AND

APPLICATION FOR PERMIT

TO CONSTRUCT

DISPOSAL UNIT A-2 AT

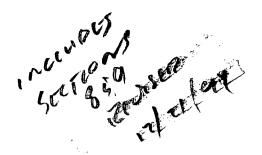
PASCO COUNTY

RESOURCE RECOVERY FACILITY

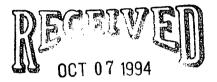
CLASS I LANDFILL

Prepared for:
PASCO COUNTY
BOARD OF COUNTY COMMISSIONERS

Prepared by:
LAW ENVIRONMENTAL, INC.
Tampa, Florida







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SOUTHWEST DISTRICT

AND
APPLICATION FOR PERMIT TO CONSTRUCT
DISPOSAL UNIT A-2 AT PASCO COUNTY
RESOURCE RECOVERY FACILITY
CLASS I LANDFILL

Prepared For:

PASCO COUNTY BOARD OF COUNTY COMMISSIONERS
Pasco County, Florida

Prepared By:

LAW ENVIRONMENTAL, INC. Tampa, Florida

Revision 1

OCTOBER 1994

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SECTION 1.0 COMPLETED APPLICATION FORM

STATE OF FLORIDA DEPARTMENT OF ENVIRONMENTAL REGULATION

APPLICATION FOR PERMIT TO CONSTRUCT, OPERATE, MODIFY OR CLOSE A SOLID WASTE MANAGEMENT FACILITY

Please Type or Print

Α.	GENERAL INFORMA	TION		
1.	Type of facility:			
	Disposal [X]			
	Class I Landfill Class II Landfill Class III Landfill Other	[X] [] []	Ash Monofill Asbestos Monofill Industrial Solid Waste	[X] [] []
	Volume Reduction	[]		
	Incinerator Composting Materials Recovery Other	[] [] []	Pulverizer/Shredder Compactor/Bailing Plant Energy Recovery	[]
2	Type of application:			
	Construction Operation	[X] []	Construction/Operation Closure	[]
3.	Classification of applica	tion:		
	New Renewal	[]	Substantial Modification Minor Modification	[]
4.	Facility name: Pasco C	ounty Resource I	Recovery Facility Class I L	andfill
5. 6.	DER ID number:Facility location (main e	entrance): <u>Haye</u>	s Road, Springhill, Florid	<u>a</u> .
7.	Location coordinates:			
٠	Section: <u>24,25,26</u> To	wnship: <u>245</u> R	ange: <u>17E</u>	
	UTMs: Zone	km E	km N	
8.	Latitude: 28° 22' 30 Applicant name (operati	<u>)"</u> Longi ing authority):	tude: 82° 34' 00" Pasco County Board of C	County Commissioners
		36 State Street	New Port Richey	

	Contact person: Douglas Bramlett Telephone: (813) 847-8145
	Title: Assistant County Administrator - Utilities Services Branch
9.	Authorized agent/Consultant: Law Environmental, Incorporated
	Mailing address: 1715 North Westshore Blvd., Suite 875 Tampa Florida 33607 Street or P.O. Box City State Zip
	Contact person: Richard E. Mayer Telephone: (813) 289-9491
	Title: Principal Engineer
10.	Landowner (if different than applicant): N/A
	Mailing address: N/A Street or P.O. Box City State Zip
	Contact person: N/A Telephone: N/A
11.	Cities, towns and areas to be served: Entire Pasco County including the six incorporated municipalities
12.	Population to be served:
	Current: 295,000 Five-Year Projection: 318,000
13.	Volume of solid waste to be received: 225 (ash) tons/day
14.	Date site will be ready to be inspected for completion:
15.	Estimated life of facility: 5 for Disposal Unit A-2 years
16.	Estimated costs:
	Total Construction: \$2,000,000 Closing Costs: \$
17.	Anticipated construction starting and completion dates:
	From: <u>1/95</u> To: <u>7/96</u>
	* For Disposal Unit A-2

B. DISPOSAL FACILITY GENERAL INFORMATION

١.	Provide brief description of disposal facility design and operations planned by this application:				
	The West Pasco Landfill Disposal Unit A-2, an ash monofill designed to meet the requirements of Chapters 17-701 and 17-702, F.A.C., is to provide continued disposal of MSW ash from the Pasco County Resource Recovery Facility. The disposal unit is part of a 160-acre landfill footprint permitted under the Power Plant Siting Act.				
2.	Facility site supervisor: Mr. Vince Mannella				
	Title: Solid Waste Facility Manager Telephone: (813) 861-3006				
3.	Disposal area: Total 160 acres; Used 20 acres; Available 140 acres				
4.	Weighing scales used: Yes [X] No []				
5.	Security to prevent unauthorized use: Yes [X] No [] (Fenced w/ locked gates)				
6.	Charge for waste received: N/A \$/yds³\$/ton				
7.	Surrounding land use, zoning:				
	Residential [x] Industrial [] Agricultural [x] None [] Commercial [] Other []				
8.	Types of waste received:				
	Residential [] C & D debris [] Commercial [] Shredded/cut tires [] Incinerator/WTE ash [X] Yard trash [] Treated biohazardous [] Septic tank [] Water treatment sludge [] Industrial [] Air treatment sludge [] Industrial sludge [] Agricultural [] Domestic sludge [] Asbestos [] Other [] See Section 3				
9.	Salvaging permitted: Yes [] No [X]				
10.	Attendant: Yes [X] No [] Trained operator: Yes [X] No []				
11.	Spotters: Yes [X] No [] Number of spotters used: 1				
12.	Site located in: Floodplain [] Wetlands [] Other [x] Uplands				
13.	Property recorded as a Disposal Site in County Land Records: Yes [X] No []				
14.	Days of operation: Seven (7) days per week.				
15.	Hours of operation: 7 AM - 5 PM				
16.	Days Working Face covered: N/A				

17.	Elevation of water table: 37.0 Ft. NGVD						
18.	Number of monitoring wells: Twelve (12) existing, additional Seven (7) proposed.						
19.	Number of surface monit	Number of surface monitoring points: None					
20.	Gas controls used: Yes	[X] No[]	Type controls: Active [] Pass	sive [X]			
	Gas flaring: Yes [] N	No [X]	Gas recovery: Yes [] No [2	K]			
21.	Leachate control method	- liner type:	·				
	Natural soils Single clay liner Single geomembrane Single composite Slurry wall Other	[] [] [] []	Double geomembrane Geomembrane & composite Double composite None	[X] [] [] []			
22.	Leachate collection meth	od:					
	Collection pipes Geonets Well points Perimeter ditch Other	[X] [X] [] []	Sand layer Gravel layer Interceptor trench None	[X] [] []:			
23.	Leachate storage method	l:					
	Tanks Other	[X _.]	Surface impoundments	[]			
24.	Leachate treatment meth	od:					
	Oxidation Secondary Advanced Other	[] [] [] [X]	Chemical treatment Settling None Off-site @ an applicant-owner	[] [] [] d WWTP			

25.	Leachate disposal method:
	Recirculated [] Pumped to WWTP [X] Transported to WWTP [] Discharged to surface water [] Injection well [] Evaporation (ie: Perc Pond) [] Other []
26.	For leachate discharged to surface waters:
	Name and Class of receiving water: N/A
27.	Storm Water:
	Collected: Yes [X] No [] Type of treatment: Retention pond
	Name and Class of receiving water: N/A
28.	Management and Storage of Surface Waters (MSSW) Permit number or status:

VOLUME REDUCTION FACILITY GENERAL INFORMATION [NOT APPLICABLE] 1. Provide brief description of volume reduction facility design and operations planned by this application: 2. Facility site supervisor: Title: Telephone () Disposal area: Total ____ acres; Used ____ acres; Available ____ acres 3. Security to prevent unauthorized use: Yes () No () 4. Site located in: Floodplain () Wetlands () Other () 5. Days of operation: 6. 7. Hours of operation: Number of operating staff: 8. Expected useful life: years 9. Weighing scales used: Yes (x) No () ıIJ. 11. Normal processing rate: Maximum processing rate; ____ yd³/day ____ tons/day ____ gal/day 12. 13. Charge for waste received: Type of facility (check one or more): 14. Composting Incinerator Pulverizer/shredder () Materials recovery Energy recovery Compactor/baling · () **Pyrolysis** Sludge concentration () Other () Material recovered, tons/week: 15. Glass Paper Ferrous metals Non-ferrous metals ___ Aluminum Plastics __ Other: Energy recover, in units shown: 16.

Chilled water, gal.hr

____ Oil, gal/hr

____ High pressure steam, lb/hr

____ Low pressure steam, lb/hr

Resource Recovery Facility - Class I Landfill Law Environmental Project 573565

	Electricity, kw/hr Gas, ft³/hr Other: Gas, BTU/hr Gas, BTU/hr	
17.	Process water management:	
	Recycled: Yes () No ()	
	Treatment method used:	
	Discharged to: Surface water () Underground () Other ()	
	Name and Class of receiving water:	
18.	Storm Water:	
	Collected: Yes () No () Type of treatment:	
	Name and Class of receiving water:	
19.	MSSW Permit number or status:	
20.	Final residue produced:	
	% of normal processing rate	, ,
	% of maximum processing rate	
	Disposed of at (Site name):	
21.	Supplemental fuel used:	
	Type: Quantity used/hour:	
22.	Costs:	
	Estimated operating costs (material-energy revenue): \$	
	Total cost/ton \$ Net cost/ton: \$	
23.	State pollution control bond financing amount: \$	
24	Estimated amount of tax exemption that will be requested: \$	

LOCATION N/A N/C

D.	SOLID WASTE MANAGEM (17-701.320,FAC)	ENT FACIL	ITY PERMIT GENERAL REQUIREMENTS		
<u>X</u>	Section 2.1	1.	Six copies at minimum, of the completed application form. all supporting data and reports; (17-701.320(5)(a), FAC).		
<u>X</u>	Section 2.2	2.	Engineering and/or professional certification (signature, date and seal) provided on the applications and all engineering plans, reports and supporting information for the application; (17-701.320(6), FAC).		
<u>X</u>	Section 2.3	3.	A letter of transmittal to the Department; (17-701.320(7)(b),FAC).		
<u>X</u>	Section 2.4	4.	A completed application for dated and signed by the applicant; (17-701.320(7)(c).FAC).		
	Section 2.5 X	_ 5.	Permit fee specified in Rule 17-4.050,FAC and Rule 17-7-1.320(5)(c), FAC in check or money order, payable to the Department; 17-701.320(7)(c),FAC).		
<u>X</u>	Section 2.6	6.	An engineering report addressing the requirements of this rule and with the following format: a cover sheet, text printed on 8 1/2 inch by 11 inch consecutively numbered pages, a table of contents or index, the body of the report and all appendices including an operation plan, contingency plan, illustrative charts and graphs, records or logs of tests and investigations, engineering calculations; (17-701.320(7)(d), FAC)		
<u>X</u>	Section 2.7	7.	Operation Plan; (17-701.320(7)(e)1,FAC).		
<u>X</u>	Section 2.8	8.	Contingency Plan; (17-701.320(7)(e)2,FAC).		
		9.	Plans or drawings for the solid waste management facilities in appropriate format (including sheet size restrictions, cover sheet, legends, north arrow, horizontal and vertical scales, elevations referenced to NGVD) showing; (17-702.320(7)(f),FAC).		
<u>X</u>	Drawing Sheet No.2		a) A regional map or plan with the project location;		
<u>X</u>	Drawing Sheet No.4		b) A vicinity map or aerial photograph no more than 1 year old;		
<u>X</u>	Drawing Sheet No.4		c) A site plan showing all property boundaries certified by a registered Florida land surveyor;		
<u>X</u>	D.S. Nos. 1-13		d) Other necessary details to support the engineering report.		
<u>X</u>	Section 2.10	10.	Proof of property ownership or a copy of appropriate agreements between the facility operator and property owner authorizing use of property; (17-701.320(7)(g),FAC).		
	X	_ 11.	For facilities owned or operated by a county, provide a description of how, is any, the facilities covered in this application will contribute to the county's achievement of recycling goals contained in Section 403.706.FS; (17-701.320(7)(h),FAC).		

S	LOCATION	N/A	N/C			
		<u>X</u> _		12.	Depart orders	e a history and description of any enforcement actions taken by the ment against the applicant for violations of applicable statutes, rules, or permit conditions relating to the operation of any solid waste ement facility in this state; (17-701.320(7)(i),FAC).
		<u>X</u> _		13.	applica	of publication in a newspaper of general circulation of notice of ation for a permit to construct or substantially modify a solid waste ement facility; (17-702.320(8),FAC)
		<u>X</u> _		14.	Provid includi	le a description of how the requirements for airport safety will be achieved ing proof of required notices if applicable; (17-701.320(12),FAC).
E. L	ANDFILL PERM	IT GENEI	RAL R	EQUIR	EMENT	TS (17-701.330,FAC)
<u>X</u>	Section 3.1			1.	scale s suffici other	ty map or aerial photograph no more than 1 year old and of appropriate showing land use and local zoning within one mile of the landfill and of ent scale to show all homes or other structures, water bodies, and roads significant features of the vicinity. All significant features shall be d; (17-701.3340(4)(c),FAC).
<u>X</u>	Section 3.2			2.	that	ty map or aerial photograph no more than 1 year old showing all airports are located within five miles of the proposed landfill; (17-30(4)(b),FAC).
<u>X</u>	D.S. 4,5 & 7			3.		olan with a scale not greater than 200 feet to the inch showing; (17-30(4)(c),FAC).
<u>x</u>	Section 3.3.1			_	a)	Dimensions,
<u>X</u>	Section 3.3.2			_	b)	Locations of proposed and existing water quality monitoring wells;
<u>X</u>	Section 3.3.3			_	c)	Locations of soil borings;
<u>x</u>	Section 3.3.4			-	d)	Proposed plan of trenching or disposal areas;
<u>X</u>	Section 3.3.5		- —	_	e)	Cross sections showing original elevations and proposed final contours which shall be included either on the plot plan or on separate sheets;
<u>X</u>	Section 3.3.6	 ,		_	f)	Any previously filled waste disposal areas;
<u>X</u>	Section 3.3.7			_	g)	Fencing or other measures to restrict access.
	·					with a scale not greater than 200 feet to the inch with 5-foot contou 17-701.330(4)(d),FAC).
<u>x</u>	Section 3.4.1			_	a)	Proposed fill areas;
<u>X</u>	Section 3.4.2			_	b)	Borrow areas;
<u>X</u>	Section 3.4.3			_	c)	Access roads;
x	Section 3 4 4				d)	Grades required for proper drainage;

	LOCATION	N/A	N/C		
<u>X</u>	Section 3.4.5			e)	Cross sections of lifts;
X	Section 3.4.6	x_		f) ·	Special drainage devices if necessary;
<u>x</u>	Section 3.4.7			g)	Fencing;
X	Section 3.4.8			h)	Equipment facilities.
		5	. A report o	on the land	fill describing the following (17-701.330(4)(e),FAC).
<u>X</u>	Section 3.5.1			a)	The current and projected population and area to be served by the proposed site;
<u>X</u>	Section 3.5.2			b)	The anticipated type, annual quantity,m and source of solid waste, expressed in tone;
<u>X</u>	Section 3.5.3			c)	The anticipated facility life;
<u>X</u>	Section 3.5.4			_ d)	The source and type of cover material used for the landfill.
<u>X</u>	Section 3.6		6.	monit	de evidence that an approved laboratory shall conduct water quality oring for the facility in accordance with Rule 17-160,FAC; (17-30(4)(h),FAC).
<u> </u>	Section 3.7		7.	Provide for the	de a statement of how the applicant will demonstrate financial responsibility e closing and long-term care of the landfill; (17-701.330(4)(i),FAC)
F.	GENERAL CRIT	TERIA F	OR LAND	FILLS (17	-701.340,FAC).
<u>X</u>	Section 4.1		1.	availa 100-y the te	ibe (and show on a Federal Insurance Administration flood map, if ble) how the landfill or solid waste disposal unit shall not be located in the ear floodplain where it will restrict the flow of the 100-year flood, reduce emporary water storage capacity of the floodplain unless compensating te is provided, or result is a washout of solid waste;
<u>X</u>	Section 4.2		2.	landfi	ibe how the minimum horizontal separation between waste deposits in the land the landfill property boundary shall be 100 feet, measured from the the proposed final cover slope; 17-701.340(4)(c),FAC)
<u>X</u>	Section 4.3		3.		ibe what method shall be taken to screen the landfill from public view such screening can practically be provided; (17-701.340(4)(d),FAC).
G. I	LANDFILL CONSTI	RUCTIO	N REQUIR	REMENTS	(17-701,400,FAC)
<u>X</u>	Section 5.1		1.	be co	ibe how the landfill shall be designed so that solid waste disposal units will instructed and closed at planned intervals throughout the design period of indfill; (17-701.400(2).FAC)

S	LOCATION	N/A	N/C		
			2. Land a.	Ifill liner requirements; (General construction re (17-701.400(3)(a),FAC	quirements;
X	Section 5.2.1		<u>.</u>	(1)	Provide test information and documentation to ensure that liner will be constructed of materials that have appropriate physical, chemical, and mechanical properties to prevent failure;
<u>X</u>	Section 5.2.2			(2)	Document foundation is adequate to prevent liner failure;
<u>X</u>	Section 5.2.3			(3)	Constructed so bottom liner will not be adversely impacted by fluctuation of the ground water;
	Section 5.2.4	<u> X</u> _		_ (4)	Designed to resist hydrostatic uplift if bottom liner located below seasonal high ground-water table;
<u>X</u>	Section 5.2.5			(5)	Installed to cover all surrounding earth which could come into contact with the waste or leachate.
			b.	Composite liners; (17-	701.400(3)(b),FAC).
		<u>X</u>		(1)	Upper geomembrane thickness and properties;
		<u>X</u>		(2)	Design leachate head for primary LCRS including leachate recirculation if appropriate;
		<u>X</u> _		(3) Double liners; (17-701.4)	Design thickness in accordance with Table A and number of lifts planned for lower soil component.
			C. I		
<u>X</u>	Section 5.3.1			(1)	Upper and lower geomembrane thicknesses and properties;
<u>X</u>	Section 5.3.2			(2)	Design leachate head for primary LCRS to limit the head to one foot above the liner;
<u>X</u>	Section 5.3.3			(3)	Lower geomembrane sub-base design;
<u>X</u>	Section 5.3.4			(4)	Leak detection and secondary leachate collection system minimum design criteria (k 1 cm/sec, head on lower linear linch, head not to exceed thickness of drainage layer);
			d.	Standards for geomem (17-701.400(3)(d),FAC	
<u>X</u>	Section 5.4.1			(1)	Field seam test methods to ensure all field seams are at least 90 percent of the yield strength for the lining material;
X	Section 5.4.2			(2)	Design of 24-inch-thick protective layer above upper geomembrane liner;

	LOCATION	N/A	N/C		
<u>X</u>	Section 5.4.3	 		(3)	Describe operational plans to protect the liner and leachate collection system when placing the first layer of waste above 24-inch-thick protective layer.
			e.	Geosynthetic specification (17-701.400(3)(e),FAC)	
X	Section 5.5.1			(1)	Definition and qualifications of the designer, manufacturer, installer, QA consultant and laboratory, and QA program;
<u>X</u>	Section 5.5.2			(2)	Material specifications for geomembranes, geotextiles, geogrids, and geonets;
X	Section 5.5.3			(3)	Manufacturing and fabrication specifications including geomembrane raw material and roll QA, fabrication personnel qualifications, seaming equipment and procedures, overlaps, trial seams, destructive and nondestructive seam testing, seam testing location, frequency, procedure, sample size and
<u>X</u> _	Section 5,5.4			(4)	geomembrane repairs; Geomembrane installation specifications including earthwork, conformance testing, geomembrane placement, installation personnel qualifications, field seaming and testing, overlapping and repairs, materials in contact with geomembrane and procedures for system acceptance;
<u>X</u>	Section 5.5.5			(5)	Geotextile and geogrid specifications including handling and placement, conformance testing, seams and overlaps, repair, and placement of soil materials;
<u>X</u>	Section 5.5.5			(6)	Geonet specifications including handling and placement, conformance testing, stacking and joining, repair and placement of soil materials;
			f.	Standards for soil comp 17-701.400(3)(f),FAC)	onents;
<u>X</u>	Section 5.6.1			(1)	Description of construction procedures including over- excavation and backfilling to preclude structural inconsistencies and procedures for placing and compacting soil component in layers;
<u>X</u>	Section 5.6.2			(2)	Demonstration of compatibility of the soil component with actual or simulated leachate in accordance with EPA Test Method 9100 or an equivalent test method;
<u>X</u>	Section 5.6.3			(3)	Procedures for testing in-situ soils to demonstrate they meet the specifications for soil liners;
				(4) Specifications	for soil component of liner including at a minimum:
<u>x</u>	Section 5.6.4				(a) Allowable particle size distribution, Atterberg limits, shrinkage limit;

S	LOCATION	N/A	N/C			
<u> </u>	Section 5.6.4				(b)	Placement moisture and dry density criteria;
<u>X</u>	Section 5.6.4				(c)	Maximum laboratory-determined saturated hydraulic conductivity using simulated leachate;
<u>X</u> _	Section 5.6.4				(d)	Minimum thickness of soil liner;
X	Section 5.6.4				(e)	Lift thickness;
<u>x</u>	Section 5.6.4				(f)	Surface preparation (scarification);
<u>X</u>	Section 5.6.4				(g)	Type and percentage of clay mineral within the soil component;
<u>X</u>	Section 5.6.4			(5)	docum	dures for constructing and using a field test section to nent the desired saturated hydraulic conductivity and ess can be achieved in the field.
		:	(17-701 a. T	te collection and remo 1.400(4)(a),FAC) The primary and secon 17-701.400(4)(a),FAC	dary LCF	
<u>X</u>	Section 5.7.1			(1)	Const leacha	ructed of materials chemically resistant to the waste and te;
<u>X</u>	Section 5.7.2			(2)		sufficient mechanical properties to prevent collapse pressure.
<u>X</u>	Section 5.7.3			(3)	Have clogg	granular material or synthetic geotextile to preventing;
<u>X</u>	Section 5.7.4			(4)		method for testing and cleaning clogged pipes or agent designs for rerouting leachate around failed areas;
	·			Primary LCRS require 17-701.400(4)(b),FAC		
<u>X</u>	Section 5.8.1			(1)	Botto cm/se	m 12 inches having hydraulic conductivity 1 x 10 ² c;
<u>X</u>	Section 5.8.2			(2)		thickness of 24 inches of material chemically resistant to aste and leachate;
<u>X</u>	Section 5.8.3			(3)	Botto	m slope design to accommodate for predicted settlement;
		<u>X</u> _		(4)	equiv	onstration that synthetic drainage material, if used, is alent or better than granular material in chemical atibility, flow under load and protection of geomembrane

^{4.} Leachate recirculation (17-701.400(5),FAC).

	LOCATION	N/A	N/C				
		_X		a.	Describe	general	procedures for recirculating leachate;
_ .		<u>X</u> _		b.			ares for controlling leachate runoff and minimizing e runoff with stormwater;
		<u>X</u> _		c.	Describe buildup;		res for preventing perched water conditions and gas
		<u>X</u>		d.	recircula	ited due t	e methods for leachate management when it cannot be so weather of runoff conditions, surface seeps, wind- elevated levels of leachate head on the liner;
		<u>x</u> _		e.	Describe		s of gas management to control odors and migration
		_X		f.	standard provide	s for leac	ation is proposed, describe treatment methods and chate treatment prior to irrigation over final cover and tation that irrigation does not contribute significantly ation.
		:	5. Leachate stor (17-701.400(-	s and leac	hate surf	ace impoundments;
		_		impound .400(6),F	dment req	uirement	s;
		<u>X</u> _			(1)		ntation that the design of the bottom liner will not be y impacted by fluctuations of the ground water;
		<u>X</u> _			(2)	_	d in segments to allow for inspection and repair as without interruption of service;
	****	<u>X</u> _			(3)	General	design requirements;
		<u>X</u> _				(a)	Double liner system consisting of an upper and lower 60-mil minimum thickness geomembrane;
		<u>X</u> _				(b)	Leak detection and collection system with hydraulic conductivity 1 cm/sec;
		<u>x</u> _				(c)	Lower geomembrane placed on sub-base 6 inches thick with k 1x10 ⁻⁵ cm/sec;
		<u>X</u> _				(d)	Design calculation to predict potential leakage through the upper liner;
		<u>X</u> _				(e)	Daily inspection requirements and notification and corrective action requirements leakage rates exceed that predicted by design calculations;
		X			(4)	Descrip	tion of procedures to prevent uplift, if applicable;

S	LOCATION	N/A N/C		
		<u>x</u>	(5)	Design calculations to demonstrate minimum two feet o freeboard will be maintained;
		<u>X</u>	(6)	Procedures for controlling vectors and off-site odors.
		b.	Above-ground leachate	e storage tanks; (17-701.400 (6) (c), FAC)
		<u>x</u>	(1)	Describe tank materials of construction and ensure foundation is sufficient to support tank;
		<u>X</u>	(2)	Describe procedures for cathodic protection if needed for th tank;
		<u>x</u>	(3)	Describe exterior painting and interior lining of the tank t protect it from the weather and the leachate stored;
		_X	(4)	Describe secondary containment design to ensure adequate capacity will be provided and compatibility of materials of construction;
		<u>X</u>	(5)	Describe design to remove and dispose of stormwater from the secondary containment system;
		_X	(6)	Describe an overall prevention system such as level sensors gauges, alarms, and shutoff controls to prevent overfilling;
			(7) Inspections, co	corrective action and reporting requirements;
		_X		(a) Overfill prevention system weekly;
		_X		(b) Exposed tank exteriors weekly;
		<u>X</u>		(c) Tank interiors when tank is drained or at least even three years;
		<u>x</u>		(d) Procedures for immediate corrective action if failure detected;
				(e) Inspection reports available for department review
		c.	Underground leachate (17-701.400 (6) (d), F	
		<u> X</u>	(1)	Describe materials of construction;
		X	(2)	A double-walled tank design system to be used with the following requirements;
		x		(a) Interstitial space monitoring at least weekly;

	LOCATION	N/A	N/C				
		<u>X</u> _				(b)	Corrosion protection provided for primary tank interior and external surface of outer shell;
		<u>x</u> _	· 			(c)	Interior tank coatings compatible with stored leachate;
		<u>x</u> _	<u> </u>			(d)	Cathodic protection inspected weekly and repaired as needed;
		_X			(3)	gauge	be an overfill prevention system such as level sensors, s, alarms, and shutoff controls to prevent overfilling and le for weekly inspections;
		<u>X</u> _			(4)	Inspec	tion reports available for department review.
		<u>X</u> _	<u> </u>	d.	Sched (e), F.	-	ded for routine maintenance of LCRS; (17-701.400 (6)
		ć	6. Liner	systems cons	struction qu	ality ass	urance (CQA): (17-701.400 (7), FAC)
			a.	Provide CQA	A Plan incl	uding:	
<u>X</u>	Section 5.9.1				(1)	Specia	ications and construction requirements for liner system;
- -	Section 5.9.1				(2)		ed description of quality control testing procedures and encies;
<u>x</u>	Section 5.9.1				(3)	Identi	fication of supervising professional engineer;
<u>X</u>	Section 5.9.1				(4)		fy responsibility and authority of all appropriate izations and key personnel involved in the construction at;
<u>X</u>	Section 5.9.1				(5)	State person	qualifications of CQA professional engineer and support nnel;
<u>X</u>	Section 5.9.1				(6)	Descr	iption of CQA reporting forms and documents;
<u>X</u> ·	Section 5.9.2			b.			nt laboratory experienced in the testing of geosynthetics juired testing;
		,	7. Surfa	ce Water mar	nagement s	ystems;	(17-701.400 (9), FAC)
X	Section 5.10			a.			face water management system to isolate surface water led areas and to control stormwater run-off;
<u>X</u>	Section 5.10			b.			ormwater control design including retention ponds, ds, and drainage ways;
			8. Gas c	ontrol systen	ns; (17-70)	1.400 (10)), FAC)
	Section 5.11	X		· a.			for gas control system including collection pipes and sive venting or vacuum extraction details;

S	LOCATION	N/A	N/C				
-		<u>X</u>			b.		entation that the gas control system will not impact the liner or e control system;
 -		_X			c.		ed methods of odor control including flaring designs in ance with Chapter 17-210, FAC;
			d.		otion of a y includ		gas monitoring program to ensure gas control system is operating
		_X				(1)	Location of monitoring points;
		<u>X</u>				(2)	Requirements for quarterly sampling of all monitoring points;
		<u>X</u>				(3)	Description of corrective measures to be completed within 60 days of detection of elevated levels of explosive gases;
		<u>X</u>			e.	Descri	ption of condensate collection and disposal methods.
			9. Lan	dfill gas	recovery	/ facilitie	s; (17-701.400 (11), FAC)
		<u>X</u>			a.	Inform supplie	nation required in Rules 17-701.320 (7) and 17-701.330 (4), FAC ed;
		<u>X</u>			b.		nation required in Rule 17-701.600 (4), FAC supplied where nt and practical;
		X			c.		ate of current and expected gas generation rates and description densate disposal methods provided;
		X			d.		ption of procedures for condensate sampling, analyzing and dataing provided;
		X			e.		re plan provided describing methods to control gas after recovery v ceases operation;
		<u> </u>			f.		mance bond provided to cover closure costs if not already ed in other landfill closure costs.
		<u> </u>		10.	will p	rovide a	esigned in ground water, provide documentation that the landfill degree of protection equivalent to landfills designed with bottom ontact with ground water; (17-701.400 (12), FAC)
Н.	HYDROGEOL	OGICAI	L INVE	STIGAT	TON RI	EQUIRE	MENTS (17-701.410, FAC)
				omit a h		logical i	nvestigation and site report including at least the following
X	Section 6.1.1		<u>.</u>	_	a.	Regio	nal and site specific geology and hydrogeology;
<u>x</u>	Section 6.1.2			_	b.		tion and rate of ground-water and surface-water flow including

	LOCATION	N/A	N/C		
X	Section 6.1.3			c.	Background quality of ground water and surface water;
X	Section 6.1.4			d.	Any on-site hydraulic connections between aquifers;
<u>X</u>	Section 6.1.5			e.	Site stratigraphy and aquifer characteristic for confining layers, semi- confining layers, and all aquifers below the landfill site that may be affected by the landfill;
X	Section 6.1.6			f.	Site topography and soil characteristics;
X	Section 6.1.7		_	g.	Inventory of all public and private water wells within a one-mile radius of the landfill including well top of casing and bottom elevations, name of owner, age and usage of each well, stratigraphic unit screened, well construction technique and static water level;
	Section 6.1.8	X_		h.	Existing contaminated areas on landfill site.
<u>x</u>	Section 6.2		2.	Repor	t signed, sealed and dated by PE or PG.
I.	GEOTECHNICA	L INVE	STIGATI	ON REQUI	REMENTS (17-701.420, FAC)
		1		a geotechnica g at least the	al site investigation report defining the engineering properties of the site following:
<u>X</u>	Section 7.1			a.	Description of subsurface conditions including soil stratigraphy and ground-water table conditions;
X	Section 7.2			b.	Investigate for the presence of muck, previously filled areas, soft ground, lineaments and sink holes;
<u>X</u>	Section 7.3			c.	Estimates of average and maximum high water table across the site;
			d. Fo	oundation and	alysis including:
<u>X</u>	Section 7.4.1		****		(1) Foundation bearing capacity analysis;
<u>X</u>	Section 7.4.2				(2) Total and differential subgrade settlement analysis;
<u>X</u>	Section 7.4.3				(3) Slope stability analysis;
<u>X</u>	Section 7.5			e.	Description of methods used in the investigation and includes soil boring logs, laboratory results, analytical calculations, cross sections, interpretations and conclusions;
<u>X</u>	Section 7.6			f.	Report signed, sealed and dated by PE or PG.
J.	VERTICAL EX	PANSIO	N OF LAI	NDFILLS (17-701.430, FAC)
		<u>X</u>	1.	leakag	ibe how the vertical expansion shall not cause or contribute to leachate ge from the existing landfill or adversely affect the closure design of the ng landfill;

S	LOCATION	N/A	N/C	
		<u>X</u> _	2.	Describe how the vertical expansion over unlined landfills will meet the requirements of Rule 17-701.400, FAC with the exceptions of Rule 17-701.430 (1) (c), FAC);
		<u>X</u> _	3.	Provide foundation and settlement analysis for the vertical expansion;
		_X	4.	Provide total settlement calculations demonstrating that the final elevations of the lining system, that gravity drainage, and that no other component of the design will be adversely affected;
		<u>X</u> _	5.	Minimum stability safety factor of 1.5 for the lining system component interface stability and deep stability;
		<u>X</u> _	6.	Provide documentation to show the surface water management system will not be adversely affected by the vertical expansion;
		<u>X</u> _	7.	Provide gas control designs to prevent accumulation gas under the new liner for the vertical expansion.
K.	LANDFILL O	PERATIO	N REQUIRE	MENTS (17-701.500, FAC)
<u>X</u>	Section 8.1		1.	Provide documentation that landfill will have at least one trained operator during operation and at least one trained spotter at each working face; (17-701.500 (1), FAC)
		2	2. Provide a	landfill operation plan including procedures for: (17-701.500 (2), FAC)
<u>X</u>	Section 8.2.1			a. Designating responsible operating and maintenance personnel;
<u>X</u>	Section 8.2.2			b. Contingency operations for emergencies;
<u>X</u>	Section 8.2.3			c. Controlling types of waste received at the landfill;
<u>X</u>	Section 8.2.4			d. Weighing incoming waste;
<u>X</u>	Section 8.2.5			e. Vehicle traffic control and unloading;
<u>X</u>	Section 8.2.6			f. Method and sequence of filling waste;
<u>X</u>	Section 8.2.7			g. Waste compaction and application of cover;
<u>X</u>	Section 8.2.8			h. Operations of gas, leachate, and stormwater controls;
<u>X</u>	Section 8.2.9			i. Water quality monitoring.
<u>X</u>	Section 8.3		3.	Provide a description of the landfill operation record to be used at the landfill (17-701.500 (3), FAC)
<u>X</u>	Section 8.4		4.	Describe the waste records that will be compiled monthly and provided to the Department quarterly; (17-701.500 (4), FAC)

	LOCATION	N/A	N/C		
<u>x</u> _	Section 8.5		5.	Describ	pe methods of access control; (17-701.500 (5), FAC)
X	Section 8.6	<u> </u>	6.	Describ disposa	be load checking program to be implemented at the landfill to discourage al of unauthorized wastes at the landfill; (17-701.500 (6), FAC)
		7	7. Describer pr 701.500 (7),		for spreading and compacting waste at the landfill that include: (17-
<u>X</u>	Section 8.7.1			a.	Waste layer thickness and compaction frequencies;
X	Section 8.7.2			b.	Special considerations for first layer of waste placed above liner and leachate collection system;
X	Section 8.7.3			c.	Slopes of disposal unit working face and side grades above land surface, planned lift depths during operation;
X	Section 8.7.4			d.	Maximum width of working face;
X	Section 8.7.5		·	e.	Procedures for applying initial cover including minimum cover frequencies;
S	LOCATION	N/A I	N/C		
	Section 8.7.6			f.	Procedures for applying intermediate cover;
X	Section 8.7.7			g.	Time frames for applying final cover;
X	Section 8.7.8			h.	Description of litter policing methods;
<u>X</u>	Section 8.7.9			i.	Erosion control procedures.
		8	8. Describe op	erational	procedures for leachate management including; (17-701.500 (8), FAC)
X	Section 8.8.1			a.	Leachate level monitoring, sampling, analysis and data results submitted to the Department;
<u>X</u>	Section 8.8.2			b.	Operation and maintenance of leachate collection and removal system, and treatment as required;
<u>X</u>	Section 8.8.3			c.	Procedures for managing leachate if it becomes regulated as a hazardous waste;
X	Section 8.8.4			d.	Agreements for off-site discharge and treatment of leachate;
<u>X</u>	Section 8.8.5			e.	Contingency plan for managing leachate during emergencies or equipment problems;
<u>X</u>	Section 8.8.6			f.	Procedures for recording quantities of leachate generated in gal/day;
X	Section 8.8.7			g.	Procedures for comparing precipitation experienced at the landfill with

S	LOCATION	N/A	N/C		
<u>X</u>	Section 8.9			9.	Describe routine gas monitoring program for the landfill as required by Rule 17-701.400 (10), FAC; (17-701.500 (9), FAC)
<u>X</u>	Section 8.10			10.	Describe procedures for operating and maintaining the landfill stormwater management system to comply with the standards of Chapters 17-3, 17-302, and 17-25, FAC; (17-701.500 (10), FAC)
		1	1.	Equipm	nent and operation feature requirements; (17-701.500 (11), FAC)
<u>X</u>	Section 8.11.1				a. Sufficient equipment for excavating, spreading, compacting and covering waste;
<u>X</u>	Section 8.11.2				b. Reserve equipment or arrangements to obtain additional equipment within 24 hours of breakdown;
<u>X</u>	Section 8.11.3				c. Communications equipment;
<u>X</u>	Section 8.11.4				d. Personnel shelter and sanitary facilities, first aid equipment;
<u>X</u>	Section 8.11.5				e. Dust control methods;
<u>X</u>	Section 8.11.6				f. Fire protection capabilities and procedures for notifying local fire department authorities in emergencies;
- <u>X</u>	Section 8.11.7				g. Litter control devices;
<u>X</u>	Section 8.11.8				h. Signs indicating operating authority, traffic flow, hours of operation, disposal restrictions.
<u>X</u>	Section 8.12			12.	Provide a description of all-weather access road, inside perimeter road and other roads necessary for access which shall be provided at the landfill; (17-701.500 (12), FAC)
			13.	Additio	onal record keeping and reporting requirements; (17-701.500 (13), FAC)
<u>X</u>	Section 8.13.1				a. Records used for developing permit applications and supplemental information maintained for the design period of the landfill;
<u>x</u>	Section 8.13.2				b. Monitoring information, calibration and maintenance records, copies of reports required by permit maintained for at least 10 years;
<u>X</u>	Section 8.13.3				c. Background water quality records shall be maintained for the design period of the landfill;
<u>X</u>	Section 8.13.4	-			d. Maintain annual estimates of the remaining life of constructed landfills and of other permitted areas not yet constructed and submit this estimate annually to the Department.

