City, FL

BORING

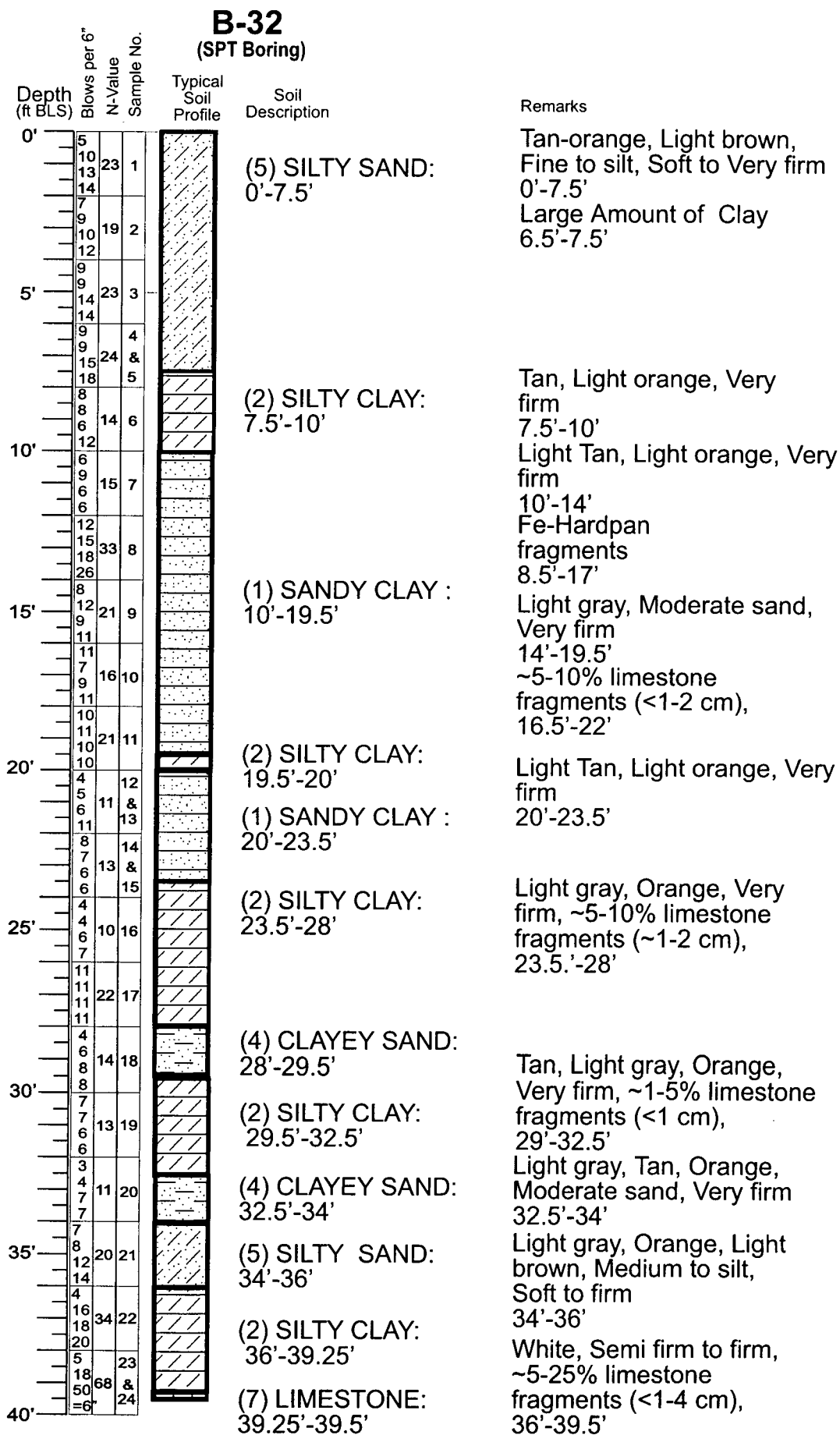
GR. ELEV: 75.05' NGVD
DIA-TYPE: 3 3/4"-Mud Rotary
DEPTH: 39.5' BLS
DATE STARTED: 9-3-03
DATE ENDED: 9-3-03

DRILLING

RIG TYPE: CME 45
CREW: Amdrill

LEGEND

- (1) SANDY CLAY
- (2) SILTY CLAY
- (3) CLAY
- (4) CLAYEY SAND
- (5) SILTY SAND
- (6) LS MARL
- (7) LIMESTONE



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DISPOSAL FACILITY
DADE CITY, FLORIDA

FIGURE
18

B-33 (SPT Boring)

PROJECT

NUMBER: 99.0331.007
NAME: Enterprise Recycling
 & Disposal Facility
LOCATION: Dade City, FL

BORING

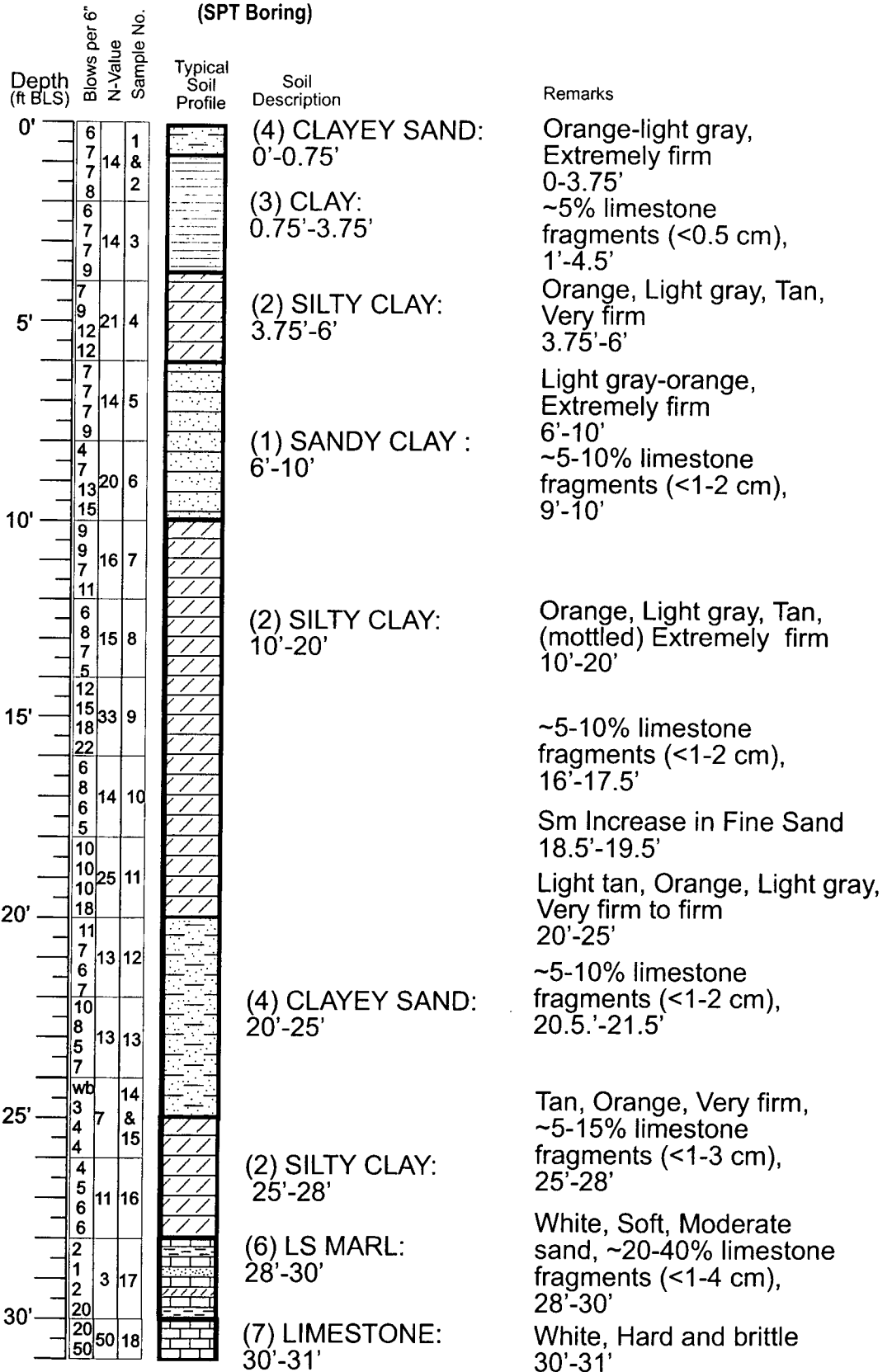
GR. ELEV: 75.51' NGVD
DIA-TYPE: 3 3/4" Mud Rotary
DEPTH: 31' BLS
DATE STARTED: 9-4-03
DATE ENDED: 9-4-03

DRILLING

RIG TYPE: CME 45
CREW: Amdrill

LEGEND

- (1) SANDY CLAY
- (2) SILTY CLAY
- (3) CLAY
- (4) CLAYEY SAND
- (5) SILTY SAND
- (6) LS MARL
- (7) LIMESTONE



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**STRATIGRAPHIC COLUMN
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 DISPOSAL FACILITY
 DADE CITY, FLORIDA**

**FIGURE
 19**

B-34 (SPT Boring)

PROJECT

NUMBER: 99.0331.007
NAME: Enterprise Recycling
 & Disposal Facility
LOCATION: Dade City, FL

BORING

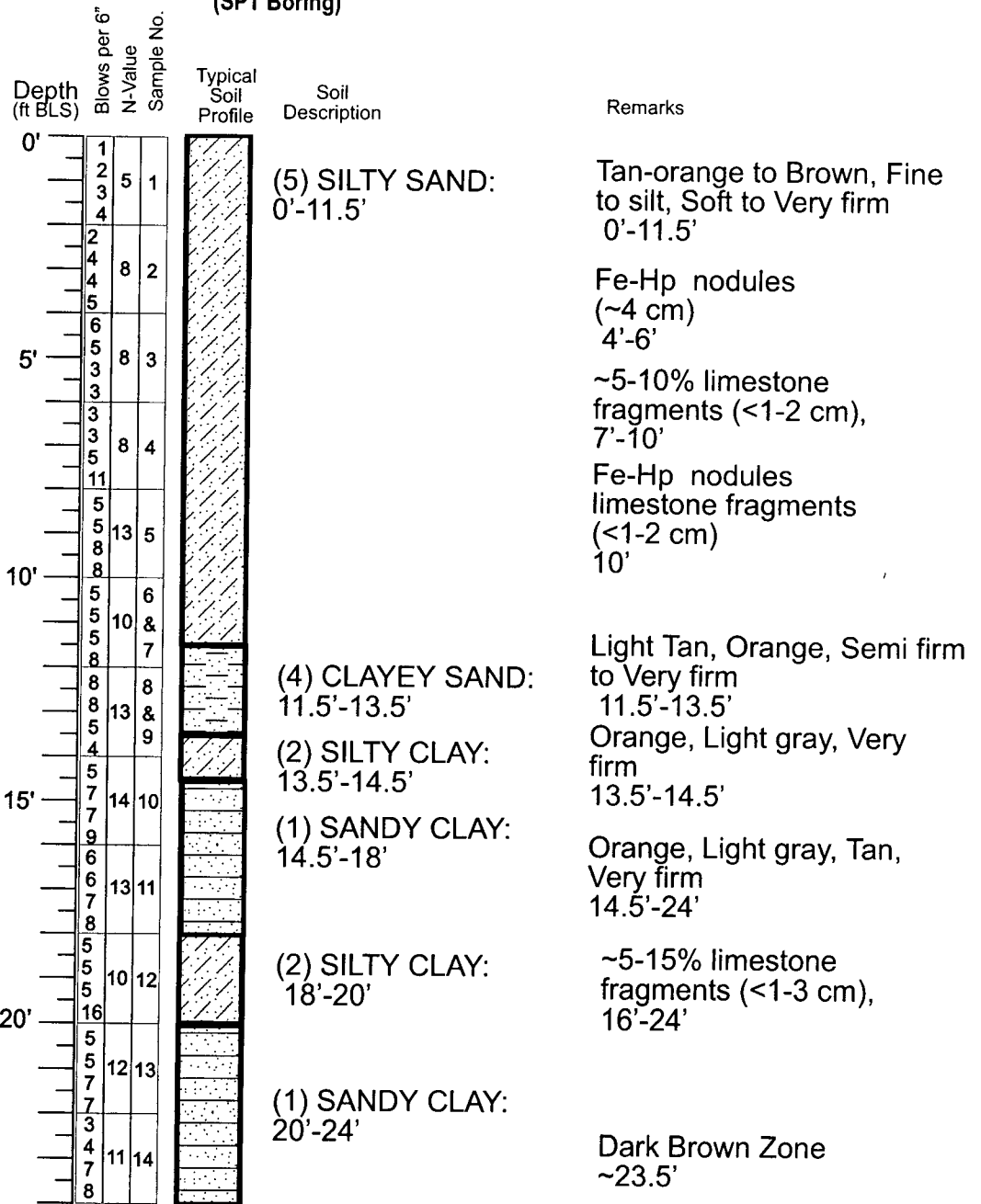
GR. ELEV: 75.43' NGVD
DIA-TYPE: 3 3/4"-Mud Rotary
DEPTH: 24' BLS
DATE STARTED: 9-4-03
DATE ENDED: 9-4-03

DRILLING

RIG TYPE: CME 45
CREW: Amdrill

LEGEND

| | |
|-----------------|--|
| (1) SANDY CLAY | |
| (2) SILTY CLAY | |
| (3) CLAY | |
| (4) CLAYEY SAND | |
| (5) SILTY SAND | |
| (6) LS MARL | |
| (7) LIMESTONE | |



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 DADE CITY, FLORIDA**

**FIGURE
 20**

SSA-14

(Solid-stem Auger Boring)

PROJECT

NUMBER: 99.0331.007
NAME: Enterprise Recycling
 & Disposal Facility
LOCATION: Dade City, FL

BORING

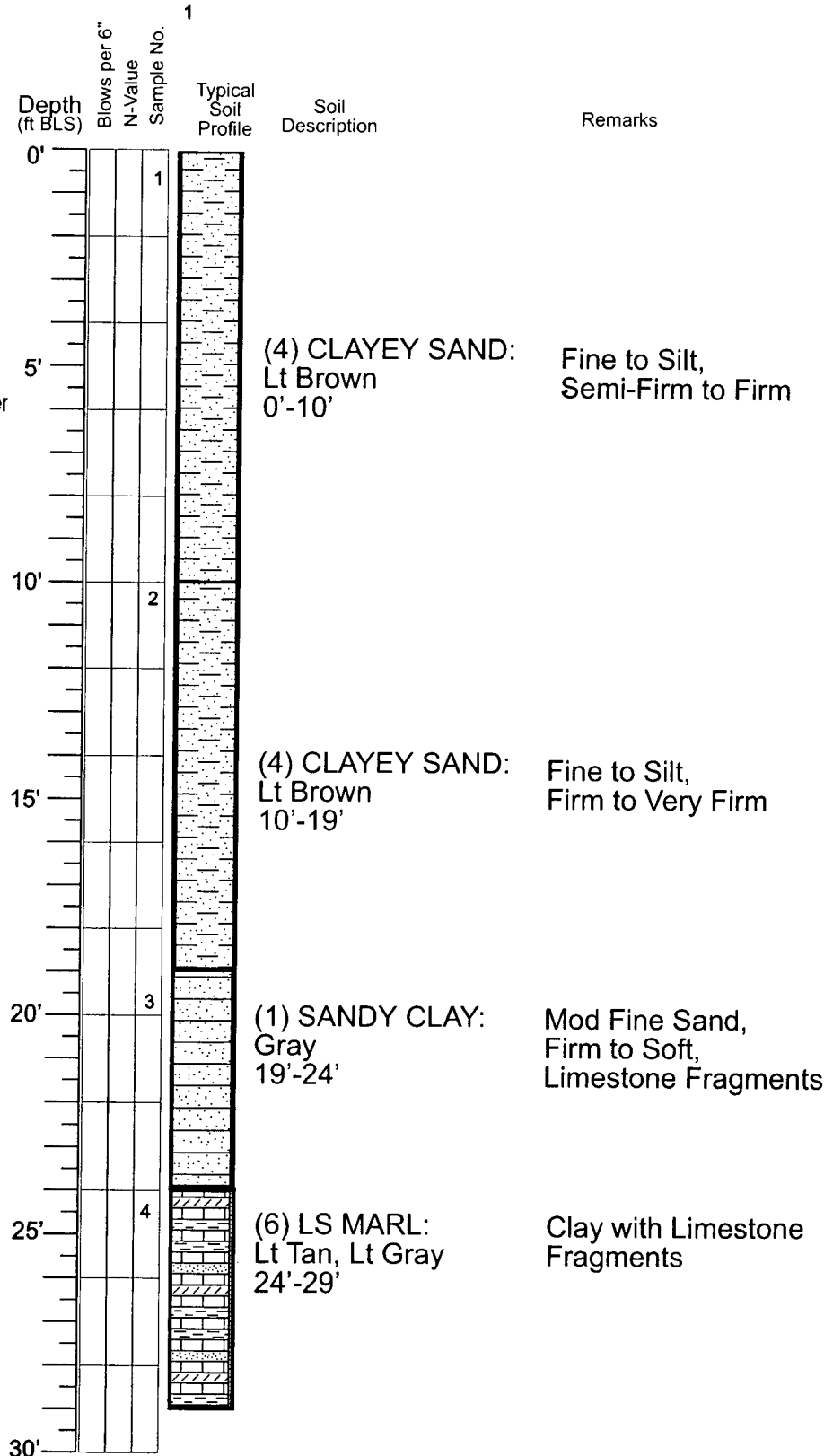
GR. ELEV: 99' NGVD
DIA-TYPE: 3"-Solid Stem Auger
DEPTH: 29' BLS
DATE STARTED: 7-31-03
DATE ENDED: 7-31-03

DRILLING

RIG TYPE: CME 45
CREW: EDS

LEGEND

- (1) SANDY CLAY
- (2) SILTY CLAY
- (3) CLAY
- (4) CLAYEY SAND
- (5) SILTY SAND
- (6) LS MARL
- (7) LIMESTONE



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& DISPOSAL FACILITY
DADE CITY, FLORIDA

FIGURE
14

SSA-15 (Solid-stem Auger Boring)

PROJECT

NUMBER: 99.0331.007
NAME: Enterprise Recycling
 & Disposal Facility
LOCATION: Dade City, FL








BORING

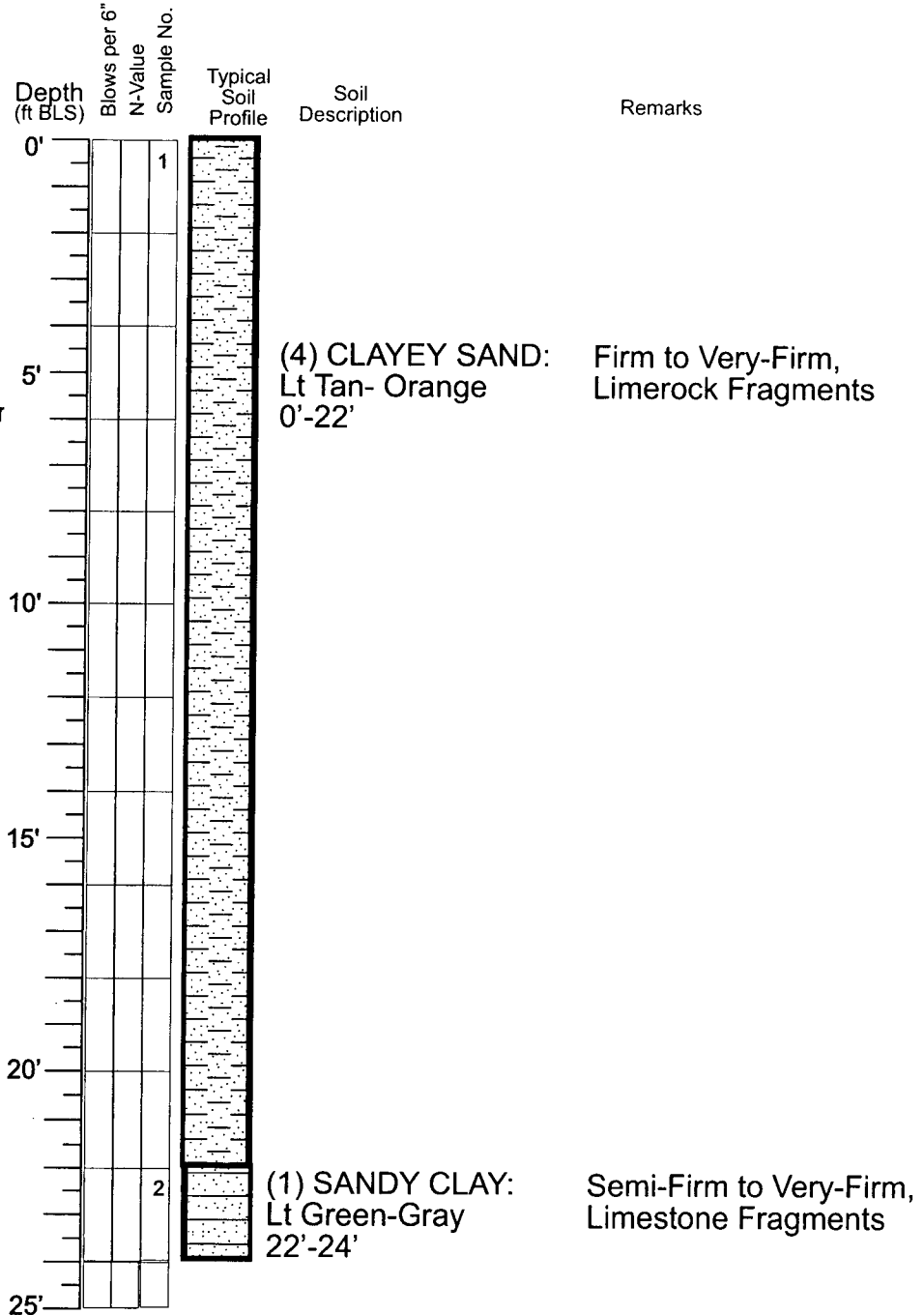
GR. ELEV: 103' NGVD
DIA-TYPE: 3"-Solid Stem Auger
DEPTH: 24' BLS
DATE STARTED: 7-31-03
DATE ENDED: 7-31-03

DRILLING

RIG TYPE: CME 45
CREW: EDS

LEGEND

- (1) SANDY CLAY 
- (2) SILTY CLAY 
- (3) CLAY 
- (4) CLAYEY SAND 
- (5) SILTY SAND 
- (6) LS MARL 
- (7) LIMESTONE 



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**STRATIGRAPHIC COLUMN
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 DADE CITY, FLORIDA**

**FIGURE
 15**

SSA-16 (Solid-stem Auger Boring)

PROJECT

NUMBER: 99.0331.007
NAME: Enterprise Recycling
 & Disposal Facility
LOCATION: Dade City, FL








BORING

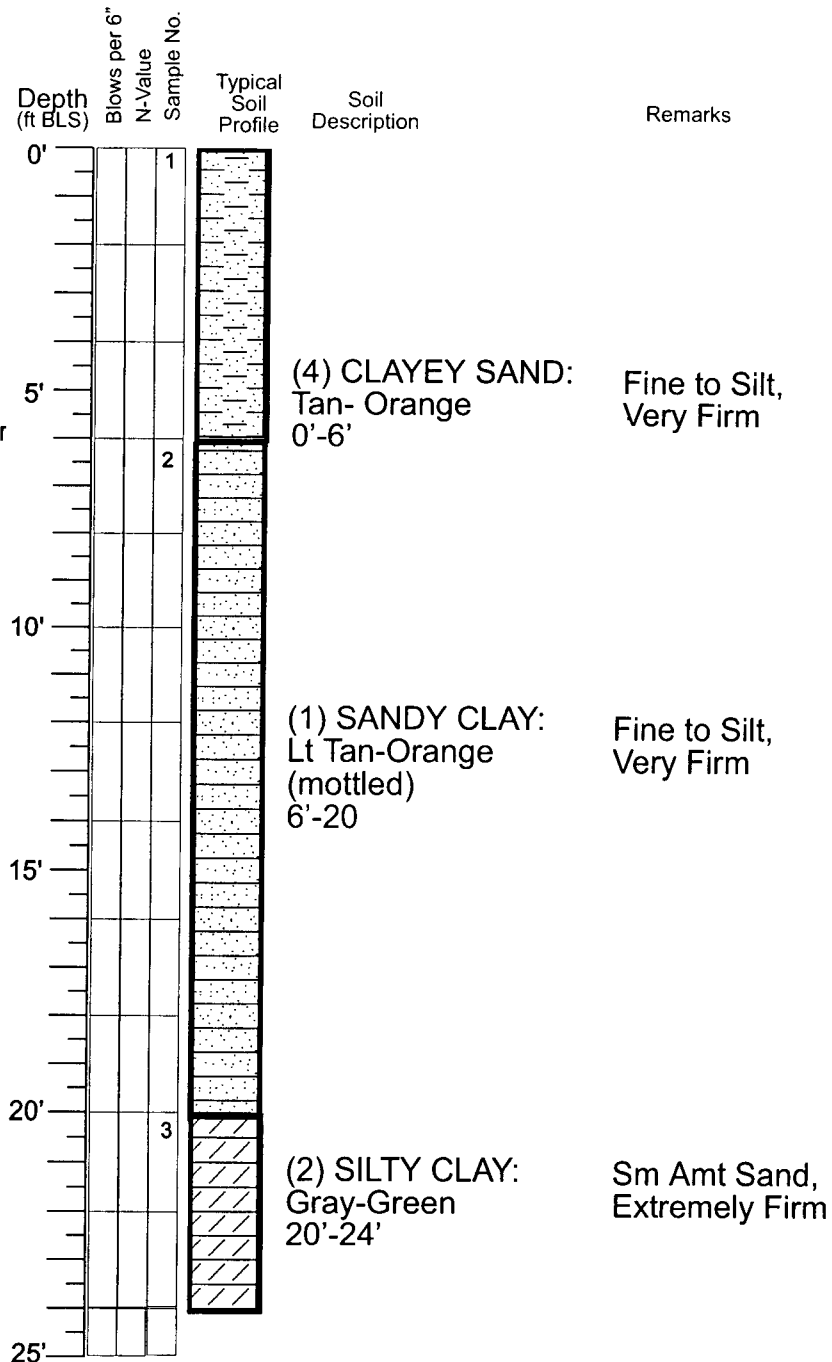
GR. ELEV: 104' NGVD
DIA-TYPE: 3"-Solid Stem Auger
DEPTH: 24' BLS
DATE STARTED: 7-31-03
DATE ENDED: 7-31-03

DRILLING

RIG TYPE: CME 45
CREW: EDS

LEGEND

- (1) SANDY CLAY 
- (2) SILTY CLAY 
- (3) CLAY 
- (4) CLAYEY SAND 
- (5) SILTY SAND 
- (6) LS MARL 
- (7) LIMESTONE 



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**STRATIGRAPHIC COLUMN
 ENTERPRISE RECYCLING
 & DISPOSAL FACILITY
 DADE CITY, FLORIDA**

**FIGURE
 16**

SSA-17

(Solid-stem Auger Boring)

PROJECT

NUMBER: 99.0331.007
NAME: Enterprise Recycling
 & Disposal Facility
LOCATION: Dade City, FL