LOCATION N/A N/C

L.	WATER QUALIT	Y AND	LEAG	CHATE MONIT	ORING	REQUIREMENTS (17-701.510, FAC)
X	Section 9.1			propose	d ground	nd leachate monitoring plan shall be submitted describing the d water, surface water and leachate monitoring systems and shall e following requirements;
<u>X</u>	Section 9.1.1			a.	and sig	on the information obtained in the hydrogeological investigation ened, dated and sealed by the PG or PE who prepared it; (17-0 (2) (a), FAC)
<u>X</u>	Section 9.1.2			b.	Depart	ampling and analysis performed by organizations having ment approved Comprehensive Quality Assurance Plans; (17-0 (2) (b), FAC)
X	Section 9.1.3			c.	Ground	d-water monitoring requirements;(17-701.510 (3), FAC)
<u>X</u> _	Section 9.1.3.1		 		(1)	Detection wells located downgradient from and within 50 feet of disposal units;
X	Section 9.1.3.2				(2)	Downgradient compliance wells as required;
<u>X</u>	Section 9.3.1.3				(3)	Background wells screened in all aquifers below the landfill that may be affected by the landfill;
<u> </u>	Section 9.3.1.4				(4)_	Location information for each monitoring well;
<u>X</u> _	Section 9.3.1.5			-	(5)	Well spacing no greater than 500 feet apart for downgradient wells and no greater than 1500 feet apart for upgradient wells unless site specific conditions justify alternate well spacings;
<u>x</u> _	Section 9.3.1.6			-	(6)	Well screen locations properly selected;
<u>X</u> _	Section 9.1.3.7			-	(7)	Procedures for properly abandoning monitoring wells;
	Section 9.3.1.8	_X_		-	(8)	Detailed description of detection sensors if proposed.
			d.	Surface-water m (17-701.510 (4)		ng requirements;
<u>X</u>	Section 9.1.4.1			-	(1)	Location of and justification for all proposed surface-water monitoring points;
<u>X</u>	Section 9.1.4.2			-	(2)	Each monitoring location to be marked and its position determined by a registered Florida land surveyor;
<u>X</u>	Section 9.1.5			e. (17-701.510 (5)		ate sampling locations proposed;
			f.	Routine samplir (17-701.510 (6)		ency and requirements;

S	LOCATION	N/A	N/C				
<u>X</u> _	Section 9.1.6.1		. <u> </u>			(1)	Background ground-water and surface-water sampling and analysis requirements;
<u>X</u>	Section 9.1.6.2		·			(2)	Leachate quarterly and annual sampling and analysis requirements;
X	Section 9.1.6.3					(3)	Detention well quarterly sampling and analysis requirements;
<u>X</u>	Section 9.1.6.4					(4)	Compliance well sampling and analysis as per requirements;
S	LOCATION	N/A	N/C				
<u>X</u>	Section 9.1.6.5					(5)	Surface-water sampling and analysis requirements.
<u>X</u>	Section 9.1.7				g.		pe procedures for implementing assessment monitoring and ive action as required; (17-701.510 (7), FAC)
			h.	Water	quality m	onitorin	g report requirements; (17-701.510 (9), FAC)
<u>x</u>	Section 9.1.8.1		·			(1)	Quarterly report requirements;
<u>X</u>	Section 9.1.8.2					(2)	Annual report requirements including a report submitted to the Department which is signed, dated and sealed by PG or PE.
M.	SPECIAL WAS	TE HAN	DLING	REQU	IREME	NTS (17	7-701.520, FAC)
_		<u>X</u>		1.	Describ	e proced	lures for managing motor vehicles; (17-701.510 (1), FAC
		<u>X</u>		2.	Describ	oe proced	lures for landfilling shredded waste; (17-701.520 (3), FAC)
		<u>X</u>	···	3.	Describ	oe proced	dures for asbestos waste disposal; (17-701.520 (4), FAC)
N.	LANDFILL FI	NAL CLO	SURE	REQUI	REMEN	TS (17	-701.600, FAC)
			1. Clos	ure sche	dule requ	iirements	s; (17-701.600 (2), FAC)
<u>X</u>	Section 10.1	 .			a.	will be	nentation that a written notice including a schedule for closure provided to the Department at least one year prior to final of wastes;
<u>X</u>	Section 10.1				b.	Notice	to user requirements within 120 days of final receipt of wastes;
<u>X</u>	Section 10.1				c.	Notice	to public requirements within 10 days of final receipt of wastes.
			2. Clos	ure pern	nit genera	al require	ements; (17-701.600 (3), FAC)
<u>X</u>	Section 10.2				a.		ation submitted to Department at least 90 days prior to final of wastes;

	LOCATION	N/A	N/C				
			b.	Closure plan sha	ll includ	e the following;	
X	Section 10.2				(1)	Closure report;	
X	Section 10.2				(2)	Closure design plan;	
<u>X</u>	Section 10.2				(3)	Closure operation plan;	
S	LOCATION	N/A N	I/C				
X	Section 10.2		 		(4)	Closure procedures;	
<u>X</u>	Section 10.2				(5)	Plan for long term care;	
X	Section 10.2				(6)	A demonstration that proof of financial responsibility for long term care will be provided.	
	3. Closure report requirements; (17-701.600 (4), FAC)						
	a. General information requirements;						
	Section 10.3	X			(1)	Identification of landfill;	
_	Section 10.3	<u>X</u>			(2)	Location, description and vicinity map;	
	Section 10.3	<u>X</u> _			(3)	Total acres of disposal areas and landfill property;	
	Section 10.3	X_			(4)	Legal property description;	
	Section 10.3	<u>X</u> _			(5)	History of landfill;	
	Section 10.3	X_			(6)	Identification of types of waste disposed of at the landfill.	
—	Section 10.3	X		b.		hnical investigation report and water quality monitoring planed by Rule 17-701.330 (4), FAC);	
—	Section 10.3	<u> X</u>		c.	Land use information report indicating: identification of adjacent landowners; zoning; present land uses; and roads, highways right-of-way, or easements.		
	Section 10.3	X		_ d.	biodeg	on actual or potential gas migration at landfills containing radable wastes including detailed description of test and gation methods used;	
	Section 10.3	X		_ e.	includi water	assessing the effectiveness of the landfill design and operationing results of geotechnical investigations, surface water and storm management, gas migration and concentrations, condition of g cover, and nature of waste disposed of at the landfill;	
	4. Closure design requirements to be included in the closure design plan: (17-701.600 (5), FAC						
<u>X</u>	Section 10.4.1		_	_ a.	Plan sl	heet showing phases of site closing;	

S	LOCATION	N/A	N/C				
<u>X</u>	Section 10.4.2			b.	Drawing	gs showing existing topography and proposed final grades;	
<u>X</u>	Section 10.4.3	- —	-	_ c.	Provisio	ons to close units when they reach approved design dimensions;	
<u>X</u>	Section 10.4.4	- —		d.	Final elevations before settlement;		
<u>X</u>	Section 10.4.5			_ e.	Side slope design including benches, terraces, down slope drainage ways, energy dissipators and discussion of expected precipitation effects;		
			f.	Final cover insta	allation p	lans including:	
	Section 10.4.6	<u>X</u>		-	(1)	CQA plan for installing and testing final cover;	
<u>X</u>	Section 10.4.6			-	(2)	Schedule for installing final cover after final receipt of waste;	
<u>X</u>	Section 10.4.6			-	(3)	Description of drought-resistant species to be used in the vegetative cover;	
<u>X</u>	Section 10.4.6		<u></u>	_	(4)	Top gradient design to maximize runoff and minimize erosion;	
<u>X</u>	Section 10.4.6			-	(5)	Provisions for cover material to be used for final cover maintenance.	
	- .		g.	Final cover desi	gn requir	rements:	
<u>X</u>	Section 10.4.7			_	(1)	Protective soil layer design;	
_	Section 10.4.7	<u>X</u>		_	(2)	Barrier soil layer design;	
<u>X</u>	Section 10.4.7			_	(3)	Erosion control vegetation;	
<u>X</u>	Section 10.4.7			_	(4)	Geomembrane barrier layer design.	
<u>X</u>	Section 10.4.8	_		_ h.	Propose	ed method of stormwater control;	
<u>X</u>	Section 10.4.9	_		_ i.	Proposed method of access control;		
<u>X</u>	Section 10.4.10			_ j.	Description of proposed final use of the closed landfill, if any;		
		5. Closure operation plan shall include: (17-701.600 (6), FAC)					
<u>X</u>	Section 10.5.1			_ a.	Detaile	d description of actions which will be taken to close the landfill;	
<u>X</u>	Section 10.5.2			_ b.	Time se	chedule for completion of closing and long term care;	
<u>X</u>	Section 10.5.3			_ c.	Describ	be proposed method for demonstrating financial responsibility;	
<u>X</u>	Section 10.5.4	_	. <u> </u>	_	d.	Indicate any additional equipment and personnel needed to complete closure.	

	LOCATION	N/A	N/C				
		<u>X</u>	6.	Justification for and detailed description of procedures to be following for temporary closure of the landfill, if desired; (17-701.600 (7), FAC)			
o.	CLOSURE PROCEDURES (17-701.610, FAC)						
	Section 11.0	X	1.	Survey monuments; (17-701.610 (2), FAC)			
	Section 11.0	<u>X</u>	2.	Final survey report; (17-701.610 (3), FAC)			
	Section 11.0	X	3. (17-701.61	Certification of closure construction completion; 0 (4), FAC)			
	Section 11.0	_ <u>X</u> _	4.	Declaration to the public; (17-701.610 (5), FAC)			
	Section 11.0	<u>X</u>	5.	Official date of closing (17-701.610 (6), FAC)			
	Section 11.0	_ <u>X</u> _	6.	Use of closed landfill areas; (17-701.610 (7), FAC)			
P.	LONG TERM CARE REQUIREMENTS (17-701.620, FAC)						
	Section 12.0	X	1. (17-701.62	Right of property access requirements; 0 (4), FAC)			
_	Section 12.0	<u>X</u>	2.	Successors of interest requirements; (17-701.620 (5), FAC)			
-	Section 12.0	X	3.	Requirements for replacement of monitoring devices; (17-701.620 (7), FAC)			
Q.	FINANCIAL R	ESPONSI	BILITY REG	QUIREMENTS (17-701.630, FAC)			
<u>X</u>	Section 13.1		1.	Provide cost estimates for closure costs and long term care costs estimated by a PE for a third party performing the work, on a per unit basis, with the source of estimates indicated; (17-701.630 (3), FAC)			
<u>X</u>	Section 13.2		2.	Describe procedures for providing annual cost adjustments to the Departmen based on inflation and changes in the closure and long-term care plans; (17-701.630 (4), FAC)			
<u>X</u>	Section 13.3		3.	Describe funding mechanisms for providing proof of financial responsibility and include appropriate financial responsibility forms; (17-701.630 (5) & (6), FAC			
R.	VOLUME RED	UCTION	PLANT RE	QUIREMENTS (17-701.700, FAC)			
_		<u>X</u>	1.	Proof of posting a performance bond payable to the Department to cover closing costs, if required; (17-701.700 (4), FAC)			
		<u>X</u>	2. (17-701.70	Materials recovery facility requirements; 00, FAC)			
		<u>X</u>		a. Submit information required in Rule 17-701.320, FAC			
		X		b. Submit an engineering report including the following:			

S	LOCATION	N/A N/	C		
		_X	(1)		otion of the solid waste proposed to be collected, stored, ed or disposed;
		<u>X</u>	(2)		ion with assumptions for waste types and quantities d in future years;
 -		_ <u>x</u>	(3)		otion of operation and functions of all processing ent with design criteria and expected performance;
		_X	(4)	operati	otion of flow of solid waste, expected regular facility ons, procedures for start up and shut down, potential nazards and control methods including fire protection;
		_X	(5)	Descrip	otion of loading, unloading, processing areas;
		<u>X</u>	(6)	for ma	cation and capacity of temporary on-site storage areas aterials handled and provisions for solid waste and e containment;
		<u>x</u>	(7)		cation of potential ground-water and surface-water ination;
		<u>X</u>	(8)		or disposal of unmarketable recyclables and residue and gencies for waste handling during breakdowns.
		c.	Submit the following of	perational	information:
		_X	(1)	Operat	ion and maintenance manual;
		_X	(2)	Waste	control plan to manage unauthorized wastes;
		_X	(3)	Contin	gency plan for emergencies;
		_X	(4)	Closur	e plan including the following:
	·	_ <u>X</u>		(a)	Notification to Department 180 days prior to closure;
		_X		(b)	Procedures for removal of all waste within 30 days of receipt of final waste;
		<u>X</u>		(c)	Completion of closure activities within 180 days of receipt of final waste and notification to the Department that closure is complete.

S. CERTIFICATION BY APPLICANT AND ENGINEER OR PUBLIC OFFICER

A. Applicant

The undersigned applicant or authorized representative of <u>Pasco County</u>, <u>Florida</u> is aware that statements made in this form and attached information are an application for a <u>landfill construction</u> Permit from the Florida Department of Environmental Regulation and certifies that the information in this application is true, correct and complete to the best of his knowledge and belief. Further, the undersigned agrees to comply with the provisions of Chapter 403, Florida Statutes,

Resource Recovery Facility - Class I Landfill Law Environmental Project 573565

S. CERTIFICATION BY APPLICANT AND ENGINEER OR PUBLIC OFFICER

A. Applicant

The undersigned applicant or authorized representative of <u>Pasco County</u>, <u>Florida</u> is aware that statements made in this form and attached information are an application for a <u>landfill construction</u> Permit from the Florida Department of Environmental Regulation and certifies that the information in this application is true, correct and complete to the best of his knowledge and belief. Further, the undersigned agrees to comply with the provisions of Chapter 403, Florida Statutes, and all rules and regulations of the Department. It is understood that the Permit is not transferable, and the Department will be notified prior to the sale or legal transfer of the permitted facility.

Signature of Applicant or Agent

Douglas S. Bramlett, Assistant County Administractor Name and Title

Tunic and Time

Date: July 8, 1994

Attach letter of authorization if agent is not a governmental official, owner, or corporate officer.

B. Professional Engineer Registered in Florida or Public Officer as required in Section 403.707 and 403.707 (5), Florida Statutes.

This is to certify that the engineering features of this solid waste management facility have been designed/examined by me and found to conform to engineering principals applicable to such facilities. In my professional judgement, this facility, when properly maintained and operated, will comply with all applicable statutes of the State of Florida and rules of the Department. If it agreed that the undersigned will provide the applicant with a set of instructions of proper maintenance and operation of the facility.

Richard & Mary

4919 West Laurel Street

Mailing Address

ichard E. Mayer, Principal Engineer

Tampa, Florida 33607

City, State, Zip Code

ame and Title (please type)

(813) 289-0750

Telephone Number

Date: July 8, 1994

DER Form 17-701.900(1)

Effective 01-06-93

SECTION 2.0

SOLID WASTE MANAGEMENT FACILITY PERMIT GENERAL REQUIREMENTS

SECTION 2.0

SOLID WASTE MANAGEMENT FACILITY PERMIT GENERAL REQUIREMENTS

- 2.1 Six copies at minimum, of the completed application form with all supporting data and reports Six copies are provided.
- 2.2 Engineering and/or professional certification (signature, date and seal) provided on the applications and all engineering plans, reports and supporting information for the application

The data have be properly sealed.

~2.3 A letter of transmittal to the Department

The transmittal letter has been provided.

✓2.4 A completed application dated and signed by the applicant

The application is complete and signed.

Permit fee specified in Rule 17-4.050,FAC and Rule 17-7-1.320(5)(c), FAC in check or money order, payable to the Department

Not required.

An engineering report addressing the requirements of this rule and with the following format: a cover sheet, text printed on 8 1/2 inch by 11 inch consecutively numbered pages, a table of contents or index, the body of the report and all appendices including an operation plan, contingency plan, illustrative charts and graphs, records or logs of tests and investigations, engineering calculations.

The Engineering Report is provided in Sections 3 - 13.

2.7 Operation Plan

The Operation Plan is provided in Appendix 8

2.8 Contingency Plan

Solid waste due to a natural disaster will be stockpiled for later removal to the Resource Recovery Facility, or debris may be isolated and burned in an area adjacent to the operational phase in an unused area. Cooled ash would then be landfilled. Special access roads can be built at the landfill to allow normal operations under adverse conditions. Cooperative lending agreement with other Pasco County departments will be made for backup equipment and manpower.

▶2.9 Plans or drawings for the solid waste management facilities in appropriate format (including sheet size restrictions, cover sheet, legends, north arrow, horizontal and vertical scales, elevations referenced to NGVD) showing:

A regional map or plan - See Drawing Sheet No. 2

A vicinity map - See Drawing Sheet No. 4

A site plan with property boundaries certified by a registered land surveyor - See Drawing Sheet No. 4.

Drawing sheets to support the engineering report - Refer to the attached drawing set.

✓2.10 Proof of property ownership or a copy of appropriate agreements between the facility operator and property owner authorizing use of property -

Not applicable, proof provided in Power Plant Site Certification Application

2.11 For facilities owned or operated by a county, provide a description of how, if any, the facilities covered in this application will contribute to the county's achievement of recycling goals.

The facility is an integral part of comprehensive solid waste management plan which includes numerous recycling programs designed to assist the County in meeting State goals.

2.12 Provide a history and description of any enforcement actions taken by the Department against the applicant for violations of applicable statutes, rules, orders or permit conditions relating to the operation of any solid waste management facility in this state. -

If required information will be provided by the County under separate cover.

2.13 Proof of publication in a newspaper of general circulation of notice of application for a permit to construct or substantially modify a solid waste management facility

Not applicable, provided in Power Plant Site Certification permit application.

2.14 Provide a description of how the requirements for airport safety will be achieved including proof of required notices if applicable.

Not applicable, provided in Power Plant Site Certification permit application.

SECTION 3.0

LANDFILL PERMIT GENERAL REQUIREMENTS

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SECTION 3.0

LANDFILL PERMIT GENERAL REQUIREMENTS

Vicinity map or aerial photograph, no more than one year old and of appropriate scale, showing land use and local zoning within one mile of the landfill and of sufficient scale to show all homes or other structures, water bodies, roads, and other significant features of the vicinity. All significant features should be labeled; (Ch. 17-701.330 (4) (a), FAC).

Aerial photographs of the site were provided with the Application for Power Plant Site Certification on Drawing Sheets Nos. 2, 3, 4, and 5. Reductions of these original drawings are provided in Appendix 3. No expansion of the landfill area is being requested. Disposal Unit A-2 is within the 160 acre footprint of permitted landfill, which is well buffered inside the 800 acre site.

Vicinity map or aerial photograph, no more than one year old, showing all airports that are located within five miles of the proposed landfill: (Ch. 17-701.330 (4) (b), FAC).

The landfill is located more than ten thousand feet from any airport, as stated in the Power Plant Site Certification and shown on Drawing Sheet No. 2, Appendix 3. The nearest airport, Pilots County Estates Airport, is located approximately 10 miles southeast of the landfill near U.S. Highway 41 and State Road 52. Hidden Lake Airport (PVT) is located approximately 14 miles southwest of the landfill near Moon Lake Road and the Hernando County Airport is approximately 15 miles to the northeast.

3.3 Plot Plan with a scale not greater than 200 feet to the inch showing: (Ch. 17-701.330(4)(c), FAC).

3.3.1 Dimensions

Dimensions of Disposal Unit A-2 are shown on Drawing Sheet No. 5 of this application. Disposal Unit A-2 dimensions are approximately 390 feet by 970 feet. The disposal unit covers approximately ten acres.

3.3.2 Locations of Proposed and Existing Water Quality Monitoring Wells

Water quality monitoring wells are shown on Drawing Sheet No. 4 and on Figure 1 in Appendix 9. The Florida Aquifer monitoring wells are designated 4MW and the Surficial Aquifer monitoring wells 2MW.

3.3.3 Location of Soil Borings

Location of the soil borings are shown on Drawing Sheet No. 6.

3.3.4 Proposed Plan of Trenching or Disposal Areas

The location of Disposal Unit A-2 is shown on Drawing Sheet No. 5. The facility layout, depicting location of sixteen contiguous disposal units within the permitted foot print of the landfill, is shown on reduced copies of the Application for Site Certification Permit Drawings provided in Appendix 3.0 (see Drawing Sheet No. 8).

3.3.5 Cross Sections Showing Original Elevations and Proposed Final Contours Which Shall Be Included on the Plot plan or on Separate Sheet.

Original elevations and contours for Disposal Unit A-2 are shown on Drawing Sheet No. 7. Original elevations and proposed final contours for the entire permitted landfill remain unchanged from the closure plan submitted in Application for Power Plant Site Certification (see Appendix 3.0, Drawing Sheet Nos. 9, 11, and 15). Proposed final elevation is 152.0 feet NGVD, which is approximately 100 feet above surrounding ground level at 48 feet NGVD.

3.3.6 Any previously Filled Waste Disposal Areas

To date only Disposal Unit A-1 has received solid waste (see Drawing Sheet No. 5).

3.3.7 Fencing or Other Measures to Restrict Access

A perimeter fence, constructed of barbed wire and chain link, encloses the entire 800 acre site. Internally, the Resource Recovery Facility, the West Pasco Class III Landfill and Recycling Center and the West Pasco Class I Landfill are separated by additional chain link fencing. The Resource Recovery Facility and the West Pasco Class III Landfill, and Recycling Center have separate entrance gates that restrict access to the sites. No external access to the West Pasco Class I Landfill exists. Entry into the West Pasco Class I Landfill can be made through internal gates which are locked during non-operating hours: (See Drawing Sheet No. 4).

3.4 Topographic Maps with a Scale Not Greater than 200 feet to the inch with 5-foot Contour Intervals Showing:

3.4.1 Proposed Fill Areas

The layout for the planned disposal units are shown on the permit drawings submitted with the Power Plant Site Certification Permit Application (see Appendix 3, Drawing Sheet Nos. 8 and 9). The proposed fill area, Disposal Unit A-2, is located adjacent to Disposal Unit A-1 and is shown on Drawing Sheet No. 7.

3.4.2 Borrow Areas

Borrow areas remain unchanged from the Power Plant Site Certification Application and are shown on the permit drawings submitted with that application (See Appendix 3, Drawing Sheet No. 7).

3.4.3 Access Roads

The access road is shown on Drawing Sheet Nos. 4 and 5 and with contours on Drawing Sheet No. 7. Access into Disposal Unit A-2 will be through the entrance ramp to Disposal Unit A-1.

3.4.4. Grades Required for Proper Drainage

The stormwater management system has been constructed. Currently swales around Disposal Unit A-1 direct stormwater runoff from surrounding area through a cultvert under the access road in a stormwater retention pond. Existing swales will be modified to direct stormwater runoff from areas surrounding both disposal units to the retention pond. Both the existing swales and the proposed modifications are shown on Drawing Sheet No. 7.

3.4.5 Cross Sections of Lifts

See Appendix 3, Drawing Sheet No. 10

3.4.6 Special Drainage Devices If Necessary

A temporary side slope riser and pump arrangement will be used to pump uncontaminated stormwater from the disposal unit during the initial stages of filling. The bottom of the disposal unit is sloped from the southwest corner to the northeast corner. The leachate collection system piping will be laid to drain the same direction. Filling of the unit will begin in the east (lowest) part and proceed westward. Therefore, stormwater that accumulates in the areas to the west of the filling area would not come into contact with the ash and can be removed and disposed of without treatment. In order to accomplish the removal, two valves will be placed in the primary leachate collection header pipe and be closed to block flows from upstream collector pipes. One, valve will be located at a point approximately one-third of the distance from the west end of the disposal unit. The second valve will be located at two-thirds of the distance (refer to Drawing Sheet No. 8). A vertical piece of geomembrane extending from the surface of the drainage layer down two feet to the primary liner will block upstream stormwater flow (see Drawing Sheet No. 10). This geomembrane piece or "flap" will be long enough to extend across the disposal unit width and will be welded to the top liner. Stormwater will be pumped from the area upstream from the closed valve and geomembrane flap using a side slope riser and pump (refer to Section 5 for a description of the side slope riser and pump). Water pumped from the disposal unit will be piped across the width of the disposal unit and discharged into the swale on the south side of the unit. The side slope riser will initially be located at the valve nearest the west end. Then, as filling of the unit proceeds westward to the first withdrawal point the pump will be removed from the first riser and inserted into the riser at the next withdrawal point. Refer to Drawing Sheet Nos. 13 and 14 for details.

The pump is sized to move the volume of water that will accumulate from a one in 25-year, 24-hour storm and an area two-thirds of the total unit size or 1.6 million gallons in seven days.

3.4.7 Fencing

See response to 3.3.7.

3.4.3 Access Roads

The access road is shown on Drawing Sheet Nos. 4 and 5 and with contours on Drawing Sheet No. 7. Access into Disposal Unit A-2 will be through the entrance ramp to Disposal Unit A-1.

3.4.4. Grades Required for Proper Drainage

The stormwater management system has been constructed. Currently swales around Disposal Unit A-1 direct stormwater runoff from surrounding area through a culvert under the access road in a stormwater retention pond. Existing swales will be modified to direct stormwater runoff from areas surrounding both disposal units to the retention pond. Both the existing swales and the proposed modifications are shown on Drawing Sheet No. 7.

3.4.5 Cross Sections of Lifts

See Appendix 8, Figure 2.

3.4.6 Special Drainage Devices If Necessary

A temporary side slope riser and pump arrangement will be used to pump uncontaminated stormwater from the disposal unit during the initial stages of filling. The bottom of the disposal unit is sloped from the southwest corner to the northeast corner. The leachate collection system piping will be laid to drain the same direction. Filling of the unit will begin in the east (lowest) part and proceed westward. Therefore, stormwater that accumulates in the areas to the west of the filling area would not come into contact with the ash and can be removed and disposed of without treatment. In order to accomplish the removal, two valves will be placed in the primary leachate collection header pipe and be closed to block flows from upstream collector pipes. One, valve will be located at a point approximately one-third of the distance from the west end of the disposal unit. The second valve will be located at two-thirds of the distance (refer to Drawing Sheet No. 8). A vertical piece of geomembrane extending from the surface of the drainage layer down two feet to the primary liner will block upstream stormwater flow (see Drawing Sheet No. 10). This geomembrane piece or "flap" will be long enough to extend across the disposal unit width and will be welded to the top liner. Stormwater will be pumped from the area upstream from the closed valve and geomembrane flap using a side slope riser and pump (refer to Section 5 for a description of the side slope riser and pump). Water pumped from the disposal unit will be piped across the width of the disposal unit and discharged into the swale on the south side of the unit. The side slope riser will initially be located at the valve nearest the west end. Then, as filling of the unit proceeds westward to the first withdrawal point, the pump will be removed from the first riser and inserted into the riser at the next withdrawal point. Refer to Drawing Sheet Nos. 13 and 14 for details.

The pump is sized to remove the volume of water that will accumulate from a one in 25-year, 24-hour storm and an area two-thirds of the total unit size or 1.6 million gallons in seven days.

3.4.7 Fencing

See response to 3.3.7.

3.4.8 Equipment Facilities

A maintenance facility is located on-site immediately southwest of landfill adjacent to the access road (see Drawings Sheet Nos. 4 and 5). The facility consists of two work bays for repair and maintenance of heavy equipment and office and restroom facilities for the landfill supervisor and staff.

3.5 Report on the Landfill Describing the Following (17-701.330(4)(e), FAC)

3.5.1 Current and Projected Population and Area to be Served by the Proposed Site;

The area to be served by the West Pasco Class I Landfill includes the incorporated and unincorporated areas of Pasco County. Each of the six municipalities located within the County has an Interlocal Agreement with the County for the purpose of designating the County as the lead solid waste agency.

The projected population for Pasco County, Florida is presented below:

<u>Year</u>	<u>Population</u>	
1994	290,000	
2000	340,000	

These population figures are based on projections of Florida population by county as reported by the University of Florida, College of Business Administration, Bureau of Economic and Business Research.

In addition, Pasco County has agreements with Hernando Co., Hillsborough Co., and the City of Plant City for solid waste to be processed at the Resource Recovery Facility. Ash from the combustion of solid waste provided under these interlocal agreements will be disposed of at the West Pasco Class I Landfill. No non-processible waste will be received under these agreements.

3.5.2 Anticipated Type, Annual Quantity, and Source of Solid Waste

The West Pasco Class I Landfill will accept only solid waste which may be disposed at a Class I Landfill as defined in Chapter 17-701, FAC. Hazardous or biohazardous waste as defined by the U.S. EPA and FDEP will not be accepted. The landfill will accept ash residue, non-processible waste that cannot be directed to the West Pasco Class III Landfill, East Pasco Landfill, or recycling activities (i.e. scrap metal) and raw solid waste which exceeds the resource recovery facility capacity. Only ash will go to the ash monofill disposal units.

The projected annual quantities of solid waste are presented in below.

<u>Year</u>	Ash (in tons)
1994	82,125
2000	82,125

The solid waste is primarily from domestic and commercial sources.

3.5.3 Anticipated Facility Life

The anticipated life of the West Pasco Class I Landfill is in excess of 50 years. Disposal Unit A-1 initial filling phase provided for approximately five years of disposal capacity. Disposal Unit A-2 initial filling phase also is anticipated to provide for five years of disposal capacity of ash produced at the Resource Recovery Facility.

Disposal Unit SW-1 is designed to accept solid waste only if the input volume exceeds the capacity of the Resource Recovery Facility and has not been placed in service. The operating capacity of the Resource Recovery Facility has not been exceeded partly due to alternative disposal methods provided by the County at the West Pasco Class III Landfill and Recycling Center, the East Pasco Transfer Station and Recycling Center, and the East Pasco Landfill.

3.5.4 Source and Type of Cover Material Used for the Landfill

The source of daily and intermediate cover material is from the on-site excavation of surface water management pond and was stockpiled during initial site construction. Additional soil material is available from potential borrow areas outside the 160 acre landfill site but within the 800 acre facility site.

The type of material is native sandy soils. These soils are unconsolidated fine-grained sands, in part silty and/or clayey.

3.6 Evidence that an Approved Laboratory Shall Conduct Water Quality Monitoring for the Facility in Accordance with Rule 17-160, FAC.

Pasco County Environmental Laboratory has been and will continue to conduct the required water quality sampling and analyses for the West Pasco Class I Landfill. The laboratory's approved Comprehensive Quality Assurance Plan is FDEP 870167G.

Chemical analyses that cannot be performed by the County laboratory are contracted out under an annual contract. Currently, the County has five FDEP approved laboratories under annual contract.

3.7 Provide a Statement of how the Applicant will Demonstrate Financial Responsibility for the Closing and Long-Term Care of the Landfill

Pasco County has established a landfill management escrow account for the closure and long term care of the West Pasco Class I Landfill. Cost estimates for closure and long term care (30 years) are made by a professional engineer based on third-party performance of the work for each disposal unit. See Section 13 for detail costs. Annual cost adjusts for closure and long-term care are also made pursuant to Chapter 17-701.630, FAC.

The County's procedure has been approved by the Department. Annual adjustments and statements are on file with the Department.

- SECTION 4.0 GENERAL CRITERIA FOR LANDFILLS

SECTION 4.0

GENERAL CRITERIA FOR LANDFILLS

4.1 Describe how landfill or solid waste disposal unit shall not be located in the 100-year floodplain where it will restrict the flow of the 100-year flood, reduce the temporary water storage capacity of the floodplain unless compensating storage is provided or result is a washout of solid waste.

The West Pasco Class I Landfill is not located in the 100-year floodplain. The landfill is on a topographically high area with an average elevation of 50 feet above mean sea level.

Describe how the minimum horizontal separation between waste deposits in the landfill and the landfill property boundary shall be 100 feet measured from the toe of the proposed final cover slope.

The West Pasco Class I Landfill planned disposal area covers approximately 160 acres and is situated within a larger 800-acre solid waste management site. The horizontal separation is well in excess of required 100-foot buffer (see Drawing Sheet No. 4).

24.3 Describe what method shall be taken to screen the landfill from public view where such screening can practically be provided.

The West Pasco Class I Landfill is obscured from public view by pine trees and other vegetation on the east, south, and west boundaries. The landfill is partially obscured from public view on the north boundary. Additional trees and vegetation have been planted to further limit the landfill from public view.

SECTION 5.0 LANDFILL CONSTRUCTION REQUIREMENTS

SECTION 5.0

LANDFILL CONSTRUCTION REQUIREMENTS

5.1 Describe how the landfill shall be designed so that disposal units will be constructed and closed at planned intervals throughout the design period of the landfill

The landfill was designed to be constructed in a phased series of sixteen-10 acre disposal units each with a capacity sufficient to contain the ash generated during a 5-year period. These disposal units are prepared by construction of a perimeter berm using borrow from limited excavation of soil within the disposal unit area and from other sources. An on-site source has been identified and is shown in Drawing Sheet No. 5. As each disposal unit is filled to the top of perimeter berm, an adjacent disposal unit will be constructed. Intermediate cover will be applied to the filled disposal unit and disposal will begin in the new unit. The process will be repeated until all the disposal units are filled. Refer to Appendix 3 for an illustration of closure procedures.

5.2 Landfill liner requirements

A double liner system is proposed for this disposal unit. The system includes two High Density Polyethylene (HDPE) geomembranes placed on a compacted soil foundation.

5.2.1 General Construction Requirements

The liner system has been designed to meet the requirements contained in Chapter 17 - 701.400(3) of the Florida Administrative Code (FAC). Test results are provided at the end of this section (Appendix 5, Section A 5.1) to demonstrate liner compatibility, physical, chemical and mechanical properties.

5.2.2 Document foundation is adequate to prevent liner failure

A geotechnical investigation has been conducted to evaluate the foundation to avoid liner failure. Total settlement of the landfill at closure height is anticipated to be 1.5 to 2.0 feet. If the settlement were to be 9 to 12 inches in the middle of a disposal unit, the elongation of the liner would be 0.2 % which is within tolerance for the geomembrane proposed (maximum elongation is 13 %). Settlement in a concentrated area could be the critical condition in regard to potential failure. This type of settlement could occur as a result of sinkhole activity or an unrealized "soft spot". The potential for future sinkhole activity was evaluated using soil borings and ground penetrating radar survey. The results of this survey indicate a few anomalies in the foundation. The foundation will be proof-rolled prior to placement of the geomembrane to consolidate localized soft spots. Refer to Section 7 for the complete geotechnical investigation and results and to Section 6.1.1.2.

√5.2.3 Constructed so bottom liner will not be adversely by fluctuation of the ground water

The elevation of the bottom of the liner system is above the seasonal ground-water level. The seasonal ground-water elevation was determined using water level measurements from existing monitoring wells located around the site.

5.2.4 Design to resist hydrostatic uplift

Not Applicable. See response in Section 5.2.3.

5.2.5 Cover to protect surrounding earth

The berms will be constructed to contain the waste within each disposal unit and these berms and the entire area within the berms will be lined with the geomembrane. The geomembrane will be made continuous across disposal units by welding existing and new geomembranes together. Special precautions will be to avoid folds and creases in the membrane during placement of the cover soils and in corners of the disposal unit. The liner will have no penetrations. Leachate and uncontaminated stormwater will be pumped from the disposal unit.

5.3 Double Liner system details

5.3.1 Upper and lower geomembrane thickness and properties

The proposed primary and secondary liner will be 60 mils nominal thickness with properties as listed in Table 1 of Section 02280 of the specifications included in Appendix 5, Section 5.2 located at the end of this section.

5.3.2 Head over liner

The primary leachate collection and removal system is designed to limit the head on the top liner to 12 inches or less. This system consists of a 24-inch thick granular soil drainage layer above the geomembrane with a hydraulic conductivity of not less than 1×10^{-3} cm/sec. Leachate that passes through the drainage layer will be collected by perforated pipes. These pipes are sloped to allow leachate to drain to a collection sump. Leachate that collects in the sump will be pumped out of the disposal unit. An estimate of the head over the liner using the proposed design configuration was made using the US Army Corps of Engineers computer model Hydrologic Evaluation of Landfill Performance (HELP). The results of the calculations using the model are in Appendix 5, Section A 5.3 at the end of this section.

The sump is divided into two areas; one for the primary system and the second for the leak detector or secondary system (refer to Drawing Sheet No. 12). These sump areas will be created by extending the double liner through an area excavated below the grade of the bottom of the disposal unit. For additional protection a geosynthetic clay liner will be added beneath each liner in the sump area. The sump will be filled with gravel. A geotextile will be placed between the gravel and the liners for extra protection. The dimensions of the sump (30' x 30' x 2.5' deep) will provide 6,000 gallons in the primary and 2,250 gallons in the secondary sump (assuming a 50 percent void ratio in the gravel in the sump) refer to the calculations in Appendix 5 for sizing of the sumps.

Leachate will be pumped from the sumps using submersible pumps. The pumps are fitted with wheels and are lowered into the sump area down a pipe laid on the side slope of the outer berm of the disposal unit. The bottom end of the pipe is closed off with a screen to allow water but not gravel or sand to enter into the pump intake. At the top the riser is open for access for withdrawing pump and leachate level measurements.

✓5.2.4 Design to resist hydrostatic uplift

Not Applicable. See response in Section 5.2.3.

▶5.2.5 Cover to protect surrounding earth

The berms will be constructed to contain the waste within each disposal unit and these berms and the entire area within the berms will be lined with the geomembrane. The geomembrane will be made continuous across disposal units by welding existing and new geomembranes together. Special precautions will be to avoid folds and creases in the membrane during placement of the cover soils and in corners of the disposal unit. The liner will have no penetrations. Leachate and uncontaminated stormwater will be pumped from the disposal unit.

5.3 Double Liner system details

5.3.1 Upper and lower geomembrane thickness and properties

The proposed primary and secondary liner will be 60 mils nominal thickness with properties as listed in Table 1 of Section 02280 of the specifications included in Appendix 5, Section 5.2 located at the end of this section.

5.3.2 Head over liner

The primary leachate collection and removal system is designed to limit the head on the top liner to 12 inches or less. This system consists of a 24-inchethick granular soil drainage layer above the geomembrane with a hydraulic conductivity of not less than 1 x/10⁻³ cm/sec. Leachate that passes through the drainage layer will be collected by perforated pipes. These pipes are sloped to allow leachate to drain to a collection sump. Leachate that collects in the sump will be pumped out of the disposal unit. An estimate of the head over the liner using the proposed design configuration was made using the US Army Corps of Engineers computer model Hydrologic Evaluation of Landfill Performance (HELP). The results of the calculations using the model are in Appendix 5, Section A 5.3 at the end of this section.

The sump is divided into two areas; one for the primary system and the second for the leak detector or secondary system (refer to Drawing Sheet No. 12). These sump areas will be created by extending the double liner through an area excavated below the grade of the bottom of the disposal unit. For additional protection a geosynthetic clay liner will be added beneath each liner in the sump area. The sump will be filled with gravel. A geotextile will be placed between the gravel and the liners for extra protection. The dimensions of the sump (30' x 30' x 2.5' deep) will provide 12,000 gallons in the primary and 4,500 gallons in the secondary sump.

Leachate will be pumped from the sumps using submersible pumps. The pumps are fitted with wheels and are lowered into the sump area down a pipe laid on the side slope of the outer berm of the disposal unit. The bottom end of the pipe is closed off with a screen to allow water but not gravel or sand to enter into the pump intake. At the top the riser is open for access for withdrawing pump and leachate level measurements.

The side slope riser pumping system has a level switch that activates the pump at a pre-set leachate level. The lower level switch shuts off the pump to prevent damage. The pumping rate is recorded as well as the leachate level fluctuations. The elevation of leachate in the sumps (disposal unit) can also be measured manually through the riser pipe.

The pumps and sump were sized based on data from the existing system. Currently, stormwater flowing across the ash is collected in the southeast corner of Disposal Unit A-1. This contaminated stormwater passes through the primary collection system and discharges into the manholes and flows to a holding tank. Two pumps, each having a capacity of 150 gallons per minute, pump leachate to a pump station at Disposal Unit SW-1. Once A-2 is opened, the ash in Unit A-1 will be shaped for drainage and intermediate cover will be placed over the ash (refer to Drawing Sheet 7A). The flow rates from A-1, with intermediate cover, will be 15 to 20 gpm or about ten percent of the current pumping capacity. Therefore, the pump used to draw leachate from the primary sump in A-2 will have a capacity of 150 gpm. A spare riser pipe will be installed into the primary sump to allow adding a second pump, if necessary.

The capacity of the secondary pump is 5 to 10 gpm. The County had a study performed to determine the quantity of secondary leachate, and the results indicate 5 to 10 gpm will be more than adequate.

5.3.3 Lower geomembrane sub-base

The sub-base beneath the lower geomembrane will be six inches thick and have a saturated hydraulic conductivity of less than or equal to 1 x 10⁻⁵ cm/sec. The stability of this sub-base will be evaluated by proof rolling the area. Prior to placing the geomembrane the liner installation contractor will walk the entire area and certify that the base is free of sharp objects that might penetrate the geomembrane (refer to the liner quality assurance document in Section 5.9).

5.3.4 Leak detection

Leachate that may pass through the top liner will be collected using a geonet place between the two geomembranes. This geonet will be sloped to drain to collection pipes which flow into a sump in the northeast corner of the disposal unit for pump out. The geonet will have a hydraulic conductivity of 10 cm/sec and the system will limit the head over the secondary liner to less than the thickness between the liners (refer to Section 5.8.4). The transmissivity of the geonet can be maintained even under full loading (15,000 pounds per square foot). The selected manufacturer provided catalogue data to show this to be true. See Calculations in Section A5.4 of Appendix 5.

5.4 Standards for geomembrane

5.4.1 Field seam test methods

Field seam testing includes non-destructive and destructive testing and these are described in Section 02280 of the specifications enclosed in Appendix 5, Section A5.2

.5.4.2 Protective layer design

The protective layer will be designed to meet minimum requirements for the drainage layer (5 x 10⁻³ cm/sec hydraulic conductivity). No, separate 12-inch thick protective layer is proposed. Instead, the drainage layer above the geomembrane will be 24 inches thick. Refer to the calculations in Appendix 5, Section A5.4.

25:4.3 Operational plans during initial waste placement

The first layer of waste will be placed in a compacted lift four feet in compacted thickness and it will consist of selected waste containing no large rigid objects that could damage the geomembrane or leachate collection system. The ash material is screened prior to hauling to the disposal unit so the materials required for the initial layer should be readily available.

5.5 Geosynthetic specifications

5.5.1 Definition and qualifications

The duties and qualifications of the designer, manufacturer, installer, QA consultant and laboratory, and QA program are presented in the liner quality assurance program in Section A.5.2 of Appendix 5.

5.5.2 Material Specifications

Geomembranes - Refer to Part 2, Section 02280 of the specifications provided in Appendix 5, Section A5.2

Geotextiles - Refer to Part 2, Section 02278 of the specifications provided in Appendix 5, Section A5.2

Geonet - Refer to Part 2, Section 02279 of the specifications provided in Appendix 5, Section A5.2

5.5.3 Manufacturing and fabrication specifications

Manufacturing and fabrication specifications are provided in Section 02280 of the specifications provided in Appendix 5, Section A5.2.

5.5.4 Geomembrane installation specifications

Earthwork specifications are provided in Section 02200 and remaining specifications for installation of the membrane are provided in Section 02280 of the specifications see Appendix 5, Section A5.2.

5.5.5 Geotextile and geonet specifications

Specifications for the geotextile and geonet are provided in Sections 02278 and 02279, respectively located in Section 5.2 of Appendix 5.

5.6 Standards for soil components

5.6.1 Construction procedures

Specifications for proof-rolling the liner subbase prior to liner placement and correcting potential problem areas are discussed in Section 02200 of Appendix 5, Section A5.2.

5.6.2 Compatibility

The sand and clay are compatible with the ash material. No adverse impacts have been noted during the placement of this waste in Disposal Unit A-1 over the last five years.

5.6.3 Testing procedures

Testing procedures are provided in Sections 02200 and 02215 of the specifications in Appendix 5, Section A5.2.

5.6.4 Specifications for soil

Specifications for soil components and earthwork are provided in Sections 02200 and 02215 of the specification (see Appendix 5, Section A5.2).

5.6.5 Field test section

A field test section is not planned.

5.7 Leachate collection and removal system (LCRS).

5.7.1 Compatibility

The piping and geonet are to be manufactured of HDPE materials similar to the liner and will therefore be resistant to the waste and leachate. Refer to Appendix 5, Section A5.1.

5.7.2 Mechanical properties

Design calculations for pipe loading are enclosed at the end of this section of the application in Appendix 5, Section 5.4.

5.7.3 Design to avoid clogging

The drainage materials and geotextiles have been designed to prevent clogging. Fabric will be used as a cushion to protect the geomembrane but not as a filter material. Filtration to prevent clogging will be accomplished by gradation design of adjacent soils. The calculations are enclosed in Appendix 5, Section 5.4.

5.7.4 Testing collection pipes

Cleanouts are provided at both ends of header pipes and collector pipes. The condition of collection and header pipes can be verified using television cameras similar to those used in sewer applications. Clean out could also be accomplished using methods currently employed for sewers.

5.8 Primary LCRS requirements

5.8.1 Drainage layer

A 24-inch drainage layer is proposed with a saturated hydraulic conductivity of 5 x 10⁻³ cm/sec will be installed.

5.8.2 Drainage Protective layer thickness

The total thickness of the drainage layer will be 24 inches with a minimum hydraulic conductivity of 5 x 10⁻³ cm/sec, and the soil material will be chemically resistant to the waste and the leachate.

5.8.3 Allowance for Settlement

The bottom slope of collection pipes and header is 0.5%, which is adequate to allow flow after the predicted settlement has taken place (refer to Appendix 5, Section A5.4).

5.9 Liner System Construction Quality Assurance (CQA)

5.9.1 CQA Plan

A CQA Plan is provided in Section A5.2 of Appendix 5. The plan includes:

- Specifications and construction requirements for liner system
- Detailed description of quality control testing procedures and frequency
- Identification of supervising professional engineer
- Qualifications of CQA professional engineer and support personnel
- Description of CQA reporting forms

5.9.2 Independent Laboratory Testing

The County will contract with a third-party laboratory experienced in geosynthetic liner testing to conduct independent testing. Coupons for destructive tests will be split, splits will be given to the third party testing laboratory and the installer. A third split will be retained on site for reference.

5.10 Surface Water Management System

The surface water management system has been designed to isolate surface water from waste filled areas and to control storm water runoff. All the designed stormwater management system has been constructed with the exception of stormwater retention pond No. 4 which was designed to receive stormwater runoff from future disposal units to be constructed on the west side of the permitted landfill area.

Details of the stormwater control design including retention ponds and drainage ways are shown on Drawing Sheets No. 4 and 5.

5.11 Gas Control System

Designs for passive gas venting in solid waste monofill disposal units were included in the Power Plant Site Certification permit application. No gas control system is needed for the ash monofill disposal units.

SECTION 6.0

HYDROGEOLOGICAL INVESTIGATION REQUIREMENT

SECTION 6.0

HYDROGEOLOGICAL INVESTIGATION REQUIREMENT

- 6.1 Hydrogeological Investigation and Site Report for the West Pasco Class I Landfill
- 6.1.1 Regional and Site Specific Geology and Hydrogeology
- 6.1.1.1 Regional Geology and Hydrogeology

In Pasco County, undifferentiated surficial deposits of Pliocene to recent age sands of variable thickness overlie Cretaceous and Tertiary carbonates and clays. The Tertiary carbonates (primarily limestones) deposited during the Oligocene and Eocene periods, are called the Suwannee, Ocala, and Avon Park Formations in descending order. Separating the surficial sands from the underlying carbonates are clayey sands, sandy clays, and clays which are remnants of or a residual from the Hawthorn and Tampa Formations of upper and lower Miocene age. Both the Hawthorn and the Tampa Formation are thicker further to the south and east.

The importance of the residual clay of the Hawthorn and Tampa Formations, where present, is that the unit comprises the confining layer separating the underlying Floridan aquifer system from the overlying surficial aquifer. Regionally, the Floridan aquifer is known to be only semi-confined, which is attributed to the presence of karst features (sinkholes) and the thinning or absence of the confining layer. See Table 1 in Appendix 6 for the regional stratigraphic and hydrogeologic nomenclature.

6.1.1.2 Site Specific Geology and Hydrogeology

In addition to the extensive site investigation conducted prior to and during the Resource Recovery Facility Power Plant Site certification in 1989, a more localized hydrogeological investigation was made in the immediate vicinity of the next landfill disposal unit to be constructed, Disposal Unit A-2. The purpose of the additional site work was to define further the characteristics of the surficial aquifer, the presence of the confining layer, and the attitude of underlying limestone.

Sixteen soil borings were conducted to achieve this goal; six piezometers also were installed locally in the surficial aquifer. A confining layer of clay was encountered in all soil borings except LB-2 which was not augured deep enough to penetrate the clay or underlying limestone. The piezometers were installed with ten foot screened intervals at the base of the surficial sands to detect even minimal water levels in the surficial aquifer. The piezometers were checked periodically during the hydrogeologic investigation and were found to be dry even in the deepest screened interval (22.6 to 32.6 feet, N.G.V.D. in piezometer LB-3).

Generally, the confining layer clay was found to be gradational with the overlying surficial sands, coarsening upward from a clay to sandy clay to a clayey sand, enough so that the actual base of the surficial aquifer unit was very high and dry (see Drawing Sheet No. 6, Geologic Data). Soil boring lithologic descriptions and piezometer construction details are provided in the Appendix 6. Geologic cross-section depicting the site stratigraphy is shown in Figure 1 in Appendix 6.

6.1.2 Direction and Rate of Ground-Water and Surface-Water Flow Including Seasonal Variations

Water level data from the existing ground-water monitoring system which has been in place since the start up of the Resource Recovery Facility and landfill operations in 1990, were evaluated. Additional ground-water monitoring data was available from an adjacent Class III Landfill Site (see Table 2 in Appendix 6).

Two Floridan aquifer potentiometric surface maps for September 1992 and March 1993 were drawn. Both maps show a northwest direction for ground-water flow for the Floridan aquifer towards Weeki Wachi Springs (see Figures 2 & 3 in Appendix 6). Two surficial aquifer water table maps also were drawn for September 1991 and March 1992. The flow direction in the surficial aquifer is less definitive, but appears to be to the northwest or north, probably deflecting around the existing high the landfill is sited on (see Figures 4 & 5 in Appendix 6).

The hydraulic gradient for the surficial aquifer based on the September 1991 water table map is approximately 0.001. The hydraulic gradient for the Floridan aquifer appears to be higher at the end of the wet period (based on the September 1992 potentiometric surface map) than it is near the end of the dry season (based on the March 1993 map). The hydraulic gradients ranged from approximately 0.002 in September 1992 to 0.0015 in March 1993.

6.1.3 Background Quality of Ground-Water and Surface Water

Ground-water monitoring wells, 2MW-1 through 2MW-4 and 4MW-1 through 4MW-4 were installed in 1981 prior to construction of the landfill. Analytical results from the 1987 sampling event are provided in Appendix 6.

Ground-water monitoring wells, 2MW-5, 2MW-6, 4MW-5, and 4MW-6, were installed in 1991 after construction of Disposal Units A-1 and SW-1. Analytical results from the 1991 sampling event are provided in Appendix 6.

Monitoring wells designated 2MW are surficial aquifer wells. Wells designated 4MW are Floridan Aquifer monitoring wells. Well locations are shown on Figures 2 through 5 in Appendix 6. No surface water bodies exist on the landfill consequently no surface water samples have been taken.

6.1.4 Any On-Site Hydraulic Connections Between Aquifers

There does not appear to be any hydraulic connections between the aquifers within the footprint of the landfill site. All soil borings advanced deep enough during this investigation encountered clay before limestone which is an indication of the continuity of the confining layer. Soil boring LB-2 was terminated at 23.8 feet, N.G.V.D., before reaching the confining layer clay of the Floridan aquifer limestones. Outside the landfill area to the southwest are known sinkholes which provide a connection between the aquifers.

Soil boring LB-5 encountered water 12.5 feet into the clay layer after penetrating 21.5 feet of surficial sands. The estimated elevation of the water is 18.1 feet, N.G.V.D., which is well below any water level reported for the surficial aquifer or the Floridan aquifer in the routine groundwater monitoring. No water was encountered above the top of the clay unit at 30.6 feet, N.G.V.D. It was concluded that the surficial aquifer was dry and that their was no hydraulic connection between aquifers at this location.

Soil boring LB-2 was also reported to have encountered water at 18 feet below ground level or at an elevation of 31.8 feet, N.G.V.D. The boring was terminated short of penetrating clay or limestone at a total depth of 26 feet below ground level or at an elevation of 23.8 feet, N.G.V.D. The next soil boring LB-3,located less than 150 feet from LB-2, was continued deeper until it penetrated clay at 23.6 feet, N.G.V.D. A piezometer was

installed with a screened interval from 22.6 to 32.6 feet, N.G.V.D., which was deep enough to have intercept the water reported in soil boring LB-2 at 31.8 feet, N.G.V.D. No water was encounter in LB-3. It was concluded that the aquifer was dry and that no hydraulic connection between aquifers exits in the vicinity of soil boring LB-3. It was also concluded that if water was actually encountered in soil boring LB-2 that it should have been encountered in LB-3.

In locations where ground-water monitoring wells for the surficial aquifer and the Floridan aquifer are close or clustered and the surficial is partially saturated, the hydraulic level of the surficial aquifer is generally slightly higher than in the underlying Floridan aquifer and the differential between them is greater as the distance away from the sinkholes increases (see Table 2 in Appendix 6). This was concluded to be additional evidence that an effective hydraulic separation exists between the aquifers in the vicinity of the landfill.

6.1.5 Site Stratigraphy and Aquifer Characteristics for Confining Layers, Semi-Confining Layers, and All Aquifers Below the Landfill Site That May Be Affected by the Landfill.

The site stratigraphy is illustrated on Figure 1 in Appendix 6. The stratigraphy at the site differs only slightly from the regional stratigraphy. The undifferentiated surficial deposits are present, but slightly thinner. The Hawthorn and Tampa Formations are either absent or not distinguishable and probably represented by the residual clay, which grades upward into the surficial sands. The residual clay lies unconformably on the underlying Suwannee Formation. Limestones of the Suwannee Formation have only been partially penetrated by some of the soil borings and by the installation of the Floridan Aquifer Monitoring Wells. Thicknesses of these units are given in Table 3 in Appendix 6 along with other pertinent characteristics.

Hydraulic conductivities were determined for three undisturbed samples taken from the confining layer clay penetrated in soil borings LB-4, LB-5, and LB-8. The values ranged from 2.2x10⁻⁶ feet per day to 6.9x10⁻⁶ feet per day.

The confining layer below the landfill site is assumed to be continuous. No soil borings penetrated the Floridan aquifer limestone without first penetrating the confining layer clay. Hydrogeologically, the Floridan aquifer is considered to be only semi-confined in West Pasco due to the regional thinning of its confining unit from the southeast to the northwest out of Hillsborough County, and due to the presence of karst features.

Water levels in both the surficial aquifer and Floridan aquifer have remained below 40 feet NGVD at the site during the period from 1991 to 1993 (see Figure 6 in Appendix 6). The surficial aquifer has been generally dry. Only in Monitoring Well MW-2 has water been consistently present.

6.1.6 Site Topography and Soil Characteristics

The West Pasco Class I Landfill is located in a relatively flat section of the Pamlico Terrace of the Gulf Coastal Lowlands physiographic province along the west coast of Florida. Topographic relief across the site ranges from about 20 to 30 feet in the southwestern portion and about 10 feet throughout the remainder of the site and within the footprint of the landfill.

Numerous lakes and ponds exist to the south of the permitted landfill area, but none occur within the landfill area itself. The area is drained primarily by Buckhorn Creek and the Pithachascotee River. The major surface water feature in the vicinity of the landfill is Crews Lake located approximately 1.5 miles to the east. Crews Lake, a lenticular body and headwaters of the Pithachascotee River, forms a northeast-trending lineament with the river.

The soil characteristics for the landfill area are given as Candler and Tavares sands, which are described as nearly level to sloping, excessively drained soils that are sandy throughout with occasional thin lamellae of loamy sand below a depth of 50 inches in the soil survey of Pasco County, Florida. This is consistent with the lithologic description and grain-size analysis performed on samples taken from the surficial sands in the soil borings. The sands appear to be fine-grained, well sorted sands until they begin to grade into the underlying clay confining unit.

6.1.7 Inventory of All Public and Private Water Wells within a One-Mile Radius of the Landfill including Top of Well Casing and Bottom Elevations, Name of Owner, Age and Usage of Each Well, Stratigraphic Unit Screened, Well Construction Technique and Static Water Level.

A listing of all public and private water wells within a one-mile radius of the landfill was obtained from the Southwest Florida Water Management District for Sections 13, 14, 23, 24, 25, 26, 35 & 36, Township 24 South, Range 17 East, and Sections 18, 19, 30 & 31, Township 24 South, Range 18 East. Each section comprises one square mile and the required one--mile radius of the landfill falls within these twelve (12) sections. The listing is provided in the Appendix 6. In Pasco County properties are listed on the tax rolls by identication numbers based on its specific township, range and section location. A listing can be derived from the tax roll by sorting on section, township and range and used to contact residents within the one-mile radius of the landfill if the necessity arises.

6.1.8 Existing Contaminated Areas on Landfill Site

There are no known existing contaminated areas on the landfill site.

6.2 Report Signed, Sealed and Dated by PE or PG

The hydrogeological investigation and site report for the West Pasco Class I Landfill is signed, sealed and dated by George W. Ellsworth, a licensed Professional Geologist (No. 0000848) in the State of Florida.

SECTION 7.0

GEOTECHNICAL INVESTIGATION REQUIREMENTS

SECTION 7.0

GEOTECHNICAL INVESTIGATION REQUIREMENTS

7.1 Submit a geotechnical site investigation report defining the engineering properties of the site including at least the following:

A report of the results of the geotechnical site investigation is provided in Appendix 7. A report of the results of the hydrogeological site investigation is provided in Appendix 6.

7.1.1 Description of subsurface conditions including soil stratigraphy and ground-water conditions.

Test boring records for the five soil borings LB-1, LB-3, LB-5, LB-7 and LB-11 are provided in geotechnical site investigation report. The soil borings penetrated unconsolidated surficial sands and clays. Two of the borings were terminated after reaching limestone. Detailed lithologic descriptions and a discussion regarding the water table are provided in the hydrogeological site investigation report. The surficial aquifer was unsaturated.

7.2 Investigation for presence of muck, previously filled areas, soft ground, lineaments and sink holes.

A detailed discussion regarding the presence of the above referenced features was provided in the Power Plant Site Certification Application. The 160 acre landfill footprint was selected as the area most suitable considering the above criteria. No anomalies were encountered during this site investigation.

7.3 Estimates of average and maximum high water table across the site.

Based on site specific water table levels, the surficial aquifer is not expected to be saturated above 40 feet, NGVD. A detailed discussion is provided in the hydrogeological site investigation provided in Appendix 6.

7.4 Foundation Analysis including:

7.4.1 Foundation bearing capacity analysis

The stress from filling Disposal Unit A-2 to 100 feet above grade was calculated to be 8.5 ksf. The ultimate bearing capacity was calculated to be over 15 ksf, which is sufficient to support the calculated stress.

7.4.2 Total and differential subgrade settlement analysis

The total subgrade settlement is calculated 1.5 to 2.0 feet. The differential settlement between the interior portion of the disposal unit and the exterior portion will not be enough to effect the leachate collection system designed flow.

7.4.3 Slope stability analysis.

The calculations are given in Appendix 7 and are attached to the geotechnical report.

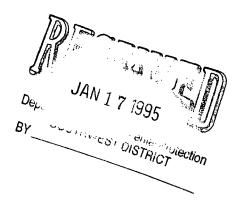
7.5 Description of the methods used in the investigation and includes soil boring logs, laboratory results, analytical calculations and conclusions;

The report, Results of Geotechnical Exploration is provided in Appendix 7. Supporting data is included in the report.

7.6 Report signed, sealed and dated by PE or PG.

The report is signed, sealed and dated by a Professional Engineer.





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SECTION 8.0 LANDFILL OPERATION REQUIREMENTS



SECTION 8.0

LANDFILL OPERATIONS

The landfill addressed in this application is an integral unit of the Pasco County Solid Waste System ("System"). The System is comprised of: a mass-burn resource recovery facility; the West Pasco Class I Landfill, the West Pasco Class III Landfill and Recycling Center, the East Pasco Transfer Station and Recycling Center, and the East Pasco Class I Landfill. The Resource Recovery Facility, the West Pasco Class I Landfill, and the West Pasco Class III Landfill and Recycling Center are co-located on an 800-acre site. The Resource Recovery Facility and the West Pasco Class I Landfill are permitted under the Florida Electrical Power Plant Siting Act, while the West Pasco Class III Landfill and Recycling Center was permitted separately under Chapter 17-701, F.A.C.

The Resource Recovery Facility is designed to receive and process 1,050 tons per day of waste generated by residential, commercial and industrial sources. Three separate combustion units with a capacity of 350 tons per day and a boiler system generate steam for conversion to electrical energy. Emissions controls include dry scrubbers and fabric filter baghouses for each combustion unit. The residue ash handling system is completely enclosed. Bottom ash and grate siftings from the combustion units, as well as fly ash and spent scrubber reagent, are collected and quenched. Ash is moved by conveyor through a scalper screen to remove large materials and through a magnetic separator to remove ferrous metal. Processed residue (MSW ash) is loaded into trucks for disposal in an ash monofill disposal unit at the adjacent West Pasco Class I Landfill.

Currently no delivery of municipal solid waste (MSW) is made directly to the West Pasco Class I Landfill. Deliveries are accepted at the Solid Waste Resource Recovery Facility (SWRRF) 10 hours each day, Monday through Saturday, except legal holidays. Refuse is delivered to the SWRRF in standard packer vehicles, open body dump trucks, semi-truck transfer trailers, and by smaller private vehicles. The waste transferring vehicles pass through an entrance and exit over an automated truck scale system. The scale system is operated by an adjacent scale house with a computerized record keeping system that maintains an accurate accounting of all refuse delivered and ash residue removed from the building.

All processible waste received is dumped inside the Resource Recovery Facility in a refuse storage pit with the exception of some waste from small private vehicles which are directed to a public drop-off area outside the building. Inside the facility building on the tipping floor rollover containers are provided for removing of non-processible waste. The County provides a trained spotter on the tipping floor to observe refuse dumping. The spotter has communication links with the scale house and the facility operators to advise them of the delivery of an unacceptable waste.

The West Pasco Class I Landfill was designed and permitted to be constructed in a phased series of individual disposal units, with a total of 16 disposal units. Six disposal units (A-1 to A-6) are designated for ash disposal, eight disposal units (SW-1 to SW-8) for non-processible or by-pass waste, and two disposal units (I-1 and I-2) were left undesignated. The layout of the disposal units is shown in Figure 8.1. The disposal area covers approximately 160 acres, each disposal unit is approximately 10 acres in size. The initial phase of construction was completed in 1990, with the construction of disposal units SW-1 and A-1, eastern portion of the perimeter access road, retention ponds 1 and 2, an equipment maintenance building, and other associated drainage work.

The entire 800-acre site is enclosed by chain-link and barbed wire fence to limit access. To further limit access the Resource Recovery Facility, the West Pasco Class I Landfill, and the West Pasco Class III Landfill and Recycling Center are separated internally by a chain-link and barbed wire fence to control movement between the units.

8.1 Operating Personnel Training

The Pasco County Utilities Services Branch (PCUSB) has a pro-active approach to training and certification of all landfill personnel and currently has trained operators who have satisfied the requirements of Chapter 17-703 FAC. Additionally, Pasco County currently has other staff members who have attended the TREEO Solid Waste Landfill Operator Short Course and are used as trained spotters at the landfill and elsewhere in the solid waste management system. Copies of their course completion certificates are kept on file. The landfill will have at least one trained operator at the landfill during all times when the landfill receives waste. At least one trained spotter will be at each working face at all times when the landfill receives waste other than ash to detect unauthorized wastes.

8.2 Landfill Operation Plan

8.2.1 Designated Responsible Operating and Maintenance Personnel

The Pasco County Board of County Commissioners sets policy for the administration and management of the disposal of solid waste in the County. Douglas S. Bramlett, Assistant County Administrator, Utilities Services Branch coordinates solid waste management in the County. He is assisted by Vince Mannella, Solid Waste Facilities Manager, who manages the operation and maintenance of the solid waste management facilities.

The following current schedule is typical of the staffing for the West Pasco Class I Landfill.

Certified Landfill Operators Jim Gerger, Supervisor Walter Dransky	Six Days* MTWTFS
Equipment Operator/Spotters Keith Wallace Barry Wright	MTWTWTFS

^{*}Landfill is closed on Sundays. No ash is hauled to ashfill disposal unit.

8.2.2 Contingency Operations for Emergencies

8.2.2.1 Fire Emergency Procedures

In the highly unlikely event that an uncontrollable fire does occur at the landfill site:

- field staff will contact scale attendant by 2-way radio and provide details;
- scale attendant will contact 911 to request fire department assistance;
- scale attendant will contact Landfill Supervisor;
- Landfill Supervisor will direct additional equipment and manpower to the scene as necessary.

If the fire is controllable:

- field staff will contact scale attendant by 2-way radio and provide details;
- field staff will snuff out fire using landfill equipment and soil from an on-site stockpile maintained for suppressing fires.
- scale attendant will contact Landfill Supervisor;
- Landfill Supervisor will inspect scene.

8.2.2.2 Natural Disasters Procedure

If notice is available of a pending natural disaster (tornado, hurricane, etc.) the Landfill Supervisor will direct staff to:

- Check stormwater management system for any blockages at culverts, pipes, etc.
- Check leachate management system levels, pumping units, etc.
- Apply daily cover to working face where appropriate.
- Secure equipment where appropriate.

After the natural disaster has occurred, the Landfill Supervisor will direct staff to assess damage to and operational status of:

- Access roads
- Stormwater management system
- Leachate management system
- Landfill equipment
- Disposal units

8.2.2.3 Equipment Failure Procedures

If equipment fails the Landfill Supervisor will be notified so that arrangements can be made for the equipments repair. If the downtime is expected to hinder landfill operations, the Landfill Supervisor will obtain backup equipment under established cooperative lending agreements with other solid waste management facilities or other County departments.

8.2.2.4 End of Work Week Procedures

At the end of the work week, prior to shut down, the Landfill Supervisor will direct staff to:

- Check stormwater management system for any blockages at culverts, pipes, etc.
- Check leachate management system levels, pumping units, etc.
- Apply daily cover to working face where appropriate.
- Secure equipment where appropriate.

At the beginning of the work, immediately after opening, the Landfill Supervisor will direct staff to observe the condition of and record deficiencies of:

- Access roads
- Stormwater management system
- Leachate management system
- Landfill equipment
- Disposal units

Particular attention is to be paid to the leachate management system pumps, operability and the leachate levels in the disposal units.

8.2.3 Controlling Types of Waste Received at Landfill

One spotter will be located at each working face receiving wastes to inspect waste being dumped at the working face. In the event Resource Recovery Facility is not receiving waste and waste is being by-passed to landfill or other permitted facilities, the spotter(s) assigned to the Resource Recovery Facility tipping floor will be reassigned by the landfill supervisor to the specific Class I landfill.

If in the highly unlikely case a hot load of ash is spotted, the vehicle will be directed to return to the ash handling facility for requenching or the load may be dumped on the paved entry to the disposal unit and allowed to cool. The Landfill Supervisor will be notified so that the Resource Recovery Facility Manager can be advised of the receipt of the hot load and quenching operations be checked.

If prohibited types of waste are observed by the spotter in any by-pass waste, the Landfill Supervisor will be notified so that arrangement for the observed wastes can be removed.

Batteries, tires, and used oil can be removed to the adjacent Class III Landfill and Recycling Center, which has facilities for handling these prohibited wastes. Hazardous and medical wastes can be removed under existing arrangements for the proper handling and disposal. These wastes should be removed under the direction of the County Hazardous Waste Coordinator.