BORING

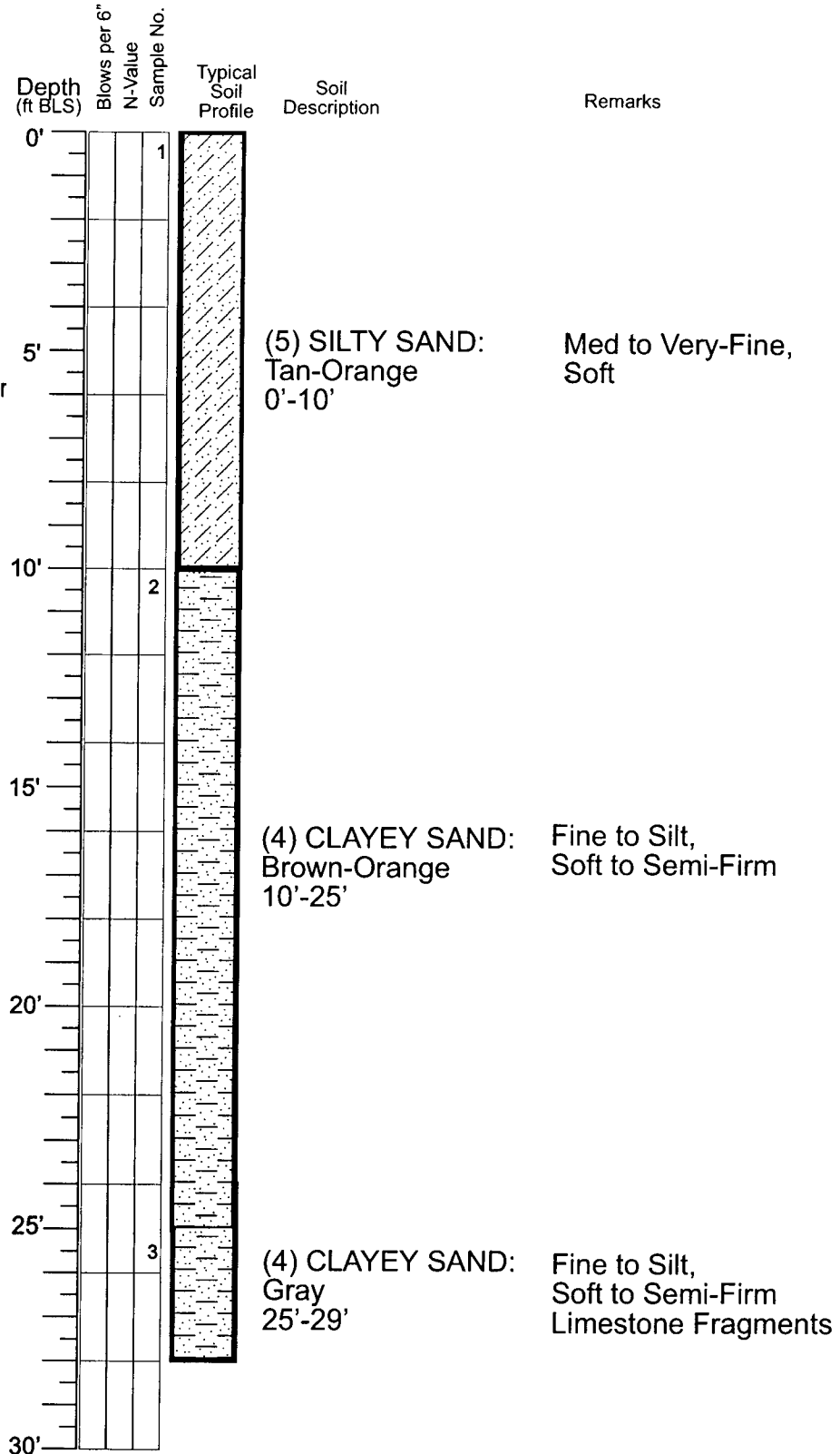
GR. ELEV: 107' NGVD
DIA-TYPE: 3"-Solid Stem Auger
DEPTH: 29' BLS
DATE STARTED: 7-31-03
DATE ENDED: 7-31-03

DRILLING

RIG TYPE: CME 45
CREW: EDS

LEGEND

- (1) SANDY CLAY
- (2) SILTY CLAY
- (3) CLAY
- (4) CLAYEY SAND
- (5) SILTY SAND
- (6) LS MARL
- (7) LIMESTONE



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STRATIGRAPHIC COLUMN
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DADE CITY, FLORIDA

FIGURE
17

SSA-18 (Solid-stem Auger Boring)

PROJECT

NUMBER: 99.0331.007
NAME: Enterprise Recycling
 & Disposal Facility
LOCATION: Dade City, FL

BORING

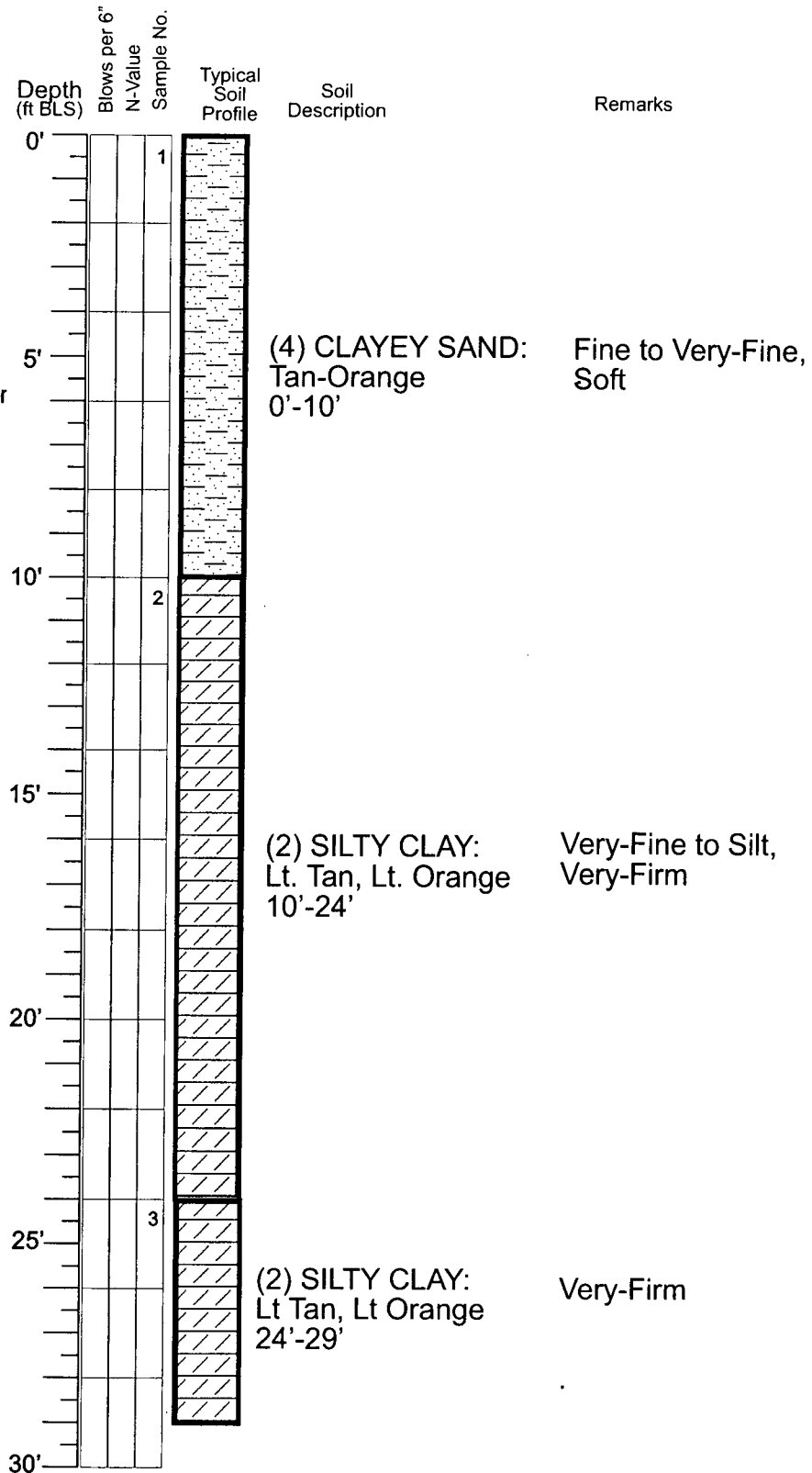
GR. ELEV: 108' NGVD
DIA-TYPE: 3"-Solid Stem Auger
DEPTH: 29' BLS
DATE STARTED: 7-31-03
DATE ENDED: 7-31-03

DRILLING

RIG TYPE: CME 45
CREW: EDS

LEGEND

- (1) SANDY CLAY
- (2) SILTY CLAY
- (3) CLAY
- (4) CLAYEY SAND
- (5) SILTY SAND
- (6) LS MARL
- (7) LIMESTONE



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 & DISPOSAL FACILITY
 DADE CITY, FLORIDA**

**FIGURE
 18**

SSA-19

(Solid-stem Auger Boring)

PROJECT

NUMBER: 99.0331.007

NAME: Enterprise Recycling
& Disposal Facility

LOCATION: Dade City, FL

BORING

GR. ELEV: 84.89' NGVD

DIA-TYPE: 3"-Solid Stem Auger

DEPTH: 20' BLS

DATE STARTED: 8-23-03

DATE ENDED: 8-23-03

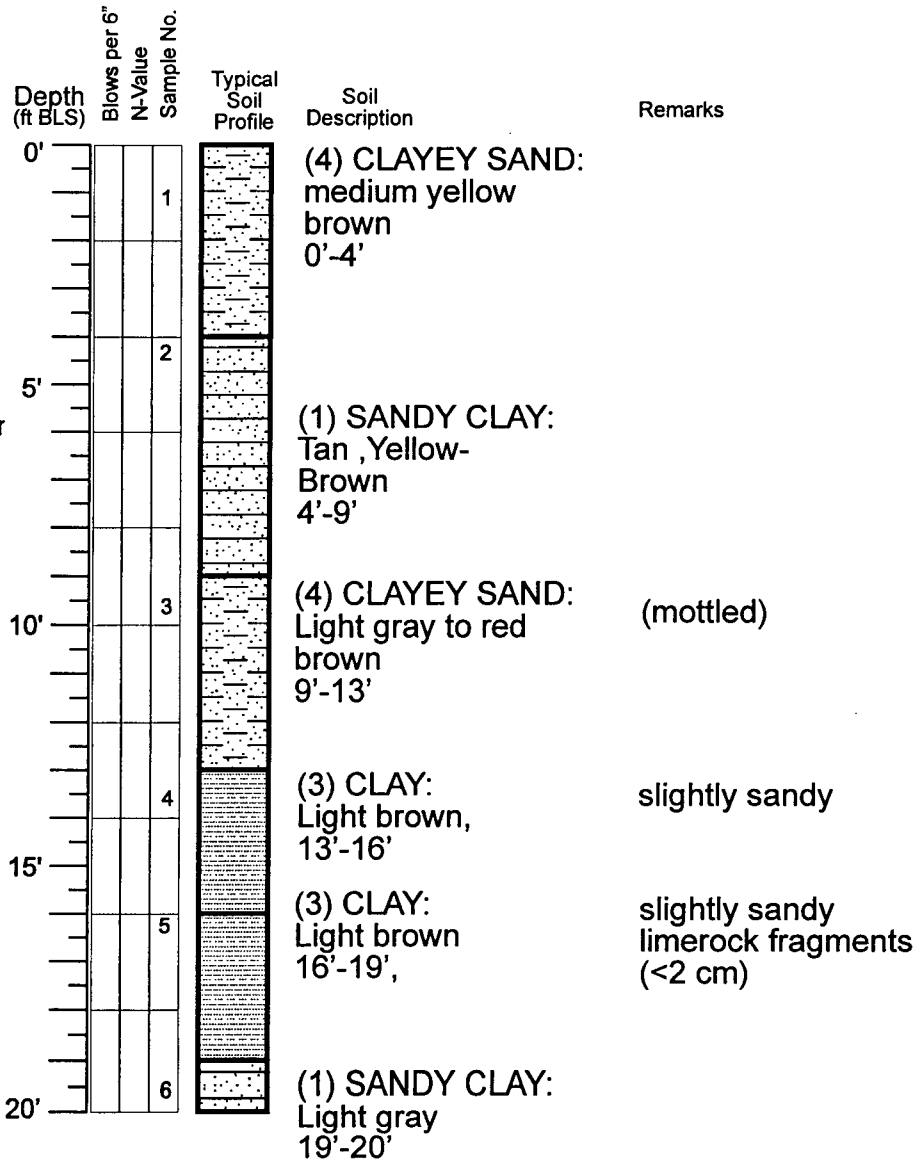
DRILLING

RIG TYPE: CME 45

CREW: Amdrill

LEGEND

| | |
|-----------------|--|
| (1) SANDY CLAY | |
| (2) SILTY CLAY | |
| (3) CLAY | |
| (4) CLAYEY SAND | |
| (5) SILTY SAND | |
| (6) LS MARL | |
| (7) LIMESTONE | |



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DADE CITY, FLORIDA**

**FIGURE
19**

SSA-20 (Solid-stem Auger Boring)

PROJECT

NUMBER: 99.0331.007
NAME: Enterprise Recycling
 & Disposal Facility
LOCATION: Dade City, FL

BORING

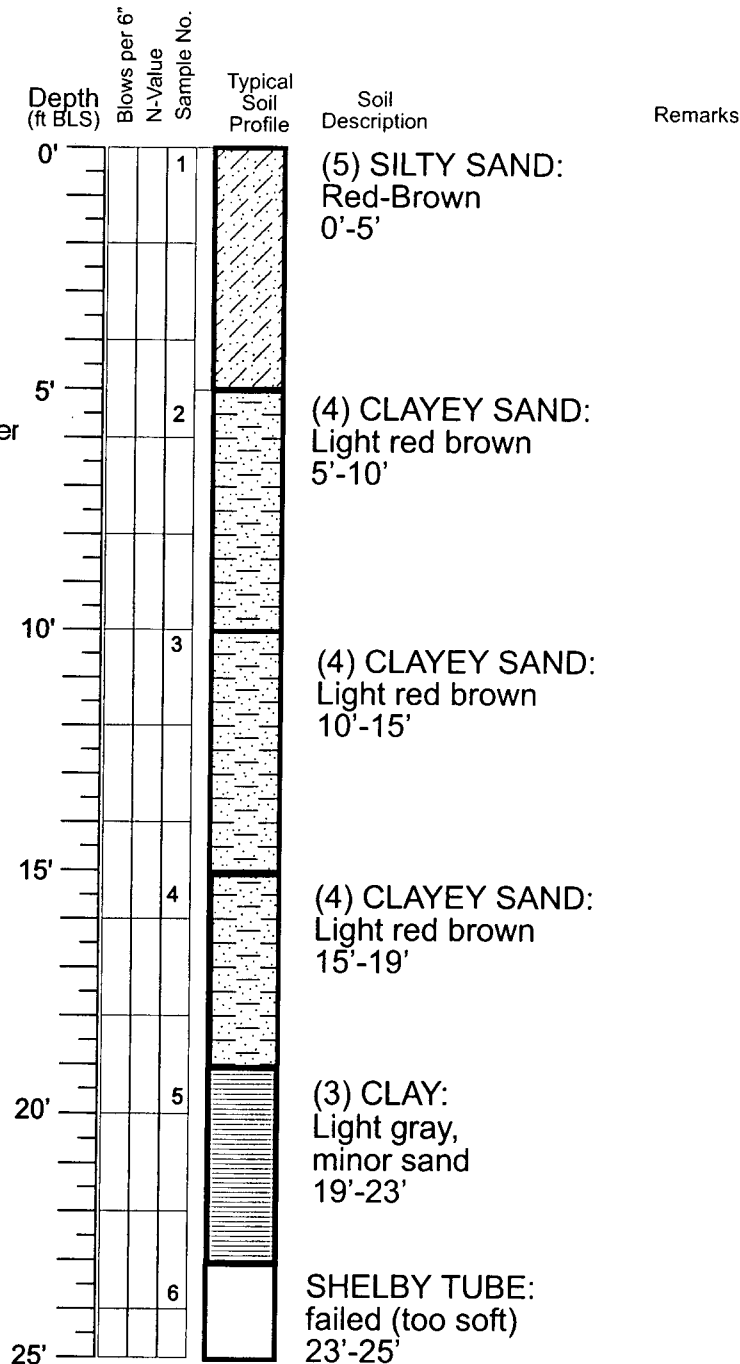
GR. ELEV: 84.43' NGVD
DIA-TYPE: 3"-Solid Stem Auger
DEPTH: 25' BLS
DATE STARTED: 8-23-03
DATE ENDED: 8-23-03

DRILLING

RIG TYPE: CME 45
CREW: Amdrill

LEGEND

| | |
|-----------------|--|
| (1) SANDY CLAY | |
| (2) SILTY CLAY | |
| (3) CLAY | |
| (4) CLAYEY SAND | |
| (5) SILTY SAND | |
| (6) LS MARL | |
| (7) LIMESTONE | |



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**STRATIGRAPHIC COLUMN
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 DADE CITY, FLORIDA**

**FIGURE
 20**

SSA-21

(Solid-stem Auger Boring)

PROJECT

NUMBER: 99.0331.007
NAME: Enterprise Recycling
 & Disposal Facility
LOCATION: Dade City, FL

BORING

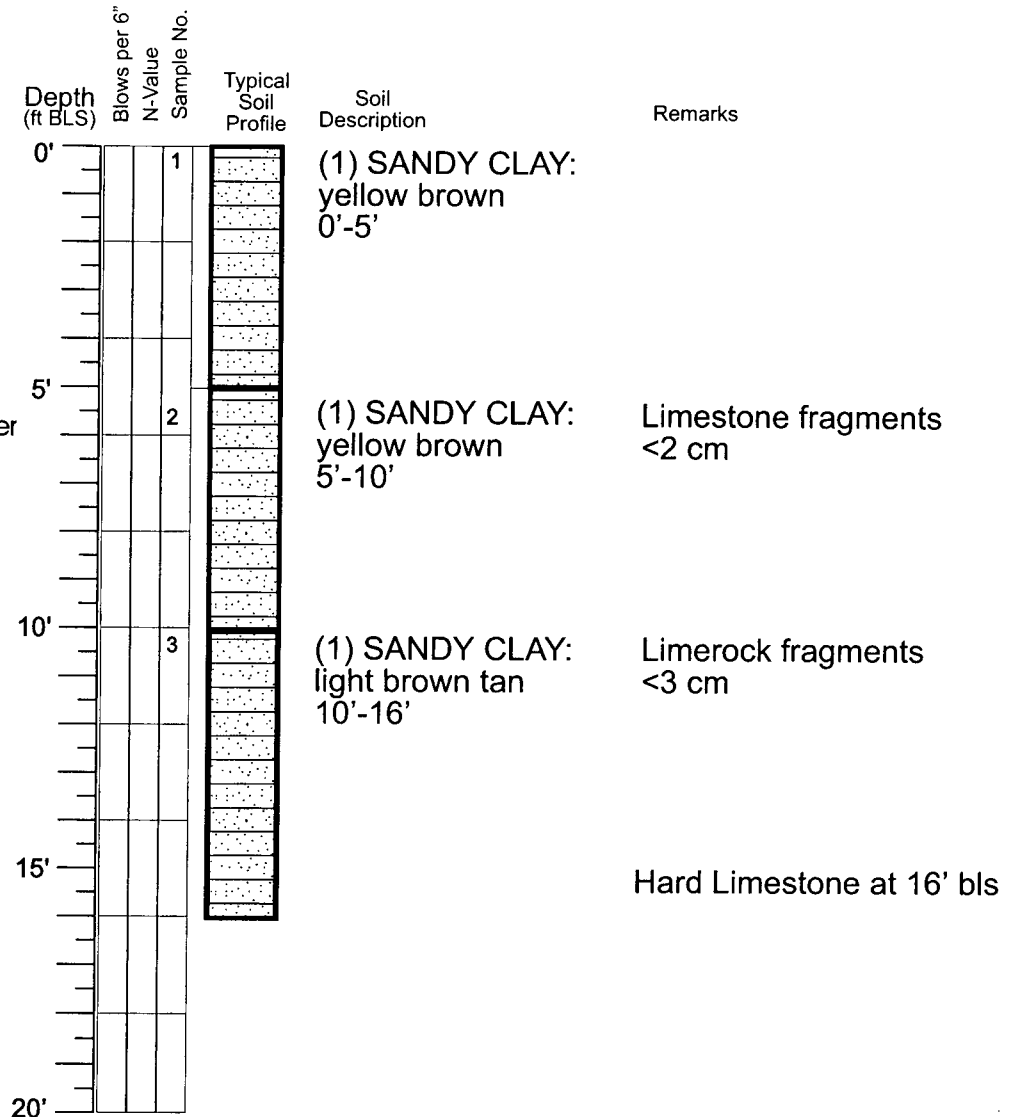
GR. ELEV: 82.77' NGVD
DIA-TYPE: 3"-Solid Stem Auger
DEPTH: 16' BLS
DATE STARTED: 8-23-03
DATE ENDED: 8-23-03

DRILLING

RIG TYPE: CME 45
CREW: Amdrill

LEGEND

| | |
|-----------------|--|
| (1) SANDY CLAY | |
| (2) SILTY CLAY | |
| (3) CLAY | |
| (4) CLAYEY SAND | |
| (5) SILTY SAND | |
| (6) LS MARL | |
| (7) LIMESTONE | |



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 DADE CITY, FLORIDA**

**FIGURE
 21**

SSA-22

(Solid-stem Auger Boring)

PROJECT

NUMBER: 99.0331.007

NAME: Enterprise Recycling
& Disposal Facility

LOCATION: Dade City, FL

BORING

GR. ELEV: 82.26' NGVD

DIA-TYPE: 3"-Solid Stem Auger

DEPTH: 20' BLS

DATE STARTED: 8-23-03

DATE ENDED: 8-23-03

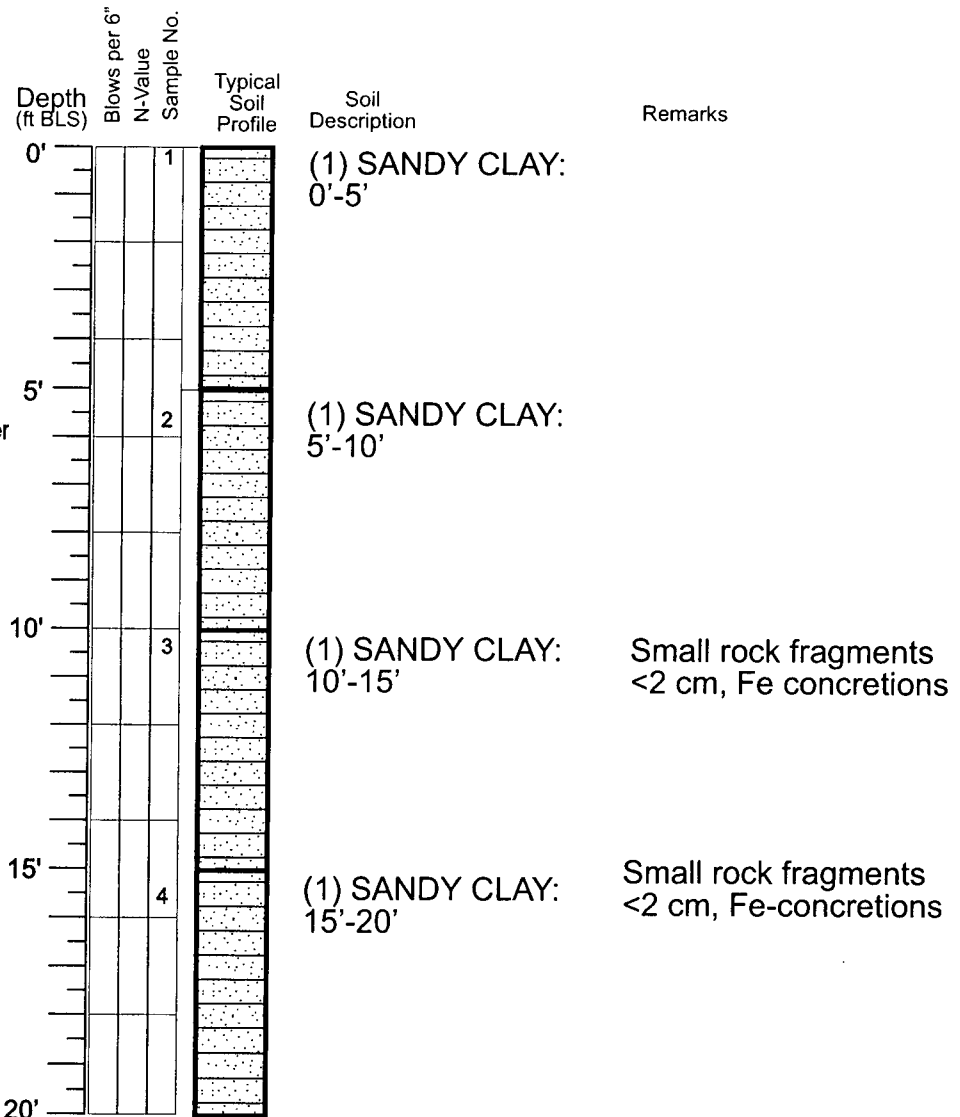
DRILLING

RIG TYPE: CME 45

CREW: Amdrill

LEGEND

| | |
|-----------------|--|
| (1) SANDY CLAY | |
| (2) SILTY CLAY | |
| (3) CLAY | |
| (4) CLAYEY SAND | |
| (5) SILTY SAND | |
| (6) LS MARL | |
| (7) LIMESTONE | |



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**STRATIGRAPHIC COLUMN
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DADE CITY, FLORIDA**

**FIGURE
22**

SSA-23

(Solid-stem Auger Boring)

PROJECT

NUMBER: 99.0331.007
NAME: Enterprise Recycling
 & Disposal Facility
LOCATION: Dade City, FL

BORING

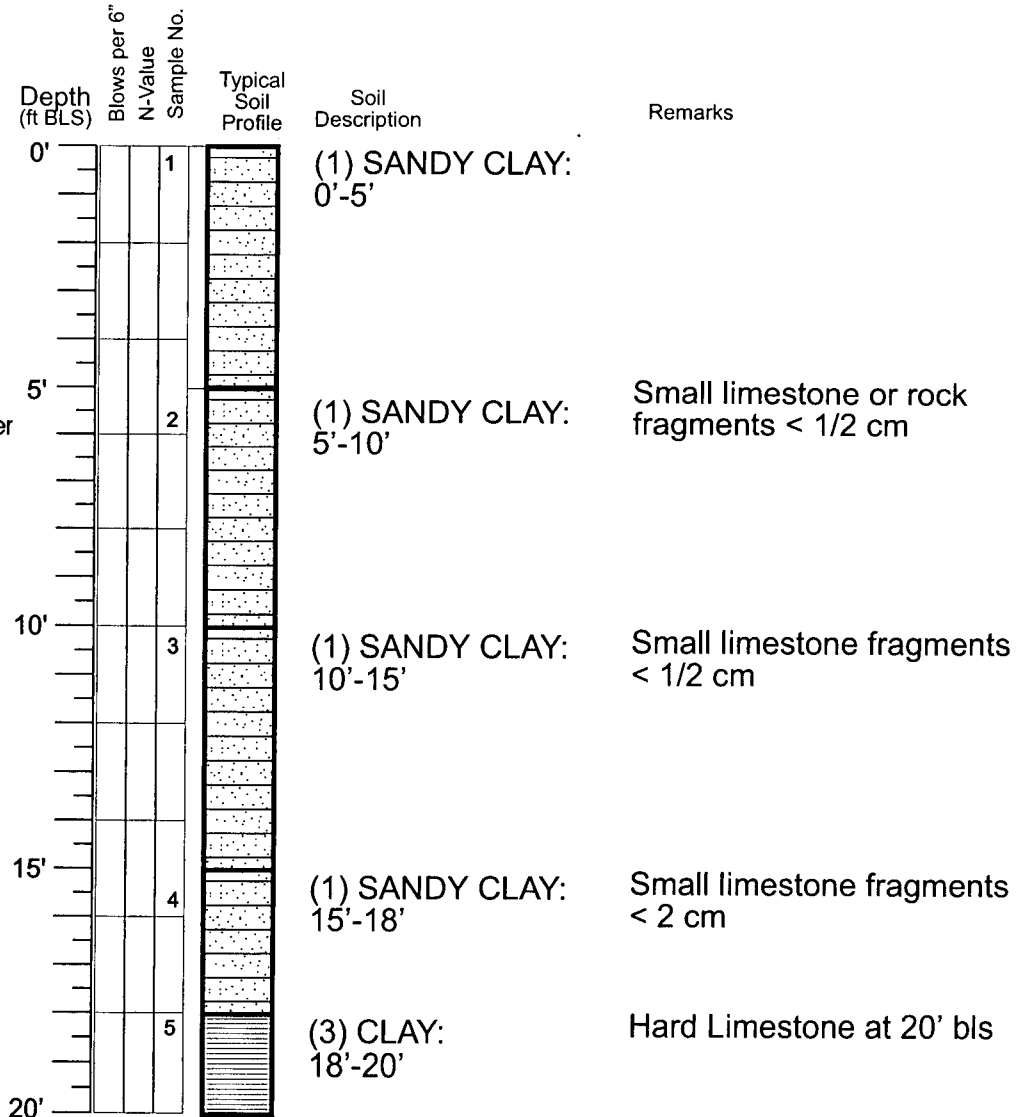
GR. ELEV: 80.70' NGVD
DIA-TYPE: 3"-Solid Stem Auger
DEPTH: 20' BLS
DATE STARTED: 8-23-03
DATE ENDED: 8-23-03

DRILLING

RIG TYPE: CME 45
CREW: Amdrill

LEGEND

| | |
|-----------------|--|
| (1) SANDY CLAY | |
| (2) SILTY CLAY | |
| (3) CLAY | |
| (4) CLAYEY SAND | |
| (5) SILTY SAND | |
| (6) LS MARL | |
| (7) LIMESTONE | |



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STRATIGRAPHIC COLUMN
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DADE CITY, FLORIDA

FIGURE
23

SSA-24

(Solid-stem Auger Boring)

PROJECT

NUMBER: 99.0331.007
NAME: Enterprise Recycling
 & Disposal Facility
LOCATION: Dade City, FL








BORING

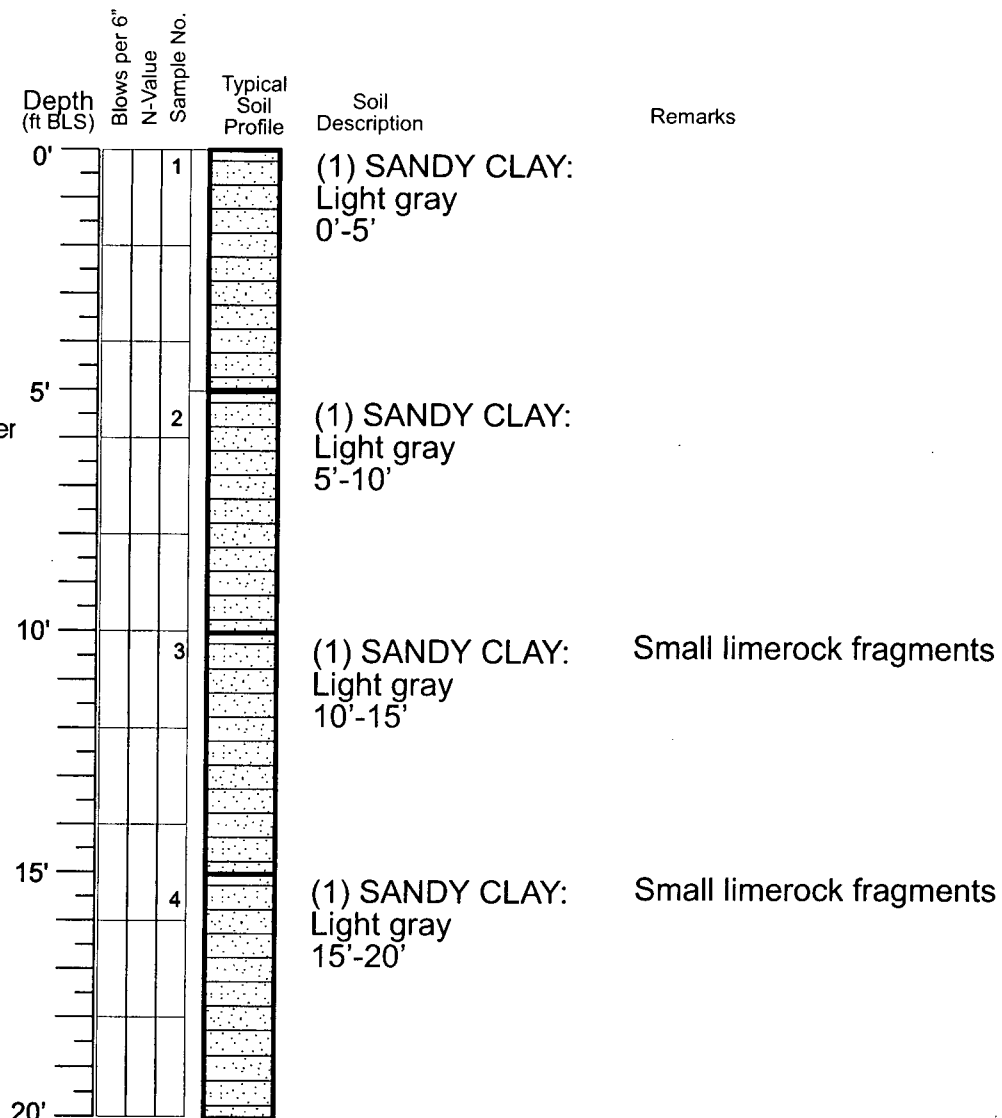
GR. ELEV: 80.11' NGVD
DIA-TYPE: 3"-Solid Stem Auger
DEPTH: 20' BLS
DATE STARTED: 8-23-03
DATE ENDED: 8-23-03

DRILLING

RIG TYPE: CME 45
CREW: Amdrill

LEGEND

- (1) SANDY CLAY 
- (2) SILTY CLAY 
- (3) CLAY 
- (4) CLAYEY SAND 
- (5) SILTY SAND 
- (6) LS MARL 
- (7) LIMESTONE 



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STRATIGRAPHIC COLUMN
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DADE CITY, FLORIDA

FIGURE
24

SSA-25

(Solid-stem Auger Boring)

PROJECT

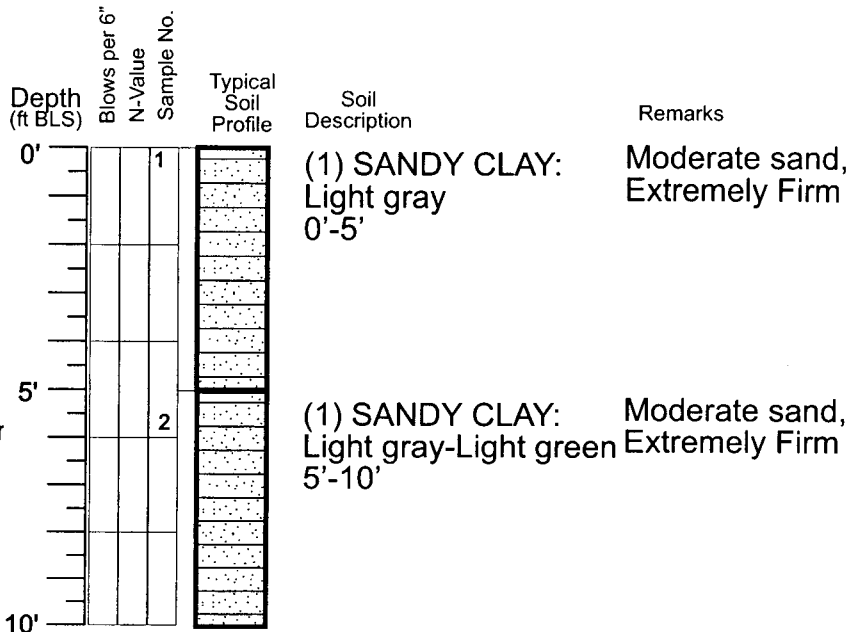
NUMBER: 99.0331.007
NAME: Enterprise Recycling
 & Disposal Facility
LOCATION: Dade City, FL

BORING








GR. ELEV: 75.25' NGVD
DIA-TYPE: 3"-Solid Stem Auger
DEPTH: 10' BLS
DATE STARTED: 9-3-03
DATE ENDED: 9-3-03

DRILLING

RIG TYPE: CME 45
CREW: Amdrill



LEGEND

| | |
|-----------------|---|
| (1) SANDY CLAY |  |
| (2) SILTY CLAY |  |
| (3) CLAY |  |
| (4) CLAYEY SAND |  |
| (5) SILTY SAND |  |
| (6) LS MARL |  |
| (7) LIMESTONE |  |

N:\HYDROLARKIN\Boring logs\SSA-25.CDR



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STRATIGRAPHIC COLUMN
ENTERPRISE RECYCLING
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DADE CITY, FLORIDA

FIGURE
25

SSA-26

(Solid-stem Auger Boring)

PROJECT

NUMBER: 99.0331.007
NAME: Enterprise Recycling
 & Disposal Facility
LOCATION: Dade City, FL

BORING

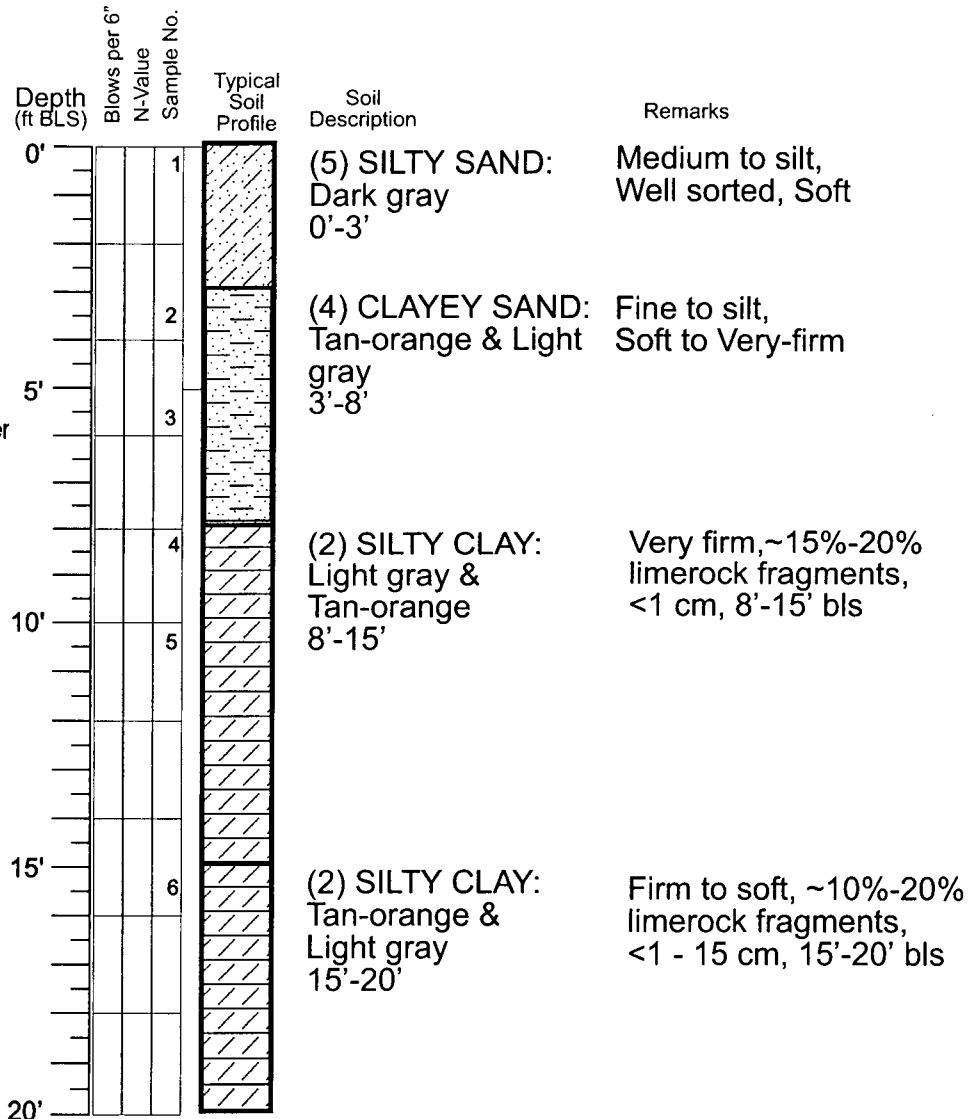
GR. ELEV: 75.01' NGVD
DIA-TYPE: 3"-Solid Stem Auger
DEPTH: 20' BLS
DATE STARTED: 9-3-03
DATE ENDED: 9-3-03

DRILLING

RIG TYPE: CME 45
CREW: Amdrill

LEGEND

| | |
|-----------------|--|
| (1) SANDY CLAY | |
| (2) SILTY CLAY | |
| (3) CLAY | |
| (4) CLAYEY SAND | |
| (5) SILTY SAND | |
| (6) LS MARL | |
| (7) LIMESTONE | |



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 DADE CITY, FLORIDA**

**FIGURE
 26**

SSA-27

(Solid-stem Auger Boring)

PROJECT

NUMBER: 99.0331.007
NAME: Enterprise Recycling
 & Disposal Facility
LOCATION: Dade City, FL








BORING

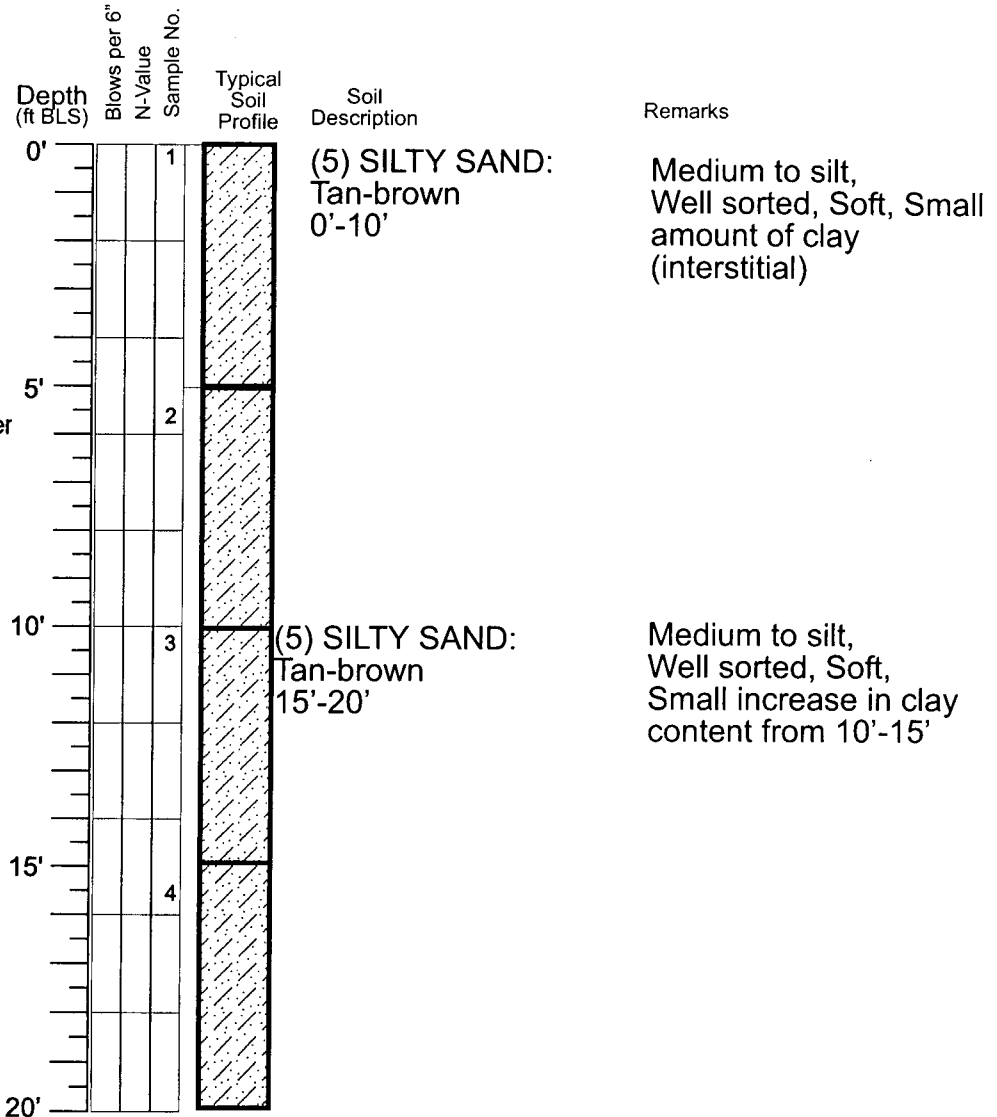
GR. ELEV: 74.79' NGVD
DIA-TYPE: 3"-Solid Stem Auger
DEPTH: 20' BLS
DATE STARTED: 9-3-03
DATE ENDED: 9-3-03

DRILLING

RIG TYPE: CME 45
CREW: Amdrill

LEGEND

- (1) SANDY CLAY 
- (2) SILTY CLAY 
- (3) CLAY 
- (4) CLAYEY SAND 
- (5) SILTY SAND 
- (6) LS MARL 
- (7) LIMESTONE 



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DADE CITY, FLORIDA

FIGURE
27

SSA-28

(Solid-stem Auger Boring)

PROJECT

NUMBER: 99.0331.007
NAME: Enterprise Recycling
 & Disposal Facility
LOCATION: Dade City, FL

BORING

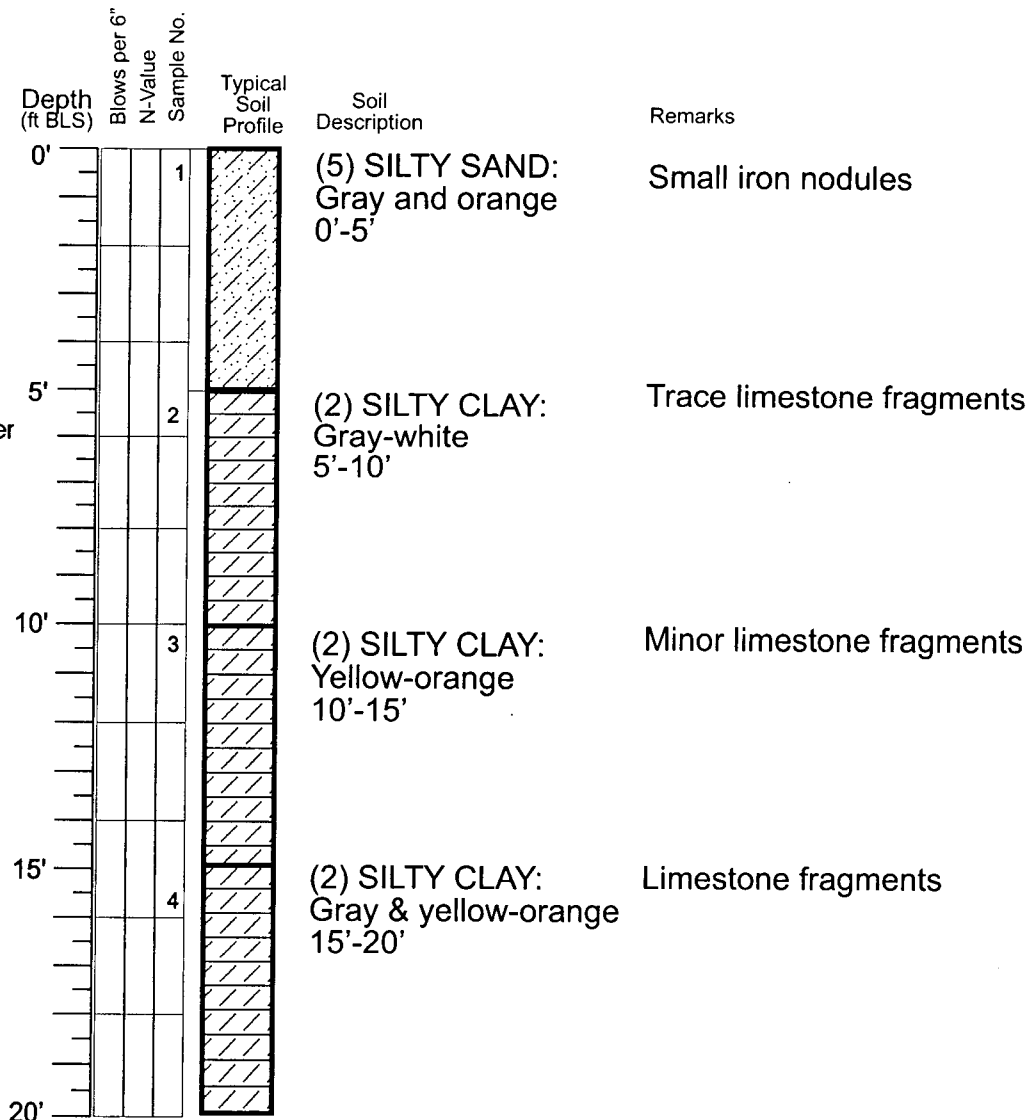
GR. ELEV: 75.07' NGVD
DIA-TYPE: 3"-Solid Stem Auger
DEPTH: 20' BLS
DATE STARTED: 9-3-03
DATE ENDED: 9-3-03

DRILLING

RIG TYPE: CME 45
CREW: Amdrill

LEGEND

| | |
|-----------------|--|
| (1) SANDY CLAY | |
| (2) SILTY CLAY | |
| (3) CLAY | |
| (4) CLAYEY SAND | |
| (5) SILTY SAND | |
| (6) LS MARL | |
| (7) LIMESTONE | |



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DADE CITY, FLORIDA

FIGURE
28

SSA-29 (Solid-stem Auger Boring)

PROJECT

NUMBER: 99.0331.007
NAME: Enterprise Recycling
 & Disposal Facility
LOCATION: Dade City, FL

BORING

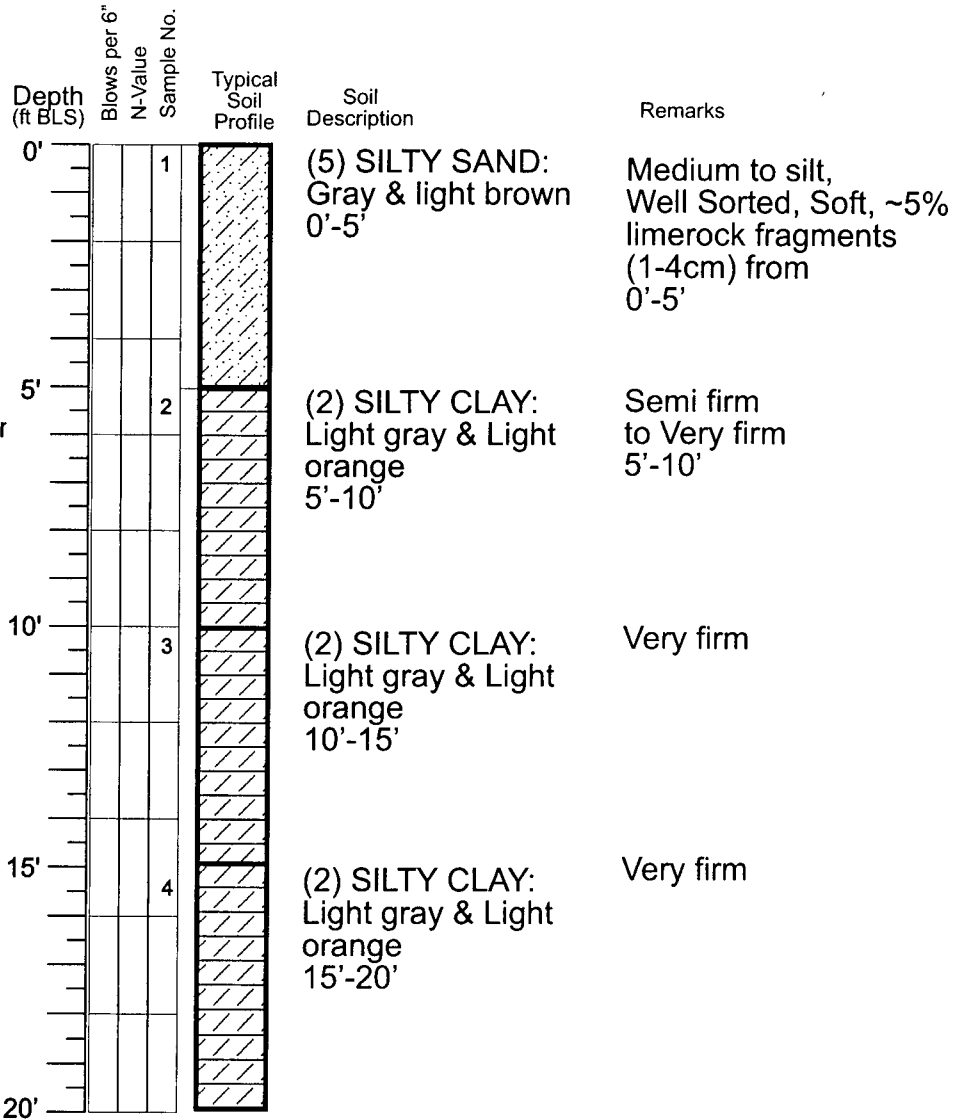
GR. ELEV: 76.77' NGVD
DIA-TYPE: 3"-Solid Stem Auger
DEPTH: 20' BLS
DATE STARTED: 9-3-03
DATE ENDED: 9-3-03

DRILLING

RIG TYPE: CME 45
CREW: Amdrill

LEGEND

| | |
|-----------------|--|
| (1) SANDY CLAY | |
| (2) SILTY CLAY | |
| (3) CLAY | |
| (4) CLAYEY SAND | |
| (5) SILTY SAND | |
| (6) LS MARL | |
| (7) LIMESTONE | |



N:\HYDROLARKIN\Boring logs\SSA-29.CDR



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 DADE CITY, FLORIDA**

**FIGURE
 29**

SSA-30

(Solid-stem Auger Boring)

PROJECT

NUMBER: 99.0331.007
NAME: Enterprise Disposal
 & Disposal Facility
LOCATION: Dade City, FL

BORING

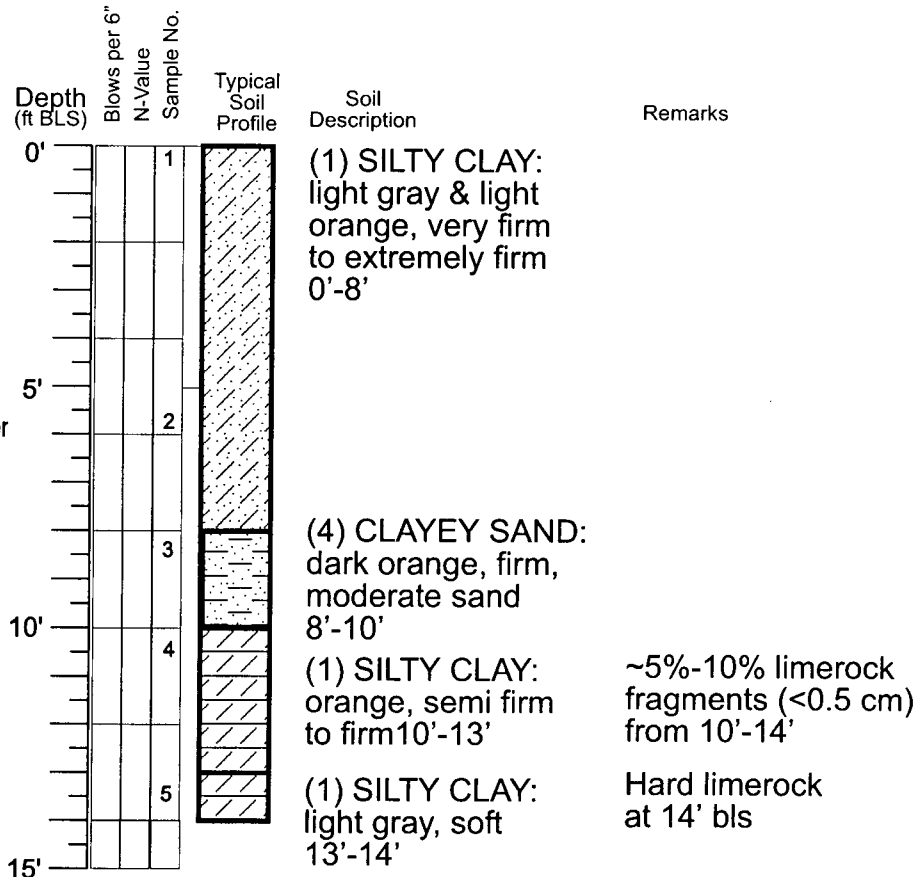
GR. ELEV: 75.12' NGVD
DIA-TYPE: 3"-Solid Stem Auger
DEPTH: 14' BLS
DATE STARTED: 9-4-03
DATE ENDED: 9-4-03

DRILLING

RIG TYPE: CME 45
CREW: Amrdrill

LEGEND

| | |
|-----------------|--|
| (1) SANDY CLAY | |
| (2) SILTY CLAY | |
| (3) CLAY | |
| (4) CLAYEY SAND | |
| (5) SILTY SAND | |
| (6) LS MARL | |
| (7) LIMESTONE | |



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DADE CITY, FLORIDA

FIGURE
30

SSA-31

(Solid-stem Auger Boring)

PROJECT

NUMBER: 99.0331.007
NAME: Enterprise Recycling
 & Disposal Facility
LOCATION: Dade City, FL