8.2.4 Weighing Incoming Waste

No waste can enter the site without passing over the weighing facilities at the Resource Recovery Facility and the West Pasco Class III Landfill. The Landfill Supervisor will periodically check ash trucks to see if they are crossing the scales by observing them as they leave the ash handling facility.

8.2.5 Vehicle Traffic Control and Unloading

Private refuse haulers are not allowed in the West Pasco Class I Landfill except during the highly unlikely event when non-processible waste and by-pass waste are being delivered to the solid waste disposal unit. During these exceptions the Landfill Supervisor will assign additional landfill staff to control traffic and direct unloading.

8.2.6 Method and Sequence of Filling Waste

The West Pasco Class I Landfill will be developed using 16 disposal units as shown on Figure 8.1. Each disposal unit is approximately 10 acres. Figure 8.2 depicts the sequencing progression of lifts within a typical disposal unit. As this sheet indicates, the liner and leachate collection system will be constructed one disposal unit at a time with temporary roads and swales for access and surface water management.

Disposal Units SW-1 through SW-8 are currently designated for disposal of solid waste. Disposal Units A-1 through A-6 are currently designated for ash disposal. Depending on future volumes, Disposal Units I-1 and I-2 may be used for either ash or solid waste. The ash and solid waste will not be co-disposed. The ash residue will be monofilled, no mixing of the two materials will be allowed.

The method of filling wastes in a individual disposal unit is described as follows. The edge of liner at the top of berm will be flagged or marked with traffic cones except at berms common between the new operating disposal unit and the adjacent filled disposal unit. Ash/solid waste will not be placed within ten feet of this flagged or marked line. All incoming ash/solid waste will be directed to the working face. Berms will be maintained around the entire working disposal area to intercept and contain leachates and divert stormwater to the surface water management system. See Figure 8.3. Ash/solid waste will be placed against the side slope of the previous day's refuse. The first row will act as a guide for the placement of refuse for the remaining rows. In each row, disposal units will be constructed having a minimum length working face to control the operation and leachate quantities, yet of sufficient length to provide adequate dumping areas and room for the landfill equipment to operate (Figure 8.3). A slope of 3 to 1 on a 50-foot wide working face will provide for centralization of operations, while providing maneuvering area for large private and commercial vehicles unloaded each day.

The sequence of filling future lined disposal unit areas with installed leachate collection systems is developed to meet the following objectives.

- Complete subsequent lifts over lower lifts frequent enough to minimize infiltration and conserve the field capacity of the lower lift disposal unit.
- Direct the surface runoff from unused portions of disposal units away from ash/solid waste using control valves and berms.
- Design landfill slopes during operation to maximize surface runoff away from the working face and minimize leachate generation.
- Provide bench terraces along side slopes to minimize erosion.

Efficient use of these techniques will reduce the need for intermediate cover, and decrease leachate volumes.

Final cover will be applied over disposal unit lifts within 180 days after the final lift over an area is completed. Final cover will consist of a 18 inches of clayey material covered with 6 inches of native soils. The top six inches will be uncompacted and vegetated with native grasses or other vegetation to promote evapotranspiration. See Figure 8.4.

8.2.7 Waste Compaction and Application of Cover

In the solid waste disposal unit sufficient cover material will be stockpiled near the working face to provide an adequate supply for at least one week of operation. No daily cover is required in the ash monofill disposal units. In areas near a borrow area, stockpiling may not be necessary. The ash/solid waste is to be placed at the bottom of the working face, within the bermed working area, and spread up toward the top in 2-foot layers. The solid waste will be compacted with a minimum of three to five passes of a compactor. The ash will be compacted as necessary by a front end loader or bulldozer. The spreading of refuse is a continuous operation.

Application of initial, intermediate, and final cover is to be performed as required per Chapter 17-701, FAC. Six inches of initial cover will be applied to the working face of the solid waste disposal unit. The ash monofill disposal unit will not require initial cover. Intermediate cover consisting of 1 foot of compacted sandy soil from an on-site borrow pit will be applied within 7 days of disposal unit completion if final cover or an additional lift is not to be applied within 180 days of disposal unit completion. All intermediate cover areas will be seeded or covered with wood chips, straw or other appropriate cover material to avoid slope erosion and sloped at 2 percent to allow stormwater to drain off and be removed from the disposal unit.

The initial intermediate and final slope on top of landfill areas will be a minimum of 2 percent and will not exceed 4 percent. The perimeter sides of all completed disposal units will have a slope of 4:1 to minimize erosion. Final cover will be applied to the landfill once the final grades are reached. Areas with final cover will be seeded or planted with grass or suitable cover vegetation.

8.2.8 Operations of Gas, Leachate, and Stormwater Controls

Since the site closure plan includes a low permeability top cap, the gas venting system in the solid waste disposal units will be installed as the disposal units are constructed. Gas vents will not be installed in the ash monofill disposal units. The detail of this gas vent is shown on Figure 8.5. The vents will provide an escape route for gases that are lighter than air, such as methane, to prevent lateral migration of these potentially explosive gases.

The leachate collection and transmission system consists of gravity drains, sumps (manholes), and isolation valves in Disposal Units SW-1 and A-1. The normal operation is by gravity drain to the leachate collection tank(see Figure 8.6). When the leachate reaches a predetermined level which is less than one foot in the storage tank(s), leachate is automatically pumped to the Pasco County Shady Hills Subregional Wastewater Treatment Plant. If testing of the leachate indicates the need for pretreatment prior to processing at the wastewater treatment plant, the necessary pretreatment will be performed.

The leachate collection system in Disposal Unit A-2 consists of gravity drains to sumps inside the primary and inside the secondary liner and isolation valves. The leachate is pumped up out of the sump through a pipe to the top of the berm into a double-walled transmission pipe to a lift station at Disposal Unit A-1. See Drawing Sheets 8 through 12.

The stormwater controls shown on Drawing Sheets 8, 13 and 14 will be operated to collect and convey runoff to surface water management areas for sedimentation control in accordance with Chapters 17-3 and 17-4 FAC. Surface water management areas will be maintained by periodic removal of sediments. Surface water control devices such as weirs and culverts will be routinely checked, and cleaned to assure proper performance.

All water coming into contact with solid waste will be intercepted and contained by terms, and will be handled as leachate. Only stormwater that has not contact ash or solid waste may be discharged to the surface water management system.

8.2.9 Water Quality Monitoring

The water quality monitoring will be performed by the Pasco County Environmental Laboratory. The water quality monitoring plan meets the requirements of Chapter 17-701.510, FAC.

If any of the ground-water monitoring wells are damaged or found to be damaged, they will be reported immediately to the Landfill Supervisor who will note the occurrence in his daily operational log. The Landfill Supervisor will also notify the Operations and Maintenance Director of the damaged well so that the Department can be notified.

8.3 Operating Record

The Operating Record shall consist of all records, reports, analytical results, demonstrations, and notifications required by Chapter 17-701, FAC, including the Department approved permit, engineering drawings, and supporting information, and the landfill operator training verifications required by Chapter 17-703, FAC. The record is considered part of the operation plan and is kept at the Pasco County Government Center Utilities Services Branch office located in New Port Richey. Duplicates of the permit, engineering drawings, and the operating plan are kept on-site at the office of the Landfill Supervisor.

The Operating Record will be available for inspection at reasonable times by Department personnel.

8.4 Waste Record

All solid waste will be weighed as it is received at the weighing facilities located at the Resource Recovery Facility. Additionally, all ash residue transported from the Resource Recovery Facility to the West Pasco Class I Landfill will be weighed at the same weighing facilities. All solid waste will be recorded in tons per day.

To the extent possible, the amount of solid waste received by the type of waste will be determined as listed under Chapter 17-701.5(4)(b), FAC. Where possible, such as ash-residue, actual weights in tons per day will be recorded. Waste reports will be completed monthly, and copies will be provided to the Department.

8.5 Access Control

To prevent unauthorized access to the 800-acre site in West Pasco, the entire site is enclosed with either barbed wire or chain-link fencing. Interior fencing separates the Resource Recovery Facility, West Pasco Class I Landfill, and the West Pasco Class III Landfill and Recycling Center. Entrance gates at the Resource Recovery and the West Pasco Class III Landfill are chain-link and are closed and secured during non-working hours. The entrance gate to the Class I Landfill is internal.

The Landfill Supervisor will check or have checked the integrity of the perimeter fencing on a regular basis. The Landfill Operators will secure the entrance gates at the end of the operating day. The Landfill Supervisor will ensure that the existing signs indicating the hours of operations and types of waste accepted are maintained.

8.6 Monitoring of Waste

Examination of the waste received is accomplished both at the East Pasco Transfer Station and at the Resource Recovery Facility tipping floor. At the East Pasco Transfer Station all loads are dumped on the transfer station tipping floor. At the Resource Recovery Facility all loads are dumped into the refuse pit, except those loads directed to dump on the floor. The SWRRF has a written plan for the identification, isolation, and handling of unacceptable materials.

Routinely, only ash residue loads are monitored at the Class I Landfill for hot loads. In the highly unlikely event that significant by-pass waste from the Resource Recovery Facility occurs, the Landfill Supervisor will establish random examination of solid waste deliveries at least three times per week. Randomly at least three loads of solid waste will be examined by the assigned spotters.

If unauthorized wastes are detected, the spotter will notify the Facility Operator who will contact the generator, hauler, or other party responsible for shipping the waste to the County facility. The facility operator will attempt to determine the identify of the waste sources, and facilitate its removal, proper disposal and correct handling in the future.

If the Facility Operator or other trained personnel determines the detected unauthorized waste to be hazardous waste, the area where the wastes are deposited will be cordoned off from public access until proper cleanup, transportation to, and/or disposal at a permitted hazardous management facility has been assured. The Facility Operator will promptly notify the Department of the person responsible for shipping the wastes to the facility, and the generator of the wastes, if known.

The information and observations resulting from each random inspection will be recorded in writing and retained at the facility for at least three years. The recorded information will include the following:

- Date and time of inspection
- Name of the hauling firm or vehicle owner
- Driver of the vehicle
- Vehicle license plate number
- Source of waste
- Observations made
- Name and signature of the inspector

8.7 Procedures for Spreading and Compacting Waste

8.7.1 Waste Layer Thickness and Compaction Frequencies

All solid waste if required will be spread in layers of approximately two (2) feet in thickness and compacted to as thin a layer as practical, depending on the type of waste received, before the next layer is applied. Ash residue will require only one or two passes with the heavy equipment. By-pass waste will require three to five passes with the heavy equipment and should be compacted to approximately one (1) foot in thickness.

8.7.2 Special Considerations for First Layer of Waste Placed in a Disposal Unit

An additional foot of protective layer soil material will be placed on the side slope and covered with a geotextile at the initial point of entry into the new disposal unit. The first layer of waste will be selected to be free of large rigid objects that may damage the liner or leachate collection system. The thickness of the first layer will be at least four feet of compacted waste. Placement of the first layer will be conducted by a trained operator.

8.7.3 Construction of Lifts

Solid waste will be placed into disposal units to construct lifts. The working face of the disposal unit, and side grades at a slope, no greater than three feet horizontal to one foot vertical rise. Lift thickness should not exceed ten (10) feet. A temporary berm will be constructed around the working face to minimize the formation of leachate (see Figure 8.3). The temporary berm will be moved as the working face/lift progresses.

8.7.4 Working Face Width

The working face will be only wide enough to accommodate vehicles dumping waste. In the ashfill disposal units and solid waste disposal units, the working face under normal operating conditions should be minimal (± 50 feet). During periods when the volume of by-pass waste is high, the size of the working face will be greater to accommodate the increased traffic.

8.7.5 Initial Cover

Initial cover will be applied to solid waste disposal units in order to minimize any adverse environmental, safety, or health effects such as those resulting from birds, blowing litter, odors, disease vectors, or fires. Initial cover will not be necessary for the ash monofill disposal units.

Initial cover at the solid waste disposal units will be applied at the end of each working day. The initial cover will be comprised of soil material and be six (6) inches in compacted thickness.

8.7.6 Intermediate Cover

Intermediate cover, in addition to six-inch initial cover, will be applied and maintained within seven days of disposal unit completion if additional solid waste will not be deposited within 180 days of disposal unit completion. The intermediate cover, when disposal to the initial fill phase and disposal activity is shifted to a new adjacent disposal unit for more than 180 days, will be graded to provide a surface slope and will also be seeded or sodded with grass to further promote runoff and minimize infiltration. When disposal activity is resumed in the disposal unit, the intermediate cover will be pushed aside and stockpiled for use as initial cover for the resumed disposal activity.

8.7.7 Final Cover

Once the solid waste disposal units have been filled to the final grades, final cover will be applied in accordance with the closure plan. The top of the landfill area will be convex with an outward slope of 2 to 4 percent from

the center. The side will be completed with slopes of 4:1. Areas with final cover will be seeded or sodded with grass or suitable cover vegetation.

8.7.8 Litter Policing Methods

Litter generated within the landfill site is expected to be nominal because the litter generating waste is currently combusted at the Resource Recovery Facility. In the event the litter generating waste by-passes the Resource Recovery Facility, the Landfill Supervisor will initiate the following litter control methods:

- Require delivery vehicles remain covered until entry into landfill
- Routine clean-up around disposal unit and access roads
- Maintain small workface and effective initial cover

Clean-up along the Resource Recovery Facility access road, Hayes Road, and within the Facility grounds, particularly around the private drop-off area will be maintained. County crews will routinely police these areas.

8.7.9 Erosion Control Procedures

Grass vegetative cover will be established and maintained on all landfill berms outer slopes, stormwater retention pond outer slopes, and along interior access roads. The Landfill Supervisor or his designee will conduct routine inspections during the wet seasons and immediately after heavy storms to detect any emerging erosion. Detected erosion will be repaired by landfill staff.

8.8 Describe Operational Procedures For Leachate Management Including:

8.8.1 Leachate Level Monitoring, Sampling, Analyses and Data Results Submitted to the Department;

The leachate sampling and analysis will be performed semi-annually by the Pasco County Environmental Laboratory as part of the Water Quality Monitoring Plan. The results will be reported to the Department. Leachate level monitoring will be performed daily (except for non-operational days). Results, including leachate generation rates, pumpage, and rainfall data will be reported to the Department quarterly. A copy of the form that will be used to record the data is included in Table 8.1.

8.8.2 Operation and Maintenance of Leachate Collection and Removal System, and Treatment as Required;

The landfill supervisor will review daily the leachate collection and removal system data to insure that the head over the liner is maintained below its maximum allowable level and that generation rates measured in the secondary leachate collection system are not excessive. Refer to Appendix A5.4 for calculations of this value. If exceedance are detected, the Operations and Maintenance Director will be notified so the exceedances can be addressed promptly.

8.8.3 Procedures for Managing Leachate if it Becomes Regulated as a Hazardous Waste

Pasco County is evaluating options for pretreating the leachate prior to it being transmitted to the Shady Hills Subregional Wastewater Treatment Plant (WWTP).

8.8.4 Agreements for Off-Site Discharge and Treatment of Leachate.

No agreement for off-site discharge and treatment is necessary. Treatment and discharge is provided by the Shady Hills WWTP. Back-up treatment and discharge will be provided by the Hudson WWTP. The West Pasco Class I Landfill and the Shady Hills and the Hudson WWTPs are owned by Pasco County and operated by the Utilities Services Branch.

8.8.5 Contingency Plan for Managing Leachate During Emergencies or Equipment Problems

If equipment problems occur such as pump failure so that leachate cannot be removed from the leachate holding tanks or leachate pumps, the landfill supervisor will be notified so that arrangements can be made for equipment repair or replacement. If problems occur with the leachate transmissive pipeline or with the WWTP, the landfill supervisor will be notified so that arrangements can be made to correct the problem and if necessary, arrangements be made to transport leachate by tanker truck to the Shady Hills WWTP or the Hudson WWTP.

8.8.6 Procedures for Recording Quantities of Leachate generated in gal/day

The landfill supervisor will direct staff to daily record the leachate levels measured in the tanks and sumps and flow meters readings. Flow meter results will be subtracted from the previous day's results to determine the quantity of leachate generated, in gallons per day. Quantities will be measured and recorded daily for each primary and secondary liner system.

8.8.7 Procedures for Comparing Precipitation Experienced at the Landfill with Leachate Generation Rates

The landfill supervisor will direct staff to daily check and record rainfall collected in an on-site rain gauge. The data will be recorded along with the leachate generation data. Leachate generation rates for each disposal unit measured and the amount of rainfall will be graphed and compared.

8.9 Describe Routine Gas Monitoring Program for the Landfill as Required

Routine gas monitoring will be initiated after the burial of putrescible waste or by-pass waste in the SW disposal units. No gas monitoring will be conducted relative to the last monofill disposal units.

8.10 Describe procedures for Operating and Maintaining the Landfill Stormwater Management System to Comply With the Standards of Chapters 17-3, 17-302, and 17-23, FAC.

The access road encompassing the landfill area and the disposal unit berms are elevated above existing ground elevations to prevent any surface water from entering the waste-filled area.

Additionally, a large swale is located at the base of the landfill slope on the interior side of the access road. The swale is designed to receive runoff from the predeveloped and any closed-out areas of the landfill and direct it to one of four major retention basins.

The bottom of the landfill disposal units are lined and positioned above the seasonal high water table to prevent any lateral flow into the waste-filled areas, if in the unlikely event that standing water was to occur in the swales. Also any closed-out disposal units will be capped with an 18-inch clay cap to inhibit vertical infiltration/percolation of rain.

The landfill supervisor will routinely inspect the stormwater management system. Particular attention will be given to inspecting the culverts under the access road for any blockage. The stormwater management system will also be inspected prior to a natural disaster if sufficient notice is available, and after any natural disaster (see Section 8.2.2.2.)

8.11 Equipment and Operation Feature Requirements.

8.11.1 Sufficient equipment for Excavating, Spreading, Compacting and Covering Waste.

The West Pasco Class I Landfill has been operating since 1990. Existing equipment has proved sufficient. The equipment available at the West Pasco Landfill is as follows:

Compactor	1
Bulldozer	2
Front-end loaders	2
Leachate Transport Truck	1
and 6,000-gallon tanker	
Dump Truck	1
Leachate Pumps	2
Dump Trucks	2

8.11.2 Reserve Equipment or Arrangements to Obtain Additional Equipment Within 24 Hours of Breakdown

Reserve equipment is available from the County's Public Works Division. All equipment on the list with the exception of the compactor are available from Public Works on a temporary basis. Additionally, the County provides for the replacement of equipment through a replacement account funded monthly during the expected life of the equipment.

8.11.3 Communication Equipment

Communication between personnel in the West Pasco Landfill Maintenance Building Resource Recovery Facility Scalehouse, and the West Pasco Class III Scalehouse and landfill staff operating equipment is maintained by two-way radios and the master communication system maintained for all County departments. Additionally, landfill staff can contact each other by two-way radios.

8.11.4 Personnel Shelter and Sanitary Facilities, First Aid Equipment

The West Pasco Landfill Maintenance Building provides the nearest shelter to the West Pasco Class I Landfill staff. The building includes office space, restrooms, and showers as well as two equipment/vehicle bays. Basic first aid is available at the maintenance building.

8.11.5 Dust Control Methods

That access road is paved. Unpaved, interior roads will be wet down with water using a spray truck on an as needed basis. Heavy equipment is enclosed and air conditioned. Dust masks, goggles, and hard hats are available to personnel working in excessively dusty area.

8.11.6 Fire Protection Capabilities and Procedures for Notifying Local Fire Department Authorities in Emergencies

Fire extinguishers are provided on all heavy equipment operating in the wastefill areas. Staff are directed to contact the Fire Department as discussed under Section 8.2.2.1 Fire Emergency Procedures.

8.11.7 Litter Control Devices

Private refuse haulers are not allowed in the West Pasco Class I Landfill except when non-processible or by-pass waste are being delivered to the solid waste disposal unit. During these exceptions, the landfill supervisor will require loads be covered, working face be kept to a minimum, cover applied efficiently, and routine clean-up occur, to control litter.

8.11.8 Signs Indicating Operating Authority, Traffic Flow, Hours of Operation, Disposal Restrictions

Signage indicating operating authority, traffic flow, hours of operation, disposal restrictions are provided at the entrances to the Resource Recovery Facility and the West Pasco Class III Landfill and Recycling Center. The landfill supervisor will ensure the signage is maintained.

8.12 Provide a Description of All-Weather Access Road, Inside Perimeter Road and Other Roads Necessary for Access Which Shall Be Provided at the Landfill.

All roads providing access to the landfill disposal units are paved with asphalt. These roads include access roads from the Resource Recovery Facility and the West Pasco Class III Landfill and Recycling Center, a perimeter road and entrance ramps to the constructed disposal units.

8.13 Additional Recordkeeping and Reporting Requirements

8.13.1 Records Used For Developing Permit Applications and Supplemental Information Maintained For the Design Period of the Landfill

Records used for developing permit applications and other Supplemental information will be maintained for the design period of the landfill in the Utilities Services Branch files.

8.13.2 Monitoring Information Calibration and Maintenance Records, Copies of Reports Required By Permit Maintained For At Least Ten Years

Reports required by the permit will be maintained for at least 10 years in the Utilities Services Branch files.

8.13.3 Background Water Quality Records Shall be Maintained for the Design Period of the Landfill

Background water quality records will be maintained for the design period of the landfill in the Utilities Services Branch files.

8.13.4 Maintain Annual Estimates of the Remaining Life of Constructed Landfills and of Other Permitted Areas Not Yet Constructed and Submit This Estimate Annually to the Department

The Operations and Maintenance Director will submit annually to the Department estimates of other remaining capacity of the constructed and unconstructed, permitted waste disposal units. Estimates will be maintained in the Utilities Services Branch Files.

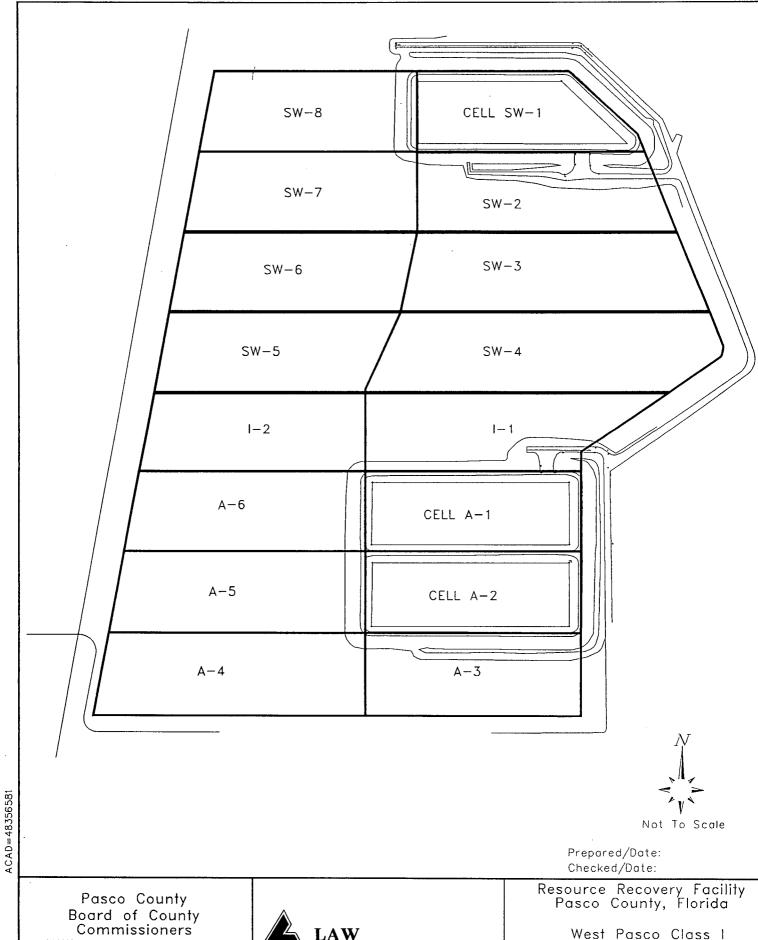
8.13.5 Annual Report Requirements Including a Report Submitted to the Department Which Is Signed, Dated and Sealed by P.G. or P.E.

A technical report, prepared, signed and sealed by a P.G. or P.E. with experience in hydrogeologic investigations, will be submitted to the Department every two years. The report will summarize and interpret the water quality data and water level measurements collected during the previous two years.

The report will also include tabular and graphical displays of any parameters detected and water level hydrographs for all monitoring wells. The report will further show trends and comparisons between zones or aquifers, comparisons between upgradient and downgradient wells, correlations between related parameters, any discussions of erratic and/or poorly correlated data. Ground-water contour maps will be interpreted as to ground-water flow direction and rates. The report will further evaluate the adequacy of the water quality monitoring frequency and sampling locations based upon the site conditions. The report will be signed, dated and sealed by a P.G. or P.E.

TABLE 8.1 WEST PASCO CLASS I LANDFILL DAILY LEACHATE FLOW METER AND RAINFALL READINGS

Mo./Yr.	Rainfall	CUMMULATIVE FLOW MET			ER READINGS in Gallo -2 CELL SW-		llions	TOTAL
D	(in inches)				Sec.	Prim.	Sec.	
Day		Prim.	Sec.	Prim.	Sec.	Friin.	Sec.	_
0			Ì	Ì				
1						<u></u>		
2								
4		 						
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7		-						
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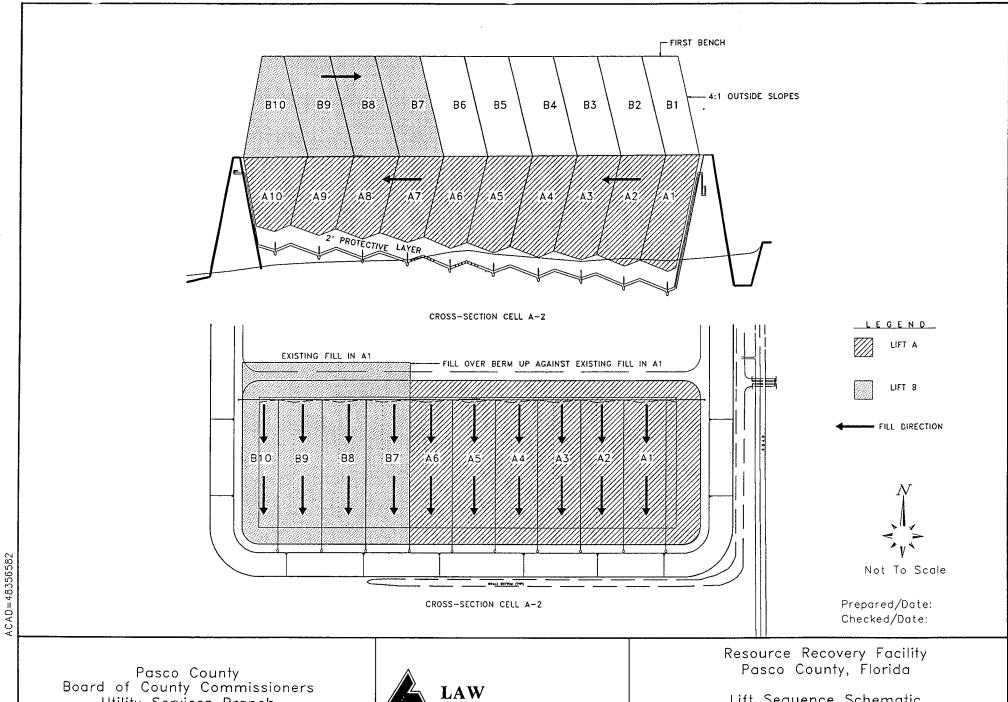


Utility Services Branch Pasco County, Florida



West Pasco Class I Landfill Footprint Schematic

Project 464-83565.01 Figure 8.1



Utility Services Branch Pasco County, Florida



Lift Sequence Schematic

Project 464-83565.01

Figure 8.2

Pasco County, Florida

464-83565.01 Figure 8.3

Project

222201-04

Pasco County
Board of County Commissioners
Utility Services Branch
Pasco County, Florida



Not To Scale

Prepared/Date: Checked/Date:

Resource Recovery Facility Pasco County, Florida

Final Cover Detail

Proje#64-83565.01

Figure 8.4

Prepared/Date: Checked/Date:

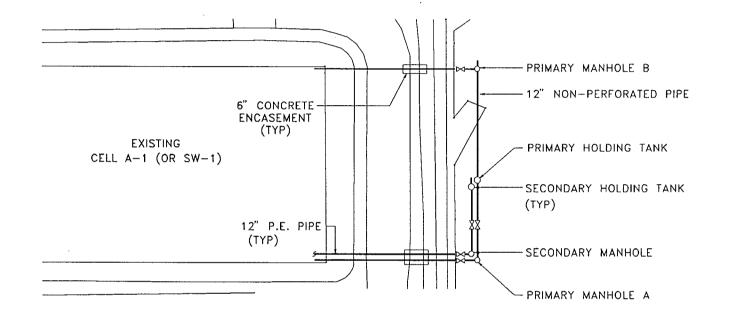
Pasco County
Board of County
Commissioners
Utility Services Branch
Pasco County, Florida



Resource Recovery Facility Pasco County, Florida

Gas Vent Detail

Project 464-83565.01 Figure 8.5



Not To Scale

Prepared/Date: Checked/Date:

Pasco County
Board of County
Commissioners
Utility Services Branch
Pasco County, Florida

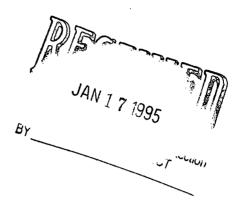


Resource Recovery Facility Pasco County, Florida Leachate Collection System Schematic Cell SW-1 and Cell A-1

Project 464-83565.01 Figure 8.6

Resource Recovery Facility - Class I Landfill Law Environmental Project 573565 November 16, 1994 Revision 2

> Jan5,1995 Revision3



SECTION 9.0

WATER QUALITY AND LEACHATE MONITORING REQUIREMENTS



SECTION 9.0

WATER QUALITY AND LEACHATE MONITORING REQUIREMENTS

9.1 Water Quality and Leachate Monitoring Plan Shall Be Submitted Describing the Proposed Ground-Water, Surface-Water and Leachate Monitoring Systems and Shall Meet at least the Following Requirements;

The following plan is designed to meet the current rule requirements in Chapter 17-701.510, F.A.C. by upgrading the existing ground-water monitoring plan as discussed in the pre-application meeting dated April 13, 1994.

9.1.1 Based on the Information Obtained in the Hydrogeological Investigation and signed, Dated and Sealed by the P.G. or P.E. Who Prepared It (17-701.510(2)(a), FAC).

The plan is based on the hydrogeological investigation provided in Section 6 of this application. The hydrogeological investigation included a review of the initial hydrogeological investigation the existing ground-water monitoring plan and subsequent water quality sampling and analysis.

9.1.2 All Sampling and Analyses Performed by Organizations Have Department-Approved Comprehensive Quality Assurance Plans; (17-701.510(2)(b), FAC).

The Pasco County Environmental Laboratory conducts the required ground-water sampling and analysis for the West Pasco County Landfill. Chemical analyses that are not performed at the county lab are contracted out under an annual agreement with other state approved labs.

Pasco County Environmental Laboratory's approved Comprehensive Quality Assurance Plan is FDEP #870167G.

9.1.3 Ground-Water Monitoring Requirements; (17-701.510(3), FAC).

9.1.3.1 Detection wells located downgradient from and within 50 feet of disposal units;

The detection wells are located downgradient from Disposal Units A-1 and A-2, with the exception of well 4MW-16D which is upgradient, and are within 50 feet of disposal units. The detection wells are depicted on Figure 9.1 and on Drawing Sheet 5 as proposed detection wells 4MW-11D through 16D and 2MW-13D. The 4MW detection wells are to be 4-inch PVC Type II monitoring wells screened in the Floridan aquifer and the 2MW detection well is to be 2-inch PVC Type II wells screened in the Surficial aquifer. Typical well construction diagrams for Type II monitoring wells are shown in Figure 9.2.

9.1.3.2 Downgradient Compliance Wells as required;

The downgradient compliance wells are located to the east, north, northwest and west of the landfill footprint, The downgradient compliance wells are existing wells and are depicted on Figure 9.1 as existing ground-water monitoring wells, 2MW-3 through 6 and 4MW-3 through 6. The 2MW wells are 2-inch PVC, Type II monitoring wells, screened in the surficial aquifer and the 4MW wells are 4-inch PVC Type II monitoring wells, screened in the Floridan aquifer.

9.1.3.3 Background wells screened in all aquifers below the landfill that may be affected by the landfill.

Background wells are depicted on Figure 9.1 as existing ground-water monitoring wells, 2MW-1 and 2 and 4MW-1 and 2. The 2MW wells are 2-inch PVC Type II monitoring wells, screened in the surficial aquifer and the 4MW wells are 4-inch PVC Type II monitoring wells screened in the Floridan aquifer.

9.1.3.4 Location Information for Each Monitoring Well.

All existing and proposed ground-water monitoring wells are shown on Figure 9.1. Location and completion data for the existing wells are provided in Table 9.1. All proposed monitoring well locations will be surveyed by a registered Florida Land Surveyor. Monitoring wells are protected by bumper posts (for example, 4MW-5 near proposed borrow area.

9.1.3.5 Well Spacing No Greater Than 500 Feet Apart for Downgradient Wells and no Greater Than 1500 Feet Apart for Upgradient Wells Unless Site Specific Conditions Justify Alternate Well Spacings;

The downgradient detection wells 4MW-11D through 4MW-15D are located 500 feet apart. The upgradient detection well, 4MW-16D is not greater than 1500 feet from the nearest detection well.

Ground-water monitoring data accumulated to date indicate the surficial aquifer to be dry in the immediate vicinity of the landfill. Consequently, the site specific conditions do not justify the installation of detection wells in the surficial aquifer on 500 feet and 1500 feet spacings. Detection well 2MW-13D is proposed as the only detection well in surficial aquifer.

9.1.3.6 Well Screen Locations Properly Selected;

The surficial aquifer ground-water monitoring wells will be screened in the base of permeable sand unit. The impermeable clayey sands encountered, immediately below the permeable, well-sorted, fine-grained sands are considered to be a part of confining layer. The screened interval will be 10 feet as depicted in the Well Construction Diagrams in Figure 9.2. The grain size of the sand pack will be adjusted in the field during installation, depending on the grain size of the formation sand in the screened interval.

The Floridan aquifer ground-water monitoring wells will be drilled and cased approximately 3 feet into the limestone. An open hole interval of approximately 10 feet will be drilled below the base of the casing into the Floridan aquifer.

9.1.3.7 Procedures for Properly Abandoning Monitoring Wells;

Any monitoring wells or piezometers which are abandoned or which will be lost due to lateral expansions of the landfill or the construction of new solid waste disposal units will be plugged as necessary so that they do not act as a conduit for any leachate release to the ground water. All such wells will be filled and sealed in accordance with the rules of the Southwest Florida Water Management District (SWFWMD), from bottom to top with neat cement grout.

The Department will be notified in writing before any monitoring wells are abandoned or plugged and will be given access, at reasonable times, to witness or inspect the plugging.

9.1.3.8 Detailed Description of Detection Sensor If Proposed

No sensors are proposed. The landfill design includes a leak detection system between the primary and secondary 60 mil HDPE liners. In the design for Disposal Unit A-2, an added barrier of protection will be placed immediately underneath the primary liner below the leachate collection header pipe. The added barrier, a geosynthetic clay liner, will provide a sealing capability to potential leaks at the hydraulic low points in the primary leachate collection system.

9.1.4 Surface-Water Monitoring Requirements; (17-701.510(4), F.A.C.).

9.1.4.1 Location of and Justification for All Proposed Surface-Water Monitoring permits.

No surface water bodies exist within the landfill site boundaries, and the stormwater management system provides for the retention of all collected stormwater runoff. The stormwater retention ponds are generally dry; consequently, no surface-water monitoring points are proposed.