BORING

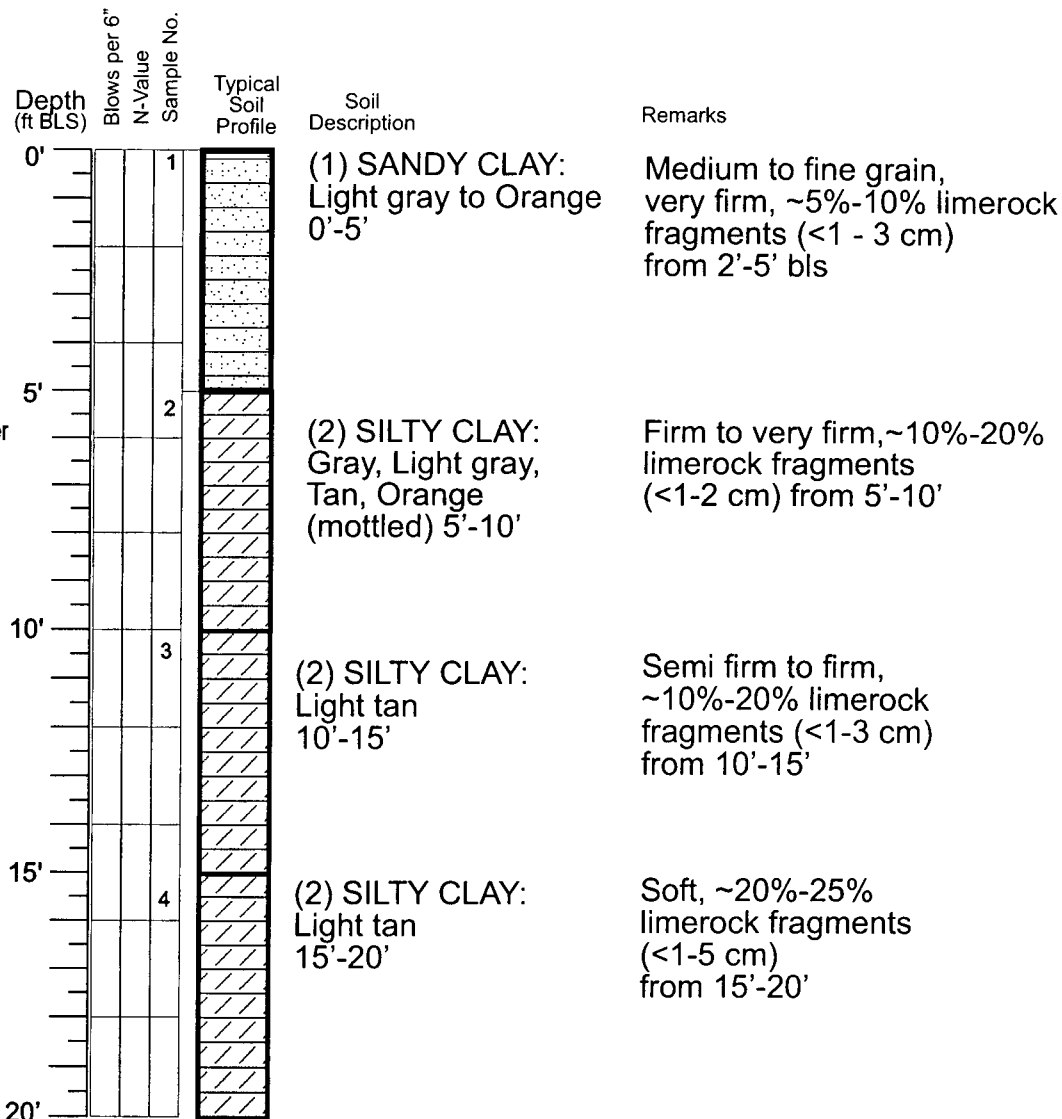
GR. ELEV: 75.53' NGVD
DIA-TYPE: 3"-Solid Stem Auger
DEPTH: 20' BLS
DATE STARTED: 9-3-03
DATE ENDED: 9-3-03

DRILLING

RIG TYPE: CME 45
CREW: Amdrill

LEGEND

- (1) SANDY CLAY
- (2) SILTY CLAY
- (3) CLAY
- (4) CLAYEY SAND
- (5) SILTY SAND
- (6) LS MARL
- (7) LIMESTONE



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DADE CITY, FLORIDA

FIGURE
31

SSA-32

(Solid-stem Auger Boring)

PROJECT

NUMBER: 99.0331.007
NAME: Enterprise Recycling
 & Disposal Facility
LOCATION: Dade City, FL








BORING

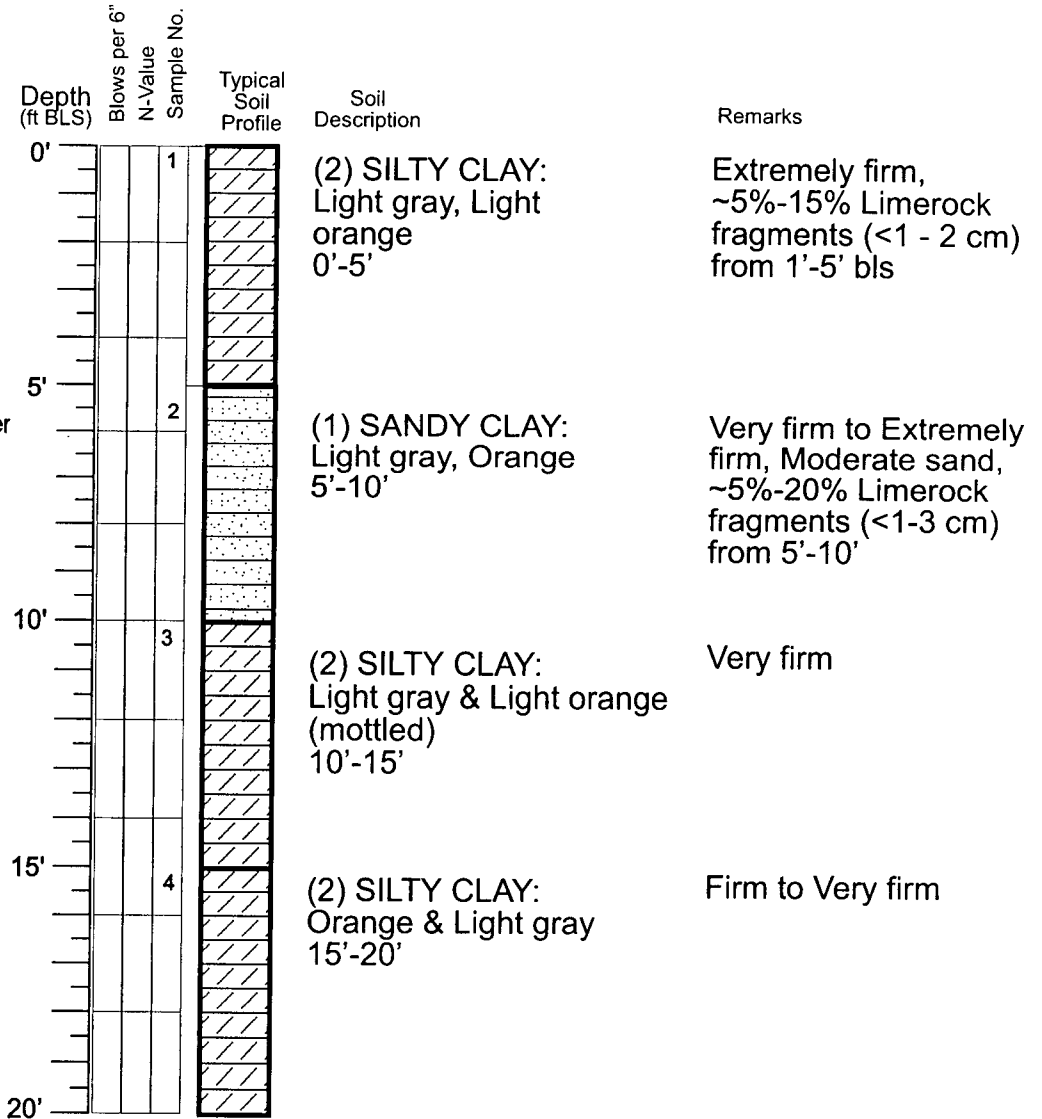
GR. ELEV: 75.03' NGVD
DIA-TYPE: 3"-Solid Stem Auger
DEPTH: 20' BLS
DATE STARTED: 9-3-03
DATE ENDED: 9-3-03

DRILLING

RIG TYPE: CME 45
CREW: Amdrill

LEGEND

- (1) SANDY CLAY 
- (2) SILTY CLAY 
- (3) CLAY 
- (4) CLAYEY SAND 
- (5) SILTY SAND 
- (6) LS MARL 
- (7) LIMESTONE 



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STRATIGRAPHIC COLUMN
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DADE CITY, FLORIDA

FIGURE
32

SSA-33

(Solid-stem Auger Boring)

PROJECT

NUMBER: 99.0331.007
NAME: Enterprise Recycling
 & Disposal Facility
LOCATION: Dade City, FL

BORING

GR. ELEV: 75.26' NGVD
DIA-TYPE: 3"-Solid Stem Auger
DEPTH: 20' BLS
DATE STARTED: 9-3-03
DATE ENDED: 9-3-03

DRILLING

RIG TYPE: CME 45
CREW: Amdrill

LEGEND

| | |
|-----------------|--|
| (1) SANDY CLAY | |
| (2) SILTY CLAY | |
| (3) CLAY | |
| (4) CLAYEY SAND | |
| (5) SILTY SAND | |
| (6) LS MARL | |
| (7) LIMESTONE | |

| Depth
(ft BLS) | Blows per 6"
N-Value | Sample No. | Typical
Soil
Profile | Soil
Description | Remarks |
|-------------------|-------------------------|------------|----------------------------|--|---|
| 0' | | 1 | | (2) SILTY CLAY:
Light gray, Orange
0'-5' | Very firm, ~5%-10%
Limerock fragments
(<0.5 cm) from
0'-5' bls |
| 5' | | 2 | | (2) SILTY CLAY:
Light tan
5'-10' | Soft, ~20% - 30% hard
limestone fragments
($<1-3$ cm) from 5'-10' bls |
| 10' | | 3 | | (2) SILTY CLAY:
Light tan
10'-15' | Soft, ~20% - 30% hard
Limestone fragments
($<1-3$ cm) from 10'-15' bls |
| 15' | | 4 | | (2) SILTY CLAY:
Light tan & Light
gray 15'-20' | Soft to firm, ~20% - 30%
hard & soft limestone
fragments ($<1-3$ cm) from
15'-20' bls |
| 20' | | | | | |

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STRATIGRAPHIC COLUMN
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DADE CITY, FLORIDA

FIGURE
33

SSA-34

(Solid-stem Auger Boring)

PROJECT

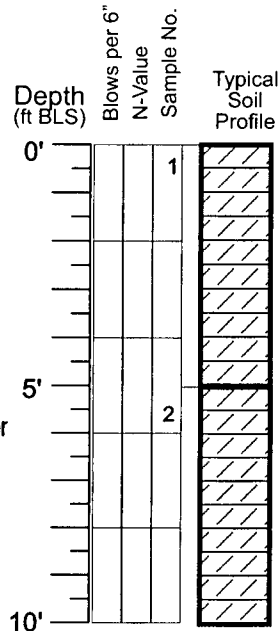
NUMBER: 99.0331.007
NAME: Enterprise Recycling
 & Disposal Facility
LOCATION: Dade City, FL

BORING

GR. ELEV: 74.93' NGVD
DIA-TYPE: 3"-Solid Stem Auger
DEPTH: 10' BLS
DATE STARTED: 9-3-03
DATE ENDED: 9-3-03

DRILLING

RIG TYPE: CME 45
CREW: Amdrill










(2) SILTY CLAY:
 Light gray, Orange,
 Dark brown (mottled)
 0'-10'

Remarks

Small amount of sand,
 Very firm, ~5%-10%
 hard & soft limerock
 fragments (<1-2 cm)
 from 4'-10' bls

LEGEND

- (1) SANDY CLAY 
- (2) SILTY CLAY 
- (3) CLAY 
- (4) CLAYEY SAND 
- (5) SILTY SAND 
- (6) LS MARL 
- (7) LIMESTONE 

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DADE CITY, FLORIDA

FIGURE
34

SSA-35

(Solid-stem Auger Boring)

PROJECT

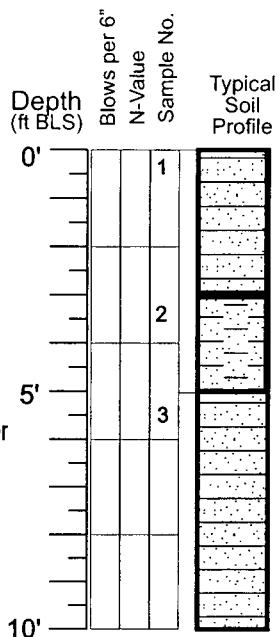
NUMBER: 99.0331.007
NAME: Enterprise Recycling
 & Disposal Facility
LOCATION: Dade City, FL

BORING

GR. ELEV: 75.27' NGVD
DIA-TYPE: 3"-Solid Stem Auger
DEPTH: 10' BLS
DATE STARTED: 9-4-03
DATE ENDED: 9-4-03

DRILLING

RIG TYPE: CME 45
CREW: Amdrill



(1) SANDY CLAY:
 Gray & Dark rust
 0'-3'

(4) CLAYEY SAND:
 Gray & Dark rust
 3'-5'

(1) SANDY CLAY:
 Light gray & Light orange,
 (mottled)
 5'-10'

Remarks

Small to moderate
 amount of sand (fine),
 Very firm
 0'-3'

Moderate amount of
 sand (fine), Very firm to
 Firm
 3'-5'

Small amount
 of sand, Very firm to
 Extremely firm
 5'-10'

LEGEND

| | |
|-----------------|--|
| (1) SANDY CLAY | |
| (2) SILTY CLAY | |
| (3) CLAY | |
| (4) CLAYEY SAND | |
| (5) SILTY SAND | |
| (6) LS MARL | |
| (7) LIMESTONE | |

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DADE CITY, FLORIDA

FIGURE
35

SSA-36

(Solid-stem Auger Boring)

PROJECT

NUMBER: 99.0331.007
NAME: Enterprise Recycling
 & Disposal Facility
LOCATION: Dade City, FL

BORING

GR. ELEV: 75.89' NGVD
DIA-TYPE: 3"-Solid Stem Auger
DEPTH: 20' BLS
DATE STARTED: 9-4-03
DATE ENDED: 9-4-03

DRILLING

RIG TYPE: CME 45
CREW: Amdrill

LEGEND

| | |
|-----------------|--|
| (1) SANDY CLAY | |
| (2) SILTY CLAY | |
| (3) CLAY | |
| (4) CLAYEY SAND | |
| (5) SILTY SAND | |
| (6) LS MARL | |
| (7) LIMESTONE | |

| Depth
(ft BLS) | Blows per 6"
N-Value | Sample No. | Typical
Soil
Profile | Soil
Description | Remarks |
|-------------------|-------------------------|------------|----------------------------|--|---|
| 0' | | 1 | | (2) SILTY CLAY:
Gray, Orange, Black
(mottled)
0'-5' | Very firm, ~5-10% Limerock
fragments (<1-2 cm) from
3.5'-5' |
| 5' | | 2 | | (2) SILTY CLAY:
Light orange
5'-10' | Firm to Semi firm from
5'-8.5'
Soft from 8.5'-10',
~5-10% Limerock
fragments (<1-2 cm) from
3.5'-10' |
| 10' | | 3 | | | |
| | | 4 | | (2) SILTY CLAY:
Light tan
10'-15' | Soft to Firm, ~10-25%
Limerock fragments
(<1-3 cm) from 10'-15' |
| 15' | | 5 | | (2) SILTY CLAY:
Gray, Light orange, Tan
15'-20' | Firm to very firm,
~20-35% Limerock
fragments from 18'-20' |
| 20' | | | | | |

N:\HYDROLARK\Boring logs\SSA-36.CDR



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DADE CITY, FLORIDA

FIGURE
36

SSA-37

(Solid-stem Auger Boring)

PROJECT

NUMBER: 99.0331.007
NAME: Enterprise Recycling
 & Disposal Facility
LOCATION: Dade City, FL

BORING

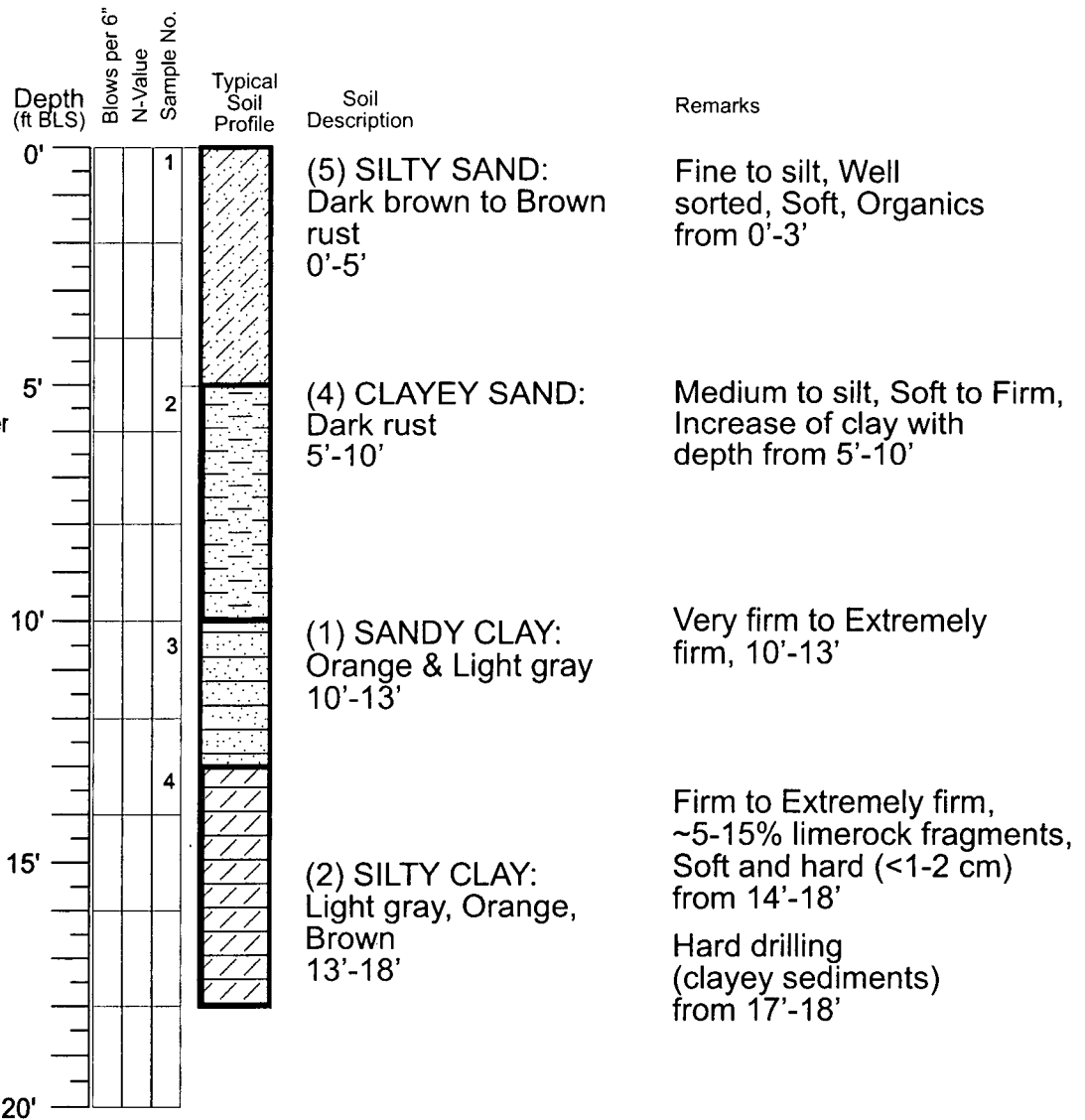
GR. ELEV: 103' NGVD
DIA-TYPE: 3"-Solid Stem Auger
DEPTH: 18' BLS
DATE STARTED: 9-4-03
DATE ENDED: 9-4-03

DRILLING

RIG TYPE: CME 45
CREW: Amdrill

LEGEND

| | |
|-----------------|--|
| (1) SANDY CLAY | |
| (2) SILTY CLAY | |
| (3) CLAY | |
| (4) CLAYEY SAND | |
| (5) SILTY SAND | |
| (6) LS MARL | |
| (7) LIMESTONE | |



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DADE CITY, FLORIDA

FIGURE
37

Bucket Auger Borings
Enterprise Recycling and Disposal Facility
Dade City, Florida

| Boring Name | Date of Boring | Cell # or Location | Total Depth, Ft BLS | Surface Elevation, Ft NGVD | Top of Confining Material Elevation, Ft NGVD | Clay Thickness, Ft | Sieve Test Elevation | % Passing No. 200 |
|---|----------------|--------------------|---------------------|----------------------------|--|--------------------|----------------------|-------------------|
| AS-1* | 7/16/03 | Cell 1 | 3 | 83.29 | 83.29 | 3 | 83 to 80 | 68.1 |
| AS-2* | 7/16/03 | Cell 1 | 3 | 82.49 | 82.49 | 3 | 88 to 79 | 56.5 |
| AS-3 | 7/16/03 | Cell 1 | 4 | 83.23 | 83.23 | 4 | 82 to 80 | 59.8 |
| | | | | | | | 80 to 78 | 56.6 |
| AS-4 | 7/16/03 | Cell 1 | 4 | 81.49 | 81.49 | 0.75 | 81 to 77 | 41.0 |
| AS-5* | 7/16/03 | Cell 1 | 2.5 | 80.90 | 80.90 | 1 | 81 to 78.5 | 52.5 |
| AS-6 | 7/16/03 | Cell 1 | 3 | 81.10 | 81.10 | 3 | 81 to 78 | 31.7 |
| AS-7* | 7/17/03 | Cell 1 | 3 | 81.03 | 81.03 | 3 | 81 to 78 | 56.7 |
| AS-8 | 7/17/03 | Cell 1 | 3.25 | 80.64 | 80.64 | 3.25 | 80 to 76.75 | 44.0 |
| AS-9* | 7/17/03 | Cell 1 | 3.25 | 80.30 | 80.30 | 3.25 | 80 to 76.75 | 60.7 |
| AS-10 | 7/17/03 | Cell 1 | 3 | 80.33 | NA | 0 | 80 to 79 | 16.4 |
| AS-11* | 7/17/03 | Cell 1 | 3 | 81.04 | 81.04 | 3 | 81 to 78 | 60.9 |
| AS-12 | 7/17/03 | Cell 1 | 3 | 80.29 | 80.29 | 3 | 80 to 79 | 52.6 |
| AS-13 | 7/23/03 | Cell 1 | 3 | 82.74 | 82.74 | 3 | 82 to 79 | 72.6 |
| AS-14* | 7/23/03 | Cell 1 | 2 | 81.77 | 81.77 | 1.5 | 81 to 79 | 49.3 |
| AS-15* | 7/23/03 | Cell 1 | 3 | 82.60 | 82.60 | 3 | 82 to 79 | 54.9 |
| AS-16 | 7/23/03 | Cell 1 | 3 | 80.55 | 80.55 | 3 | 80 to 77 | 62.7 |
| AS-17 | 7/23/03 | Cell 1 | 3 | 80.89 | 80.89 | 3 | 81 to 78 | 64.0 |
| AS-18 | 7/23/03 | Cell 1 | 3 | 81.02 | 80.02 | 1 | 80 to 78 | 48.9 |
| AS-19 | 7/23/03 | Cell 1 | 3 | 81.04 | 81.04 | 3 | 81 to 78 | 34.9 |
| AS-20* | 7/23/03 | Cell 1 | 3 | 81.05 | 81.05 | 3 | 81 to 78 | 47.0 |
| AS-21 | 7/23/03 | Cell 1 | 3 | 80.43 | 80.43 | 3 | 80 to 77 | 57.1 |
| AS-22 | 7/23/03 | Cell 1 | 3 | 79.98 | 79.98 | 3 | 80 to 77 | 67.8 |
| AS-23 | 7/23/03 | Cell 1 | 3 | 79.93 | 79.93 | 3 | 80 to 77 | 53.9 |
| AS-24 | 7/23/03 | Cell 1 | 3 | 79.88 | 79.88 | 3 | 80 to 77 | 62.2 |
| AS-25* | 7/23/03 | Cell 1 | 3 | 80.31 | 80.31 | 3 | 80 to 77 | 79.8 |
| AS-26 | 7/25/03 | Cell 1 | 3 | 96.25 | 96.25 | 3 | 96 to 93 | 29.8 |
| AS-27* | 7/25/03 | Cell 1 | 3 | 92.18 | NA | 0 | 92 to 89 | 25.0 |
| AS-28 | 7/25/03 | Cell 1 | 3 | 86.57 | 86.57 | 3 | 86 to 83 | 37.5 |
| AS-29* | 7/25/03 | Cell 1 | 3 | 84.98 | 84.98 | 3 | 85 to 82 | 47.2 |
| AS-30 | 7/25/03 | Cell 1 | 3 | 85.03 | 85.03 | 3 | 85 to 82 | 42.6 |
| AS-31 | 7/25/03 | Cell 1 | 3 | 84.87 | NA | 0 | 84 to 81 | 25.2 |
| AS-32 | 7/25/03 | Cell 1 | 3 | 85.14 | 85.14 | 3 | 85 to 82 | 80.4 |
| AS-33 | 7/25/03 | Cell 1 | 2 | 85.67 (approx.) | 85.67 (approx.) | 2 | 85 to 83 | 55.8 |
| AS-34 | 7/25/03 | Cell 1 | 3 | 82.81 | 82.81 | 3 | 82 to 79 | 55.0 |
| AS-35* | 7/25/03 | Cell 1 | 3 | 85.62 | 85.62 | 3 | 85 to 82 | 74.3 |
| AS-36 | 8/8/03 | Cell 1 | 3 | 79.98 | 79.98 | 3 | 80 to 77 | 57.9 |
| AS-37* | 8/8/03 | Cell 1 | 3 | 79.96 | 79.96 | 3 | 80 to 77 | 45.8 |
| AS-38 | 8/8/03 | Cell 1 | 3 | 80.20 | 80.20 | 3 | 80 to 77 | 40.0 |
| AS-39 | 8/8/03 | Cell 1 | 3 | 79.96 | 79.96 | 3 | 80 to 77 | 54.6 |
| AS-40 | 8/8/03 | Cell 1 | 3 | 81.07 | 81.07 | 3 | 81 to 78 | 53.2 |
| AS-41* | 8/8/03 | Cell 1 | 3 | 81.13 | 81.13 | 3 | 81 to 78 | 76.6 |
| AS-42 | 8/8/03 | Cell 1 | 4 | 81.04 | 81.04 | 4 | 81 to 77 | 15.8 |
| AS-43 | 8/8/03 | Cell 1 | 3 | 81.73 | 81.73 | 3 | 81 to 78 | 65.4 |
| AS-44 | 8/8/03 | Cell 1 | 3 | 82.25 | 82.25 | 3 | 82 to 79 | 33.5 |
| AS-45 | 8/8/03 | Cell 1 | 3 | 81.26 | 81.26 | 3 | 81 to 78 | 72.6 |
| AS-46 | 8/8/03 | Cell 1 | 3 | 81.06 | 80.06 | 2 | 80 to 78 | 52.0 |
| AS-47* | 8/8/03 | Cell 1 | 3 | 81.29 | 81.29 | 3 | 81 to 78 | 52.8 |
| Bold indicates insufficient material at the boring location | | | | | | | | |

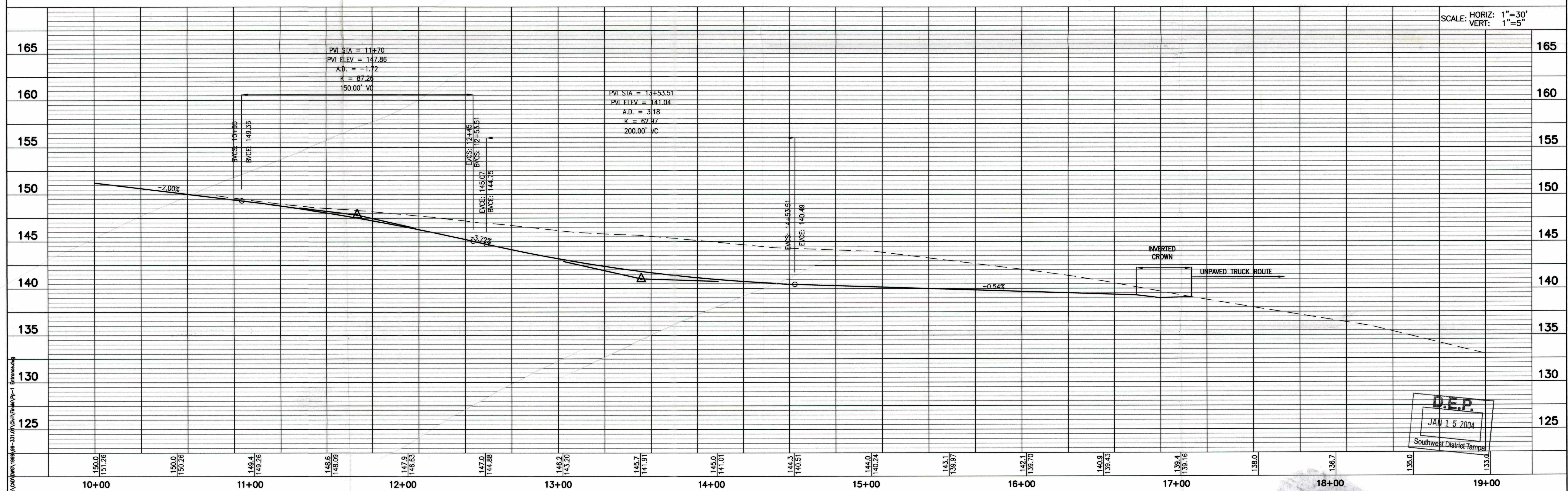
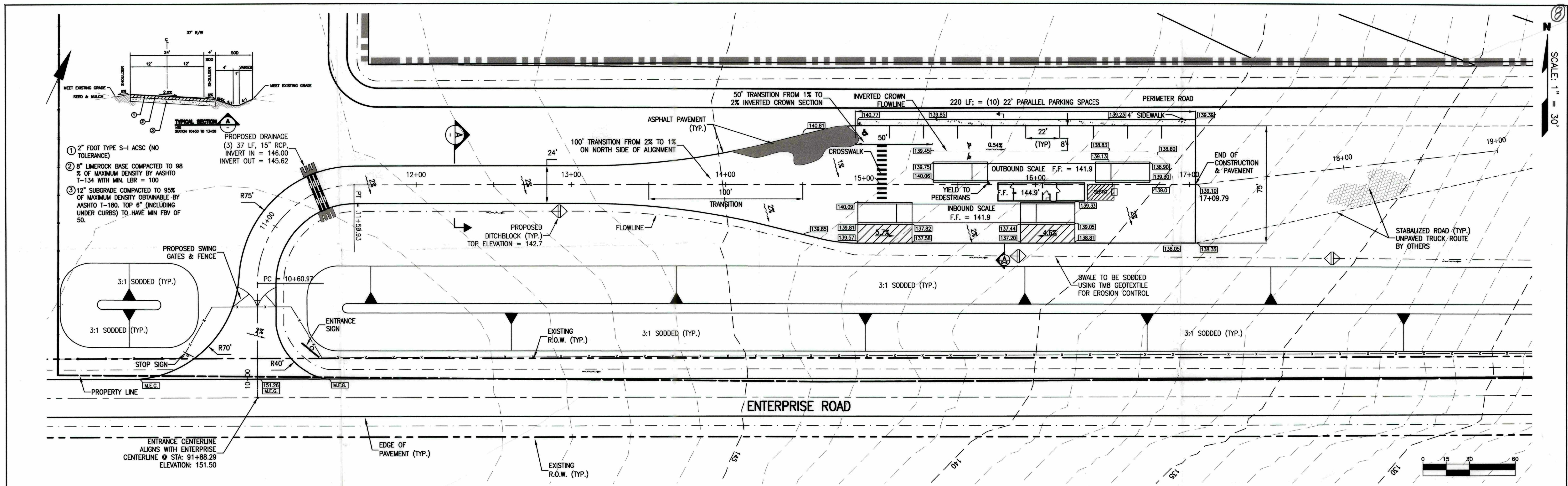
SPT Borings
Enterprise Recycling and Disposal Facility
Dade City, Florida