9.1.4.2 Each Monitoring Location to be Marked and Its Position Determined by a Registered Florida Land Surveyor;

Not applicable, there are no proposed surface-water sampling locations.

9.1.5 Leachate Sampling Locations Proposed; (17-701.510(5), F.A.C.)

Double liner systems are in place in the existing disposal units and are proposed for the construction of Disposal Unit A-2. The double liner system, in addition to consisting of an upper and lower 60 mil HDPE geomembrane, also includes a primary leachate collection and removal system lying above the upper geomembrane and a leak detection and secondary leachate collection system between the upper and lower liners. Leachate samples will be taken from both the primary and secondary leachate collection systems.

Leachate removal from existing Disposal Unit A-1 is by gravity drainage through penetrations in disposal unit berm and liners to the leachate holding tanks. In this disposal unit, leachate samples will be taken from the influent to the primary and secondary leachate collection system holding tanks.

Leachate removal from Disposal Unit A-2 will be by pumping from a sump through a riser pipe to the top of the disposal unit berm then into the leachate management system. No penetration of the disposal unit berm or liners will occur. Leachate samples will be taken from the sample ports provided at the sideslope riser pump station primary and secondary leachate collection system.

9.1.6 Routine Sampling Frequency and Requirements; (17-701.510(6), FAC)

9.1.6.1 Background Ground-Water and Surface-Water Sampling and Analysis Requirements;

Background ground-water quality data has been collected for the existing ground-water monitoring system and is provided in the Appendix to this section. Background ground-water quality for proposed detection wells in this construction application will be taken prior to accepting ash into Disposal Unit A-2. Sampling and analysis for background water quality will be for the following parameters:

Static water levels in wells before purging Specific Conductivity	Total ammonia - N Chlorides	INAPPAOLAME
Ph	Iron	FOR SW
Dissolved Oxygen	Mercury	SAMPUES
Turbidity	Nitrate	
Temperature	Sodium	
Colors and Sheens (by observation)	Total Dissolved Solids (TDS)	

Laboratory Parameters

Those parameters are listed in 40 C.F.R. Part 258, Appendix I and Appendix II.

No surface-water monitoring is required.

Field Parameters

9.1.6.2 Leachate Semi-Annual and Annual Sampling Requirements.

Leachate will be sampled and analyzed semi-annually for the following parameters:

Field Parameters	Laboratory Parameters	DOES NOT SPECTICALLY
Specific Conductivity Ph Dissolved Oxygen	Total ammonia - N Bicarbonate Chlorides Iron Mercury Nitrate Sodium Total Dissolved Solids (TDS)	SPECTICALLY INDITATE HOW MANY LEAUTHER SAMPLES WILL THE COLLECTED

Those parameters are listed in 40 C.F.R. Part 258, Appendix I.

In addition, leachate will be sampled and analyzed annually for the parameters listed in 40 C.F.R., Part 258, Appendix II.

9.1.6.3 Detection Well Semi-Annual Sampling and Analysis Requirements;

All detection wells and background wells (2MW-1 and 2MW-2, 4MW-1 and 4MW-2) will be sampled and analyzed semi-annually (as provided for lined landfills under Chapter 17-701.510(6)(c)) for the following parameters:

Field Parameters	Laboratory Parameters	
Static water levels in wells before purging Specific Conductivity Ph Dissolved Oxygen Turbidity Temperature Colors and Sheens (by observation)	Total ammonia - N Chlorides Iron Mercury Nitrate Sodium Total Dissolved Solids (TDS)	DOES NOT PRESENT SW PARAM- ETERS IN THE EVENT
Those parameters are listed in 40 C.F.R. Part 258, App	endix I.	OF A DIX(HAMLE TO SW.

9.1.6.4 Compliance Well Sampling and Analysis Requirements;

Compliance wells, 2MW-3 through 2mw-6 and 4MW-3 through 4MW-6 will be sampled and analyzed semi-annually for the same parameters listed for the detection wells.

9.1.6.5 Surface-Water Sampling and Analysis Requirements.

Not applicable; no surface-water bodies exist.

9.1.7 Procedures for Implementing Assessment Monitoring and Corrective Action as Required; (17-701.510(7), F.A.C.)

If monitoring parameters are detected in detection wells in concentrations which are significantly above background water quality or which are at levels above the Department's water quality standards or criteria specified in Chapter 17-520, F.A.C., the County will resample the wells within 15 days after the sampling data is received to confirm the data. The County will notify the Department in writing within 14 days of this finding, in addition to its scheduled water quality monitoring report. Upon notification by the Department, the County will initiate assessment monitoring as follows:

• Continue routine monitoring of monitoring wells and leachate sampling locations as required under the permit.

listed in 40 C.F.R., Part 258, Appendix II. Any new parameters detected and confirmed in the affected downgradient wells will be added to their routine ground-water monitoring suite.

- Install additional compliance monitoring wells at the compliance line of the zone of discharge and downgradient from the affected detection monitoring wells within ninety days of notice to initiate assessment monitoring. These wells will be sampled and analyzed quarterly for the routine groundwater suite plus the parameters listed in 40 C.F.R., Part 258, Appendix I and II.
- Submit a contamination assessment plan within 180 days of the notice to initiate assessment monitoring that is designed to delineate the extent and cause of the contamination, potential violations outside the zone of discharge, and evaluate methods to prevent any such violations. Upon approval by the Department the County will implement the plan and submit a contamination assessment report in accordance with the plan.
- Discontinue assessment monitoring, if for two consecutive sampling events the concentrations of all
 parameters including 40 C.F.R., Part 258, Appendixes I and II are at or below background values,
 upon receiving approval by the Department after which the County can return to routine monitoring
 requirements.
- Submit a Remedial Action Plan to the Department if the Contamination Assessment Report indicates water quality standards are likely to be violated outside the zone of discharge. Upon approval by the Department, the County will initiate the corrective actions outlined in the plan.

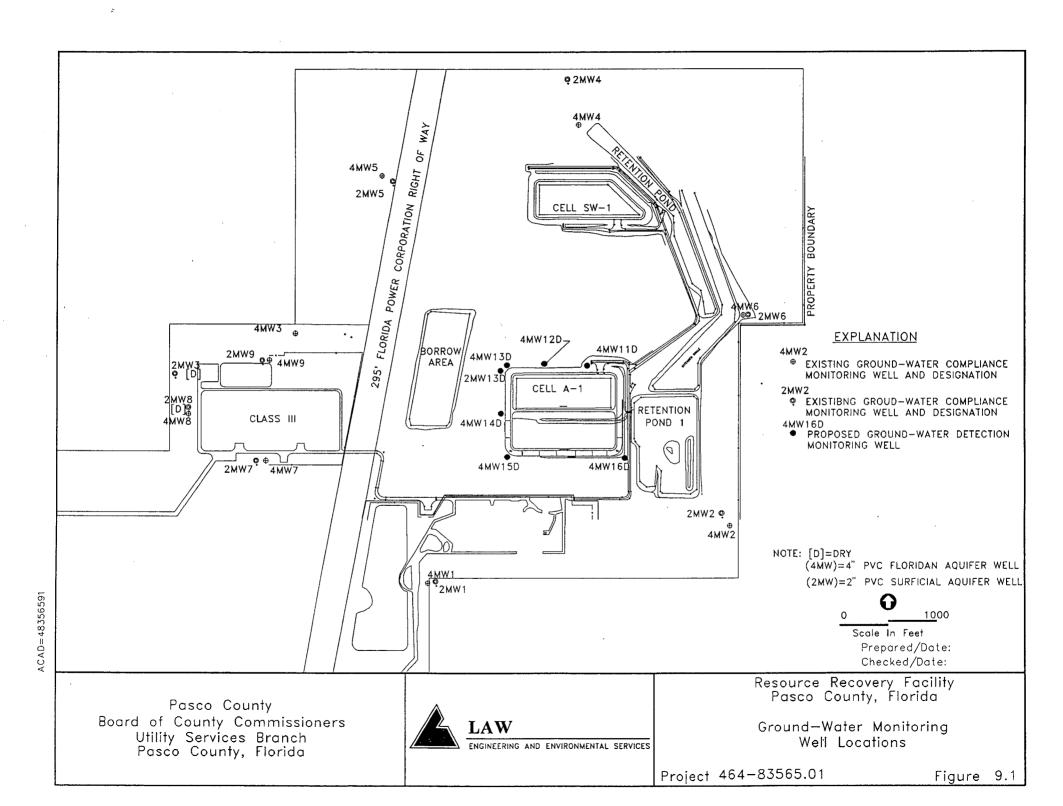
9.1.8 Water Quality Monitoring Report Requirements (17-701.510(a), F.A.C)

9.1.8.1 The County Will Report All Water Quality and Leachate Monitoring Results to the Department Semi-Annually. The County Will Notify the Department in at least 14 Days before the Sampling Is Scheduled to Occur so that the Department May Collect Split Samples. The Report Will Contain the Following Data:

Facility name and identification;
Sample collection and analyses dates;
Analytical results, including all peaks;
Identification number and designation of all monitoring points;
Applicable water quality standards;
Quality assurance, quality control notations;
Method detection limits;
Street code:

Water level data including elevation at top of casing and surface for each well on site at a precision of plus or minus 0.01 ft (NGVD);

The report will also provide updated ground-water maps at contour intervals no greater than one-foot with ground-water elevations and flow directions, and a summary of any water quality standards or criteria that have been exceeded.



ACAD=48356592

Pasco County Board of County Commissioners Utility Services Branch Pasco County, Florida



Resource Recovery Facility Pasco County, Florida

20'-30'

REVISED TO 10 FT

HOLE FOR

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Checked/Date:

Typical Well Construction Diagrams

Project 464-83565.01

Figure 9.2

TABLE 9.1
EXISTING GROUND-WATER MONITORING WELLS
LOCATION and COMPLETION DATA

Location		·	Ground * Casing *		ıg *	Screened Interval *		Total	
WELL	Latitude North	Longitude West	Level	Тор	Bottom	Тор	Bottom	Depth	
2MW-1	28 22' 05.8"	82 33' 48.1"	46.7'	49.97'	28.2'	38.2'	28.21	18'	
4MW-1	28 22' 05.5"	82 33' 48.1"	46.5'	49.61'	14.5'	open hole		60'	
2MW-2	28 22' 12.3"	82 33' 11.9"	52.8'	56.26'	17.8'	23.3'	18.3'	35'	
4MW-2	28 22' 12.2"	82 33' 11.9"	53.0'	55.76'	11.0'	open hole		70'	
2MW-3	28 22' 26.7"	82 34' 17.8"	41.7'	49.94'	26.7'	31.2'	26.2'	15'	
4MW-3A	28 22' 31.5"	82 34' 03.3"	50.6'	52.84'	28.6'	open hole		50'	
2MW-4	28 22' 57.7"	82 33'31.4"	51.3'	54.31'	35.3'	40.8'	35.8'	15'	
4MW-4	28 22' 52.5"	82 33' 30.3"	48.1'	50.52'	26.1'	open hole		50'	
2MW-5	28 22' 46.7"	82 33' 52.2"	45.3'	48.13'	37.3'	41.3'	37.3'	8'	
4MW-5	28 22' 47.2"	82 33' 53.4"	45.4'	48.26'	-22.6'	open hole		100'	
2MW-6	28 22' 32.7"	82 33' 11.1"	53.0'	55.64'	23.0'	33.0'	23.0'	20'	
4MW-6	28 22' 32.7"	82 33' 11.3"	52.4'	55.03'	-20.6'	open hole		100'	

SECTION 10.0 LANDFILL FINAL CLOSURE REQUIREMENTS

SECTION 10.0

LANDFILL FINAL CLOSURE REQUIREMENTS

10.1 Closure Schedule Requirements

A written notice, including a schedule for closure will be provided to the Department at least one year prior to final receipt of wastes. Final receipt of waste is not anticipated for at least fifty years.

The original permit for this landfill included space for monofilling solid waste and ash from the resource recovery facility. Pasco County operates the facility and are the users of the space designated for ash. Other authorized users of the solid waste management system are directed to deliver their wastes to either the adjacent Resource Recovery Facility or the West Pasco Class III Landfill and Recycling Center. If the solid waste disposal units are used in the future by other authorized users and that portion of the landfill is to be closed, the other authorized users will be notified 120 days prior to final receipt of waste.

Furthermore, the public will be notified within ten days prior to final receipt of waste. If closure of the entire facility would impact outside users notifications will be given according to the schedule for closure of the solid waste portion.

10.2 Closure permit general requirements

The application for a closure permit will be submitted to the FDEP 90 days prior to final receipt of wastes at the landfill.

A closure plan will be submitted as part of the application for closure of the facility. The closure plan will include at a minimum a closure report, a closure design plan, closure operation plan, closure procedures, and plan for long term care, and a demonstration of proof of financial responsibility for long term care will be provided.

10.3 Closure report requirements

The general information remains essentially unchanged from the Power Plant Site Certification Application. The information will be updated at the time the application is submitted for the closure permit. A geotechnical investigation will be conducted to the extent necessary at that time and the water quality monitoring plan revised accordingly. Additionally the report will include a land use information report, an actual gas migration report if biodegradable waste has been accepted at the landfill, and a report assessing the effectiveness of the landfill design and operation, as required in Ch.17-701.600(4), FAC.

10.4 Closure design requirements

A set of drawings were prepared and submitted with the original application to construct this landfill and reduced copies of these drawings are provided as an attachment.

10.4.1 Plan sheet showing phases of site closing

A reduced set of Power Plant Site Certification permit application Drawing Sheet No.8, Facilities Layout and Landfill Liner Limits and Drawing Sheet No.10, Plan of Operation is attached in Appendix 3. The facilities layout shows the individual disposal units.

10.4.2 Drawings showing existing topography and proposed final grades

Refer to Drawing Sheet 7 for the existing topography and final grading of site prior to receipt of waste.

10.4.3 Provisions to close units when they reach design dimensions

The plan of operation shows the filling sequence to the first bench level at 75 ft.,NGVD. Each Disposal Units A-1 through A-6 will be filled sequentially to the first bench level receive intermediate cover. The ashfill area will be taken up successive bench levels at 100 ft and 125 ft.,NGVD and receive intermediate cover after reaching each bench level. The final bench level will go a peak elevation of 150 ft.,NGVD and receive final cover.

10.4.4 Final elevation before settlement

The final elevation will be elevation 152 feet.

10.4.5 Side slope design including benches, terraces, downslope drainage ways, energy dissipators and a discussion of expected precipitation effects

These features are shown in the original permit application drawing set. See Drawing Sheet No. 10 in Appendix 3. Side slopes decrease upward after specific bench levels.

10.4.6 Final cover installation plans

At least 90 days prior to final receipt waste Pasco County will submit an application for final closure of the landfill. At that time the final cover installation plans will be submitted to the FDEP for approval. The plans included with the submittal of the original application will be revised at that time to reflect the FDEP requirements at the time of that submittal.

10.4.7 Final cover design requirements

The plans provided herein from the original application are in general conformance with the current design requirements. The cover design is designed to promote runoff so as to minimize infiltration and leachate generation. It includes 6 inches of top soil over 18 inches of clayey material. The design will be modified (if necessary) to conform to FDEP requirements at the time the facility is finally closed.

10.4.8 Proposed method of stormwater control

A stormwater management system was developed as part of the closure plans included in the original application submittal. The system has been permitted by the Southwest Water Management District. If it is necessary at the time of closure, this system will be modified to conform to requirements at that time.

10.4.9 Proposed method of access control

Access is restricted by a fence and manned gate at this time.

10.4.10 Proposed final use of the closed landfill

The site will be used for recreational purposes after the foundation and cover have stabilized.

10.5 Closure operation plan

10.5.1 Description of actions which will be taken to close the landfill

The sequence of activities that will be performed at final closure are:

- final grading and compaction of the top surface to conform to the slopes in the design plans
- inspection of the top surface to eliminate any objects that my damage the barrier layer
- placement of a barrier layer
- placement of the vegetative cover layer
- establishing grass on the surface
- placing survey monuments and final topographic survey

10.5.2 Time schedule for completion of closing and long term care

The estimated time required for construction of the closure should range from 200 to 270 days. The additional time required for bidding will add 100 days so the entire closure construction should take about one year after final receipt of waste. The long term care period will be 30 years unless the FDEP approves a reduced care period.

10.5.3 Describe the proposed method for demonstrating financial responsibility

Provisions for long term care will be the same as established for Disposal Unit A-1.

10.5.4 Additional equipment and personnel needed to complete the closure

Pasco County will contract to third party for the construction of the closure.

SECTION 11.0 CLOSURE PROCEDURES

SECTION 11.0

CLOSURE PROCEDURES

Not applicable at this time. The closure plan will include provision for survey monuments relative to closure. A final survey report will be prepared after closure and closure construction will be certified. Declarations to the public will be made as required. No projected closing date is available. The permitted landfill site is projected to last a minimum of fifty years. Increased recycling rates, waste stream reduction and potential ash reuse will extend the projected life of the facility even further. Eventually, when closed the site is to be considered for some form of recreation use.

SECTION 12.0 LONG-TERM CARE REQUIREMENTS

SECTION 12.0

LONG-TERM CARE REQUIREMENTS

The Department will be given the right of property access after closure as required by Ch. 17-701, FAC. Pasco County has no plans at this time to sell the property after closure. The County will maintain and replace when necessary monitoring devices installed as part of the closure.

SECTION 13.0 FINANCIAL RESPONSIBILITY REQUIREMENTS

SECTION 13.0

FINANCIAL RESPONSIBILITY REQUIREMENTS

The following cost figures are based on recent landfill closure construction bids and on supply quotes.

APPENDIX 3

REDUCED POWER PLANT SITE CERTIFICATION DRAWINGS

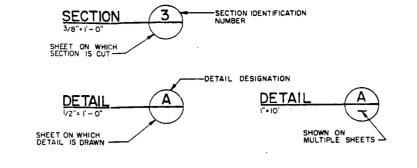
APPENDIX 3

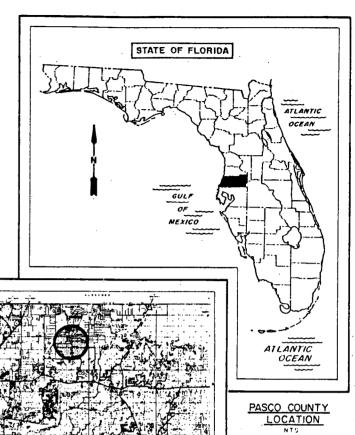
REDUCED
POWER PLANT SITE CERTIFICATION
DRAWINGS

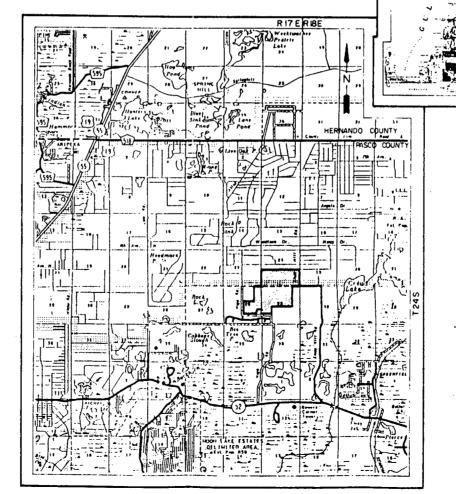
GENERAL NOTES

- 1. ALL ELEVATIONS REFER TO MEAN SEA LEVEL(MSL) BASED ON USGS DATUM.
- 2. LOCATION OF NEW ROADS AND STRUCTURES ARE APPROXIMATE.

 3. THESE PLANS ARE FOR PERMIT PURPOSES ONLY AND ARE NOT BIDDABLE PLANS.







SITE LOCATION

INDEX OF SHEETS

COVER SHEET

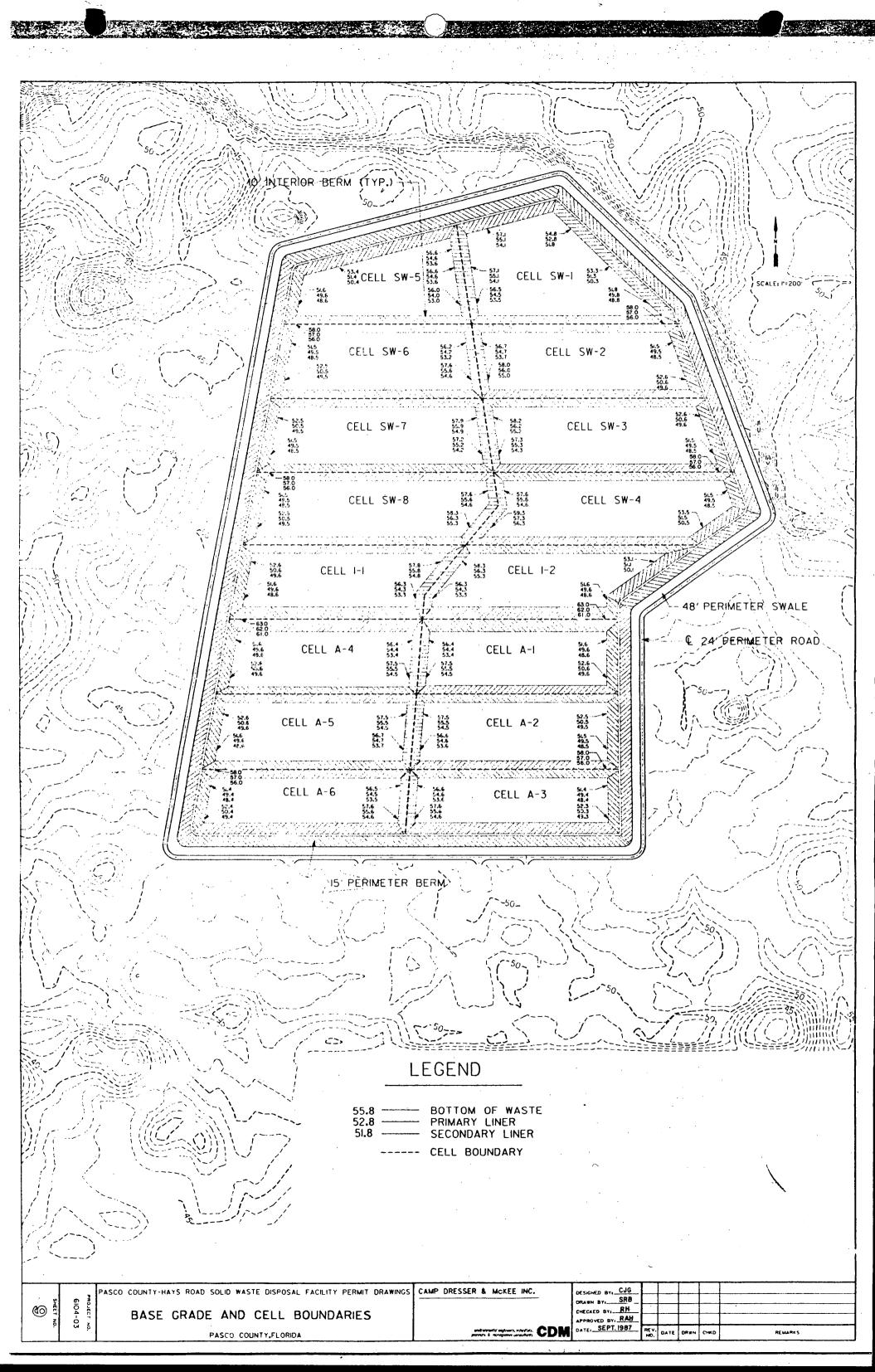
- 1. GENERAL NOTES, LOCATION MAPS AND INDEX OF SHEETS
- 2. AREA MAP
 3. EXISTING AERIAL TOPOGRAPHY (S.W.)
- 4. EXISTING AERIAL TOPOGRAPHY (S.E.) 5. EXISTING AERIAL TOPOGRAPHY (N.)

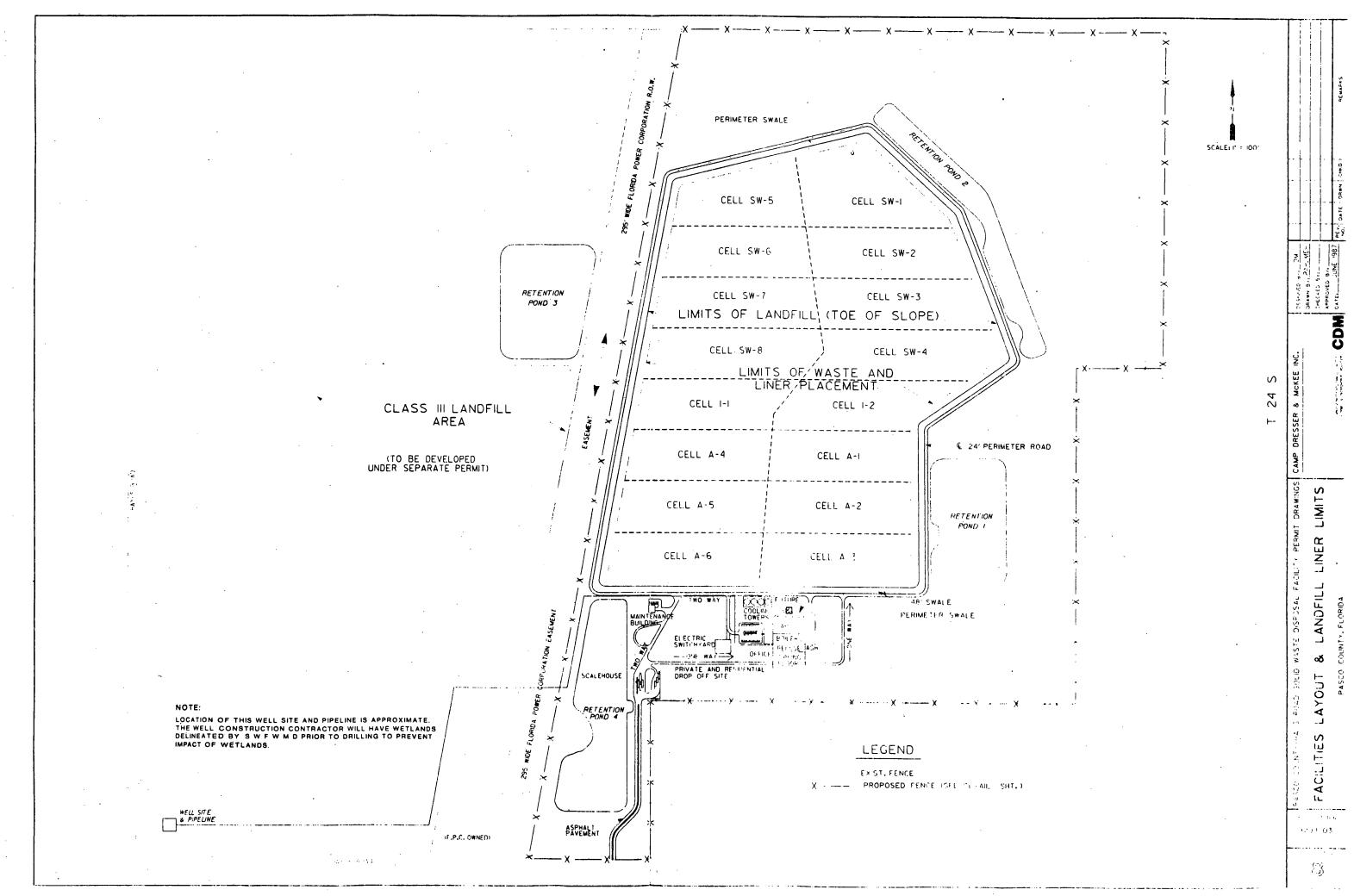
AREA LOCATION

- € DIMENSIONS AND LEGAL DESCRIPTION
- 7. SOIL BORINGS AND EXISTING ON-SITE MONITORING WELLS
- 8. FACILITIES LAYOUT AND LANDFILL LINER LIMITS
 9. BASE GRADES AND CELL BOUNDARIES
- 10. PLAN OF OPERATION
- 11. FINAL CLOSEOUT CONTOURS
- 12. LEACHATE COLLECTION SYSTEM
 13. SURFACE WATER MANAGEMENT GRADING
- 14. SURFACE WATER MANAGEMENT GRADING
- 15. CROSS SECTIONS
- 16. MISCELLANEOUS DETAILS

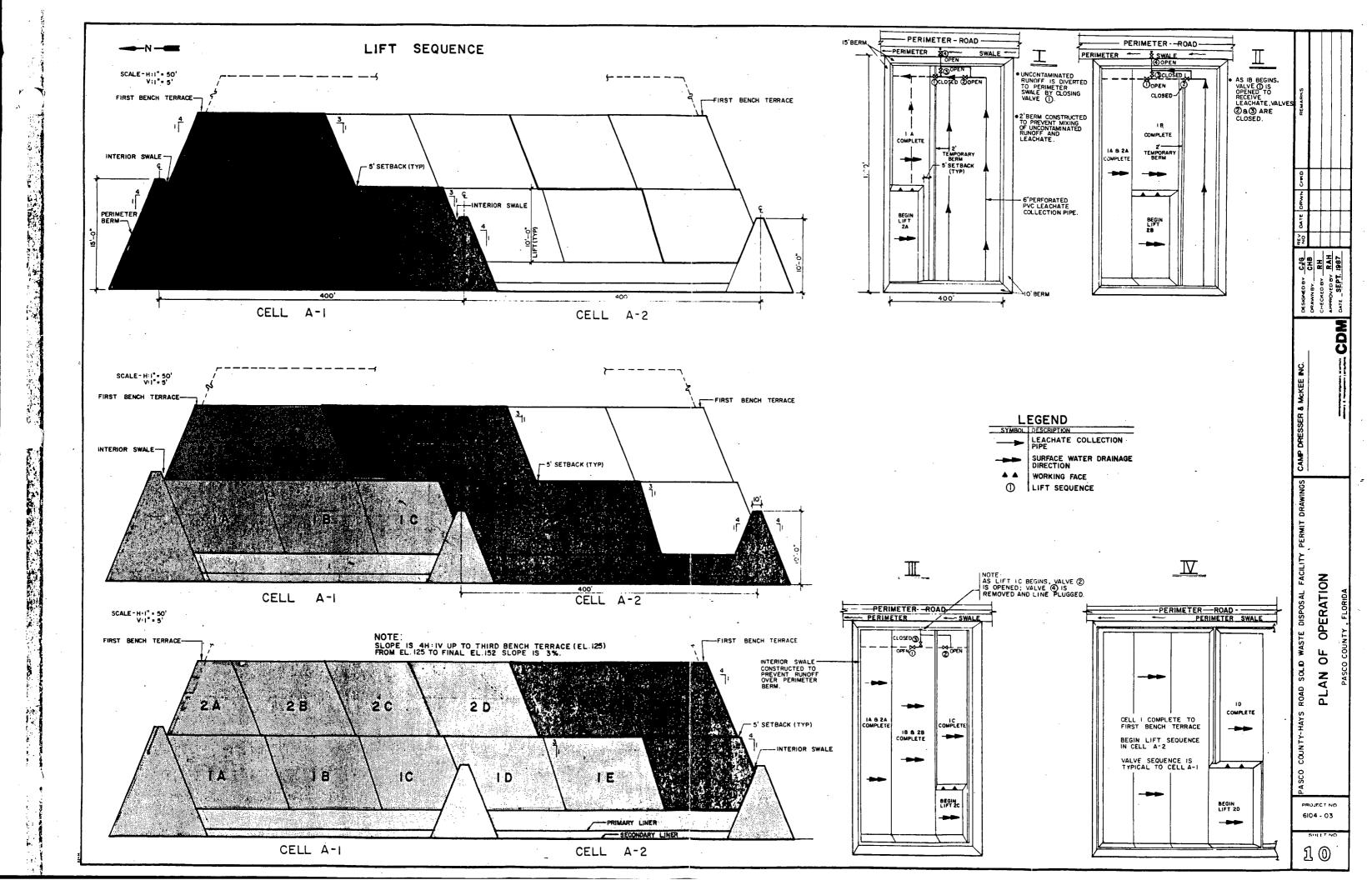
AND LOCATION MAPS OF SHEETS GENERAL

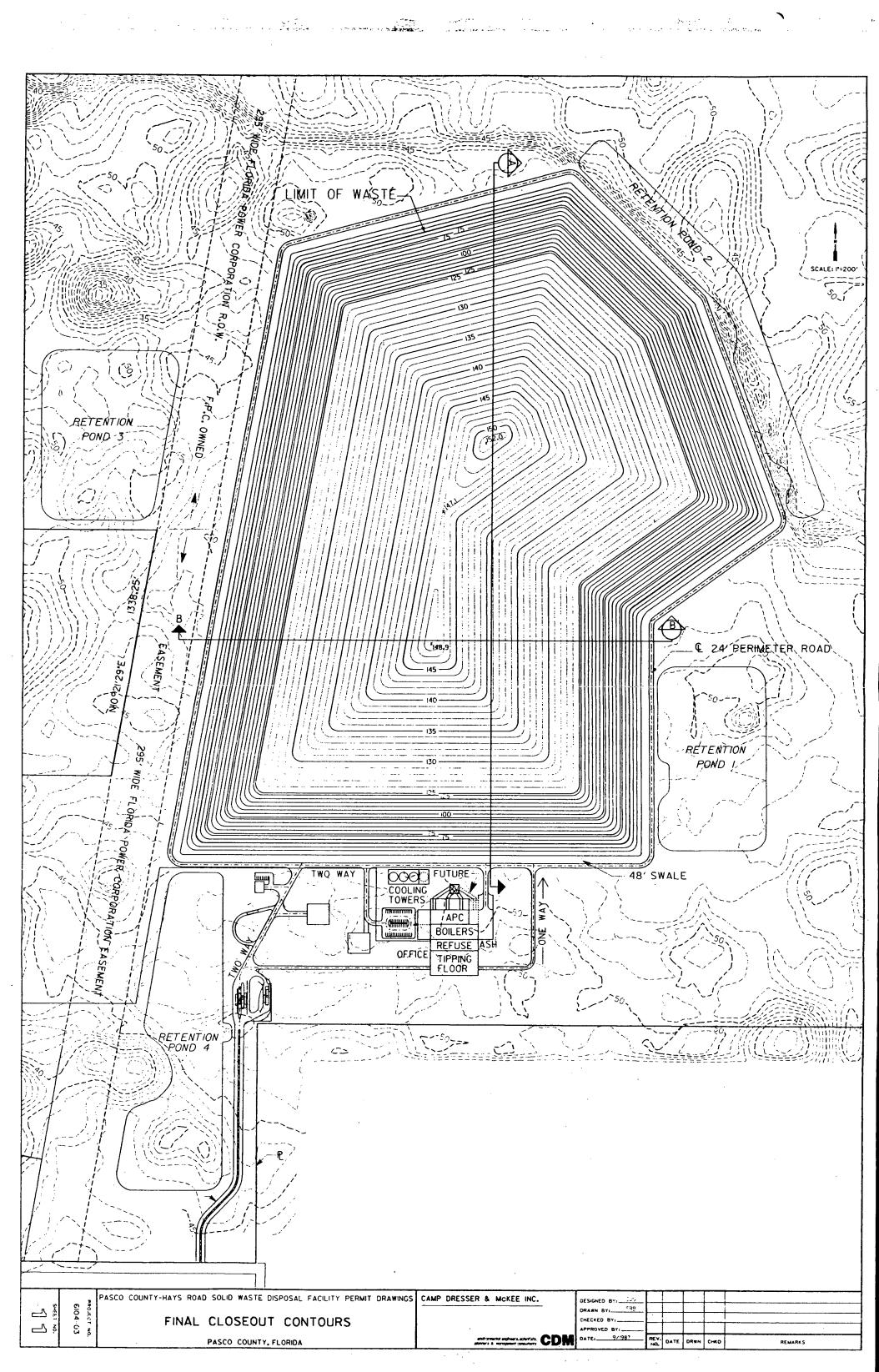
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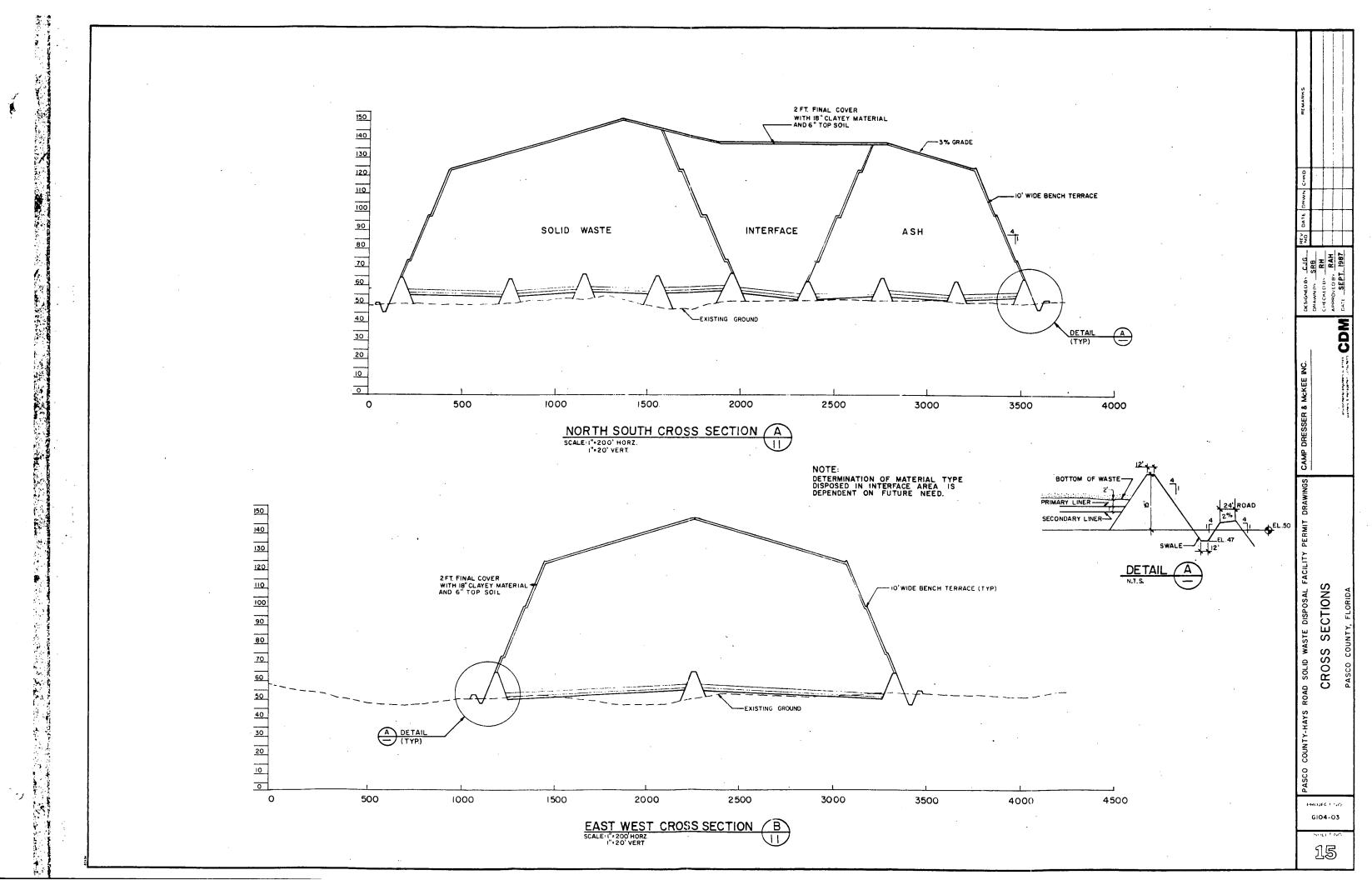




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APPENDIX 5 LANDFILL CONSTRUCTION DATA

APPENDIX 5

APPENDIX A5.1 LINER COMPACTIBILITY DATA

APPENDIX A5.1 LINER COMPATIBILITY DATA



LABORATORY REPORT #3601C

CHEMICAL RESISTANCE TESTING OF INCINERATOR ASH

DATE: 6/29/88

PROJECT:

Results of chemical compatibility (EPA Method 9090) testing of Gundline HD 60 mil. with incinerator ash sludge after 120 days immersion. The incinerator ash sludge was prepared by mixing incinerator ash with tap water in a 1:3 water:ash volume ratio.