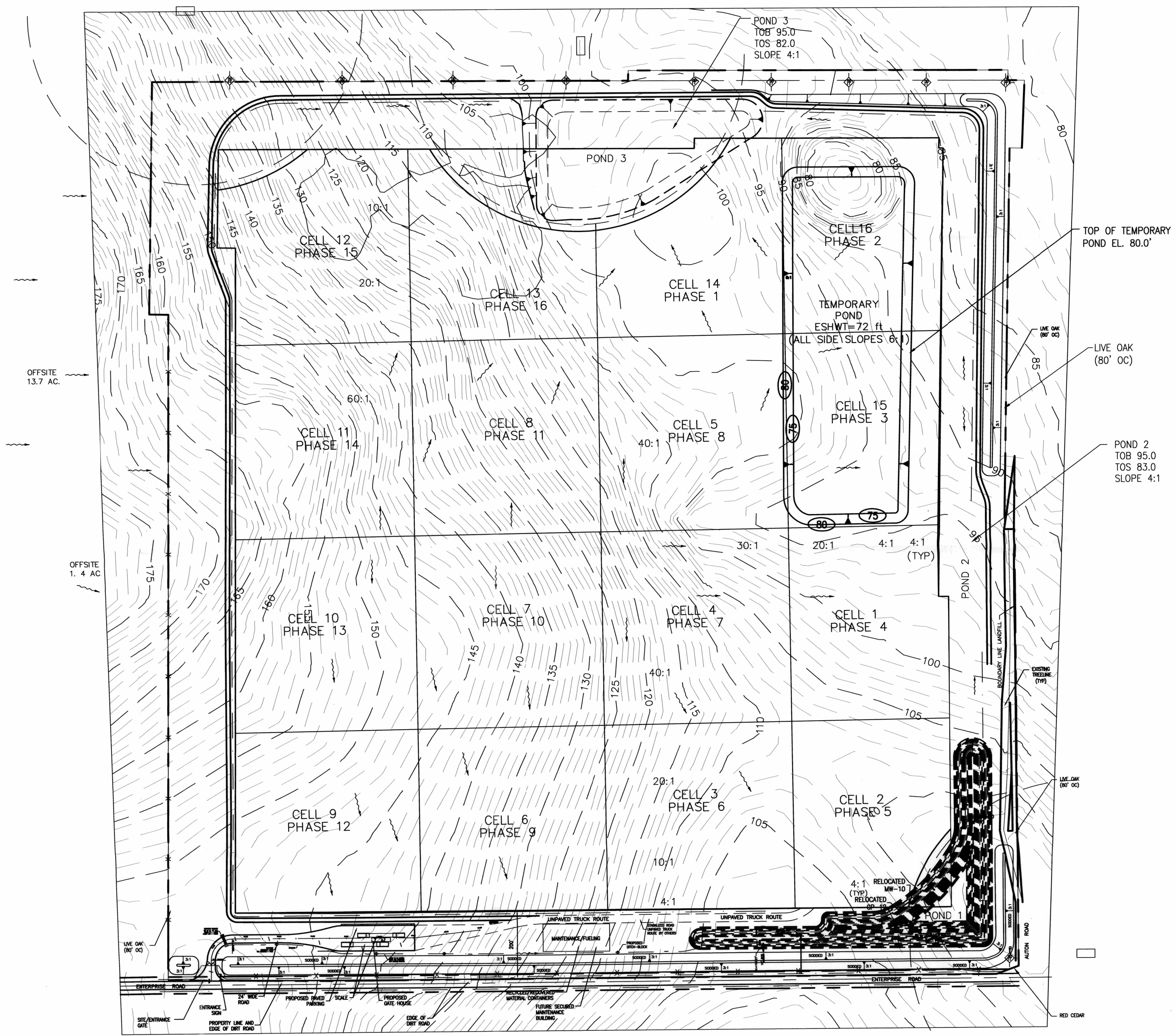
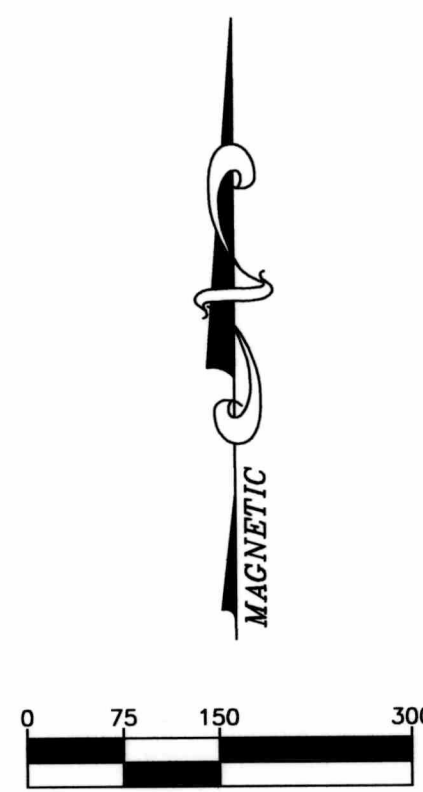
| Boring Name | Date of Boring | Cell # or Location | Total Depth, Ft BLS | Surface Elevation, Ft NGVD (Approx) | Top of Confining Material Elevation, Ft NGVD | Clay Thickness, Ft | ST Elevation, Ft NGVD | ST Sample | Permeability, cm/s | % Passing No. 200 |
|-------------------------------------|----------------|--------------------|---------------------|-------------------------------------|--|--------------------|-----------------------|-----------|--------------------|-------------------|
| B-1 | 4/7/03 | MW-1 | 60 | 115 | N/A | N/A | | | | |
| B-5* | 4/3/03 | MW-5 | 95 | 83 | 63 | 49 | | | | |
| B-6* | 4/3/03 | MW-6 | 35 | 88 | 78 | 21.5 | | | | |
| B-7* | 4/3/03 | MW-7A | 35 | 89 | 85.5 | 16.5 | | | | |
| B-8* | 4/4/03 | MW-8 | 45 | 97 | 88.8 | 30.5 | | | | |
| B-9* | 4/7/03 | MW-9 | 45 | 105 | 96.5 | 16.5 | | | | |
| B-10* | 4/7/03 | MW-10 | 43.5 | 108 | 99.5 | 6.5 | | | | |
| B-11* | 7/14/03 | MW-11 | 40 | 102 | 83.5 | 6.5 | | | | |
| B-15* | 8/22/03 | Cell 15 | 22 | 75.27 | 73.27 | 13 | 73 to 71 | ST-9 | 2.30E-08 | 42.2 |
| B-16* | 8/22/03 | Cell 1 | 20 | 80.38 | 80.38 | 12 | 74 to 72 | ST-10 | 5.20E-08 | 35.9 |
| B-17* | 8/23/03 | Cell 1 | 16 | 81.02 | 81.02 | 12 | 75 to 73 | ST-11 | 6.40E-09 | 45.7 |
| B-18* | 8/23/03 | Cell 1 | 8 | 83.43 | 83.43 | 7.5 | 79 to 77 | ST-12 | 2.40E-07 | 38.1 |
| B-19 | 8/26/03 | Cell 16 | 6 | 74.89 | 72.89 | 4 | 71 to 69 | ST-12A | 3.00E-09 | 47.4 |
| B-20* | 8/26/03 | Cell 16 | 16 | 75.23 | 69.23 | 6 | 67 to 65 | ST-13 | 4.90E-07 | 47.6 |
| B-21* | 8/26/03 | Cell 16 | 18 | 75.27 | 73.27 | 9 | 71 to 69 | ST-14 | 6.70E-08 | 52.3 |
| B-22* | 8/26/03 | Cell 16 | 46 | 75.18 | 34.18 | 4 | 49.5 to 47.5 | ST-15 | 5.20E-06 | 21.4 |
| B-22* | 8/26/03 | Cell 16 | 46 | 75.18 | 34.18 | 4 | 31 to 29 | ST-16 | 1.90E-07 | 57.2 |
| B-23* | 8/26/03 | Cell 16 | 20 | 75.00 | 73 | 18 | 73 to 71 | ST-17 | 6.90E-08 | 38.8 |
| B-24* | 8/27/03 | Cell 15 | 22 | 75.19 | 73.19 | 20 | 73 to 71 | ST-18 | 5.10E-08 | 67.6 |
| B-25* | 8/27/03 | Cell 15 | 22 | 75.62 | 73.62 | 20 | 71 to 69 | ST-19 | 4.90E-09 | 65.1 |
| B-26 | 8/28/03 | Cell 16 | 60 | 75.00 | 27 | 6 | | | | |
| B-27* | 8/29/03 | Cell 15 | 20 | 74.89 | 72.89 | 14.5 | 73 to 71 | ST-20 | 1.40E-07 | 39.8 |
| B-28* | 8/29/03 | Cell 14 | 40 | 96.00 | 64 | 8 | 64 to 62 | ST-21 | 7.50E-07 | 33.1 |
| B-29* | 8/29/03 | Cell 16 | 22 | 74.97 | 72.97 | 16 | | | | |
| B-30* | 9/2/03 | Cell 16 | 28 | 75.12 | 73.12 | 11.5 | | | | |
| B-31* | 9/2/03 | Cell 16 | 32 | 75.02 | 71.02 | 14 | | | | |
| B-32* | 9/3/03 | Cell 16 | 39.5 | 75.05 | 67.55 | 26.5 | | | | |
| B-33* | 9/4/03 | Cell 16 | 31 | 75.51 | 73.51 | 28 | | | | |
| B-34* | 9/4/03 | Cell 16 | 24 | 75.43 | 63.97 | 12.5 | | | | |
| DCL01-1 | 1/18/01 | Cell 6 | 45.5 | 122** | 83.5 | 2 | | | | |
| DCL01-2* | 1/18/01 | Cell 9 | 35.5 | 145** | 111.5 | 2 | | | | |
| DCL01-3 | 1/18/01 | Cell 10 | 40.5 | 150** | 116.5 | 7 | | | | |
| DCL01-4* | 1/18/01 | West of Cell 10 | 35.5 | 168** | 159.5 | 7 | | | | |
| DCL01-5 | 1/17/01 | Cell 11 | 50.5 | 142** | 93.5 | 2 | | | | |
| DCL01-6 | 1/17/01 | Cell 7 | 35.5 | 128** | 94.5 | 2 | | | | |
| DCL01-7 | 1/17/01 | Cell 8 | 75.5 | 112** | N/A | N/A | | | | |
| DCL01-8 | 1/17/01 | Cell 5 | 31 | 100** | 91.5 | 20 | | | | |
| DCL01-9 | 1/18/01 | Cell 4 | 40 | 104** | 79.5 | 2.5 | | | | |
| DCL01-10 | 1/18/01 | Cell 1 | 40.5 | 99** | N/A | N/A | | | | |
| DCL01-11 | 1/18/01 | Cell 3 | 30.5 | 106 | 92.5 | 16.5 | | | | |
| DCL01-12* | 1/18/01 | Cell 15 | 22.5 | 95** | 88.5 | 14 | | | | |
| DCL01-13 | 1/18/01 | Cell 15 | 35.5 | 94** | 85.5 | 21.5 | | | | |
| DCL01-14 | 1/19/01 | Cell 16 | 40.5 | 89** | 50.5 | 2 | | | | |
| B-1* | 3/3/00 | NW of Cell 10 | 95 | 170 | 159 | 9 | | | | |
| B-2* | 3/6/00 | North of Cell 7 | 76 | 136 | 136 | 15 | | | | |
| B-3 | 3/13/00 | North of Cell 13 | 55 | 95 | 79.5 | 10 | | | | |
| B-4* | 3/7/00 | Cell 14 | 50 | 98 | 87 | 29 | | | | |
| B-5 | 3/13/00 | Cell 16 | 66 | 83 | 52.5 | 30 | | | | |
| B-6* | 3/15/00 | NW of Cell 16 | 55 | 83 | 63 | 15 | | | | |
| B-7 | 3/15/00 | Cell 4 | 45 | 93 | 73 | 10 | | | | |
| B-8* | 3/16/00 | SE of Cell 15 | 50 | 93 | 63 | 14 | | | | |
| B-9* | 3/17/00 | SE of Cell 3 | 85 | 101 | 51 | 5 | | | | |
| B-10* | 3/16/00 | South of Cell 6 | 71.5 | 133 | 118 | 5 | | | | |
| * Limestone fragments in boring | | | | | | | | | | |
| ** Elevation based on NAD, not NGVD | | | | | | | | | | |

Solid Stem Auger Borings
Enterprise Recycling and Disposal Facility
Dade City, Florida

| Boring Name | Date of Boring | Cell # or Location | Total Depth, Ft BLS | Surface Elevation, Ft NGVD (Approx) | Top of Confining Material Elevation, Ft NGVD | Clay Thickness, Ft | Sieve Test Elevation | % Passing No. 200 |
|--|----------------|--------------------|---------------------|-------------------------------------|--|--------------------|----------------------|-------------------|
| SSA-1* | 7/30/03 | Cell 1 | 12 | 83 | 83 | 6 | N/A | N/A |
| SSA-2* | 7/30/03 | Cell 1 | 4 | 82.5 | 82.5 | 4 | N/A | N/A |
| SSA-3* | 7/30/03 | Cell 1 | 18 | 82.5 | 82.5 | 7 | N/A | N/A |
| SSA-4* | 7/30/03 | Cell 1 | 15 | 82.5 | 82.5 | 7 | 75.5 | 58.6 |
| SSA-5* | 7/30/03 | Cell 1 | 15 | 83 | 83 | 15 | 78 | 41.9 |
| SSA-6* | 7/30/03 | Cell 1 | 14 | 82.5 | 82.5 | 7 | 76.5 | 43.6 |
| SSA-7* | 7/31/03 | Cell 1 | 14 | 83 | 83 | 9 | 77 | 48.7 |
| SSA-8* | 7/31/03 | Cell 1 | 14 | 83 | 83 | 8 | 75 | 47.0 |
| SSA-9* | 7/31/03 | Cell 1 | 14 | 83 | 83 | 14 | 79 | 51.9 |
| SSA-10* | 7/31/03 | Cell 1 | 14 | 82.5 | 82.5 | 11 | 78.5 | 53.2 |
| SSA-11* | 7/31/03 | Cell 1 | 14 | 82 | 82 | 11 | 76 | 61.5 |
| | | | | | | | 73 | 57.5 |
| | | | | | | | 71 | 47.8 |
| | | | | | | | 68 | 42.4 |
| SSA-12* | 7/31/03 | Cell 1 | 14 | 82 | 82 | 14 | 76 | 44.8 |
| SSA-13 | 7/31/03 | Cell 1 | 24 | 95 | 95 | 24 | 71 | 21.0 |
| SSA-14* | 7/31/03 | Cell 1 | 29 | 99 | 99 | 24 | 75 | 36.9 |
| | | | | | | | 70 | 27.7 |
| SSA-15* | 7/31/03 | Cell 1 | 24 | 103 | 103 | 24 | 79 | 42.3 |
| SSA-16 | 7/31/03 | Cell 1 | 24 | 104 | 104 | 24 | 84 | 48.9 |
| | | | | | | | 80 | 57.6 |
| SSA-17* | 7/31/03 | Cell 1 | 29 | 107 | 97 | 19 | 83 | 16.1 |
| | | | | | | | 78 | 25.3 |
| SSA-18 | 7/31/03 | Cell 1 | 29 | 108 | 108 | 29 | 79 | 63.0 |
| SSA-19* | 8/23/03 | Cell 1 | 20 | 83 | 83 | 20 | 78 to 73 | 57.1 |
| SSA-20 | 8/23/03 | Cell 1 | 25 | 84 | 79 | 18 | 79 to 74 | 29.4 |
| SSA-21* | 8/23/03 | Cell 1 | 16 | 82 | 82 | 16 | 82 to 77 | 58.5 |
| SSA-22* | 8/23/03 | Cell 1 | 20 | 82 | 82 | 20 | 82 to 77 | 44.9 |
| SSA-23* | 8/23/03 | Cell 1 | 20 | 80 | 80 | 20 | 80 to 75 | 41.2 |
| SSA-24* | 8/23/03 | Cell 1 | 20 | 80 | 80 | 20 | 80 to 75 | 64.3 |
| SSA-25 | 9/3/03 | Cell 15 | 10 | 75 | 75 | 8 | 75 to 70 | 42.2 |
| SSA-26 | 9/3/03 | Cell 16 | 20 | 75 | 72 | 17 | 70 to 65 | 28.6 |
| SSA-27 | 9/3/03 | Cell 16 | 20 | 75 | N/A | 0 | 65 to 60 | 10.7 |
| SSA-28* | 9/3/03 | Cell 16 | 20 | 75 | 70 | 15 | 70 to 65 | 49.2 |
| SSA-29* | 9/3/03 | Cell 16 | 20 | 76 | 71 | 15 | 71 to 66 | 39.5 |
| SSA-30* | 9/4/03 | Cell 16 | 14 | 75 | 73 | 12 | 75 to 70 | 51.7 |
| SSA-31* | 9/3/03 | Cell 15 | 20 | 75 | 73 | 18 | 75 to 70 | 47.0 |
| SSA-32* | 9/3/03 | Cell 15 | 20 | 75 | 73 | 18 | 75 to 70 | 58.3 |
| SSA-33* | 9/3/03 | Cell 15 | 20 | 75 | 73 | 18 | 75 to 70 | 52.6 |
| SSA-34* | 9/3/03 | Cell 15 | 10 | 75 | 73 | 8 | 75 to 70 | 49.1 |
| SSA-35 | 9/4/03 | Cell 15 | 10 | 75 | 73 | 8 | 75 to 70 | 36.1 |
| SSA-36* | 9/4/03 | Cell 16 | 20 | 75 | 73 | 18 | 75 to 70 | 54.2 |
| SSA-37* | 9/4/03 | Cell 1 | 18 | 103 | 98 | 13 | 93 to 88 | 53.3 |
| * Limestone fragments in boring | | | | | | | | |
| Bold indicates insufficient material at boring location | | | | | | | | |


APPENDIX F





H:\CADD\DWG\1989\09-331\01 Civil\Plan\Entrance-2.dwg

Plotted: Jan 14, 2004 - 4:29pm by RCC

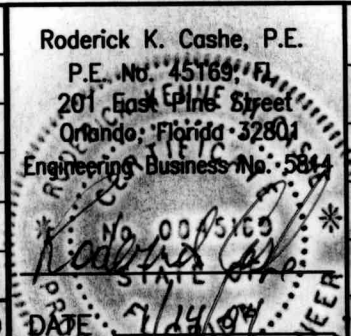


HARTMAN & ASSOCIATES, INC.
engineers, hydrogeologists, surveyors & management consultants
201 EAST PINE STREET - SUITE 1000 - ORLANDO, FL 32801
TELEPHONE (407) 839-3955 - FAX (407) 839-3790
ENGINEERING BUSINESS No. 5814

**ENTERPRISE LANDFILL
PASCO COUNTY
FLORIDA**

**ENTRANCE ROAD
& TRUCK ROUTE**

| DATE | REVISIONS | REVISED | CHECKED |
|------|-----------|---------|---------|
| | | | |
| | | | |
| | | | |
| | | | |



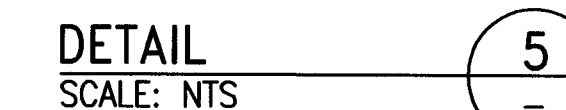
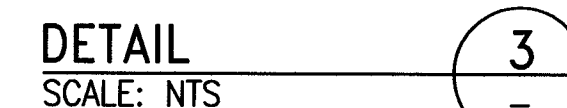
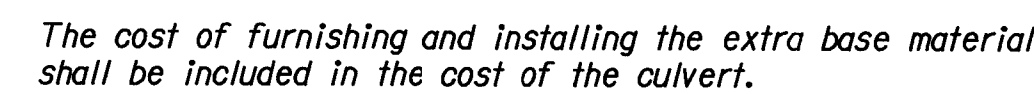
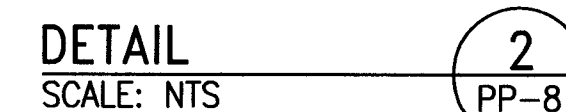
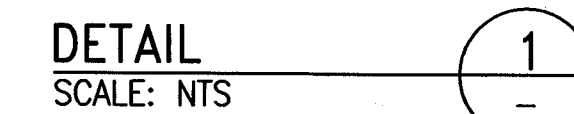
D.E.P.
JAN 15 2004
Southwest District Tampa

| | | |
|-------------|-----|------|
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| DRAWN | RCC | 6/03 |
| CHECKED | RKC | 7/03 |
| QC APPROVAL | RKC | 7/03 |

FILE: Entrance-2.dwg

| | |
|-------------|-------------|
| PROJECT NO. | 99.0331.001 |
| SCALE | 1" = 150' |
| C-1 | |
| SHEET | 2 of 4 |

Hartman & Associates, Inc.
201 E. Pine Street, Suite 1000
Orlando, FL 32801
Engineering Business No. 5814
Jennifer S. Smith, P.E.
Florida Registration No. 0000058592



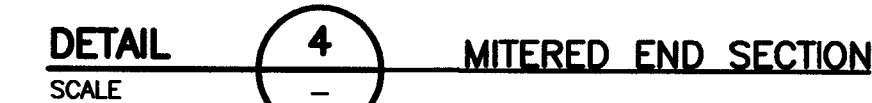
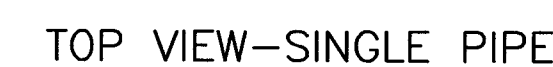
B E

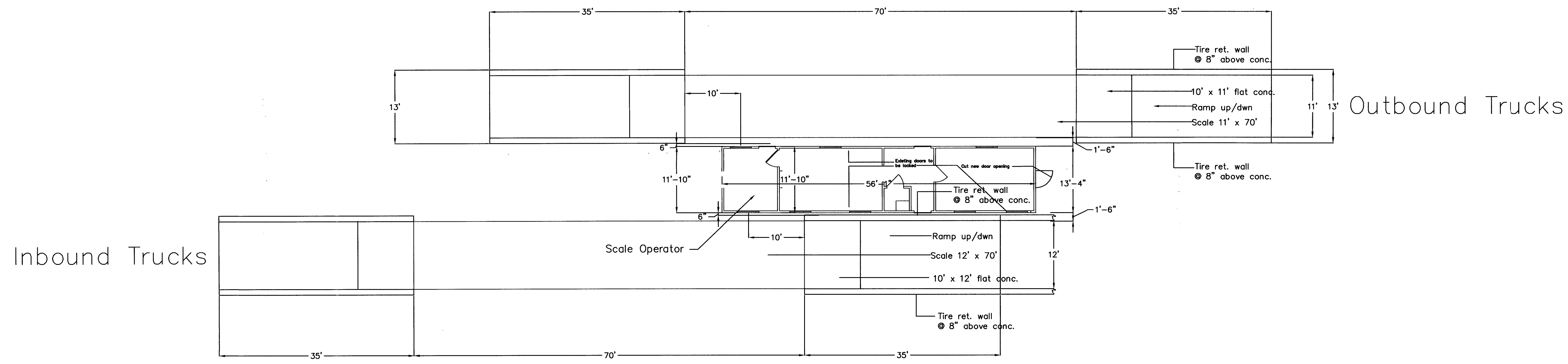
△ 6.42' △ 6.25' DIMENSIONS PERMITTED TO ALLOW USE OF 8" STANDARD PIPE LENGTHS.

◇ 10.40' ◇ 10.10' DIMENSIONS PERMITTED TO ALLOW USE OF 12" STANDARD PIPE LENGTHS.

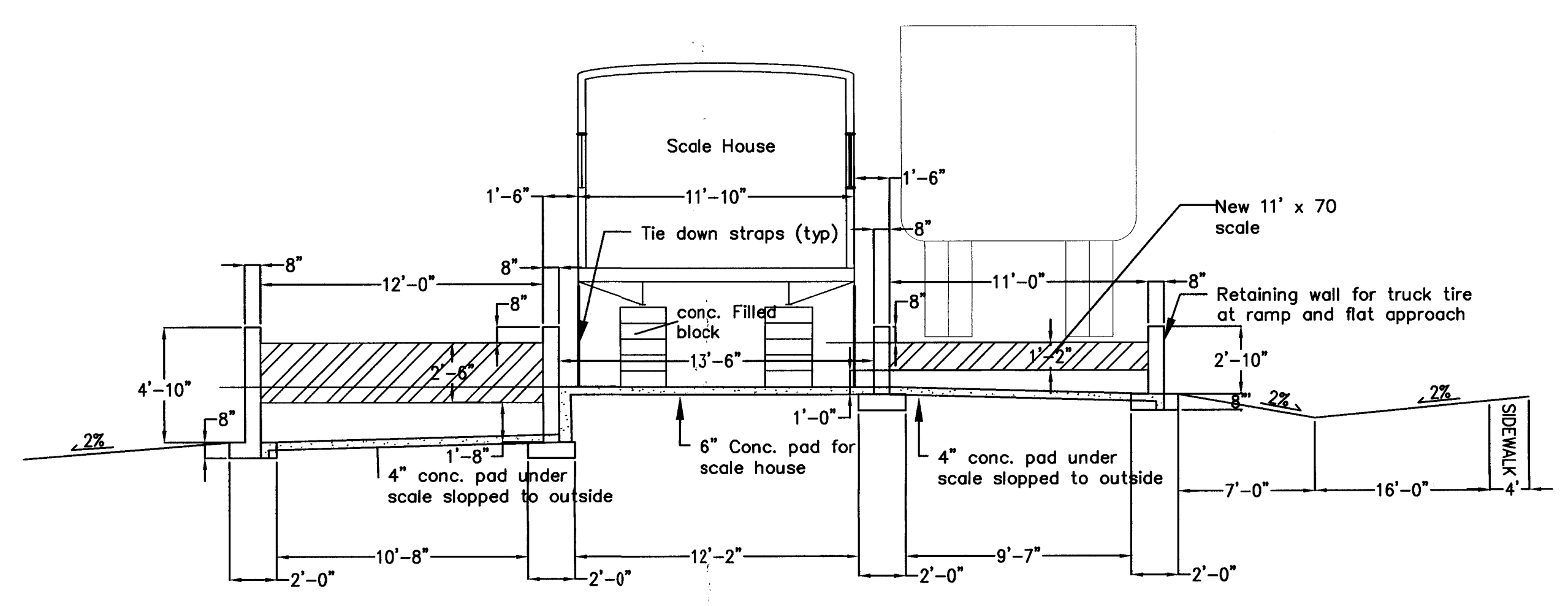
△ ◇ CONCRETE SLAB SHALL BE DEEPENED TO FORM BRIDGE ACROSS CROWN OF PIPE. SEE SECTION BELOW.

CONCRETE SLAB SHALL BE DEEPENED TO FORM BRIDGE ACROSS CROWN OF PIPE. SEE SECTION BELOW.

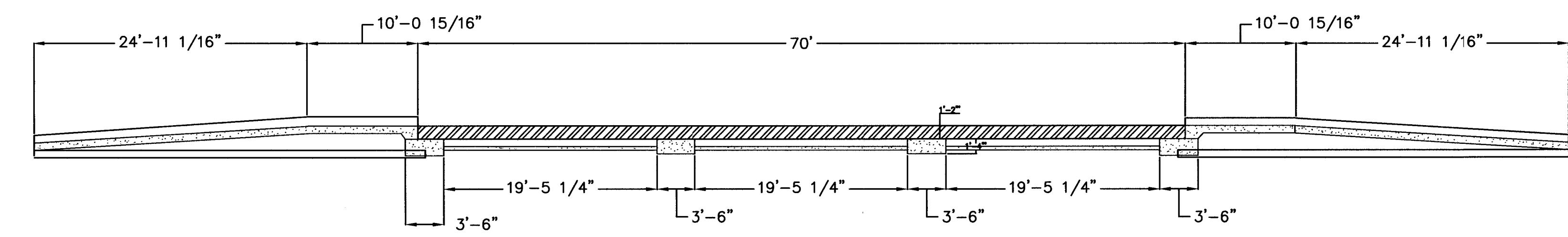




PLAN VIEW
SCALE: N.T.S.

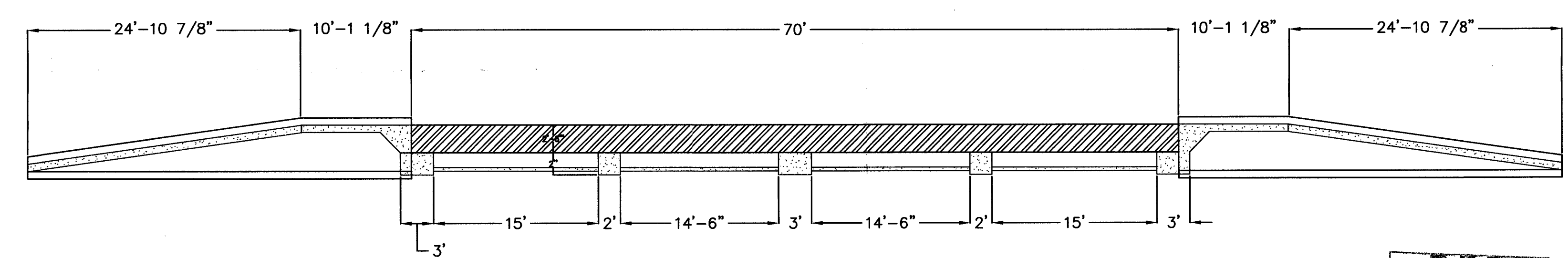


SECTION A
N.T.S.



INBOUND SCALE
(NEW SCALE)

DETAIL 6
N.T.S.



OUTBOUND SCALE
(USED SCALE)

DETAIL 7
N.T.S.

D.E.P.
JAN 15 2004
Southwest District Tampa

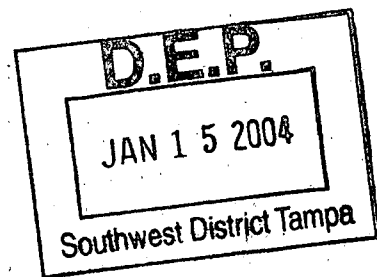
N:\040\DWG\1999\99-331\01\Civil\Final\0-2.DWG
Plotted: Jan 14, 2004 - 11:30am by ROC

HARTMAN & ASSOCIATES, INC.
engineers, hydrogeologists, surveyors & management consultants
201 EAST PINE STREET - SUITE 1000 - ORLANDO, FL 32801
TELEPHONE (407) 839-3955 - FAX (407) 839-3790
ENGINEERING BUSINESS No. 5814

**ENTERPRISE LANDFILL
PASCO COUNTY
FLORIDA**

SCALE DETAILS

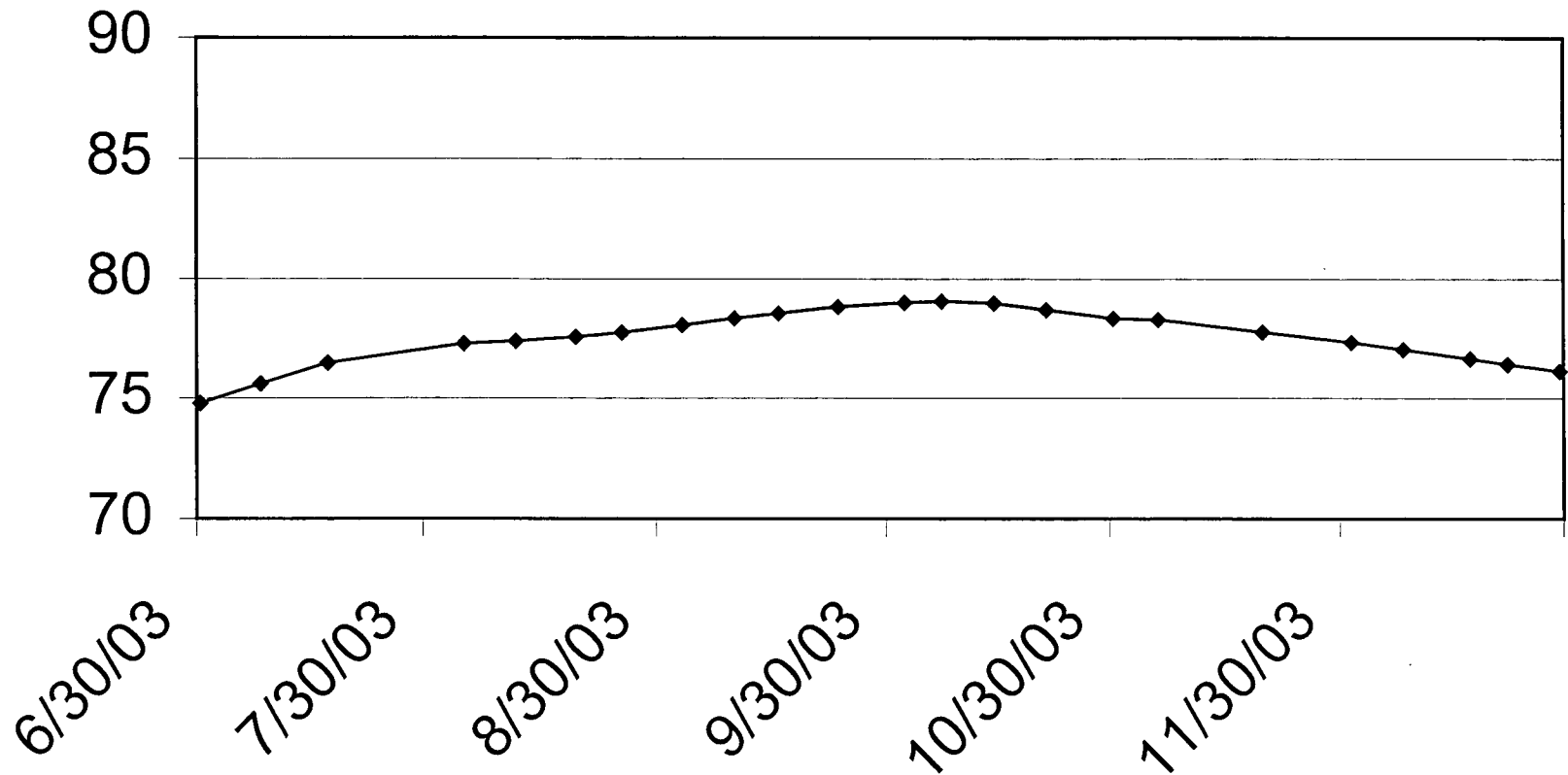
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|------|-----------|---------|---------|-------------|--------|----|------|----------------|
| DATE | REVISIONS | REVISED | CHECKED | DESIGNED | OTHERS | BY | DATE | PROJECT NO. |
| | | | | DRAWN | OTHERS | | | 99.0331.001 |
| | | | | CHECKED | OTHERS | | | SCALE AS SHOWN |
| | | | | OC APPROVAL | OTHERS | | | D-2 |
| | | | | FILE | | | | SHEET 4 OF 4 |



APPENDIX G

MW-1

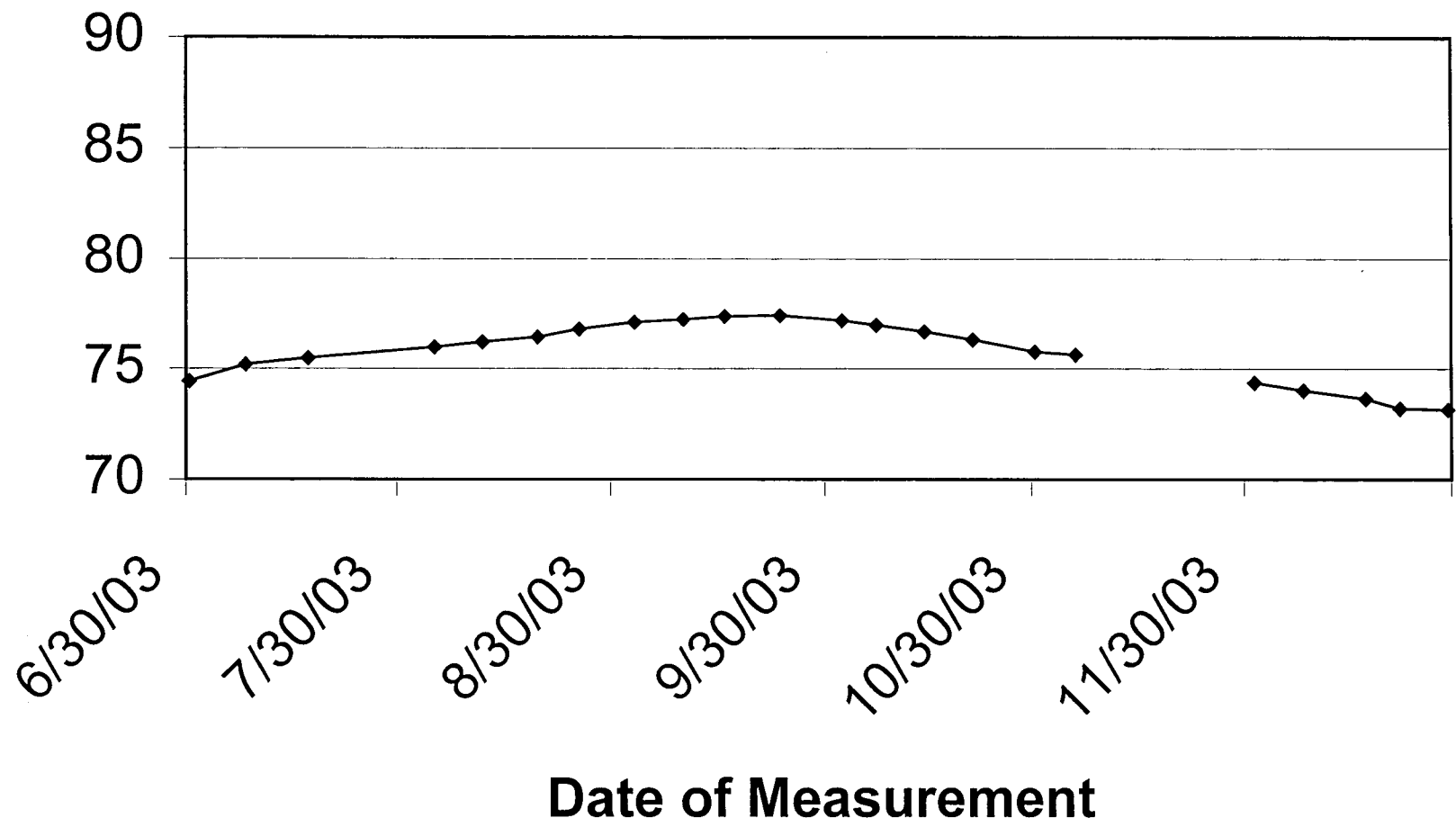
Water Level, (Ft NGVD)



Date of Measurement

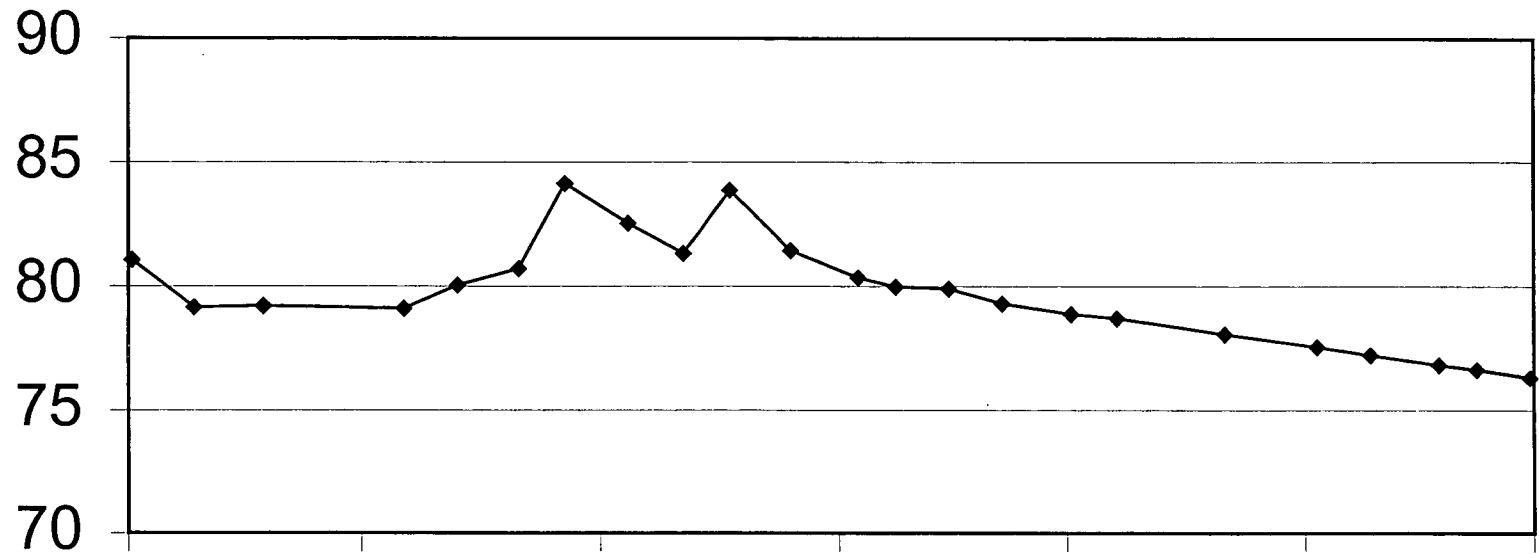
MW-1B

Water Level (ft, NGVD)

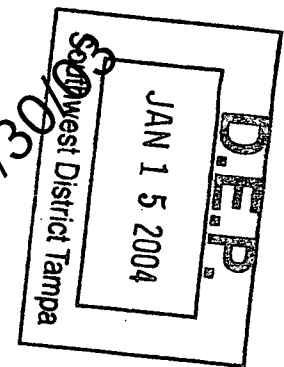


MW-5A

Water Level (ft, NGVD)

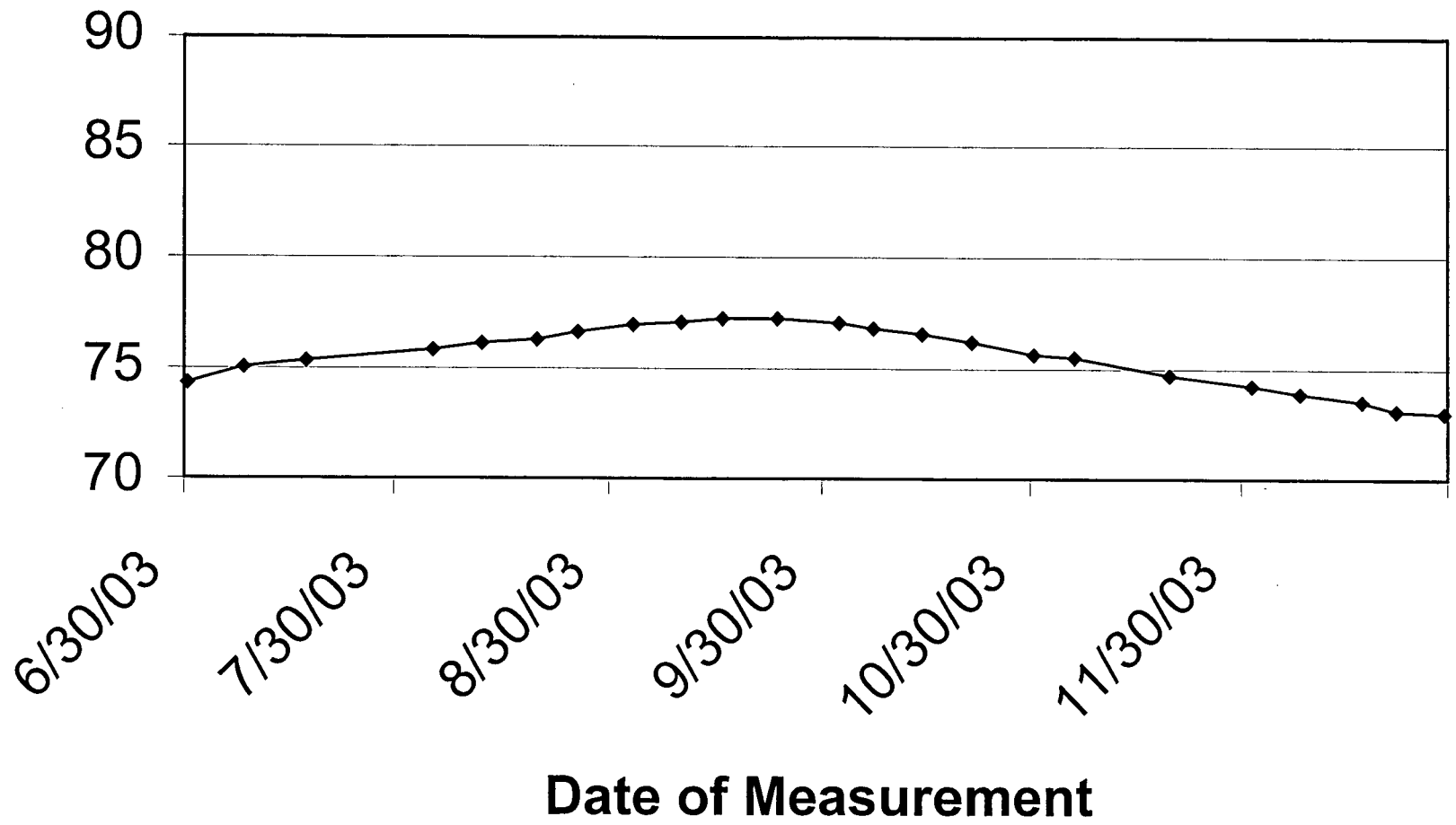


Date of Measurement

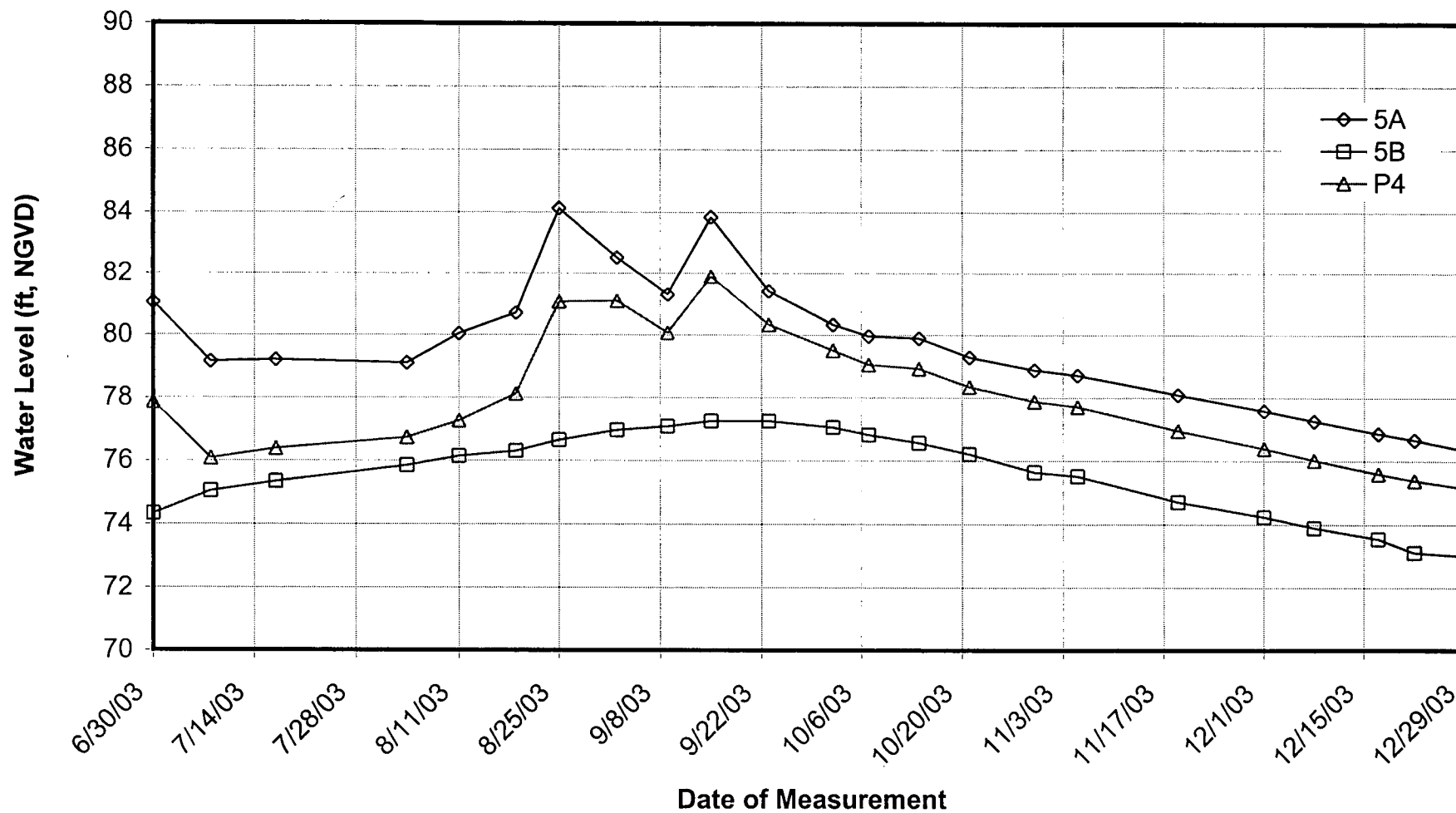


MW-5B

Water Level (ft, NGVD)

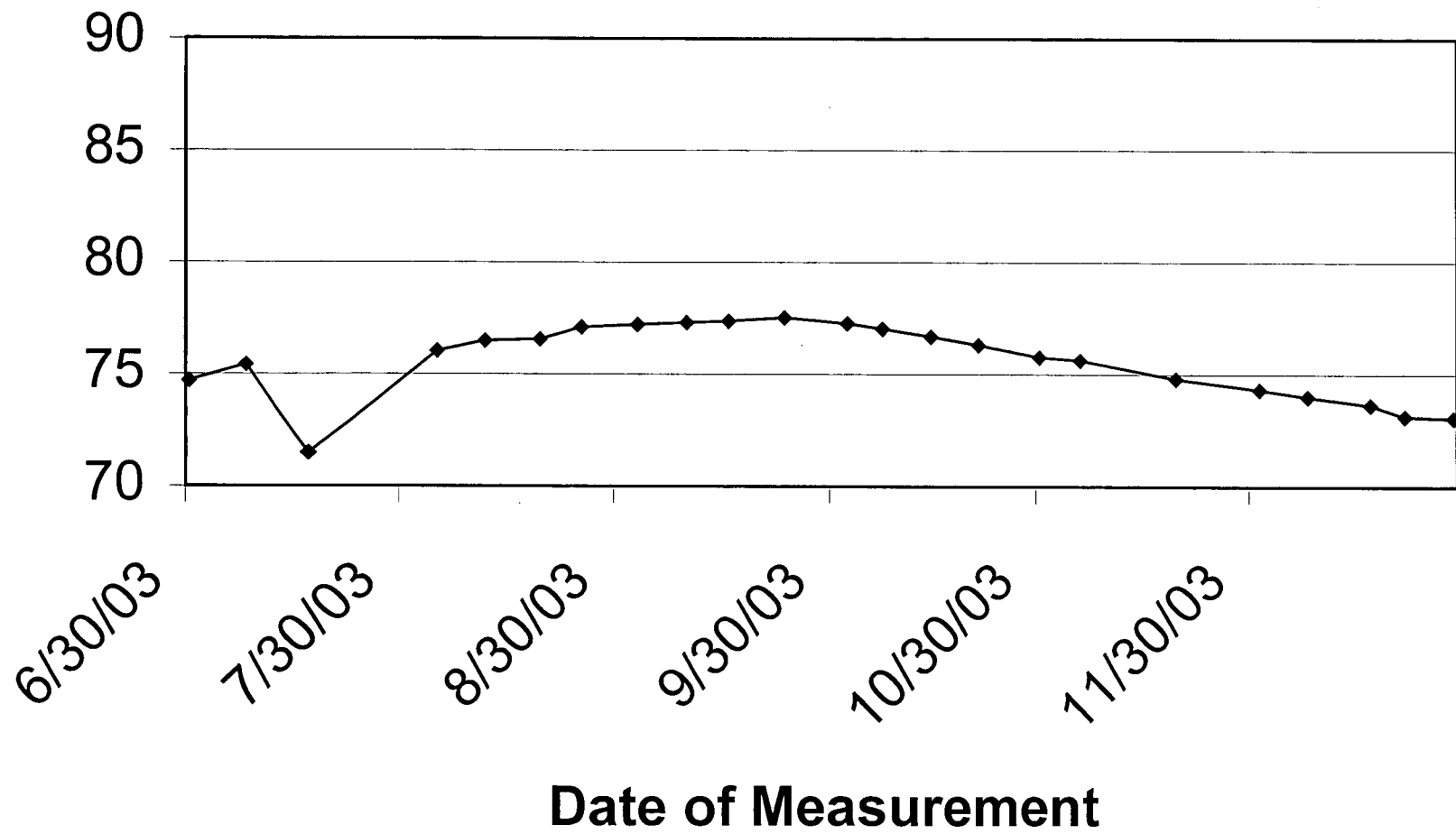


MW-5A & B

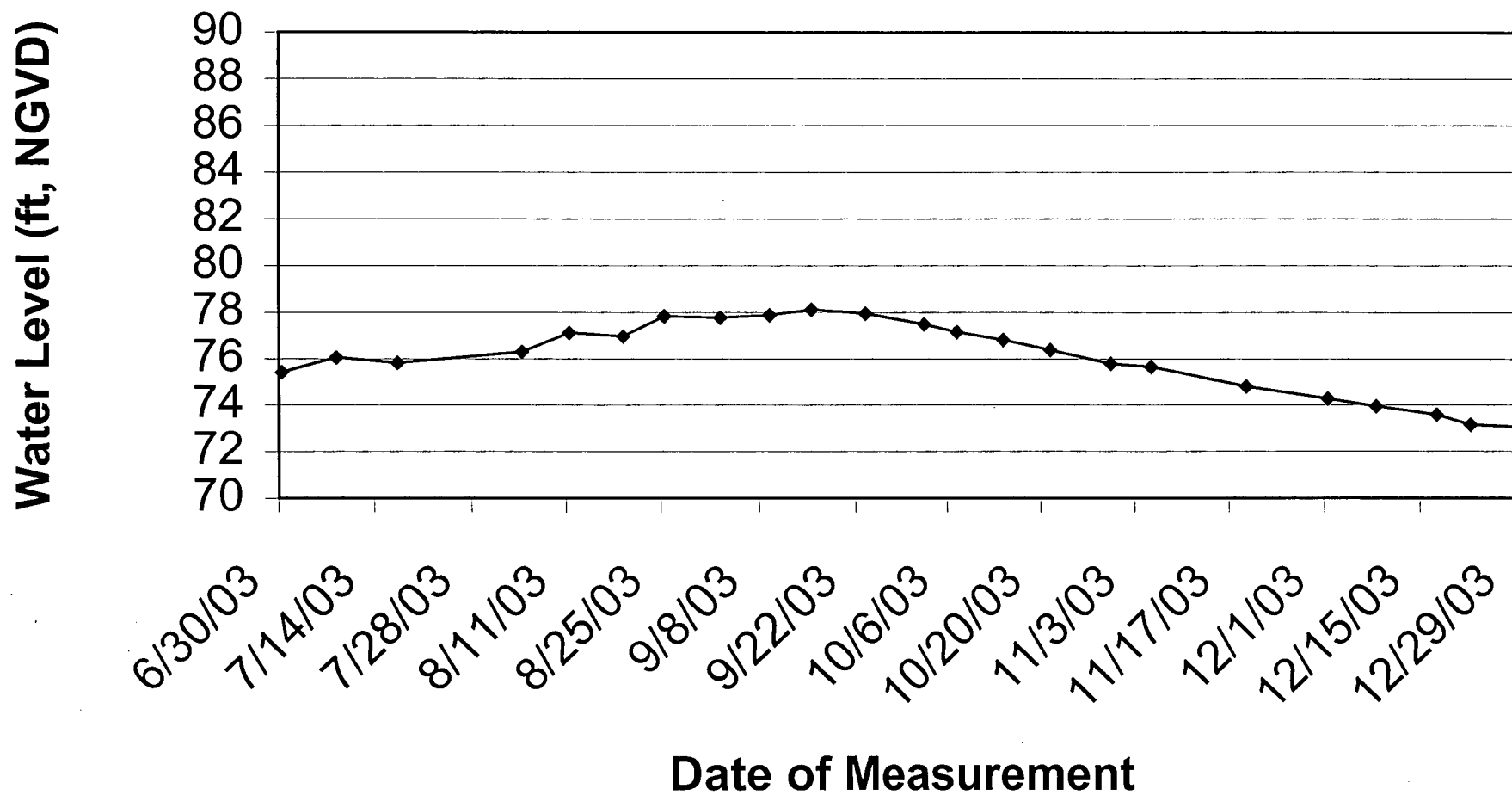


MW-6

Water Level (Ft, NGVD)