TEST METHOD:

Die-cut ASTM test specimens were placed in the waste solution and kept at a temperature of 23°C and 50°C. Samples were removed at 15 day intervals and subjected to the following tests:

TENSILE PROPERTIES ASTM D638 Type IV @ 2ipm

	23°C Machine Di	rection Averages	
	YIELD	BREAK	BREAK
	STRENGTH (PSI)	STRENGTH (PSI)	<pre>ELONGATION(%)</pre>
0 Days	3024	4477	718
15 Days	3023	4452	741
% Change	· O · ·	- .6	+3.2
30 Days	2688	4345	756
% Change	-11.1	-2.9	+5.3
45 Days	2950	4309	725
% Change	-2.4	-3.8	+1.0
60 Days	2861	*	614
% Change	-5.4	*	-14.5
90 Days	3011	4074	693
& Change	4	-9.0	-3.6
120 Days	3024	4050	705
% Change	0	-9.5	-1.8

Gundle

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1000mm 10000mm 1000mm 1000mm 1000mm 1000mm 1000mm 10000mm 10000mm 10000mm 10000mm 1000mm 10000mm 10000mm 10000mm 10000mm 10000mm 10000mm 100

	23°C Transverse D	irection Average	2S
	YIELD	BREAK	BREAK
<u>:</u>	strencth (PSI)	Strength (PSI)	elongation(%)
0 Days	3160	4686	789
15 Days	3100	4027	703
% Change	-1.9	-14.1	-10.9
30 Days	2873	*	650
% Change	-9.8	*	-17.6
45 Days	3012	3832	678
% Change	-4.7	-18.2	-14.1
60 Days	2946	4115	741
% Change	-6.8	-12.2	-6.1
90 Days	3034	3776	662
% Change	-4.0	-19.4	-16.1
120 Days	2974	4410	773
% Change	- 5.9	-5.9	-2.0
e cuande	-5.9		-2.0
	50°C Machine Di	rection Averages	
	ÄÏEľÐ	BREAK	BREAK
	Strength (PSI)	Strength (PSI)	ELONGATION(%)
O Darre	2024	4 4 77	718
0 Days	3024	4477 4751	718 773
15 Days	3131		
% Change	+3.5	+6.1	+7.7 *
30 Days	2966	*	*
% Change	-1.9	*	•
45 Days	3118	*	*
Change	+3.1	*	*
60 Days	3007	4193	697
% Change	6	-6.3	-2.9
90 Days	2802	3792	679
% Change	-7.3	-15.3	-5.4
120 Days	3117	4610	752
% Change	+3.1	+3.0	+4.7
	50°C Transverse D	irection Averso	96
	YIELD	BREAK	BREAK
	STRENGTH (PSI)	STRENGTH (PSI)	ELONGATION (%)
O' Traven	•	•	
0 Days	3160	4686	789
15 Days	3322	4413	826
<pre>% Change</pre>	+5.1	- 5.8	+4.7
30 Days	3521	4525	686
% Change	+11.4	-3.4	-13.1
45 Days	3154	3861	662
<pre>t Change</pre>	1	-17.6	-16.1
60 Days	2999	4189	735
% Change	-5.1	-11.	-6.8
90 Days	3656	*	*
% Change	+15.7	*	*
120 Days	3024	3831	695
% Change	-4.3		-11.9
e mianide	-4.3	-18.2	_TT•A



TEAR RESISTANCE INITIATION (LBS) ASTM D1004

	Machine 23 ⁰ C	Direction 50°C	Transverse 23 ^O C	Direction 50°C
0 Days	25 C 56	56 56	63	63
15 Days	58	58	71	62
% Change	+3.6	+3.6	+12.7	-1.6
30 Days	*	*	*	*
% Change	*	*	*	*
45 Days	52	52	61	63
% Change	-7.1	-7.1	-3.2	0
60 Days	54	54	54	61
% Change	-3.6	-3.6	-14.3	-3.2
90 Days	53	53	55	67
% Change	-5.4	-5.4	-12.7	+6.3
120 Days	55	56	54	66
% Change	-1.8	0	-14.3	+4.8

PUNCTURE RESISTANCE (LBS) FTMS 101B Method 2065

	23 ⁰ C	50 ⁰ C
0 Days	103	103
15 Days	86	92
% Change	-16.5	-10.7
30 Days	84	87
% Change	-18.4	-15.5
45 Days	88	89
% Change	-14.6	-13.6
60 Days	91	80
% Change	-11.7	-22.3
90 Days	87	84
% Change	-15.5	-18.4
120 Days	99	88
% Change	-3.9	-14.6

THICKNESS (MIL) CHANGES AT 23°C ASTM D1593

65
65
0
65
0
62
-4.6
64
-1.6
65
0
65
O



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THICKNESS (MIL) CHANGES AT 50°C ASTM D1592

0 Days	68
15 Days	66
% Change	-2.9
30 Days	67
% Change	-1.5
45 Days	64
% Change	-5.9
60 Days	66
% Change	-2.9
90 Days	67
% Change	-1. 5
120 Days	68
% Change	0

WEIGHT CHANGE AT 23°C

0 Days	•	2.9234
15 Days		2,9253
& Change	,	0
30 Days		2.9265
<pre>t Change</pre>		+.1
45 Days		2.9258
% Change		+.1
60 Days		2.9262
% Change		+.1
90 Days		2.9274
% Change	y.	+.1
120 Days	Ÿ.	2.9268
% Change		+.1

WEIGHT CHANGE AT 50°C

0 Days	3.0366
15 Days	3.0378
% Change	0
30 Days	3.0405
% Change	+.1
45 Days	3.0403
% Change	+.1
60 Days	3.0414
% Change	+.2
90 Days	3.0433
* Change	+.2
120 Days	3.0400
% Change	+.1

Gundle

HARDNESS AT 23°C ASTM D2240

0 Days	61
15 Days	61
% Change	0
30 Days	62
% Change	+1.6
45 Days	61
% Change	0
60 Days	61
% Change	0
90 Days	63
& Change	+3.3
120 Days	62
% Change	+1.6

HARDNESS AT 50°C ASTM D2240

0 Days	62
15 Days	61
% Change	-1.6
30 Days	62
% Change	0
45 Days	62
Change	0
60 Days	62
% Change	0
90 Days	64
% Change	+3.2
120 Days	65
% Change	+4.8

ENVIRONMENTAL STRESS CRACK ASTM D1693, INCINERATOR ASH SLUDGE

- 0% failures at 60 days of immersion (1440 hours)
 0% failures at 90 days of immersion (2160 hours)
 0% failures at 120 days of immersion (2880 hours)
- Note: These test specimens got stuck in the incinerator ash sludge because the sludge turned to cement. The specimens were too damaged for reliable testing.

90°d



DISCUSSION:

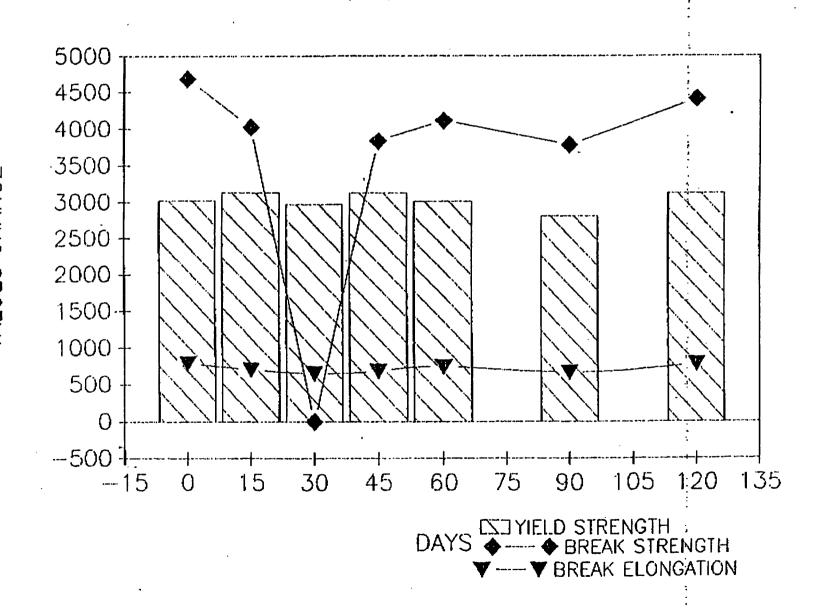
No significant changes after 120 days in either the destructive testing or the stress crack test.

Linda Ritchey Lab Technician

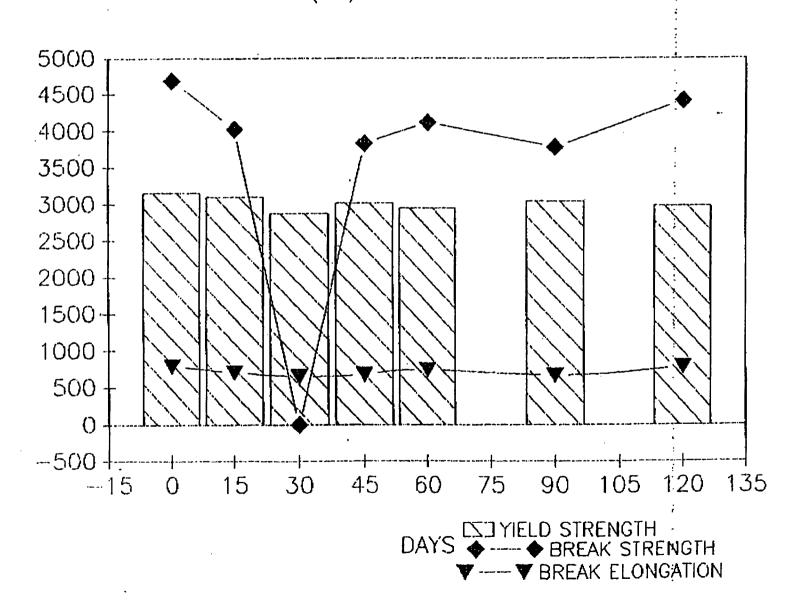
一次の変数を対しいできます。 これのない 大変なのない

Mark Cadwallader Director of Research Technical Development

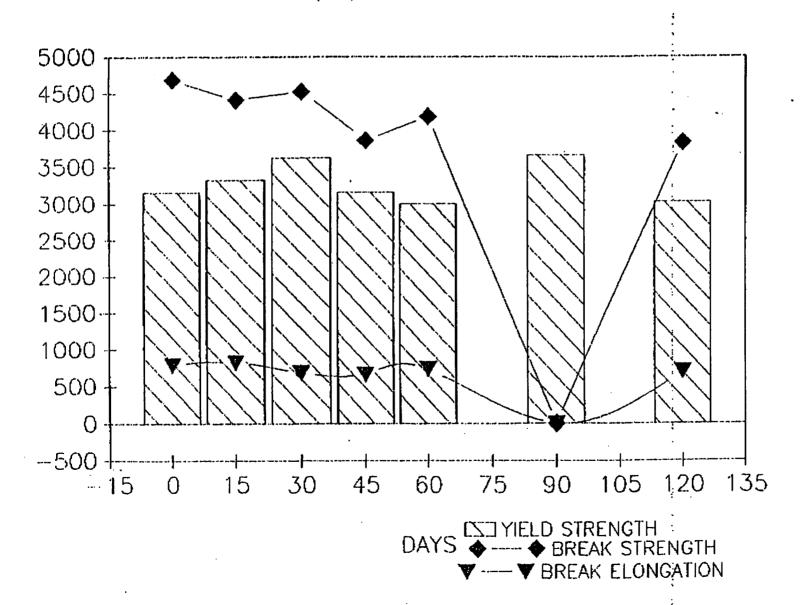
TENSILE PROPERTIES AT 50C (MD) MACHINE DIRECTION



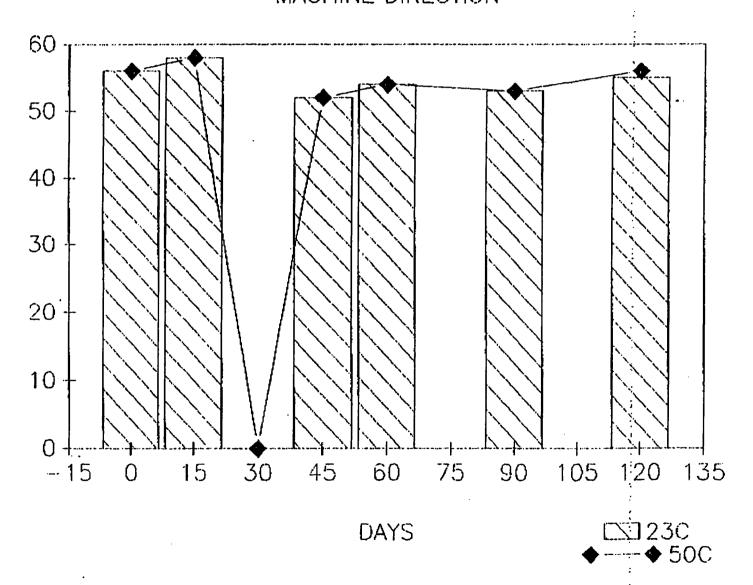
TENSILE PROPERTIES
AT 23C (TD) TRANSVERSE DIRECTION



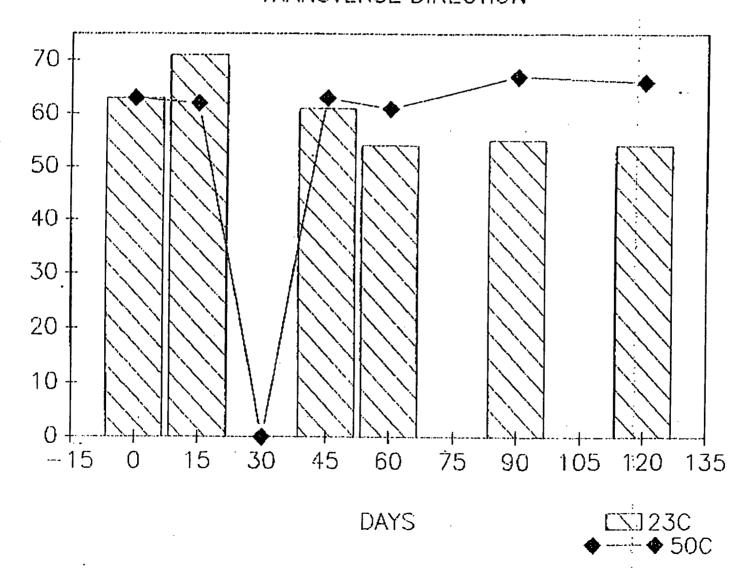
TENSILE PROPERTIES AT 50C (TD) TRANSVERSE DIRECTION



TEAR RESISTANCE INITIATION (LBS) ASTM D1004 C MACHINE DIRECTION

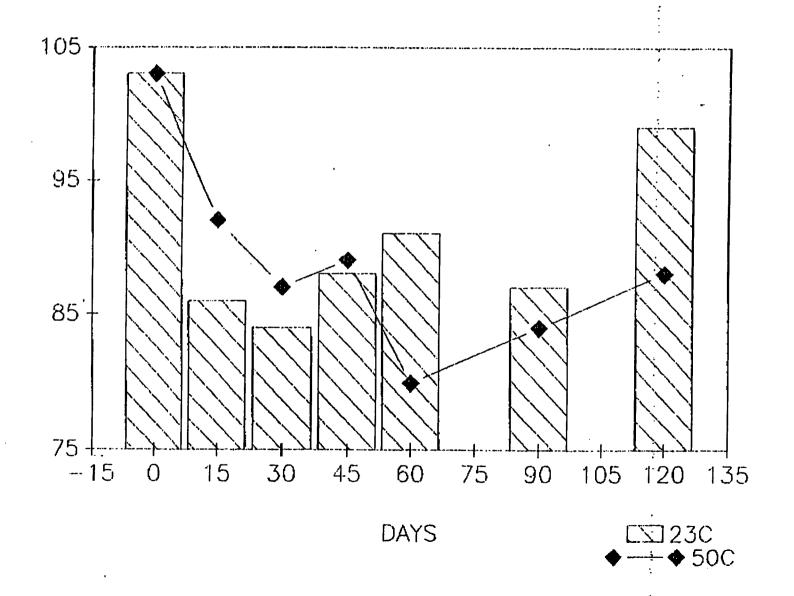


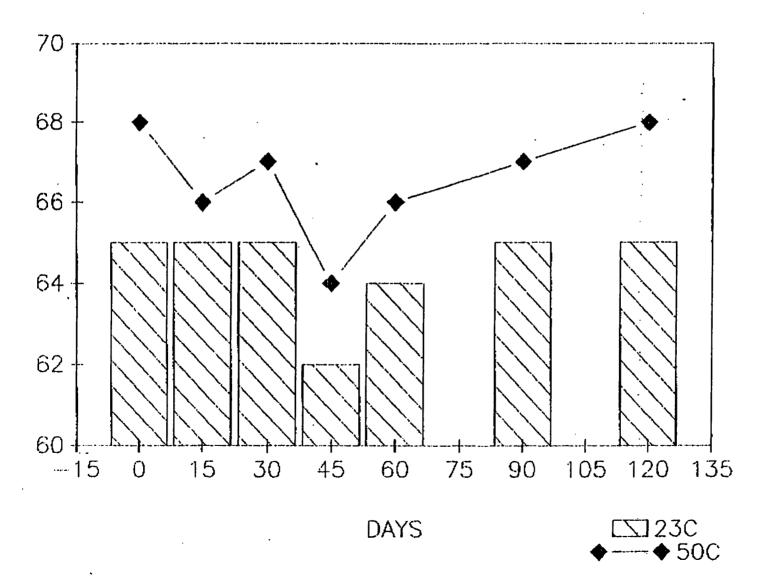
TEAR RESISTANCE INITIATION (LBS) ASTM D1004 C TRANSVERSE DIRECTION



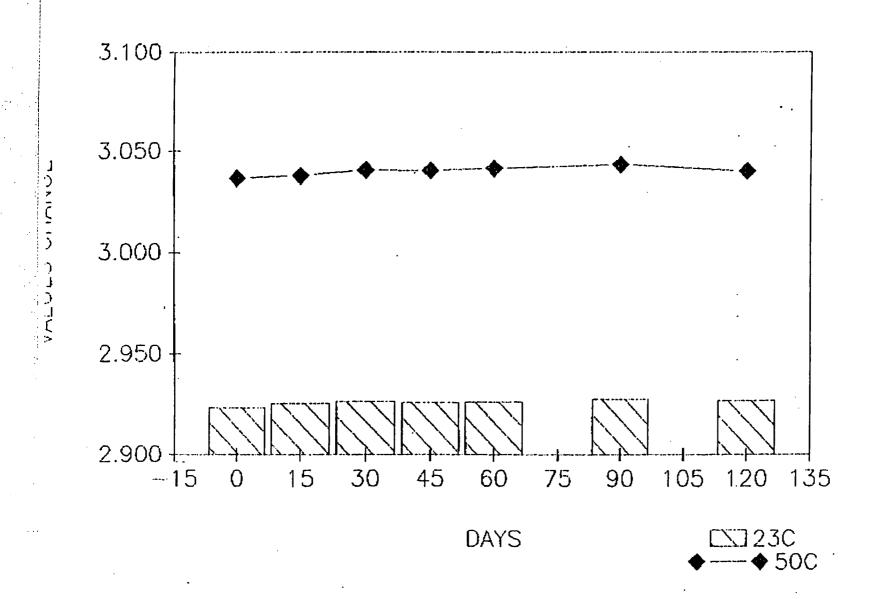
PUNCTURE RESISTANCE (LBS) FTMS 101B METHOD 2065

(1) 1977 - 《中華教育**政**學》(1975年1977) 🏙 (1) 1931年 (1) 1938年 (1) 1938年 (1) 1938年 (1) 193



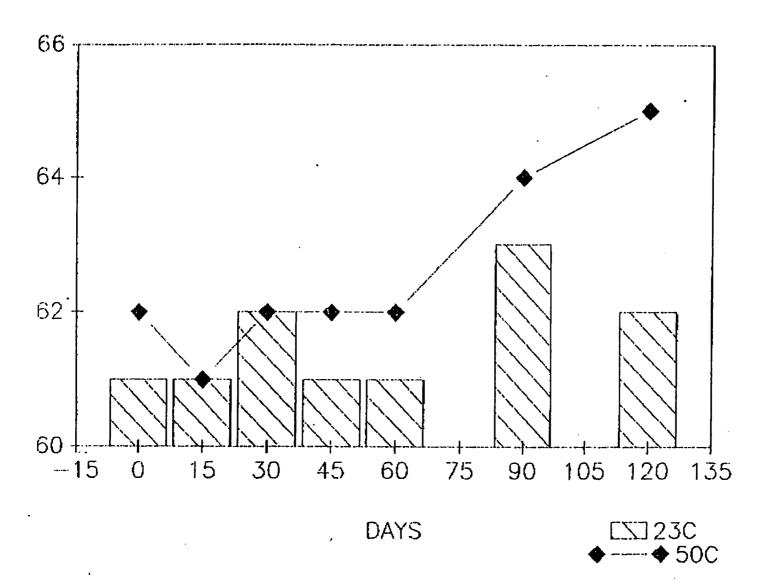


WEIGHT CHANGE

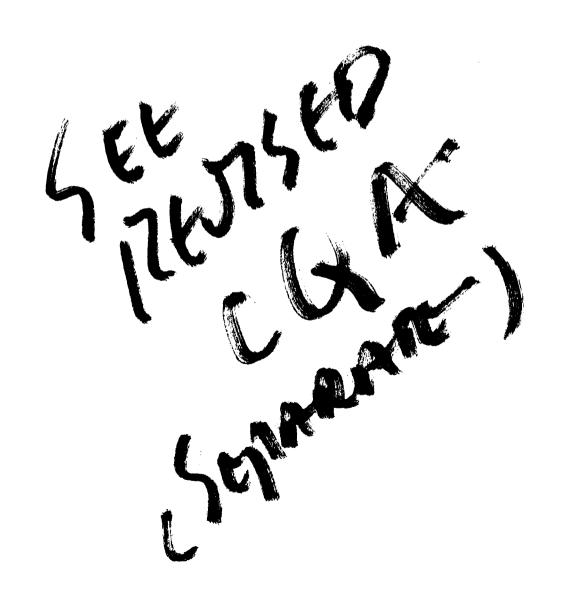


HARDNESS ASTM D2240

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APPENDIX A5.2 LINER QUALITY ASSURANCE PROGRAM



APPENDIX A5.2

LINER QUALITY ASSURANCE PROGRAM

APPENDIX A5.3 HELP MODEL RESULTS

APPENDIX A5.3

HELP MODEL RESULTS

BARE GROUND

LAYER 1

VERTICAL PERCOLATION LAYER

=	6.00 INCHES
=	0.4570 VOL/VOL
=	0.0831 VOL/VOL
=	0.0326 VOL/VOL
=	0.0831 VOL/VOL
=	0.003100000089 CM/SEC
	= = =

LAYER 2

VERTICAL PERCOLATION LAYER

THICKNESS	=	120.00 INCHES
POROSITY	=	0.4500 VOL/VOL
FIELD CAPACITY	=	0.1160 VOL/VOL
WILTING POINT	=	0.0487 VOL/VOL
INITIAL SOIL WATER CONTENT	=	0.1160 VOL/VOL
SATURATED HYDRAULIC CONDUCTIVITY	=	0.010000000708 CM/SEC

LAYER 3

LATERAL DRAINAGE LAYER

THICKNESS	=	24.00 INCHES
POROSITY	=	0.4370 VOL/VOL
FIELD CAPACITY	=	0.0620 VOL/VOL

WILTING POINT = 0.0240 VOL/VOL INITIAL SOIL WATER CONTENT =
SATURATED HYDRAULIC CONDUCTIVITY = 0.0620 VOL/VOL 0.005799999926 CM/SEC 2.00 PERCENT SLOPE = DRAINAGE LENGTH 50.0 FEET

LAYER 4

BARRIER SOIL LINER WITH FLEXIBLE MEMBRANE LINER

THICKNESS = 12.06 INCHES POROSITY 0.3509 VOL/VOL = FIELD CAPACITY 0.0705 VOL/VOL WILTING POINT 0.0326 VOL/VOL = INITIAL SOIL WATER CONTENT 0.3509 VOL/VOL =

SATURATED HYDRAULIC CONDUCTIVITY = 0.000154999900 CM/SEC LINER LEAKAGE FRACTION = 0.00010000

LAYER 5 _____

LATERAL DRAINAGE LAYER

THICKNESS 0.23 INCHES = POROSITY 0.8500 VOL/VOL FIELD CAPACITY 0.0250 VOL/VOL = = 0.0200 VOL/VOL WILTING POINT INITIAL SOIL WATER CONTENT = 0.0250 VOL/VOL SATURATED HYDRAULIC CONDUCTIVITY = 3.299999952316 CM/SEC SLOPE = 2.00 PERCENT DRAINAGE LENGTH 50.0 FEET =

LAYER 6 -----

BARRIER SOIL LINER WITH FLEXIBLE MEMBRANE LINER

THICKNESS = 6.06 INCHES 0.4224 VOL/VOL POROSITY FIELD CAPACITY 0.3495 VOL/VOL INITIAL SOIL WATER CONTENT =
SATURATED HYDRAULIC CONDUCTION WILTING POINT 0.2648 VOL/VOL 0.4224 VOL/VOL

= 0.000010000000 CM/SEC SATURATED HYDRAULIC CONDUCTIVITY

LINER LEAKAGE FRACTION 0.00010000

GENERAL SIMULATION DATA

TOTAL AREA OF COVER	=	300564. SQ FT —
EVAPORATIVE ZONE DEPTH	=	10.00 INCHES
POTENTIAL RUNOFF FRACTION	=	0.00000
UPPER LIMIT VEG. STORAGE	=	4.5420 INCHES
INITIAL VEG. STORAGE	=	0.9626 INCHES
INITIAL SNOW WATER CONTENT	=	0.0000 INCHES
INITIAL TOTAL WATER STORAGE IN		
SOIL AND WASTE LAYERS	=	22.7039 INCHES

SOIL WATER CONTENT INITIALIZED BY USER.