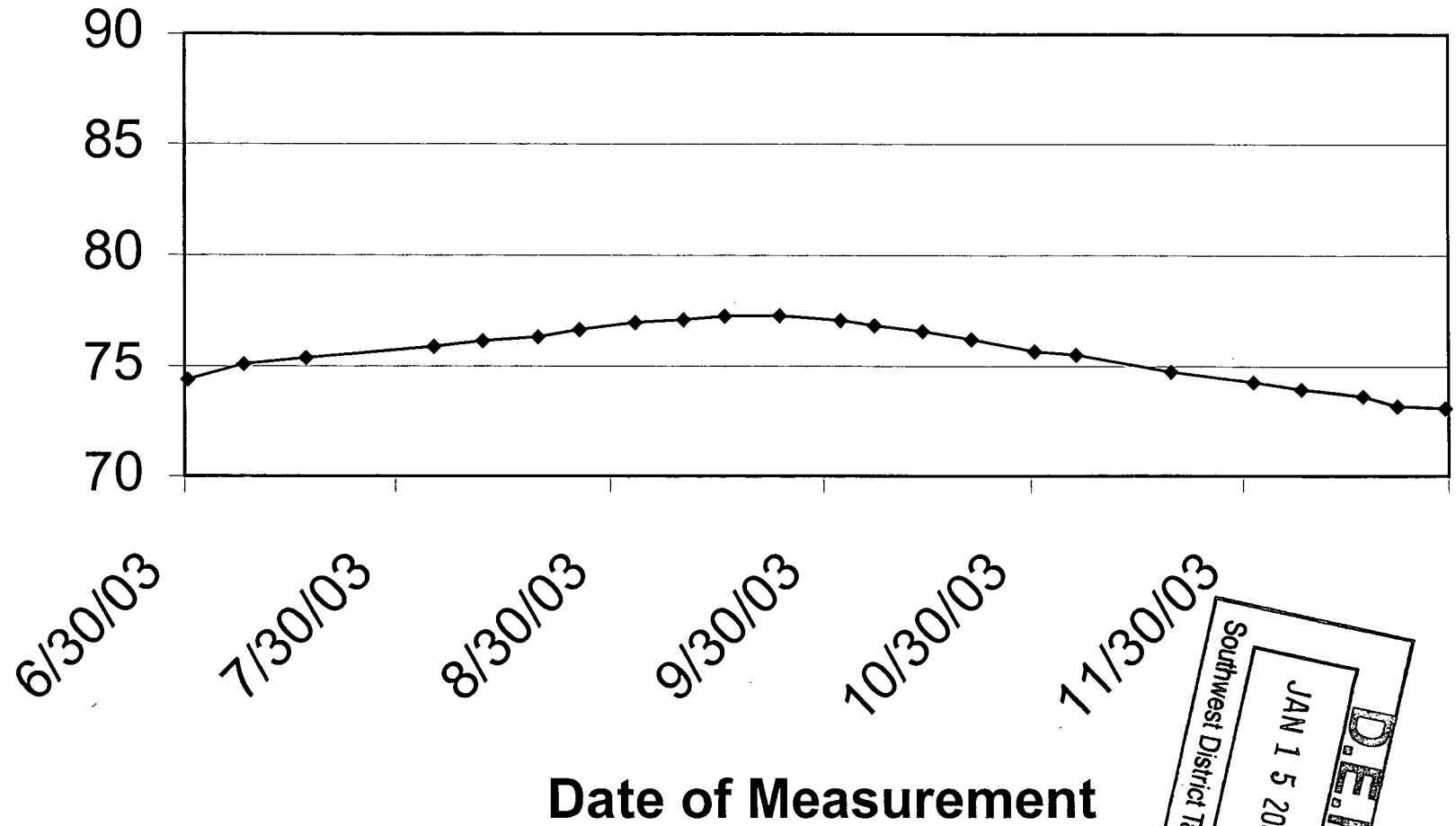


MW-7A



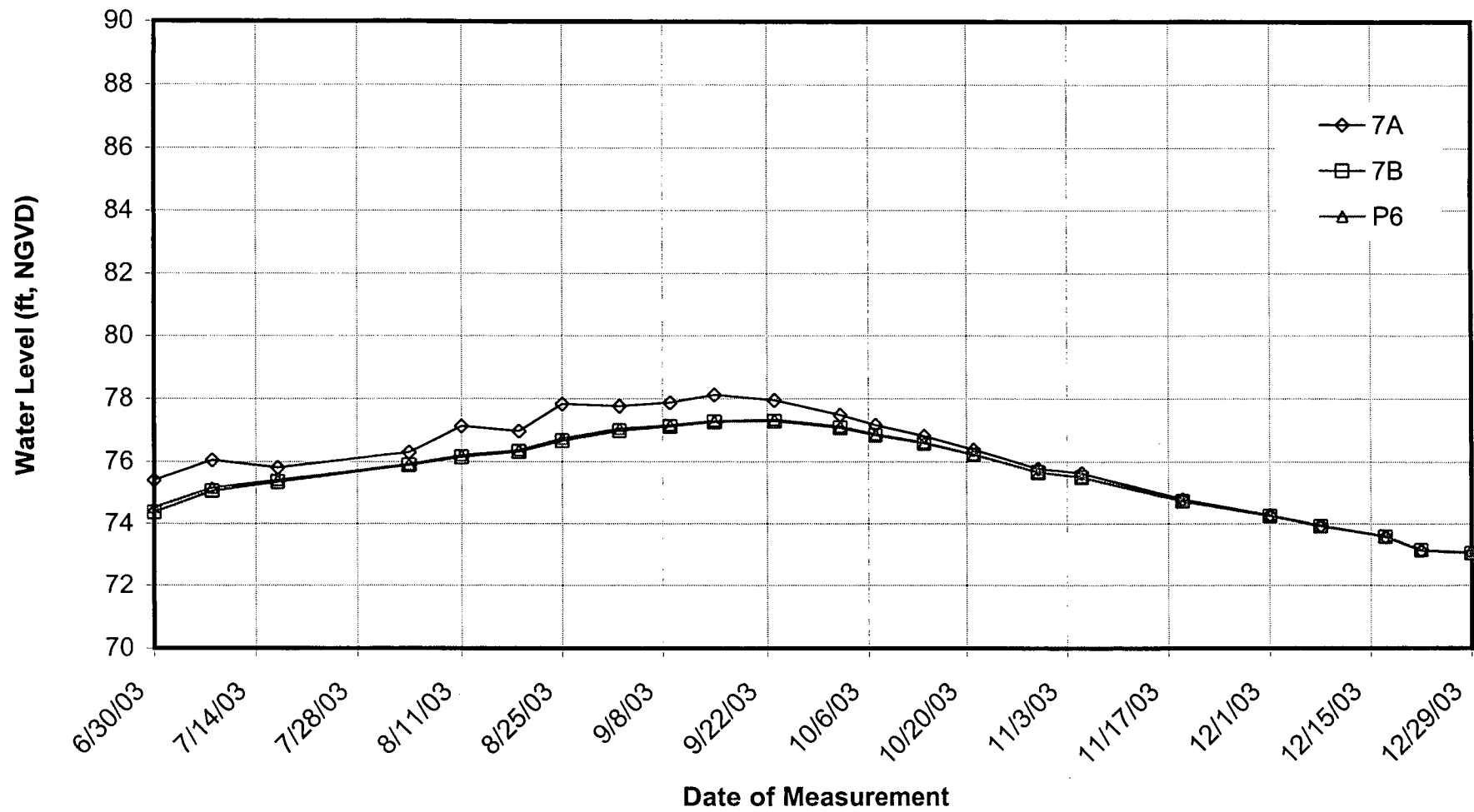
MW-7B

Water Level (ft, NGVD)



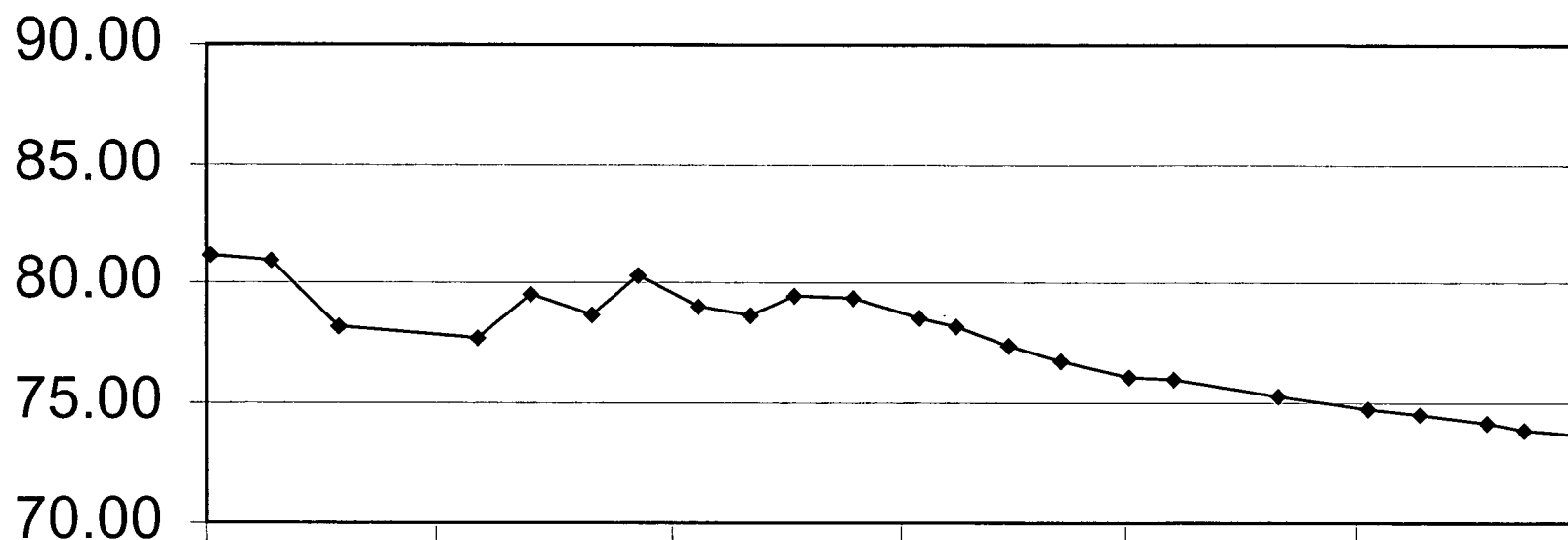
D.E.P.
JAN 15 2004
Southwest District Tampa

MW-7A & B, P6



MW-8

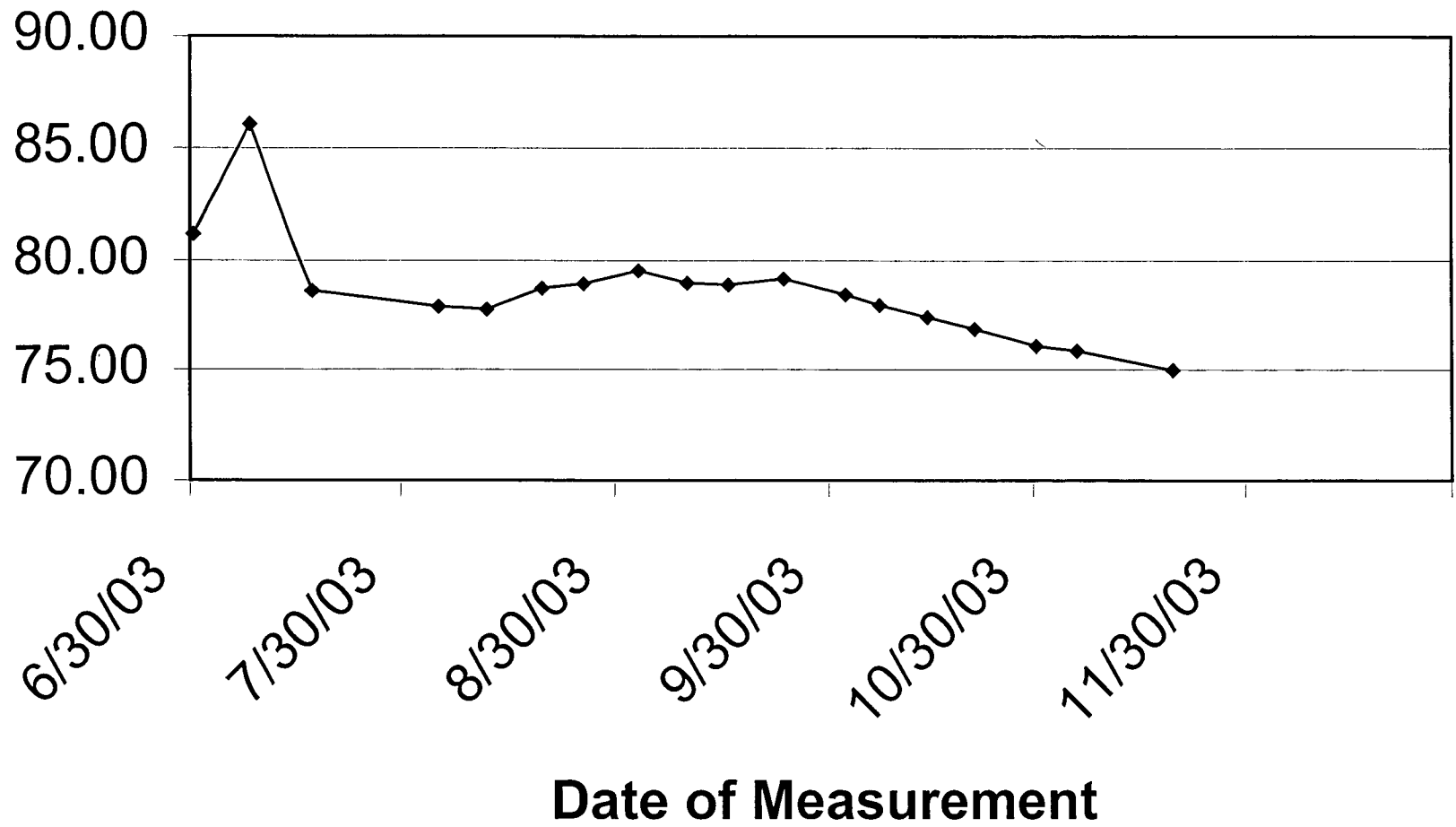
Water Level (Ft, NGVD)



Date of Measurement

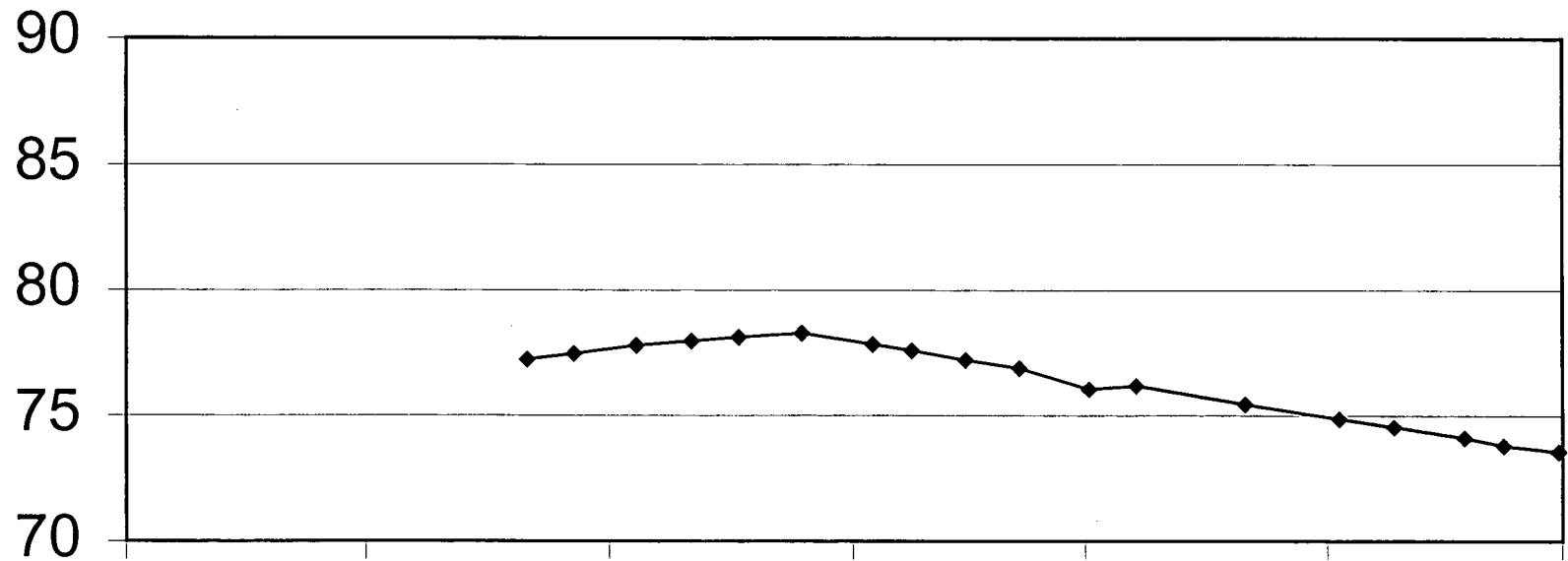
MW-10

Water Level (Ft, NGVD)



MW-11

Water Level (Ft, NGVD)

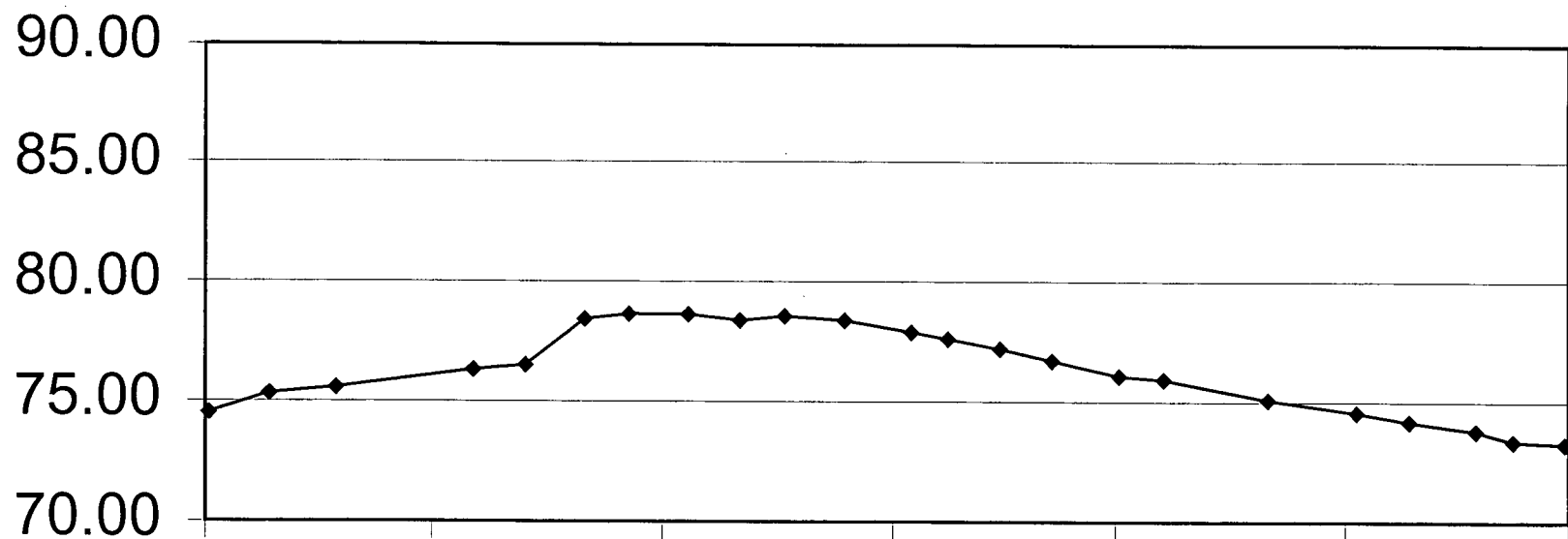


Date of Measurement

D.E.P.
JAN 15 2004
Southwest District Tampa

P-2

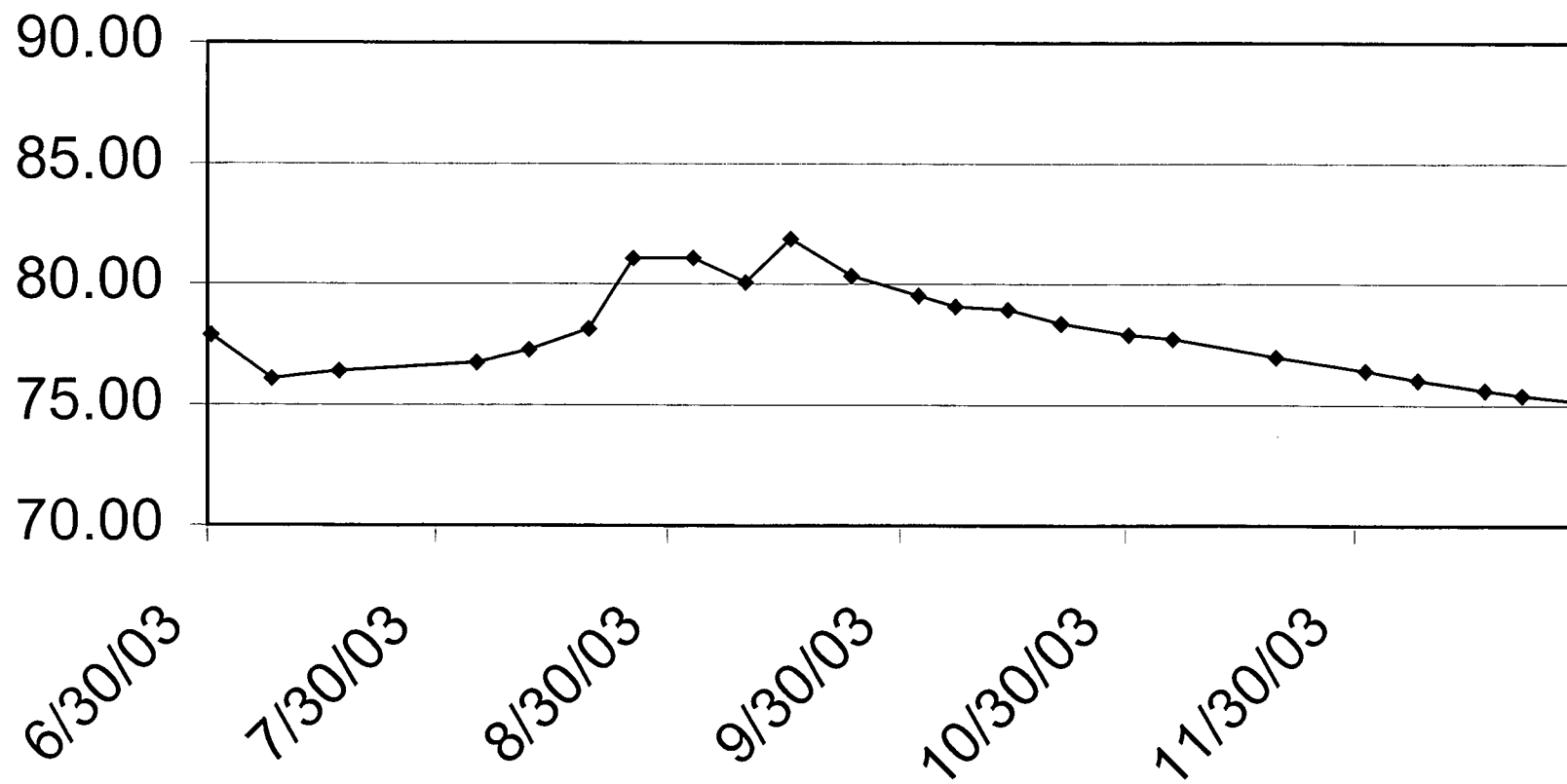
Water Level (Ft, NGVD)



Date of Measurement

P-4

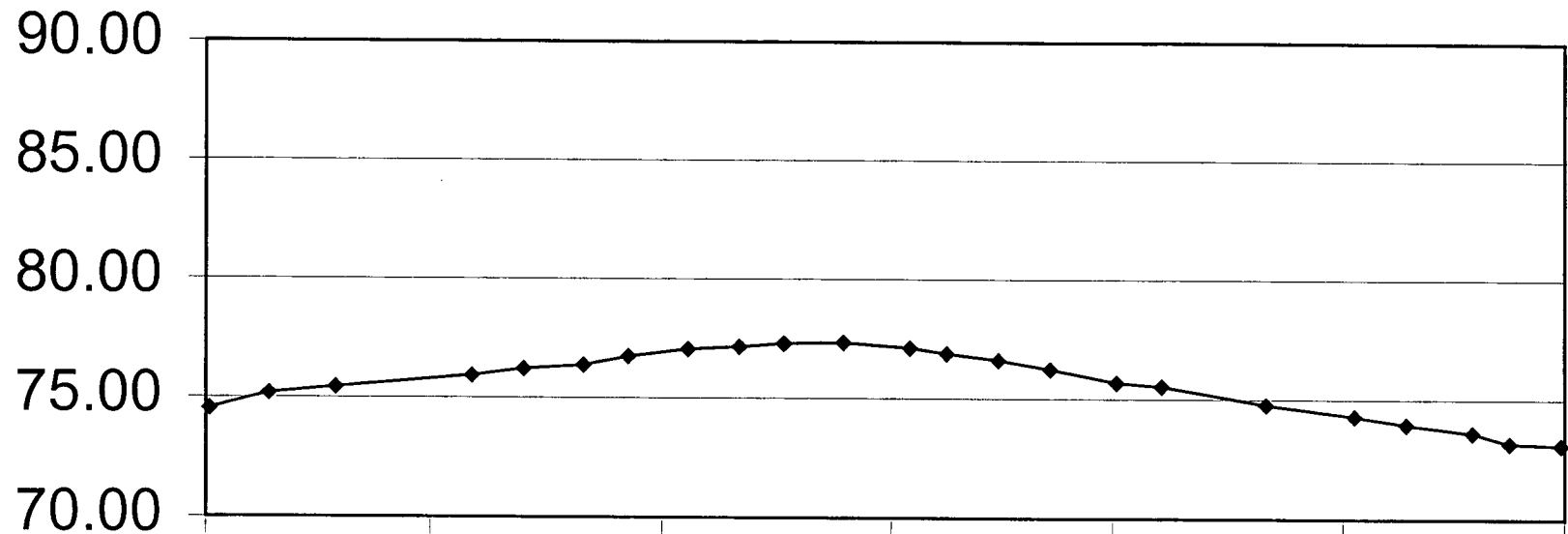
Water Level (Ft, NGVD)



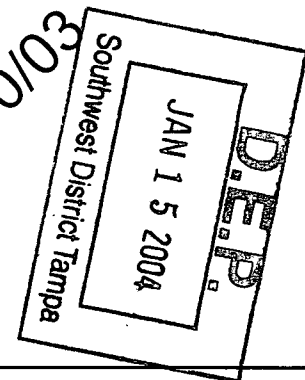
Date of Measurement

P-6

Water Level (Ft, NGVD)