CLIMATOLOGICAL DATA

USER SPECIFIED RAINFALL WITH SYNTHETIC DAILY TEMPERATURES AND SOLAR RADIATION FOR TAMPA FLORIDA

MAXIMUM LEAF AREA INDEX = 0.00START OF GROWING SEASON (JULIAN DATE) = 0 END OF GROWING SEASON (JULIAN DATE) = 367

NORMAL MEAN MONTHLY TEMPERATURES, DEGREES FAHRENHEIT

JAN/JUL	FEB/AUG	MAR/SEP	APR/OCT	MAY/NOV	JUN/DEC
59.80	60.80	66.20	71.60	77.10	80.90
82.20	82.20	80.90	74.50	66.70	61.30

******	*****	*****	*****	****	*****	*****	*
AVERAGE MONTHLY	VALUES IN	N INCHES	FOR YEAR	RS 1 '	THROUGH	5	
	JAN/JUL	FEB/AUG	MAR/SEP	APR/OCT	MAY/NOV	JUN/DEC	_
PRECIPITATION							
TOTALS	1.67 8.12	2.16 7.15	4.27 5.48	1.20 2.17	1.71 0.69	4.50 3.52	
STD. DEVIATIONS	1.00 2.52	0.60 4.17	4.41 1.52	1.41 1.26	1.50 0.46	3.39 1.06	
RUNOFF							
TOTALS	0.000	0.000	0.000	0.000	0.000	0.000	
STD. DEVIATIONS	0.000	0.000	0.000	0.000	0.000	0.000	

EVAPOTRANSPIRATION

TOTALS	1.468 4.570	1.501 4.575	2.292 3.748	1.079 1.725	1.570 0.834	3.146 2.119
STD. DEVIATIONS	0.229 0.890	0.536 1.280	0.775 0.409	0.861 0.937	1.712 0.661	1.686 0.778
LATERAL DRAINAGE FRO	M LAYER	3				
TOTALS	0.7631 0.6648	0.6073 1.1025	0.6357 1.6441	1.1299 1.7889	0.8786 1.3448	0.6296 1.0812
STD. DEVIATIONS	0.4502 0.4049	0.3525 0.5750	0.3609	1.3653 0.7979	0.9419 0.3474	0.5254 0.1947
PERCOLATION FROM LAY	ER 4					
TOTALS	0.0208 0.0204	0.0186 0.0224	0.0203 0.0237	0.0212 0.0250	0.0210 0.0230	0.0196 0.0225
STD. DEVIATIONS	0.0027 0.0023	0.0022 0.0024	0.0022	0.0050 0.0026	0.0040 0.0013	0.0027 0.0008
LATERAL DRAINAGE FRO	M LAYER	5				
TOTALS	0.0199 0.0195	0.0178 0.0215		0.0204 0.0241	0.0201 0.0221	0.0187 0.0217
STD. DEVIATIONS	0.0027 0.0023	0.0022 0.0024	0.0022	0.0050 0.0026	0.0040 0.0013	0.0027 0.0008
PERCOLATION FROM LAY	ER 6					
TOTALS	0.0009	0.0008	0.0009	0.0009	0.0009	0.0009
STD. DEVIATIONS	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
*******	******	*****	*****	*****	*****	*****

AVERAGE ANNUAL TOTALS &	(STD	. DEVIATI	101	NS) FOR Y	EARS	1 THRO	OUGH	5
		(INC	CHE	ES)	(CU.	FT.)	PERCE	NT
PRECIPITATION		42.63	(4.707)	1067	653.	100.0	0
RUNOFF		0.000	(0.000)		0.	0.0	0
EVAPOTRANSPIRATION		28.627	(2.204)	717	023.	67.1	6
LATERAL DRAINAGE FROM LAYER 3		12.2705	(5.9354)	307	339.	28.7	9
PERCOLATION FROM LAYER	4	0.2585	(0.0261)	6	475.	0.63	1
LATERAL DRAINAGE FROM		0.2480	(0.0261)	6	212.	0.58	8

LAYER 5

PERCOLATION FROM LAYER	6	0.0105	(0.0001)	263.	0.02
CHANGE IN WATER STORAGE		1.470	(3.196)	36816.	3.45

PEAK DAILY VALUES FOR YEARS	1 THROUGH	5
	(INCHES)	(CU. FT.)
PRECIPITATION	9.00	225423.0
RUNOFF	0.000	0.0
LATERAL DRAINAGE FROM LAYER 3	0.1288	3225.6
PERCOLATION FROM LAYER 4	0.0010	25.3
HEAD ON LAYER 4	11.0	
LATERAL DRAINAGE FROM LAYER 5	0.0010	24.5
PERCOLATION FROM LAYER 6	0.0000	0.7
HEAD ON LAYER 6	0.0	
SNOW WATER	0.00	0.0
MAXIMUM VEG. SOIL WATER (VOL/VOL)	0.3176	5
MINIMUM VEG. SOIL WATER (VOL/VOL)	0.037	L

FINAL WATER	STORAGE AT END	OF YEAR 5	
LAYER	(INCHES)	(VOL/VOL)	
1	0.79	0.1323	
2	19.42	0.1618	
3	3.04	0.1268	
4	4.23	0.3509	

5 0.01 0.0254
 6 2.56 0.4224

0.00

SNOW WATER

APPENDIX A5.4 CALULATIONS

APPENDIX A5.4

CALCULATIONS



JOB NO. 444.8356.0 SHEET OF
JOB NAME RELALIZE RELALIZE
BY REM DATE 9/5/94
CHECKED BY 104 DATE 9/6/94

THE double liner system includes O geomet soudwicked between two liners. This geomet must have a hydraulic Conductivity of 10 cm/see

> A. The Coading on the lines is 15,000 pounds/ square foot from geotechnical veport

8. USING F-lied Systems brand geomet + PN 3000 and Catalogue clata Check flow. Data needed:

- flow area height - · zinches

- hydraulie graduet

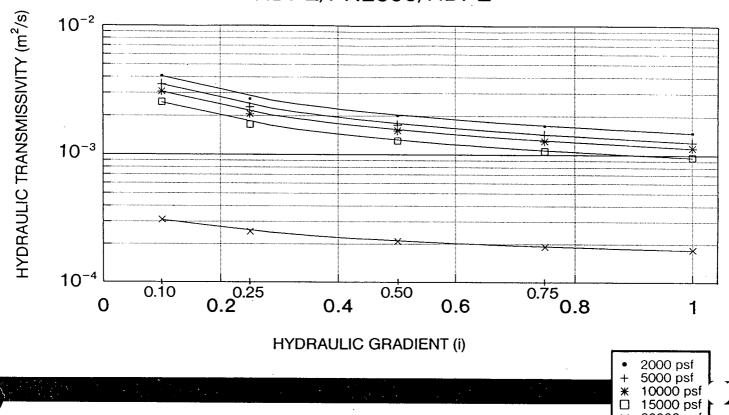
- loading - 15 K5 f

t from graph transmissivity (hydravlic Conductivity x thickness) = 3×10⁻³ m²/sec EDEP - 10cm/sec x . 2 m x 2.54cm = 5,08 cm²/sec = 5+10⁻⁴ m²/sec Therefore one geomet is 0 K

FORM 17

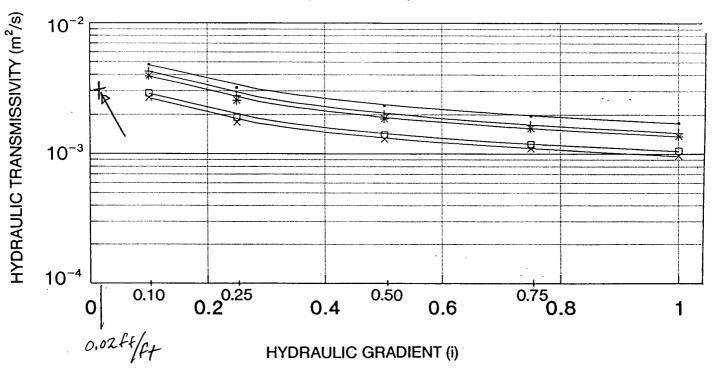
POLY-NET® TRANSMISSIVITY CHARTS

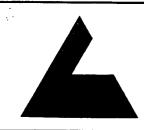
POLY-NET PN2000 HDPE/PN2000/HDPE



POLY-NET PN3000 HDPE/PN3000/HDPE

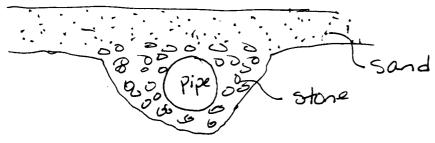
20000 psf





GEOTECHNICAL, ENVIRONMENTAL & CONSTRUCTION MATERIALS CONSULTANTS

JOB NO SHEET OF _2
JOB NAME Pasco County Land fill
BY LAM DATE 717194
CHECKED BY CIRC DATE 7/7/94



D85 y silica sand (FDOT Silica Sand section 902-2),

= 0.9mm to 2.36 mm

5 D85 = 4.5mm to 11.8 mm

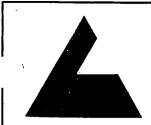
DIS Stone (NO 89) - FDOT = 1.2 mm to 4 mm

DIS (Filter) & 5 DBS (Soil)
1.2 mm to 4mm & 4.5 mm to 11.8 mm

D85 stone (N089) - FDOT = 7mm to 9mm

Do & D85(50:1)

.. pipe openings must be less than 7mm /



GEOTECHNICAL, ENVIRONMENTAL & CONSTRUCTION MATERIALS CONSULTANTS

JOB NO	$\underline{\hspace{0.5cm}}$ SHEET $\underline{\hspace{0.5cm}}$ OF $\underline{\hspace{0.5cm}}$
JOB NAME Pasco	Lounty Landfill
BY LAM	DATE
CHECKED BY	_ DATE 7/7/9/

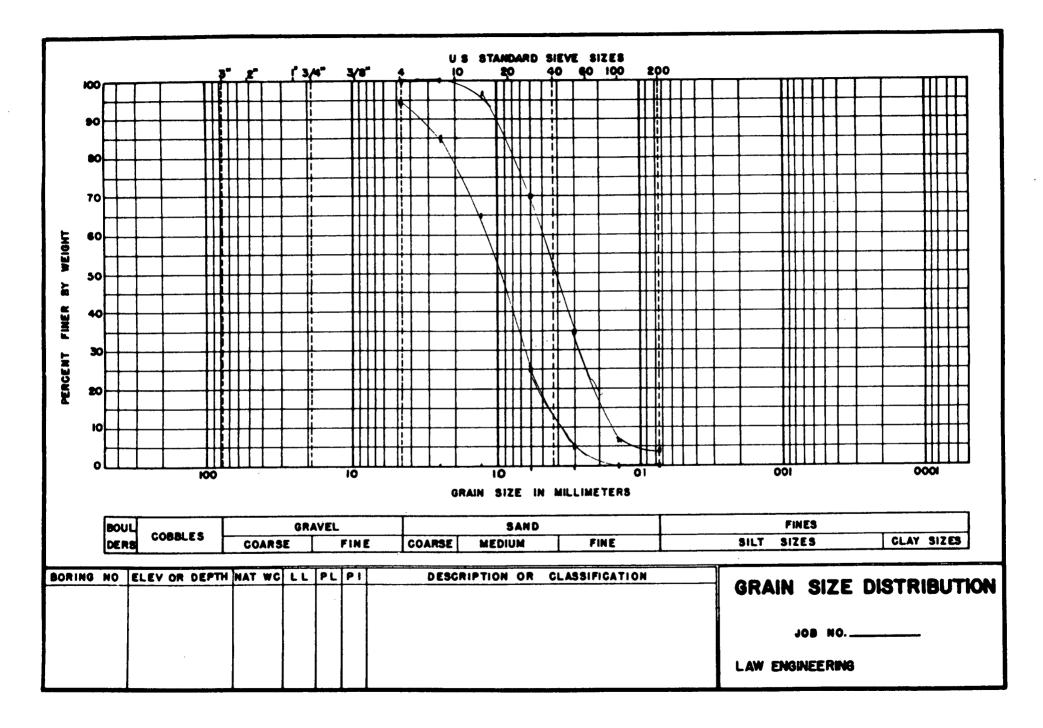
7mm = 0.7cm

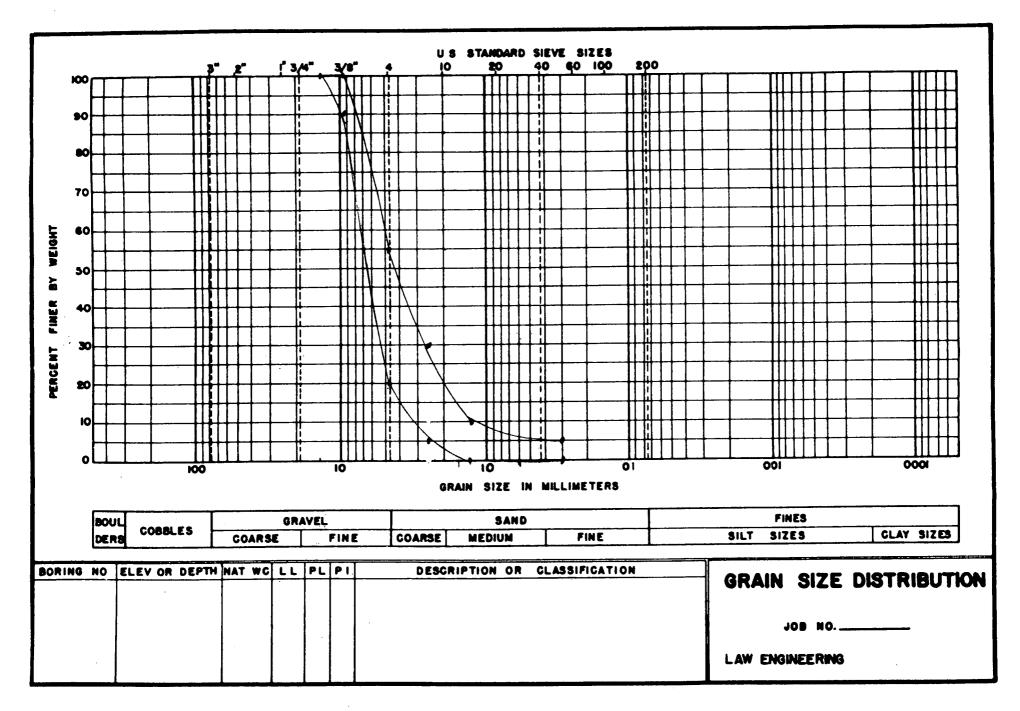
linch = 2.54cm

:. 7mm = 0.276 inches

pipe openings must be less than 0.276 inches

use 0.25 inches or less





egates to be used in asphaltic concrete may conta percent free shell. Free shell is defined as the coarse aggregate retained on the No. 4 sign of loose, whole, or broken shell, or the external mains of other marine life, having a ratio of length of the particle to the shell wall thickn ive to one. Coral, molds, or casts of other shells m and oyster shell indigenous to the formation sidered as free shell.

requirement for maximum percent of material No. 200 sieve for a lot or stockpile of any coars component shall be as follows:

any samples obtained by the Department purposes or assurances purposes at the source the average percent of material passing the of two composite samples shall not exceed in individual test shall exceed 2.0 percent.

assurance samples or acceptance samples by the Department. Obtained at the point of percent of material passing the No. 200 sieve ite samples shall not exceed 3.75 percenting est shall exceed 4.0 percent.

limitation applies only to coarse aggregates in appears as an impurity. It is not applicable which are predominantly chert.

hysical Properties:

188

aggregates shall meet the following physical uirements, except as noted herein: ∍nt

.um Sulphate) (FM 1-T 104)..... ss 1z percent*

elongated pieces...... Maximum 10 percent ce approval—Aggregates exceeding sounding ons will be rejected unless performance history ne material will not be detrimental for Portland rrete or other intended usages.

or elongated particle is defined as one having the maximum and the minimum dimensions sing prism exceeding five to one.

adation:

aggregates shall conform to the gradation of Table 1, when the stone size is specifical ole 1 is waived for those aggregates intented to uminous mixtures, provided the material ieves specified in the Standard Operation id meets uniformity and bituminous design

		:			· V	unte Miner	then Each	Laboratory	Sieve (Sur	Amounts Piner than Each Laboratory Steve (Stuare Openines), weight personn	en).				
Size	Nominal Size.	:													
		_	68	1 6		(50-mm)	1.45 1.45 1.45	1-in.	¥. o	¥-in.	i e	No.	No. 8	No. 16	No. 50
		(Fig.	(B)	E E	-	Ì	Î	(H	Î	(14.0) (11.0)		e î	1 (H	(F)	
-	3% to 1%-in. (90 to 27 f-mm)	100	90 to 100		25 to 60		0 to 15		0 to 5						
*	2% to 1%-in.	ļ		90	90 to 100	35 to 70	0 to 15		0 8 8						
ដ	2% to %-in.	į		8	90 to 100		25 to 60		0 to 10	0 10 0					
•	2 to 1-in.	į		į	8	90 to 100	35 to 70	0 to 15		0 to 5					
367	2-in. to No. 4 (50 to 4.75-mm)	i		į	100	96 to 100		35 to 70		10 to 30		0 00			
•	1% to %-in. (37.5 to 19.0-mm)			i		901	90 to 100	20 to 65	0 to 15		0 8 8				
1 9	11% to No. 4 (37.5 to 4.75-mm)	1		į		8	95 to 100		36 to 70		10 to 30	0 to 5			l
10	1 to 14-in. (25.0 to 12.5-mm)	!		į			8	90 to 100	20 to 55	0 to 10	0 to 5			į	
8	1 to %-in. (25.0 to 9.5-mm)			į			8.	90 to 100	40 to 85	10 to 40	0 to 15	0 to 5			
5	1-in. to No. 4 (25.0 to 4.75-mm)	į		i	-		100	95 to 100		25 to 60		0 to 10	0 8 8		-
•	* to *+in. (19.0 to 9.5-mm)	į		ŀ				001	90 to 100	20 to 55	0 to 15	0 to 5			j
61	K-in to No. 4 (19.0 to 4.75-mm)	!		İ				100	90 to 100		20 to .55	0 to 10	0 25 55		
8	%-in. to No. 8 (19.0 to 2.36-mm)	l		i				100	90 to 100		30 to 65	5 to 25	0 to 10	0 to 5	
٠ ;	12.5 to 4.75-mm)	!		į					100	90 to 100	40 to 70	0 to 15	0 to 6		
28	12.5 to 2.36-mm)	i		į					100	90 to 100	40 to 75	6 to 25	0 to 10	0 to 5	!
**	M-in to No. 8 (9.5 to 2.36-mm)			Ì						901	86 to 100	10 to 30	0 to 10	0 to 5	
8	%-in. to No. 16 (9.5 to 1.18-mm)	i		į						001	90 to 100	20 to 55	5 to 30	0 to 10	0 to 5
a	No. 4 to No. 16 (4.76 to 1.18-mm)	i		ļ							100	85 to 100 10 to 40 0 to 10	10 to 40		0 to 5
01	No. 4 to 0 (4.75-mm)	1		į							100	85 to 100		-	ļ

given opening, or it establishes the largest opening that can be used with a given soil. Frequently, a soil is employed as a filter. This means that the effective diameter of its voids must be less than D_{85} of the soil being filtered. Since the effective pore diameter is about $\frac{1}{5}D_{15}$, then

$$D_0 \leq D_{85(\text{soil})} \tag{3:19a}$$

$$D_{15\text{(filter)}} \leq 5D_{85\text{(soil)}} \tag{3:19b}$$

If the filter is to provide free drainage, it must be much more pervious than the soil. Since the permeability coefficient varies as the square of the grain size, then a ratio of permeabilities of over 25 to 1 can be secured by

$$D_{15\text{(filter)}} \ge 5D_{15\text{(soil)}} \tag{3:19c}$$

These criteria (Fig. 3.19) are the basis for filter design.^{3:11} In general, the filter soil should be well within these limits, and its grain size curve should be smooth and parallel to or flatter than the soil. If the soil being filtered is very fine grained, more than one filter layer will be required. The final filter layer is designed to fit between the openings in the conduit and the next finer filter. For many silty and clayey soils a well-graded concrete sand makes a satisfactory filter. A coarser pea-gravel second filter—usually described as ASTM No. 78 crushed stone— is then needed for the first.

If the soil being filtered is gap-graded, its grain size curve is redrawn, considering only the portion of the soil finer than the gap to be the total soil being filtered, and disregarding the part of the soil coarser than the gap. The filter is designed to fit the redrawn curve.

There have been many attempts to devise a "universal filter" that is small enough to filter the finest soil and yet having a D_{85} large enough so that it will not pass through the 80-mm or $\frac{5}{16}$ -in. perforations of commercial drainpipe. However, such filter materials have such a wide range of sizes (high C_{u}) that the particles segregate during handling and construction. Therefore, they should not be used unless care is taken to maintain their gradation.

The thickness of a sand or gravel filter layer is controlled by the ability of the layer to undergo distortion without rupture and by construction ease. Around drainpipes, filter layers from 0.1 to 0.2 m (4 to 8 in.) are sufficient. In dams horizontal layers should be 0.2 to 1 m (1 to 3 ft) thick and trench or chimney filters 2 to 3 m (6 to 10 ft) wide.

Fabric filters of woven metal, woven plastic, and nonwoven fiber sheets are easy to install in restricted spaces. Their cost is offset by saving labor. Reliable data on their filtering performance must be obtained from tests utilizing the same soils and gradients anticipated in the installation. Under gradients of up to 10, the nonwoven filters appear to have an effective opening of about 0.25 mm. In woven fabric, the openings are somewhat larger; they can be measured with a microscope-comparator.

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wnthe free advice of the piling salesman and against the warning of his engineer. The dam failed by shear in the downstream face that was already weakened by the increased neutral stress. The cutoff should be placed where the increased pore pressure is not harmful, at the center of the structure or upstream from the center and under the heaviest part of the structure, if possible. Incomplete cutoffs can increase seepage erosion by concentrating gradients.

Excessive water pressure can be controlled by drainage that short-circuits the flow and bleeds off the excess neutral stress at a point where it can do no harm. The trench drain, blanket drain, and toe drain (Fig. 3.18c) are used separately or in combination in earth dams to reduce neutral stresses in the downstream part of the embankment. Relief holes (Fig. 3.18d) reduce uplift on masonry dams. Relief wells (Fig. 3.18e) are used to reduce pressures in confined seams or pockets. Drainage has the disadvantage of shortening the seepage path and increasing the flow, but this can be corrected as previously described. It is essential that the drainage system be properly designed to avoid seepage erosion, as will be described in the section on filters.

FILTER DESIGN / A filter or protective filter is any porous material whose openings are small enough to prevent movement of the soil into a drain and which is sufficiently pervious to offer little resistance to seepage. Extensive experiments have shown that it is not necessary for a filter to screen out all the particles in the soil. Instead the filter openings need restrain only the coarsest 15%, or the D_{85} , of the soil. These coarser particles, D_{85} and larger, will collect over the filter opening as shown in Fig. 3.19a. Their voids will create smaller openings to trap even smaller particles of soil. Therefore, the diameter of the openings in the filter must be less than D_{85} of the soil. If the filter is a metal screen, filter fabric or holes in a perforated pipe, this limit fixes the finest soil that can be filtered by any

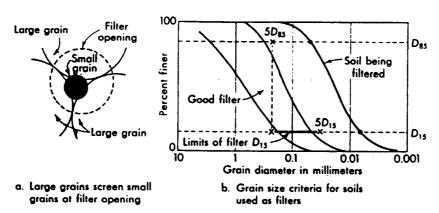


Figure 3.19 Protective filters.

Control of Mineral Aggregate Sources (Silica Sand Screenings), except as noted herein.

902-1.2 Deleterious Substances:

All fine aggregate shall be reasonably free of lumb clay, soft or flaky particles, salt, alkali, organic matter, los. or other extraneous substances. The weight of deletern. substances shall not exceed the following percentages:

Shale	1.0	
Coal and lignite	1.0	
Cinders and clinkers	0.5	
Clay Lumps	1.0	

902-2 Silica Sand. 902-2.1 Composition:

Silica sand shall be composed only of naturally occurren hard, strong, durable, uncoated grains of quartz, reasonab graded from coarse to fine, meeting the following require ments, in percent total weight.

Percent Retained Percent Passing No. 4 95 - 100 0 to 5 No. 8 0 to 15 85 - 100 65 - 97¢ 25 - 70 5 - 35 No. 16 3 to 35 No. 30 30 to 75 No. 50 65 to 95 No. 100 93 to 100 0 - 7 Maximum 4 No. 200 Minimum 96

Silica sand from any one source, having a variation Fineness Modulus greater than 0.20 either way from Fineness Modulus of target gradations established by producer, may be rejected. 902-2.2 Organic Impurities:

Silica sand shall be subject to the colorimetric test organic impurities. If the color produced is darker than standard solution, the aggregate shall be rejected unless it. be shown by appropriate tests that the impurities causing color are not of a type that would be detrimental to Portland Cement Concrete. Such tests shall be in accordance Florida Methods FM 1-T-21 and T-71. When tested for effect of organic impurities on strength of mortar, the strength ratio at 7 and 28 days, calculated in accordance with Section of FM-1-T-71, shall not be less than 95 percent.

902-3 Sands for Miscellaneous Uses. 902-3.1 Anchor Bolts and Pipe Joints:

Sand for setting anchor bolts, pipe joints or other similar uses shall meet the quality requirements of 902-2 except gradation requirements are waived.

902-3.2 Brick Sand for bri ments of 902-2 materials shall p from coarse to fi 902-3.3 Sand-Sand for sa requirements of ? material shall m

Passing No. 4 Sieve

No. 100 Siev No. 200 Siev

902-4 Filter M Silica sand for meet the require of 902-1.2 and 90

902-5 Screenir 902-5.1 Comp Screenings s either naturally resulting from th include natural (lightweight agg.

with similar cha Aggregates c following gradat.
Sieve Size

3%" No. 4 No. 200 When permi may contain up Sieve.

902-5.2 Specia 902-5.2.1 Sc

Processed sc Coarse Aggregat percent passing Aggregate Source In-service histor may be impleme



POLYNET PN-3000

PRODUCT DESCRIPTION

PN-3000 is a profiled geonet manufactured by extruding two sets of polyethylene strands to form a diamond shape. The resulting net provides superior planar water flow, is inert to biological and naturally encountered chemicals, alkalies, and acids and is resistant to UV light exposure. Polynet PN-3000 conforms to the property values listed below.

PROPERTY	<u>METHOD</u>	<u>UNITS</u>	QUALIFIER	<u>VALUE</u>
Roll Length	-	ft	Normal	300
Roll Width	-	ft	Normal	7.54
Thickness	ASTM D1777	inches	Range	0.220 <u>+</u> 0.022
Area per Roll	-	ft ²	Normal	2262
Weight per Roll	.	lbs	Normal	407
Weight per Square Foot	ASTM D3776 (option C)	lbs/ft ²	Range	0.180 <u>+</u> 0.018
Carbon Black Content	ASTM D1603	percent	Range	2.5 <u>+</u> 0.5
Polymer Density	ASTM D1505	g/cm ³	Range	0.937 <u>+</u> 0.002
Melt Flow Index	ASTM D1238 (condition E)	g/10 min.	Maximum	1.0
Tensile Strength (Machine Direction)	ASTM D1682 (modified)	ppi	Range	50 <u>+</u> 10
Transmissivity ¹ (gradient = 1.0 at 15,000 psf)	ASTM D4716	M ² /sec	Minimum	1 X 10 ⁻³

¹ Measured between two steel plates one hour after application of the confining pressure. Values may vary based on transmissivity specimen demensions and specific laboratory.



GEOTECHNICAL, ENVIRONMENTAL & CONSTRUCTION MATERIALS CONSULTANTS

JOB NO. 444-3565.0/ SHEET/ OF
JOB NAME Resource Recovery Class 1
BY REM DATE 7/6/94
CHECKED BY DATE 7/9/97

Pipe Strugth / Loading

NON- Prasuce

Piper - Collection + header - Burish elevation 125'
- 47
- 78'

- Side Slape Miser 30'

presul

- Faice main 4" winde & 5" cantainment may we 6" z if spacers are required

Preliminary truign for permit hand an Drisco pipe Exy Manueal - a capy if sumerages is attached.

1st Buried calleton / header piper living
Churt 14 Soil Type 11 sugnt to modulate
Compaction Soil madulus - 1500
Then for May Buriel 78! - use 501217

2NO Priser - Use a Striffer piper church chart
5DR 13.5 OK

4	

GEOTECHNICAL, ENVIRONMENTAL & CONSTRUCTION MATERIALS
CONSULTANTS

JOB NO. 464-35	667.0 SHEET OF
JOB NAME Resa	une Kurvey Class /
	M DATE 7/6/44
CHECKED BY	DATE

FINALLY fax Prunere pipe face main 50R-17- 100 pai, which in mad than adequate.

O DRISCOPIPE

For a given SDR the ratio of the O.D. to the minimum wall thickness remains constant. An SDR 11 means the O.D. of the pipe is eleven times the thickness of the wall. This remains true regardless of diameter. For example, a 14" diameter pipe with a wall of 1.273" is an SDR 11 pipe. An 18" diameter pipe with a wall of 1.637 is also an SDR 11 pipe. Common SDR ratios are SDR 9.3, SDR 11, SDR 13.5, SDR 15.5, SDR 17, SDR 19. SDR 21. SDR 26 and SDR 32.5. For high SDR ratios, the pipe wall is thin in comparison to the pipe O.D. For low SDR ratios, the wall is thick in comparison to the pipe O.D. Given two pipes of the same O.D., the pipe with the thicker wall will be stronger than the one with the thinner wall. Thus, high SDRs have low pressure ratings and low SDRs correspond to high pressure ratings because of the relative wall thickness.

The pressure rating of thermoplastic pipe is mathematically calculated from the SDR and an allowable hoop-stress. The allowable hoop-stress is commonly known as the long term hydrostatic design stress. It is the stress level (that has been laboratory tested and field proven) that can exist in the pipe wall continuously with a high degree of confidence that the pipe will operate under pressure for at least 50

years with safety. The formula relating SDR and hydrostatic design stress has been adopted by ISO (International Standards Organization). ASTM (American Society For Testing and Materials) and the PPI (Plastics Pipe Institute) as the standard for the industry.

The formula is:

$$P = \frac{2 St}{D-t}$$
 or $P = \frac{2 S}{SDR-1}$

Where: P = Pressure rating (psi)

D = Pipe OD (inches)t = Minimum wall thickness (inches)

S = Hydrostatic Design Stress

 $SDR = D \div t$

From the formula it can be shown that all pipes of the same SDR (regardless of diameter) will have the same pressure rating for a given design stress. Thus, 36" dia. SDR 32.5 has the same pressure rating as 14" SDR 32.5. For the design engineer's reference, the standard SDRs and their corresponding standard pressure ratings for water at 73.4°F, using a hydrostatic design stress of 800 psi, are shown in Charts 12 and 13.

Chart 12
Pipe Pressure Rating

Driscopipe 1000 (at 73.4°F)										
2.1.000, po 1000 (2.1.0.1.1)	SDR 32.5	SDR 26	SDR 21	SDR 19	SDR 17	SDR 15.5	SDR 13.5	SDR 11	SDR 9	SDR 7
Long Term Strength (psi)	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Safety Factor	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0
Hydrostatic Design Stress	800	800	800	800	800	800	800	800	800	800
Design Life (min. years)	50	50	50	50	50	50	50	50	50	50
Pressure Rating (psi)	51	64	80	90	100	110	128	160	200	267

Chart 13
Pipe Pressure Rating

Driscopipe 8600 (at 73.4°F)						
	SDR 32.5	SDR 25.3	SDR 15.5	SDR 11	SDR 9.3	SDR 8.3
Long Term Strength (psi)	1600	1600	1600	1600	1600	1600
Safety Factor	2.0	2.0	2.0	2.0	2.0	2.0
Hydrostatic Design Stress	800	800	800	800	800	800
Design Life (min. years)	50	50	50	50	50	50
Pressure Rating (psi)	51	65	110	160	190	220



Simplified Burial Design: A conservative estimate of the ability of Driscopipe pipelines to perform in a buried environment is found in Chart 24. It is based on a minimum 2:1 safety factor and 50 year design service life. A detailed burial design starts on page 37. The detailed design should be used for critical or marginal applications or whenever a more precise solution is desired.

Detailed Burial Design:

Design by Wall Crushing: Wall crushing would theoretically occur when the stress in a pipe wall, due to the external vertical pressure, exceeded the long-term compressive strength of the pipe material. To ensure that the Driscopipe wall is strong enough to endure the external pressure the following check should be made:

$$S_A = \frac{(SDR - 1)}{2} P_T$$

Values of E' Based on Soil Type (ASTM D2321) and Degree of Compaction

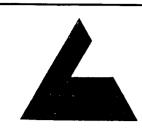
Soil Type of		E' (psi) for Degree of Compaction (Proctor Density, %)						
Initial Backfill Embedment Material	Description	Loose	Slight (70-85%)	Moderate (85-95%)	_			
1	Manufactured angular, granular materials (crushed stone or rock, broken coral, cinders, etc.)	1,000	3,000	3,000	3,000			
11	Coarse grained soils with little or no fines	N.R.	1,000	2,000	3,000			
111	Coarse grained soils with fines	N.R.	N.R.	1,000	2,000			
IV	Fine-grained soils	N.R.	N.R.	N.R.	N.R.			
V	Organic soils (peat, muck, clay, etc.)	N.R.	N.R.	N.R.	N.R.			

N.R. = Not Recommended for use by ASTM D2321 for pipe wall support

Chart 2	24
	•

	Maximum Burial Depth, ft. in dry soil of 100 lbs/cu. ft.				mum Ext ressure p		Maximum Deflection, % after installation			
SDR	Soil	Modulus	, psi*	Soil	Soil Modulus, psi*			Soil Modulus, psi*		
	1000	2000	3000	1000	2000	3000	1000	2000	3000	
32.5	25	32	37	17	22	26	1.7	0.9	0.6	
26	33	45	52	23	31	36	2.3	1.2	0.8	
21	46	61	71	32	42	49	3.2	1.6	1.1	
19	52	69	81	36	48	56	3.6	1.8	1.2	
17	61	121	181	42	84	126	4.2	2.1	1.4	
15.5	56	112	168	39	78	117	3.9	2.0	1.3	
13.5	49	98	147	34	68	102	3.4	1.7	1.1	
11	39	78	117	27	54	81	2.7	1.4	0.9	
9.3	33	68	101	23	47	70	2.3	1.2	0.8	
8.3	30	61	89	21	42	62	2.1	1.1	0.7	
7.3	26	52	79	18	36	55	1.8	0.9	0.6	

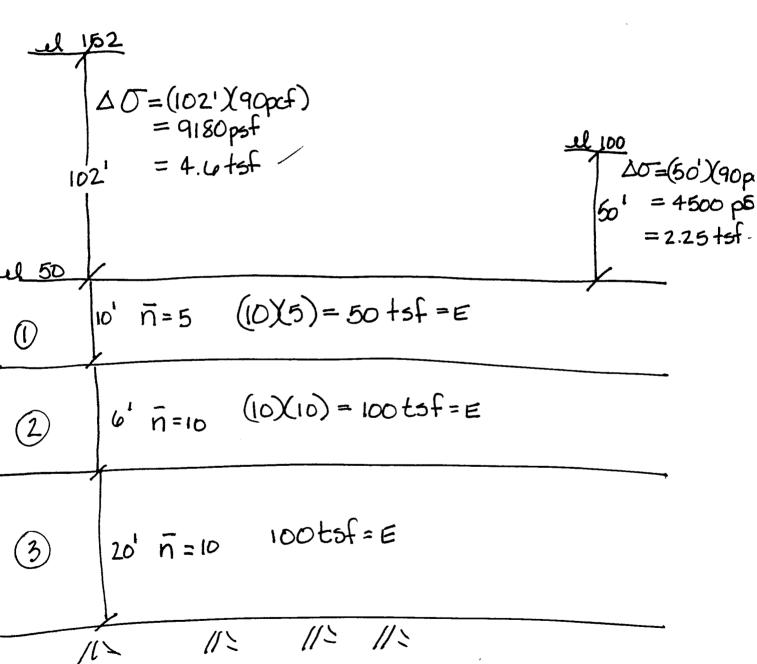
^{*}assumes no external loads



GEOTECHNICAL, ENVIRONMENTAL & CONSTRUCTION MATERIALS CONSULTANTS

1, 1, 1, 2, 5, 5,	0 . 1
JOB NO.464835650	
JOB NAME W. POSC	O Class I Land fill
BY LAH	DATE 6/22/94
CHECKED BY	- DATE 7-7-94

Suttlement Calculations



1100 feet

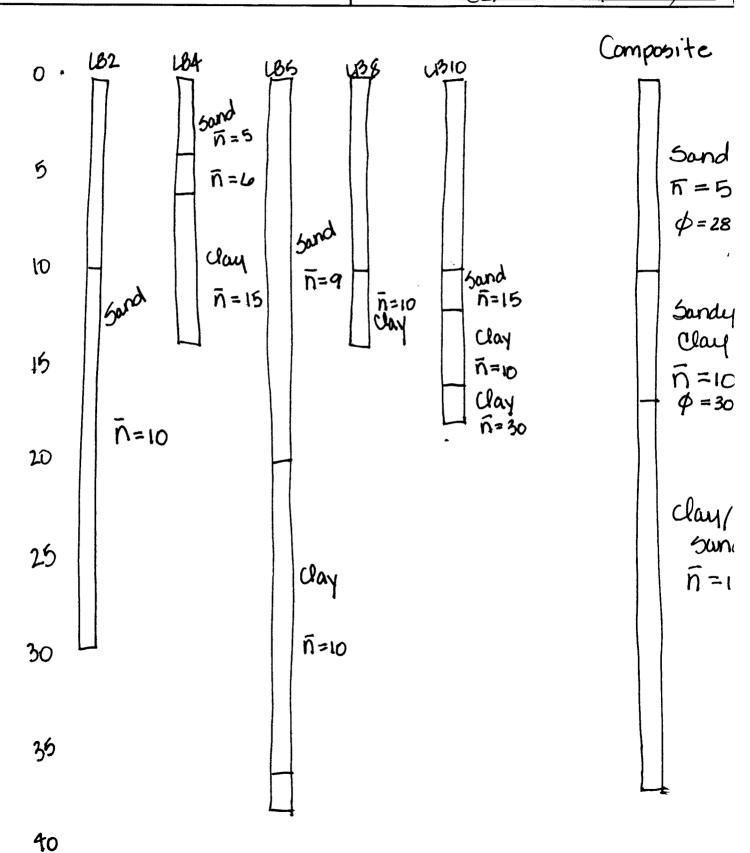


GEOTECHNICAL, ENVIRONMENTAL & CONSTRUCTION MATERIALS CONSULTANTS

JOB NO. SHEET 2 OF 4

JOB NAME W. FASCO CLASSIL AND CHECKED BY CAPE 7-794

CHECKED BY CAPE 7-794





GEOTECHNICAL, ENVIRONMENTAL & CONSTRUCTION MATERIALS CONSULTANTS

JOB NO	SHEET _	3 OF 4
JOB NAME W Pases	Class	1 Landfill
BY LHall	DATE	6/22/94
CHECKED BY	DATE	7-7-94

$$5 = \Delta \sigma H$$

at el 152

layer 1:
$$5 = \frac{(4.6 + 5f)(10')}{50 + 5f} = 0.92 ft$$
 or 11 inches

layer 2:
$$5=(4.6 + 5f)(6) = 0.28 f$$
 on 3.3 inches

"super 3:
$$5 = (4.4 \text{ tsf} \times 20') = 0.92 \text{ yr}$$
 or 11 inches

at el 100

layer 1:
$$5=(2.25+sf(10))=0.45y+0.5.4$$
 inches

layer 2:
$$5 = \frac{(2.25 + 5f)}{100 + 5f} = 0.14 + 61.6 inches$$

layer3:
$$5 = \frac{(2.25 + 5f)(20')}{100 + 5f} = 0.45 f + 0.5.4 \text{ inches}$$

•	•	

GEOTECHNICAL, ENVIRONMENTAL & CONSTRUCTION MATERIALS CONSULTANTS

JOB NO. W. Rose (SHEET 4 OF 4
BY <u>Hall</u>	DATE 6/22/94
CHECKED BY	DATE

Differential Sutlement over 1100 feet = 0,5 feet

5= 5% on 55 feet in 1100 ft

... Clading 1/2 foot of sextlement to the high side wiee not make a detrimental difference to the Slope in the pipe.