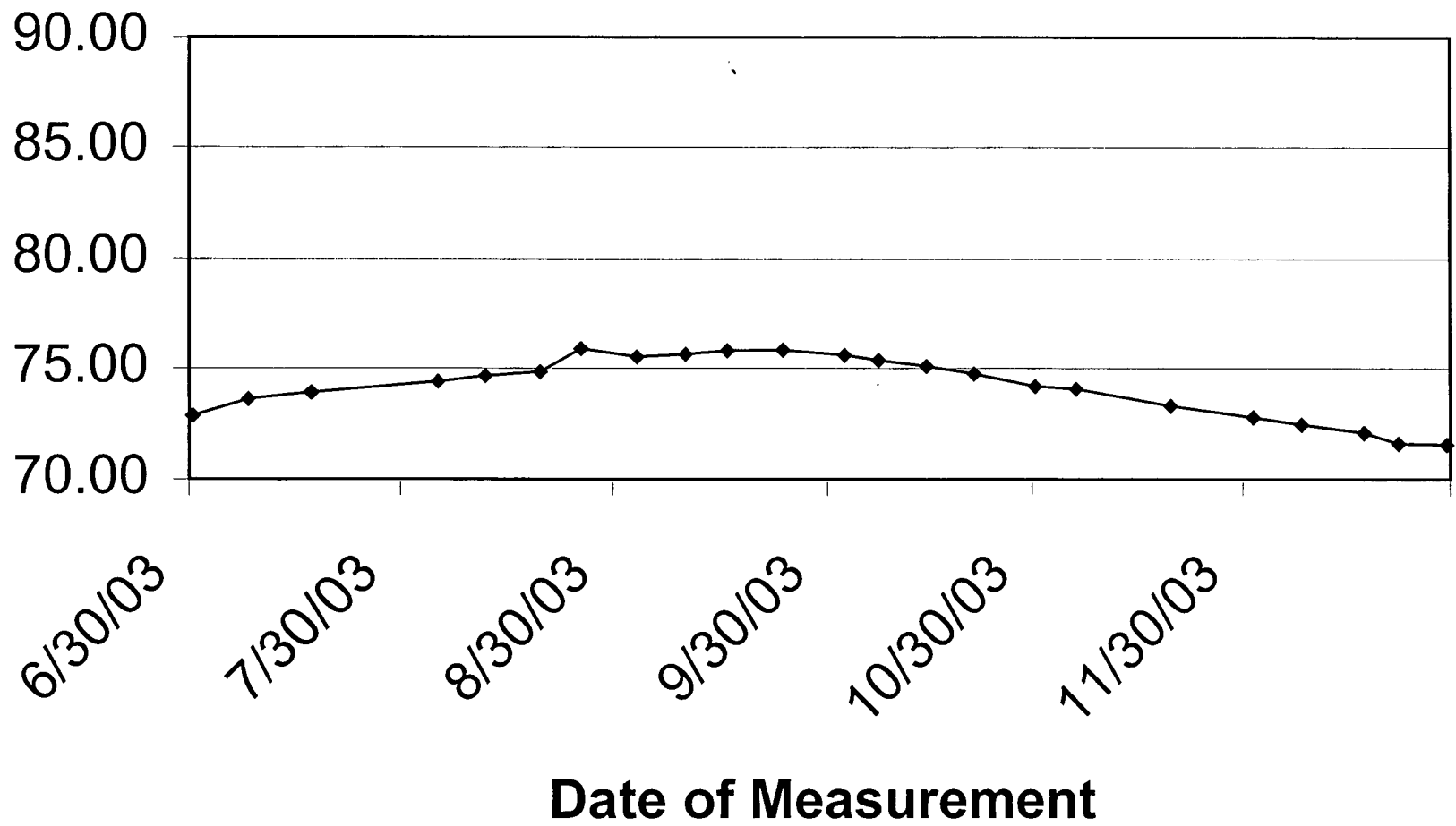


Date of Measurement



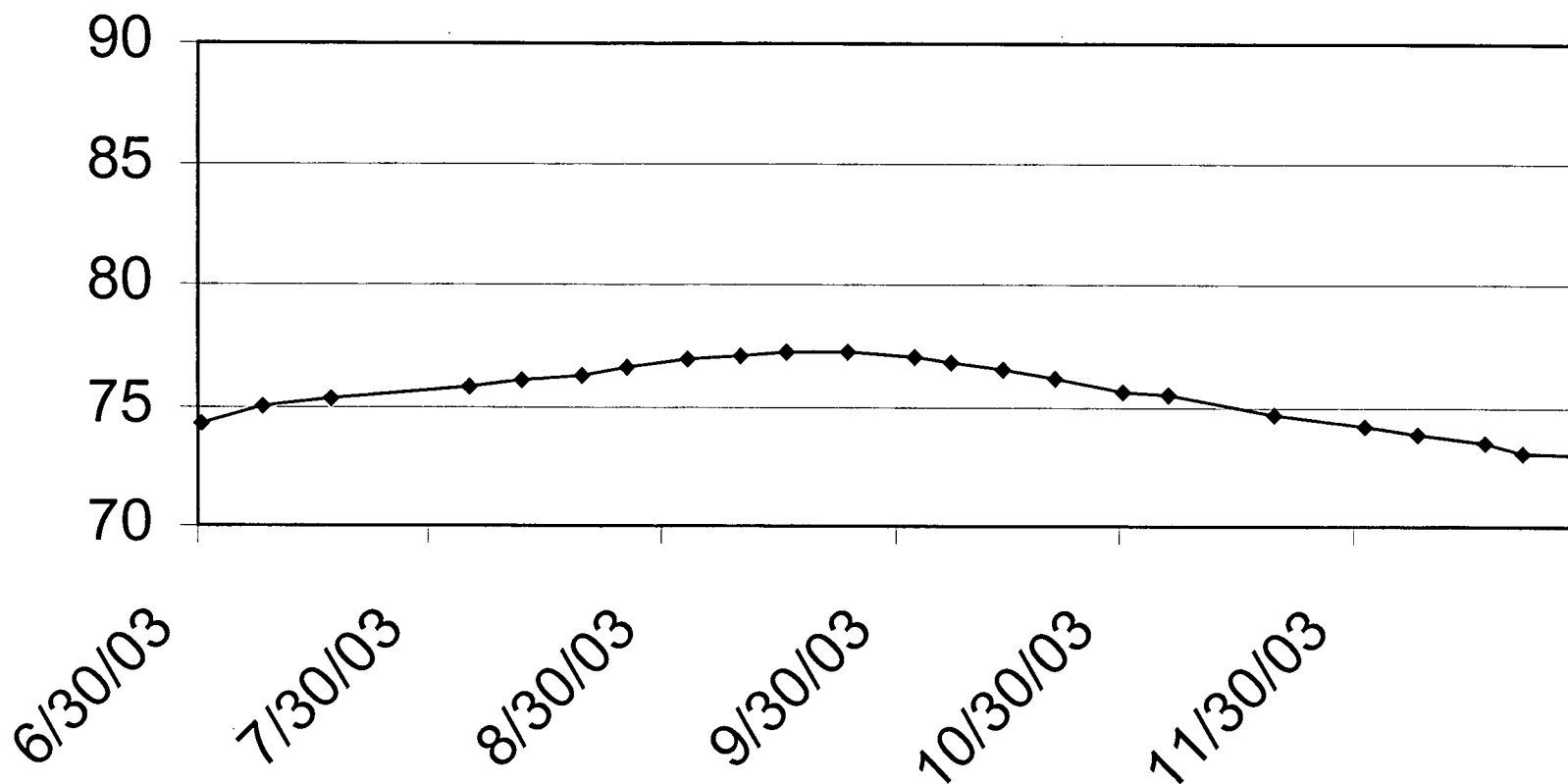
P-8

Water Level (Ft, NGVD)



P-10

Water Level (ft, NGVD)



Date of Measurement

P-11

Water Level (Ft, NGVD)

120.00
115.00
110.00
105.00
100.00

6/30/03

7/30/03

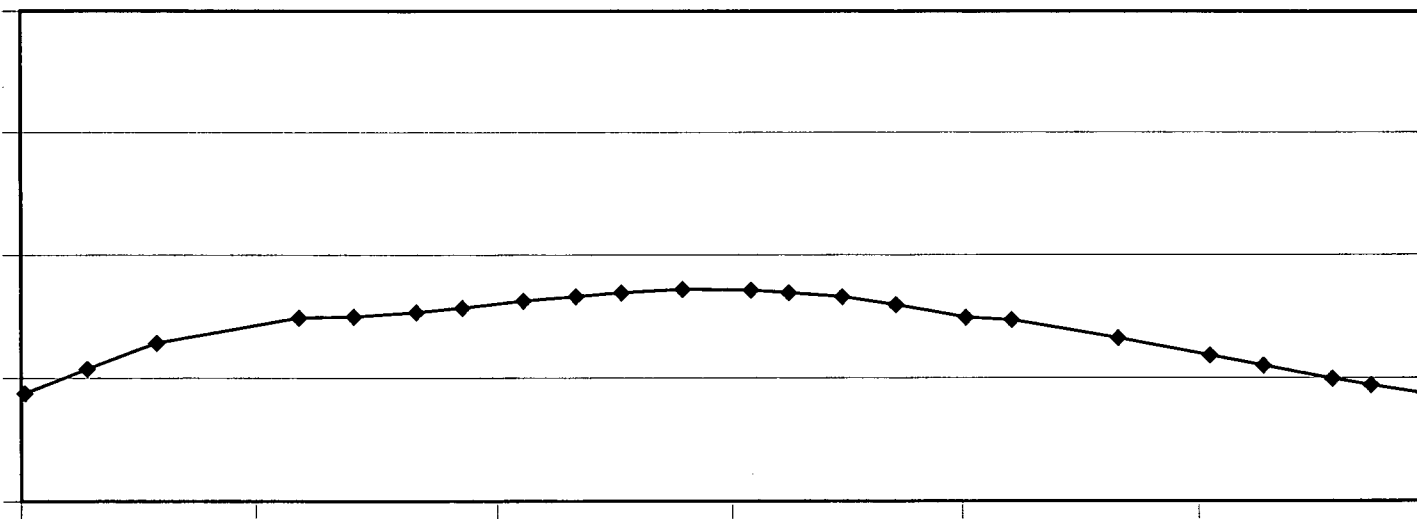
8/30/03

9/30/03

10/30/03

11/30/03

Date of Measurement



TP

Water Level (Ft, NGVD)

90.00
85.00
80.00
75.00
70.00

6/30/03

7/30/03

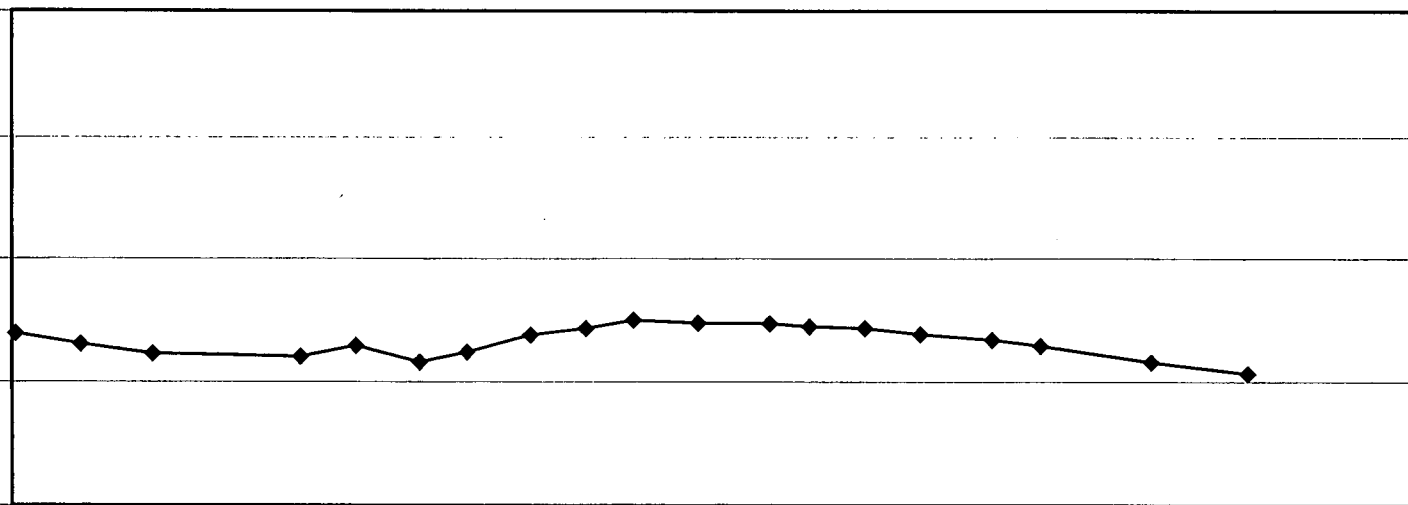
8/30/03

9/30/03

10/30/03

11/30/03

Date of Measurement



APPENDIX H

the use of the pumping-test approach is usually inappropriate. It is our opinion that the method is widely overused. Piezometer tests are simpler and cheaper, and they can provide adequate data in many cases where pumping tests are not justified.

8.7 Estimation of Saturated Hydraulic Conductivity

It has long been recognized that hydraulic conductivity is related to the grain-size distribution of granular porous media. In the early stages of aquifer exploration or in regional studies where direct permeability data are sparse, this interrelationship can prove useful for the estimation of conductivity values. In this section, we will examine estimation techniques based on grain-size analyses and porosity determinations. These types of data are often widely available in geological reports, agricultural soil surveys, or reports of soil mechanics testing at engineering sites.

The determination of a relation between conductivity and soil texture requires the choice of a representative grain-size diameter. A simple and apparently durable empirical relation, due to Hazen in the latter part of the last century, relies on the effective grain size, d_{10} , and predicts a power-law relation with K :

$$K = Ad_{10}^2 \quad (8.47)$$

The d_{10} value can be taken directly from a grain-size gradation curve as determined by sieve analysis. It is the grain-size diameter at which 10% by weight of the soil particles are finer and 90% are coarser. For K in cm/s and d_{10} in mm, the coefficient A in Eq. (8.47) is equal to 1.0. Hazen's approximation was originally determined for uniformly graded sands, but it can provide rough but useful estimates for most soils in the fine sand to gravel range.

Textural determination of hydraulic conductivity becomes more powerful when some measure of the spread of the gradation curve is taken into account. When this is done, the median grain size, d_{50} , is usually taken as the representative diameter. Masch and Denny (1966) recommend plotting the gradation curve [Figure 8.25(a)] using Krumbein's ϕ units, where $\phi = -\log_2 d$, d being the grain-size diameter (in mm). As a measure of spread, they use the inclusive standard deviation, σ_I , where

$$\sigma_I = \frac{d_{16} - d_{84}}{4} + \frac{d_5 - d_{95}}{6.6} \quad (8.48)$$

For the example shown in Figure 8.25(a), $d_{50} = 2.0$ and $\sigma_I = 0.8$. The curves shown in Figure 8.25(b) were developed experimentally in the laboratory on prepared samples of unconsolidated sand. From them, one can determine K , knowing d_{50} and σ_I .

For a fluid of density, ρ , and viscosity, μ , we have seen in Section 2.3 [Eq. (2.26)] that the hydraulic conductivity of a porous medium consisting of uniform

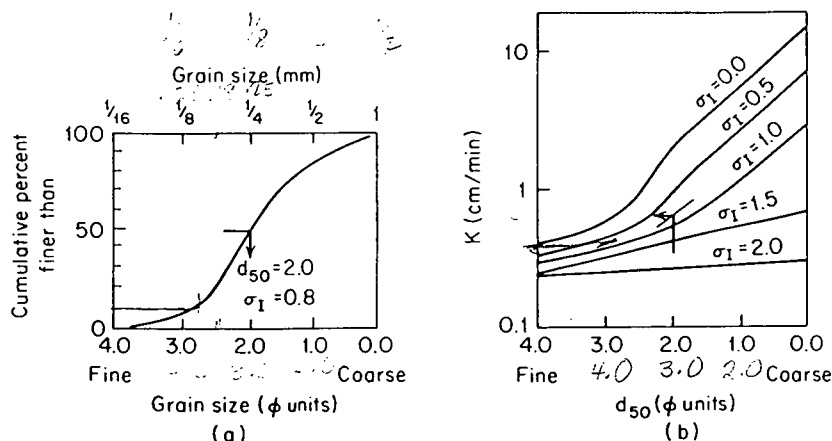


Figure 8.25 Determination of saturated hydraulic conductivity from grain-size gradation curves for unconsolidated sands (after Masch and Denny, 1966).

spherical grains of diameter, d , is given by

$$K = \left(\frac{\rho g}{\mu} \right) C d^2 \quad (8.49)$$

For a nonuniform soil, we might expect the d in Eq. (8.49) to become d_m , where d_m is some representative grain size, and we would expect the coefficient C to be dependent on the shape and packing of the soil grains. The fact that the porosity, n , represents an integrated measure of the packing arrangement has led many investigators to carry out experimental studies of the relationship between C and n . The best known of the resulting predictive equations for hydraulic conductivity is the *Kozeny-Carmen equation* (Bear, 1972), which takes the form

$$K = \left(\frac{\rho g}{\mu} \right) \left[\frac{n^3}{(1-n)^2} \right] \left(\frac{d_m^2}{180} \right) \quad (8.50)$$

In most formulas of this type, the porosity term is identical to the central element of Eq. (8.50), but the grain-size term can take many forms. For example, the *Fair-Hatch equation*, as reported by Todd (1959), takes the form

$$K = \left(\frac{\rho g}{\mu} \right) \left[\frac{n^3}{(1-n)^2} \right] \left[\frac{1}{m \left(\frac{\theta}{100} \sum \frac{P}{d_m} \right)^2} \right] \quad (8.51)$$

where m is a packing factor, found experimentally to be about 5; θ is a sand shape factor, varying from 6.0 for spherical grains to 7.7 for angular grains; P is the

percentage of sand held between adjacent sieves; and d_m is the geometric mean of the rated sizes of adjacent sieves.

Both Eqs. (8.50) and (8.51) are dimensionally correct. They are suitable for application with any consistent set of units.

8.8 Prediction of Aquifer Yield by Numerical Simulation

The analytical methods that were presented in Section 8.3 for the prediction of drawdown in multiple-well systems are not sophisticated enough to handle the heterogeneous aquifers of irregular shape that are often encountered in the field. The analysis and prediction of aquifer performance in such situations is normally carried out by numerical simulation on a digital computer.

There are two basic approaches: those that involve a *finite-difference* formulation, and those that involve a *finite-element* formulation. We will look at finite-difference methods in moderate detail; but our treatment of finite-element methods will be very brief.

Finite-Difference Methods

As with the steady-state finite-difference methods that were described in Section 5.3, transient simulation requires a discretization of the continuum that makes up the region of flow. Consider a two-dimensional, horizontal, confined aquifer of constant thickness, b ; and let it be discretized into a finite number of blocks, each with its own hydrogeologic properties, and each having a node at the center at which the hydraulic head is defined for the entire block. As shown in Figure 8.26(a), some of these blocks may be the site of pumping wells that are removing water from the aquifer.

Let us now examine the flow regime in one of the interior nodal blocks and its four surrounding neighbors. The equation of continuity for transient, saturated flow states that the net rate of flow into any nodal block must be equal to the time rate of change of storage within the nodal block. With reference to Figure 8.26(b), and following the developments of Section 2.11, we have

$$Q_{15} + Q_{25} + Q_{35} + Q_{45} = S_{11} \Delta x \Delta y b \frac{\partial h_5}{\partial t} \quad (8.52)$$

where S_{11} is the specific storage of nodal block 5. From Darcy's law,

$$Q_{15} = K_{15} \frac{h_1 - h_5}{\Delta y} \Delta x b \quad (8.53)$$

where K_{15} is a representative hydraulic conductivity between nodes 1 and 5. Similar expressions can be written for Q_{25} , Q_{35} , and Q_{45} .

FUTBA

TABLE 4.6 Ranges of intrinsic permeabilities and hydraulic conductivities for unconsolidated sediments

| Material | Intrinsic Permeability (darcys) | Hydraulic Conductivity (cm/s) |
|---------------------------------------|---------------------------------|-------------------------------|
| Clay | 10^{-6} – 10^{-3} | 10^{-9} – 10^{-6} |
| Silt, sandy silts, clayey sands, till | 10^{-3} – 10^{-1} | 10^{-6} – 10^{-4} |
| Silty sands, fine sands | 10^{-2} –1 | 10^{-5} – 10^{-3} |
| Well-sorted sands, glacial outwash | 1– 10^2 | 10^{-3} – 10^{-1} |
| Well-sorted gravel | 10 – 10^3 | 10^{-2} –1 |

4.4.3 Permeability of Sediments

Unconsolidated coarse-grained sediments represent some of the most prolific producers of ground water. Likewise, clays are often used for engineering purposes, such as lining solid-waste disposal sites, because of their extremely low intrinsic permeability. There is obviously a wide-ranging continuum of permeability values for unconsolidated sediments (Table 4.6).

The intrinsic permeability is a function of the size of the pore opening. The smaller the size of the sediment grains, the larger the surface area the water contacts (Figure 4.13). This increases the frictional resistance to flow, which reduces the intrinsic permeability. For well-sorted sediments, the intrinsic permeability is proportional to the grain size of the sediment (Norris & Fidler 1965).

For sand-sized alluvial deposits, several factors relating intrinsic permeability to grain size have been noted (Masch & Denny 1966). These observations would hold true for all sedimentary deposits, regardless of origin of deposition.

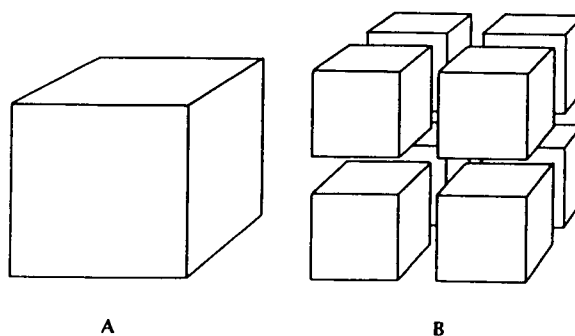


FIGURE 4.13 Relationship of sediment grain size to surface area of pore space. A. A cube of sediment with a surface area of 6 square units. B. The cube has been broken into 8 pieces, each with a diameter of one-half of the cube in part A. The surface area has increased to 12 square units—an increase of 100%.

1. As the median grain size increases, so does permeability. This is due to larger pore openings.
2. Permeability will decrease for a given median diameter as the standard deviation of particle size increases. The increase in standard deviation indicates a more poorly sorted sample, so that the finer material can fill the voids between larger fragments.
3. Coarser samples show a greater decrease in permeability with an increase in standard deviation than do fine samples.
4. Unimodal (one dominant size) samples have a greater permeability than bimodal (two dominant sizes) samples. This is again a result of poorer sorting of the sediment sizes, as the bimodal distribution indicates.

The hydraulic conductivity of sandy sediments can be estimated from the grain-size distribution curve by the **Hazen method** (Hazen 1911). The method is applicable to sands where the effective grain size (d_{10}) is between approximately 0.1 and 3.0 mm. The Hazen approximation is

$$K = C(d_{10})^2 \quad (4-19)$$

where

K is hydraulic conductivity (cm/s)

d_{10} is the effective grain size (cm)

C is a coefficient based on the following table

| | |
|----------------------------------|---------|
| Very fine sand, poorly sorted | 40-80 |
| Fine sand with appreciable fines | 40-80 |
| Medium sand, well sorted | 80-120 |
| Coarse sand, poorly sorted | 80-120 |
| Coarse sand, well sorted, clean | 120-150 |

The work of Hazen (1911) demonstrated that hydraulic conductivity could be related to the square of a characteristic dimension of a sediment. Shepherd (1989) analyzed data from 18 published studies where hydraulic conductivity had been related to grain size. He found that all studies could be related to the general formula

$$K = C d_{50}^j \quad (4-20)$$

where

C is a shape factor

d_{50} is the mean grain size (mm)

j is an exponent

The shape factor, C , and the exponent, j , were greatest for sediments that were texturally mature, as evidenced by well-sorted samples with uniformly sized

particles that had high roundness and sphericity. Both the shape factor and the exponent declined for sediments that were less texturally mature and were least for consolidated sediments.

Shepherd (1989) used the data sets to produce an idealized graph that relates hydraulic conductivity to the mean grain diameter for different sediment types (Figure 4.14). The C values on the graph are those that give the hydraulic

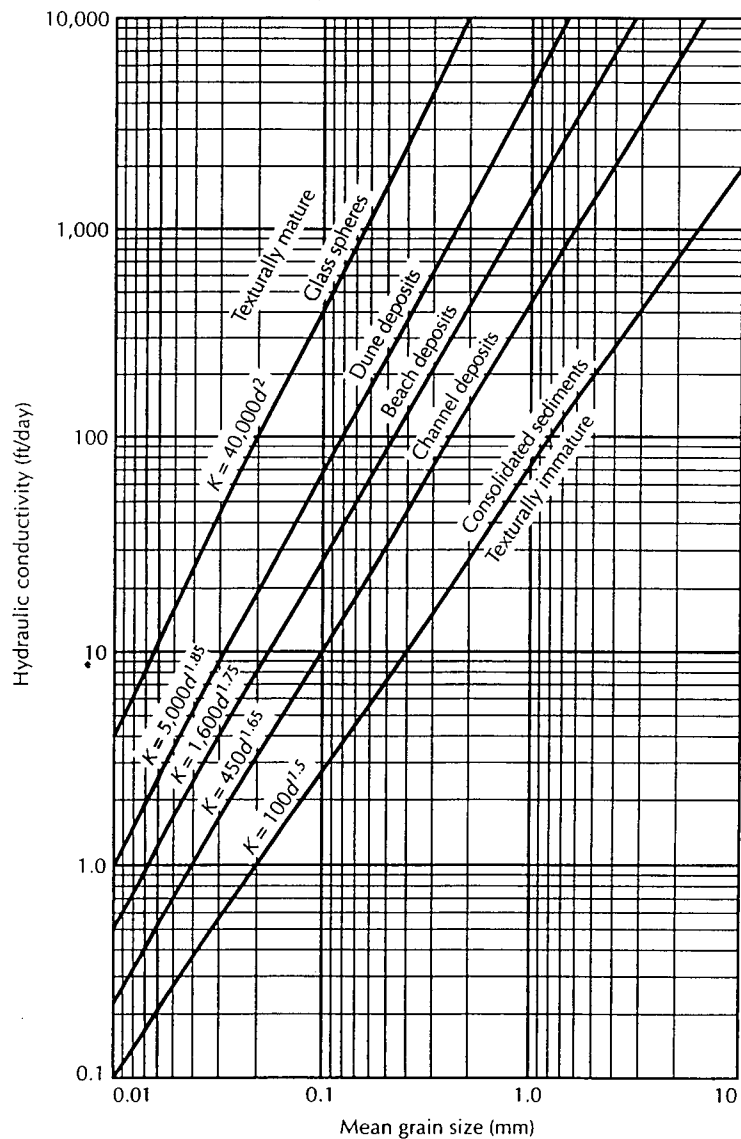


FIGURE 4.14 Graph showing the relationship of hydraulic conductivity to mean grain diameter for sediments of different textural maturity. Modified from R. G. Shepherd, *Ground Water* 27, no. 5 (1989): 633–638. Copyright © 1989 Ground Water Publishing Co.

conductivity in units of feet per day. An upper bound is presented for glass spheres, where the exponent is 2.0. Texturally mature sediments have an exponent of 1.75 or greater, whereas texturally immature sediments can have an exponent as low as 1.5.

CASE STUDY: HYDRAULIC CONDUCTIVITY ESTIMATES IN GLACIAL OUTWASH

A hazardous-waste-processing site was located on a level glacial outwash plain in southern Indiana. There were two aquifers present in the unconsolidated glacial deposits. The upper aquifer consisted of a well-sorted fine to medium sand. There were 27 ground-water monitoring wells installed in this aquifer. The lower aquifer was a poorly sorted fine to coarse sand. There were 9 monitoring wells installed in this aquifer. The grain-size analyses of the sand samples from the screen zones of the wells in each aquifer are summarized in the following table:

| | Upper Aquifer | | Lower Aquifer | |
|----------|---------------|--------------|---------------|--------------|
| | Mean | Range | Mean | Range |
| d_{10} | 0.14 mm | 0.08–0.20 mm | 0.16 mm | 0.09–0.26 mm |
| d_{60} | 0.31 mm | 0.19–0.45 mm | 2.04 mm | 0.35–6.70 mm |
| C_u | 2.29 | 1.50–3.89 | 11.01 | 3.89–33.50 |

The hydraulic conductivities of the sediments at each monitoring well were estimated by the Hazen method, using a coefficient of 100. The hydraulic conductivities of the sediments at each monitoring well were measured by means of a Hvorslev slug test performed on the well (see Section 7.5.3). The following table compares the results in centimeters per second.

| | Geometric
Mean
(cm/s) | Range (cm/s) |
|---------------|-----------------------------|---|
| | Upper Aquifer | |
| Hazen method | 1.9×10^{-2} | $4.0 \times 10^{-2} - 6.4 \times 10^{-3}$ |
| Hvorslev test | 1.9×10^{-2} | $8.9 \times 10^{-2} - 4.2 \times 10^{-3}$ |
| | Lower Aquifer | |
| Hazen method | 1.2×10^{-2} | $2.6 \times 10^{-2} - 8.1 \times 10^{-3}$ |
| Hvorslev test | 1.4×10^{-2} | $1.7 \times 10^{-1} - 2.6 \times 10^{-3}$ |

The geometric means of the data sets were used to compare the Hvorslev test results with the Hazen method results in the above case study. Hydraulic conductivity values frequently vary by more than two orders of magnitude within the same hydrogeologic unit. An arithmetic mean of such a sample population tends to give more weight to the more permeable values. Some hydrogeologists