GEOTECHNICAL, ENVIRONMENTAL & CONSTRUCTION MATERIALS CONSULTANTS

JOB NO.4648356502 S	SHEET OF
JOB NAME W. Parti	Josslandfill
BY LAH	DATE 6/22/94
CHECKED BY CIM	DATE 7-7-97

Pipe Loading
Pipe Φ = 6 inches

in suttement over pipe was assumed.

elev of top of embarkment ~+125'

Wt of soil (ash) over pipe = (+125'-+47') Yash = (78f+ \(\text{20 pcf} \) = 7020 psf or 7 Ksf



GEOTECHNICAL, ENVIRONMENTAL & CONSTRUCTION MATERIALS CONSULTANTS

JOB NO.464.3565.01	SHEET OF
JOB NAME LASOUR	Le Lee. C/AIII
BY REM	DATE
_	DATE 7/1/91/

Pump AND Forced main size

1. peak daily flow vater from HELP MODEL =

primary - 3225.06 ft 3/day × 7.48 gal/43 = 24, 127 gal/day - 6.9 Acres

Secondary- 24.5 ft] day x 7.489A1/ft3 = 183 gal/day - 6.9acres

·· primary - Z4,127 gal/day = 3500 gal/Ac.day

Secondary 183 gel/day = 26 gal/Ac.day

Fram 4 Camp Drusse & ME HEE INC report to PASCO caunty" West PASCO LANDFILL System Lines Performance CDM Project NO. 6104-17-CG3-LBR"

The Average monthly flaw (righ) 3.79 gal/hr. (law) 2.53 gal/hr.

Based on a review of data the high peak duily value was 10 gal/hr - 240 gal/10 Acrecell or 24 gal/Ac. day

... The estimate for suandary lenchete production is acceptable

GEOTECHNICAL. ENVIRONMENTAL & CONSTRUCTION MATERIALS CONSULTANTS

JOB NO. 464.3565.01 S	SHEET 2 OF
JOB NAME RESOURCE R	ecovery Class/
BY ZEM	DATE 7/4/94
CHECKED BY ROLL	

Huw data fram primary was not unaclable

2. Head and Flaw 14tes
Primary

Q - 3500 Gal/Ac-day

Q 10 Acres - 35,000 gal/day × day × 1hr = 24 gpm

VSE EPG model side slape view Pump

TSP8 - mid vange flaw rate is 25 gpm

Head - Statec 47-63' av 16'

Tynnin ADD 10' + consciruation

26'

75 P-8-1 wied wach



GEOTECHNICAL, ENVIRONMENTAL & CONSTRUCTION MATERIALS CONSULTANTS

JOB NO. 464-3565.01 SHEET 3 OF
JOB NAME RISOURCE TELOUWY CLASS 1
BY
CHECKED BY LDG DATE 7/7/94

For recaudacy use smallest size jump

Use TSP1-3

3. Face main

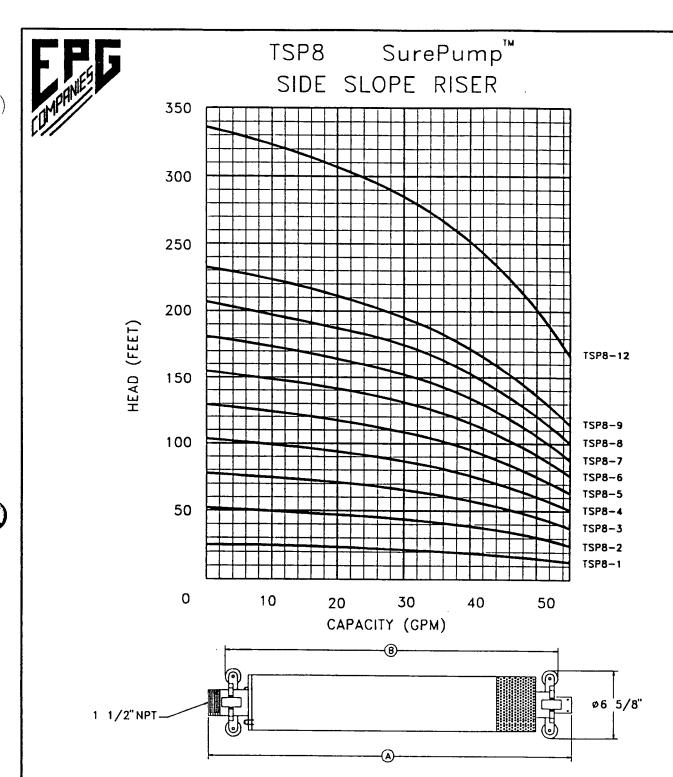
VSE nin sige - 4 inch w/ 5/nch

Containment pipe

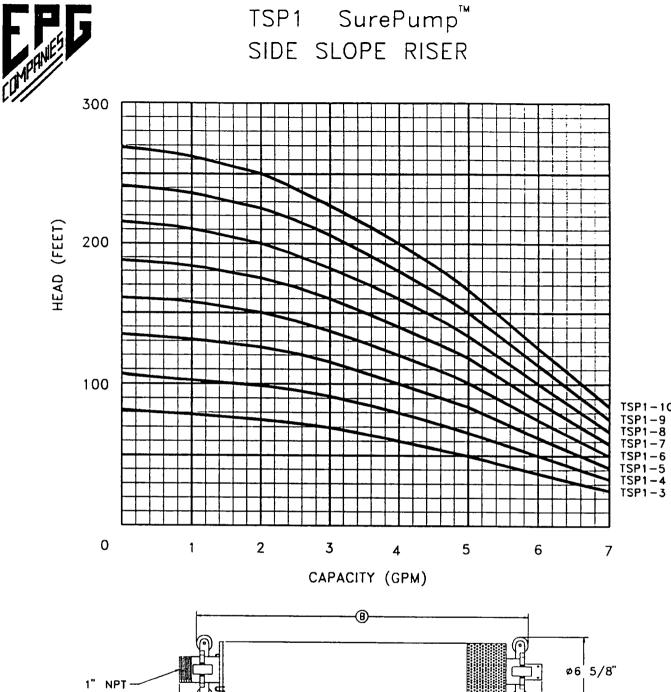
4. Tank siges Existing holding tanks are performing saturations

> Primary - 8,000 gallous ± Secondary 6,000 gallous ±

Flams viel be metered at pump so use two lines to pump to one 10,000 gallon TANE above ground in a confainment snes.



PUMP MODEL	SINGLE PHASE			Т	THREE PHASE			SHIPPING WEIGHT (LBS)	
POMP MODEL	MOTOR HP	A (in)	B (in)	MOTOR HP	A (in)	B (in)	1ø	3ø	
TSP8-1	0.50	31.50	29.50	0.50	31.50	29.50	41.0	41.0	
TSP8-2	0.75	34.50	32.50	0.75	34.50	32.50	44.7	44.7	
TSP8-3	1.00	37.00	35.00	1.00	37.00	35.00	49.6	49.6	
TSP8-4	1.00	38.75	36.75	1.00	38.75	36.75	51.0	51.0	
TSP8-5	1.50	42.25	40.25	1.50	41.25	39.25	59.1	54.7	
TSP8-6	1.50	44.00	42.00	1.50	42.75	40.75	60.6	56.2	
TSP8-7	2.00	47.00	45.00	2.00	45.75	43.75	65.5	61.0	
TSP8-8	2.00	48.75	46.75	2.00	47.25	45.25	67.5	63.0	
TSP8-9	3.00	58.75	56.75	3.00	56.00	54.00	91.9	82.1	
TSP8-12	5.00	69.75	67.75	5.00	63.75	61.75	115.1	97.4	



РИМР	MODEL	SINGLE PHASE			THREE PHASE			SHIPPING WEIGHT (LBS)	
	MODEL	MOTOR HP	A (in)	B (in)	MOTOR HP	A (in)	B (in)	1ø	3ø
TSP	1-3	0.33	30.00	28.00	0.50	30.75	28.75	39.0	41.2
TSP	1-4	0.33	31.00	29.00	0.50	31.50	29.50	40.0	42.2
TSP	1-5	0.33	31.75	29.75	0.50	32.25	30.25	41.0	43.2
TSP	1-6	0.33	32.50	30.50	0.50	33.25	31.25	42.1	44.3
TSP	1-7	0.33	33.50	31.50	0.50	34.00	32.00	43.2	45.3
TSP	1-8	0.33	34.25	32.25	0.50	34.75	32.75	44.2	46.4
TSP	1-9	0.33	35.00	33.00	0.50	35.75	33.75	45.2	47.4
TSP	1-10	0.50	36.50	34.50	0.50	36.50	34.50	48.5	48.5

GEOTECHNICAL, ENVIRONMENTAL & CONSTRUCTION MATERIALS CONSULTANTS

JOB NO. 444,1565.0] SHEET OF
JOB NAME Reserve Recury C/415/
BY REM DATE 7/7/94
CHECKED BY LOW DATE 7/7/94

ABau graund TANK INSTALLATION,

10,000 gallon tunk Appoximately a 12' Drametu tank

Cantanment area value

10,000 gallour + 1,000 gallour use 3' high walls 1,470 f+3

area of Cantonment = 1470/3 = 490

22' square

> Cancrete [22+22×(6/12) +(6/12) × 3 × 88] 150 H = 56,100 #

149,730 #

149,730 / (ZZ+ZZXIVV) = Z-1/KSi _

Typical sund unil meet this

APPENDIX 6 HYDROGEOLOGICAL INVESTIGATION REPORT

APPENDIX 6

HYDROGEOLOGICAL INVESTIGATION REPORT

HYDROGEOLOGICAL INVESTIGATION

AND

SITE REPORT

CELL A-2 AT PASCO COUNTY RESOURCE RECOVERY FACILITY CLASS I LANDFILL

Prepared For:

PASCO COUNTY BOARD OF COUNTY COMMISSIONERS
Pasco County, Florida

Prepared By:

LAW ENVIRONMENTAL, INC. Tampa, Florida

JUNE 8, 1994



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HYDROGEOLOGICAL INVESTIGATION AND SITE REPORT FOR CELL A-2 AT PASCO COUNTY RESOURCE RECOVERY FACILITY CLASS I LANDFILL

1.0 Regional and Site Specific Geology and Hydrogeology

1.1 Regional Geology and Hydrogeology

In Pasco County, undifferentiated surficial deposits of Pliocene to recent age sands of variable thickness overlie Cretaceous and Tertiary carbonates and clays. The Tertiary carbonates (primarily limestones) deposited during the Oligocene and Eocene periods, are called the Suwannee, Ocala, and Avon Park Formations in descending order. Separating the surficial sands from the underlying carbonates are clayey sands, sandy clays, and clays which are remnants of or a residual from the Hawthorn and Tampa Formations of upper and lower Miocene age. Both the Hawthorn and the Tampa Formation are thicker further to the south and east.

The importance of the residual clay of the Hawthorn and Tampa Formations, where present, is that the unit comprises the confining layer separating the underlying Floridan aquifer system from the overlying surficial aquifer. Regionally, the Floridan aquifer is known to be only semi-confined, which is attributed to the presence of karst features (sinkholes) and the thinning or absence of the confining layer. See Table 1 for the regional stratigraphic and hydrogeologic nomenclature.

1.2 Site Specific Geology and Hydrogeology

In addition to the extensive site investigation conducted prior to and during the Resource Recovery Facility Power Plant Site certification in 1989, a more localized hydrogeological investigation was made in the immediate vicinity of the next landfill disposal unit to be constructed, Cell A-2. The purpose of the additional site work was to define further the characteristics of the surficial aquifer, the presence of the confining layer, and the attitude of underlying limestone.

Sixteen soil borings were conducted to achieve this goal; six piezometers also were installed locally in the surficial aquifer. A confining layer of clay was encountered in all soil borings except LB-2 which was not augured deep enough to penetrate the clay or underlying limestone. The piezometers were installed with ten foot screened intervals at the base of the surficial sands to detect even minimal water levels in the surficial aquifer. The piezometers were checked periodically during the hydrologic investigation and were found to be dry even in the deepest screened interval (22.6 to 32.6 feet N.G.V.D. in piezometer LB-3).

Pasco County - Class I Landfill Law Environmental, Inc.

Generally, the confining layer clay was found to be gradational with the overlying surficial sands, coarsening upward from a clay to sandy clay to a clayey sand, enough so that the actual base of the surficial aquifer unit was very high and dry. Soil boring lithologic descriptions are provided in the Appendix. Geologic cross-section depicting the site stratigraphy is shown in Figure 1.

2.0 Direction and Rate of Ground-Water and Surface-Water Flow Including Seasonal Variations

Water level data from the existing ground-water monitoring system which has been in place since the start up of the Resource Recovery Facility and landfill operations in 1990, were evaluated. Additional ground-water monitoring data was available from an adjacent Class III Landfill Site (see Table 2).

Two Floridan aquifer potentiometric surface maps for September 1992 and March 1993 were drawn. Both maps show a northwest direction for ground-water flow for the Floridan aquifer towards Weeki Wachi Springs (see Figures 2 and 3). Two surficial aquifer water table maps also were drawn for September 1991 and March 1992. The flow direction in the surficial aquifer is less definitive, but appears to be to the northwest or north, probably deflecting around the existing high that the landfill is sited on (see Figures 4 and 5).

The hydraulic gradient for the surficial aquifer based on the September 1991 water table map is approximately 0.001. The hydraulic gradient for the Floridan aquifer appears to be higher at the end of the wet period (based on the September 1992 potentiometric surface map) than it is near the end of the dry season (based on the March 1993 map). The hydraulic gradients ranged from approximately 0.002 in September 1992 to 0.0015 in March 1993.

3.0 Background Quality of Ground-Water and Surface Water

Ground-water monitoring wells, 2MW-1 through 2MW-4 and 4MW-1 through 4MW-4 were installed and sampled in 1987 prior to construction of the landfill. Analytical results from the 1987 sampling event are provided in the Appendix.

Ground-water monitoring wells, 2MW-5, 2MW-6, 4MW-5, and 4MW-6, were installed in 1991 after construction of cells A-1 and SW-1. Analytical results from the 1991 sampling event are provided in the Appendix.

Monitoring wells designated 2MW are surficial aquifer monitoring wells and the monitoring wells designated 4MW are Floridan Aquifer monitoring wells. Well locations are shown on Figures 2 through 5. No surface water bodies exist on the landfill site consequently no surface water samples have been taken.

Pasco County - Class I Landfill Law Environmental, Inc.



4.0 Any On-Site Hydraulic Connections Between Aquifers

There does not appear to be any hydraulic connections between the aquifers within the footprint of the landfill site. All soil borings advanced deep enough during this investigation encountered clay before limestone which is an indication of the continuity of the confining layer. Soil boring LB-2 was terminated at 23.8 feet N.G.V.D., before reaching the confining layer clay above the Floridan aquifer limestones. Outside the landfill area to the southwest are known sinkholes which provide a connection between the aquifers.

Soil boring LB-5 encountered water 12.5 feet into the clay layer after penetrating 21.5 feet of surficial sands. The estimated elevation is 18.1 feet, N.G.V.D., which is well below any water level reported for the surficial aquifer or the Floridan aquifer in the routine groundwater monitoring. No water was encountered above the top of the clay unit at 30.6 feet, N.G.V.D.. It is concluded that the surficial aquifer was dry and that their was not a hydraulic connection between aquifers at this location.

Soil boring LB-2 was also reported to have encountered water at 18 feet below ground level or at an elevation of 31.8 feet, N.G.V.D. The boring was terminated short of penetrating clay or limestone at a total depth of 26 feet below ground level or at an elevation of 23.8 feet, N.G.V.D. The next soil boring LB-3, located less than 150 feet from LB-2, was continued deeper until it penetrated clay at 23.6 feet, N.G.V.D. A piezometer was installed with a screened interval from 22.6 to 32.6 feet, N.G.V.D., which was deep enough to have intercept the water reported in soil boring LB-2 at 31.8 feet, N.G.V.D. No water was encountered in LB-3. It was concluded that the aquifer was dry and that no hydraulic connection between aquifers exists in the vicinity of soil boring LB-3. It was also concluded that if water was actually encountered in soil boring LB-2 that it should have been encountered in LB-3.

In locations where ground-water monitoring wells for the surficial aquifer and the Floridan aquifer are close or clustered and the surficial is partially saturated, the hydraulic level of the surficial aquifer is generally slightly higher than in the underlying Floridan aquifer and the differential between them is greater as the distance away from the sinkholes increases (see Table 2).

5.0 Site Stratigraphy and Aquifer Characteristics for Confining Layers, Semi-Confining Layers, and All Aquifers Below the Landfill Site That May Be Affected by the Landfill.

The site stratigraphy is illustrated on Figure 1. The stratigraphy at the site differs only slightly from the regional stratigraphy. The undifferentiated surficial deposits are present, but slightly

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72657560 June 8, 1994

thinner. The Hawthorn and Tampa Formations are either absent or not distinguishable and probably represented by the residual clay, which grades upward into the surficial sands. The residual clay lies unconformable on the underlying Suwannee Formation. Limestones of the Suwannee Formation have only been partially penetrated by some of the soil borings and by the installation of the Floridan Aquifer Monitoring Wells. Thicknesses of these units are given in Table 3 along with other pertinent characteristics.

Hydraulic conductivities were determined for three undisturbed samples taken from the confining layer clay penetrated in soil borings LB-4, LB-5, and LB-8. The values ranged from $2.2x10^6$ feet per day to $6.9x10^6$ feet per day.

The confining layer below the landfill site is assumed to be continuous. No soil borings penetrated the Floridan aquifer limestone without first penetrating the confining layer clay. Hydrogeologically, the Floridan aquifer is considered to be only semi-confined in West Pasco due to the regional thinning of its confining unit from the southeast to the northwest out of Hillsborough County, and due to the presence of karst features.

Water levels in both the surficial aquifer and Floridan aquifer have remained below 40 feet NGVD at the site during the period from 1991 to 1993 (see Figure 6). The surficial aquifer has been generally dry. Only in Monitoring Well MW-2 has water been consistently present.

6.0 Site Topography and Soil Characteristics

The West Pasco Class I Landfill is located in a relatively flat section of the Pamlico Terrace of the Gulf Coastal Lowlands physiographic province along the west coast of Florida. Topographic relief across the site ranges from about 20 to 30 feet in the southwestern portion and about 10 feet throughout the remainder of the site and within the footprint of the landfill.

Numerous lakes and ponds exist to the south of the permitted landfill area, but none occur within the landfill area itself. The area is drained primarily by Buckhorn Creek and the Pithachascotee River. The major surface water feature in the vicinity of the landfill is Crews Lake located approximately 1.5 miles to the east. Crews Lake, a lenticular body and headwaters of the Pithachascotee River, forms a northeast-trending lineament with the river.

The soil characteristics for the landfill area are given as Candler and Tavares sands, which are described as nearly level to sloping, excessively drained soils that are sandy throughout with occasional thin lamellae of loamy sand below a depth of 50 inches in the soil survey of Pasco County, Florida. This is consistent with the lithologic description and grain-size analysis performed on samples taken from the surficial sands in the soil borings. The sands appear to

be fine-grained, well sorted sands until they begin to grade into the underlying clay confining unit.

7.0 Inventory of All Public and Private Water Wells within a One-Mile Radius of the Landfill including Top of Well Casing and Bottom Elevations, Name of Owner, Age and Usage of Each Well, Stratigraphic Unit Screened, Well Construction Technique and Static Water Level.

A listing of all public and private water wells within a one-mile radius of the landfill was obtained from the Southwest Florida Water Management District for Sections 13, 14, 23, 24, 25, 26, 35 & 36, Township 24 South, Range 17 East, and Sections 18, 19, 30 & 31, Township 24 South, Range 18 East. Each section comprises one square mile and the required one--mile radius of the landfill falls within these twelve (12) sections. The listing is provided in the Appendix.

8.0 Existing Contaminated Areas on Landfill Site

There are no known existing contaminated areas on the landfill site.

The hydrogeological investigation and site report for the West Pasco Class I Landfill is signed, sealed and dated by George W. Ellsworth, a licensed Professional Geologist (No. 0000848) in the State of Florida.

Table 4
Piezometer Construction Details

Piezometer ID	LB-1	LB-3	LB-5	LB-7	LB-8	LB-11
Top of Casing	49.6	52.6	54.1	51.9	53.8	53.4
Surface Elevation	47.6	50.6	52.1	49.9	51.8	51.4
Top of Screen	44.6	32.6	40.1	45.9	50.8	49.4
Bottom of Screen	34.6	22.6	30.1	35.9	40.8	39.4
Bottom of Casing	34.6	22.6	30.1	35.9	40.8	39.4
Total Depth of Boring	34.6	22.6	14.1	35.9	35.8	39.4
Top of Clay	35.6	23.6	30.6	36.9	41.8	41.4

Casing: Schedule 40 Pipe, 2 inch diameter

Screen: Schedule 40 Pipe, 0.01 inch slotted

Elevations: feet, NGVD

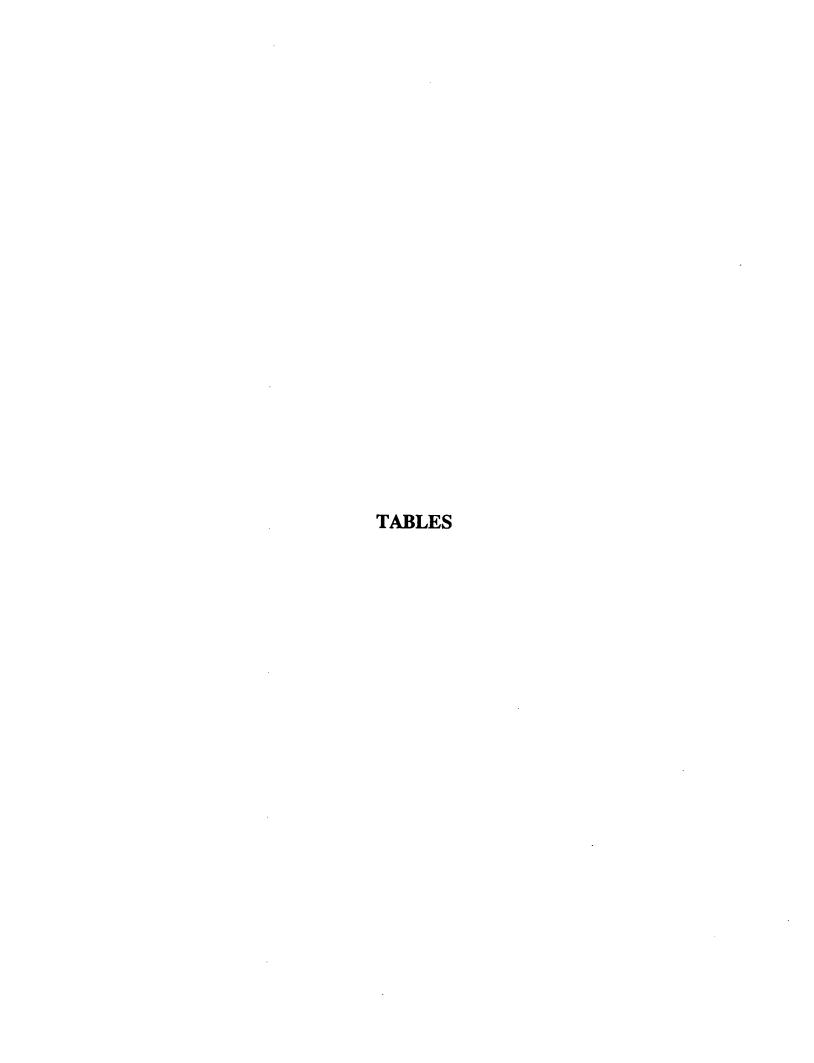


Table 1: Regional Stratigraphic and Hydrogeologic Nomenclature

STRATIGRAPHIC UNIT	HYDROGEOLOGIC UNIT	THICKNESS (FEET)	LITHOLOGY	WATER-PRODUCING CHARACTERISTICS
Undifferentiated Surficial Deposits	Surficial Aquifer System	0 - 50	Soil, sand, and clay of marine and estuarine terraces, alluvial, lake and windborne deposits.	Wells generally yield less than 20 gallons/minute.
Hawthorne Formation	Intermediate Confining Unit	0 - 50	Predominantly clay, some grayish-green, waxy, some interbedded sand and limestone, phosphatic clay, marl, calcareous sandstone limestone residuum. Limestone, sandy, fossiliferous.	Generally not a source of water due to extremely low hydraulic conductivity.
Tampa Formation	Florida Aquifer System	0 - 30	sand and clay residuum in upper part in some areas.	Many domestic and
Suwannee Formation		0 -250	fine-grained, fossiliferous,	irrigation wells produce water from the lower part.
Ocala Group		70 -250	Limestone, white to tan, fossiliferous, massive, soft,	Yield large quantities of water from wells completed
Avon Park Formation	·	200-800	Limestone and dolomite. Limestone is light-to dark-brown, highly fossiliferous; porosity is variable in lower part. Dolomite is gray to dark-brown, very fine-to micro-crystalline and contains porous fossil molds, thin beds of carbon-	above evaporites.
	Sub-Floridan Confining Unit		fragments. Formation generally contains evaporates near base.	
	UNIT Undifferentiated Surficial Deposits Hawthorne Formation Tampa Formation Suwannee Formation Ocala Group	Undifferentiated Surficial Aquifer System Hawthorne Formation Tampa Formation Suwannee Formation Ocala Group Avon Park Formation Sub-Floridan	Undifferentiated Surficial Aquifer System Hawthorne Formation Tampa Formation Plorida Aquifer System To - 30 Florida Aquifer System O - 30 Ocala Group Avon Park Formation Sub-Floridan	Unlifferentiated Surficial Aquifer System Surficial Aquifer System

Modified after Fretwell (1988).

b:\REGIONAL

Table 2: Water Level Data, West Pasco Class I and III Landfills

Mo/Yr	PARAMETER	2MW-1	4MW-1	2MW-2	4MW-2	2MW-3	4MW-3 4MW-3A**	2MW-4	4MW-4	2MW-5	4MW-5	2MW-6	4MW-6
	Top of Casing (ft.NGVD) *	49.47	49.61	56.26	55.76	49.94	52.84	54.31	50.52	48.13	48.26	55.64	55.03
FEB 91	Depth to Water (ft.) Water Elevation (ft.NGVD)	DRY	DAMAGE	23.00 33.26	23.30 32.46	DRY	24.00 28.84	DRY	24.90 25.62				
JUN 91	Depth to Water (ft.) Water Elevation (ft.NGVD)	15.80 33.67	16.30 33.31	23.85 32.41	24.00 31.76	DRY	25.50 27.34	DRY	25.40 25.12	DRY	22.80 25.46	DRY	26.80 28.23
SEP 91	Depth to Water (ft.) Water Elevation (ft.NGVD)	9.85 39.62	10.55 39.06	17.00 39.26	16.75 39.01	DRY	22.85 29.99	DRY	14.80 35.72		12.00 36.26	19.00 36.64	18.35 36.68
DEC 91	Depth to Water (ft.) Water Elevation (ft.NGVD)	13.70 35.77	14.00 35.61	19.15 37.11	19.30 36.46	DRY	18.25 34.59	DRY	20.00 30.52		17.60 30.66	DRY	21.10 33.93
MAR 92	Depth to Water (ft.) Water Elevation (ft.NGVD)	14.75 34.72	14.95 34.66	21.15 35.11	21.30 34.46	DRY	DESTROYE	DRY	22.75 27.77	DRY	20.15 28.11	DRY	23.60 31.43
JUN 92	Depth to Water (ft.) Water Elevation (ft.NGVD)	DRY	17.50 32.11	23.20 33.06	23.50 32.26	DRY	DESTROYE	DRY	25.00 0.00		22.65 25.61	DRY	26.00 29.03
SEP 92	Depth to Water (ft.) Water Elevation (ft.NGVD)	DRY	14.20 35.41	19.40 36.86	19.60 36.16	DRY	25.30 27.54	DRY	21.20 29.32		18.50 29.76	DRY	22.00 33.03
DEC 92	Depth to Water (ft.) Water Elevation (ft.NGVD)	15.45 34.02	15.65 33.96	20.65 35.61	20.90 34.86	DRY	22.00 30.84	DRY	22.50 28.02		19.95 28.31	DRY	23.15 31.88
MAR 93	Depth to Water (ft.) Water Elevation (ft.NGVD)	DRY	17.00 32.61	22.95 33.31	23.05 32.71	DRY	24.35 28.49	DRY	24.75 25.77	DRY	22.15 26.11	DRY	25.55 29.48
JUN 93	Depth to Water (ft.) Water Elevation (ft.NGVD)	DRY	17.45 32.16	22.85 33.41	23.10 32.66	DRY	24.45 28.39	DRY	24.70 25.82	DRY	22.10 26.16	DRY	25.55 29.48
AUG 93	Depth to Water (ft.) Water Elevation (ft.NGVD)	DRY	31.99 17.62	24.15 32.11	24.55 31.21	DRY	26.20 26.64	DRY	26.35 24.17	DRY	25.31 22.95	DRY	28.93 26.10
	Depth to Water (ft.) Water Elevation (ft.NGVD)												

^{*} Top of Casing Elevation is the surveyed elevation of top of steel case minus the distance between top of stell case and top of PVC casing. Raw data provide by Pasco Co. ** 4MW-3A installed in 1992.

Table 3: Characteristics of the Hydrogeologic Units Beneath the West Pasco Class I Landfill

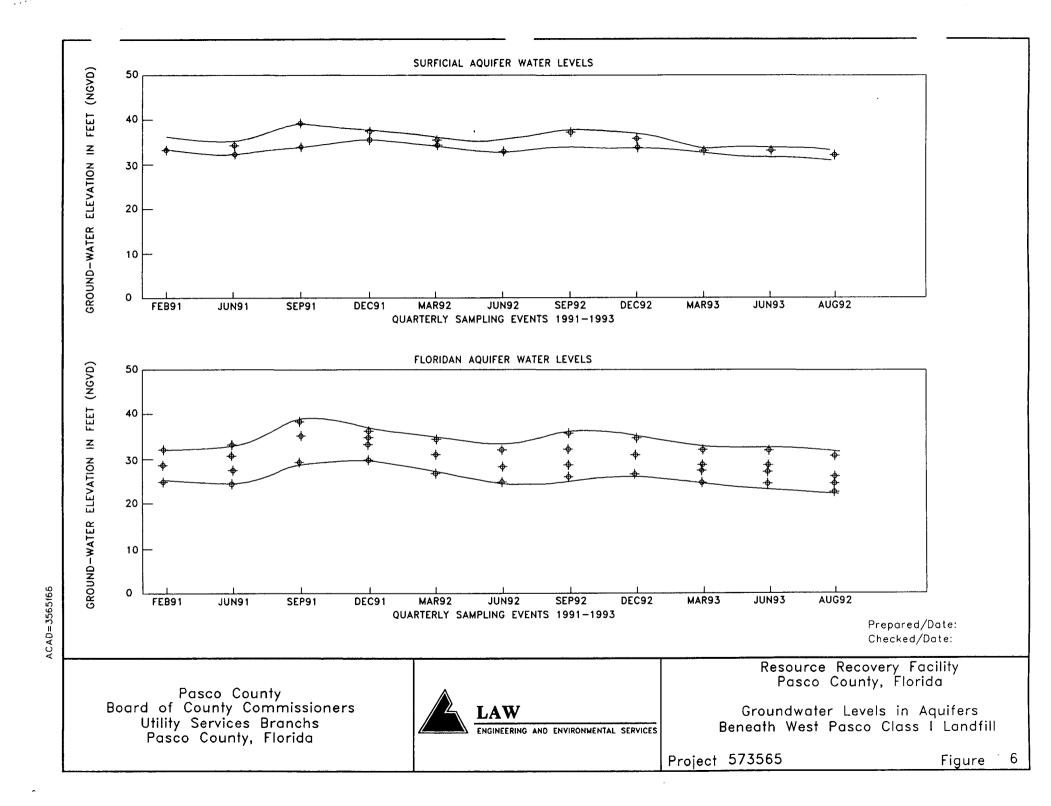
HYDROGEOLOGIC	LITHOLOGIC	THICKNESS	POROSITY	HYDRAULIC
UNIT	DESCRIPTION	(Feet)		CONDUCTIVITY
SURFICIAL	Sand, fine-grained, well-	6-36	20-40%	10-20 feet/day
AQUIFER	sorted, unconsolidated,			
	becomes finer grained			
	near base.			
CONFINING LAYER	Clay, greenish to bluish gray, in part silty and sandy, firm to plastic.	4-12		7.2 x 10E-05 cm/sec * 1.5 x 10E-09 cm/sec **
FLORIDAN AQUIFER	Limestone, thin-bedded to massive, fossiliferous, porous; dolomite in lower section.	800+	variable av.effective porosity 5%	50-150 feet/day ***

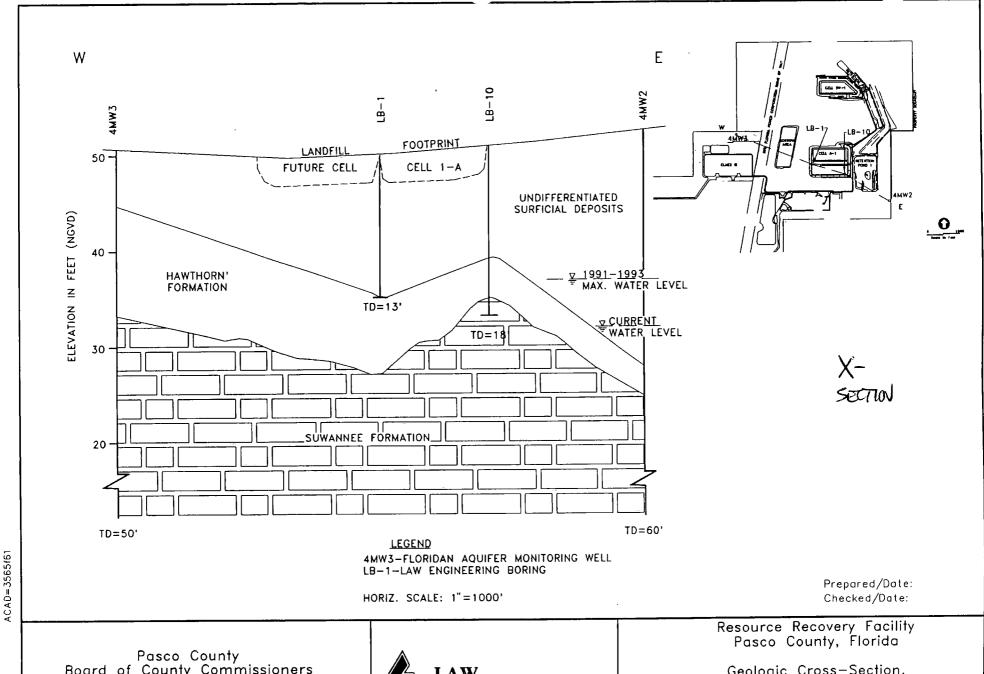
^{*} From CDM,1987, average of testing results reported in original permit application.

^{**}From Law Engineering, 1994, average of testing results on selected clay samples.

^{***}Based on a transmissivity range of 40,000-115,000 sq.ft./day (Hutchinson, 19 n). B:\CHARACT.WK3

FIGURES





Pasco County
Board of County Commissioners
Utility Services Branchs
Pasco County, Florida

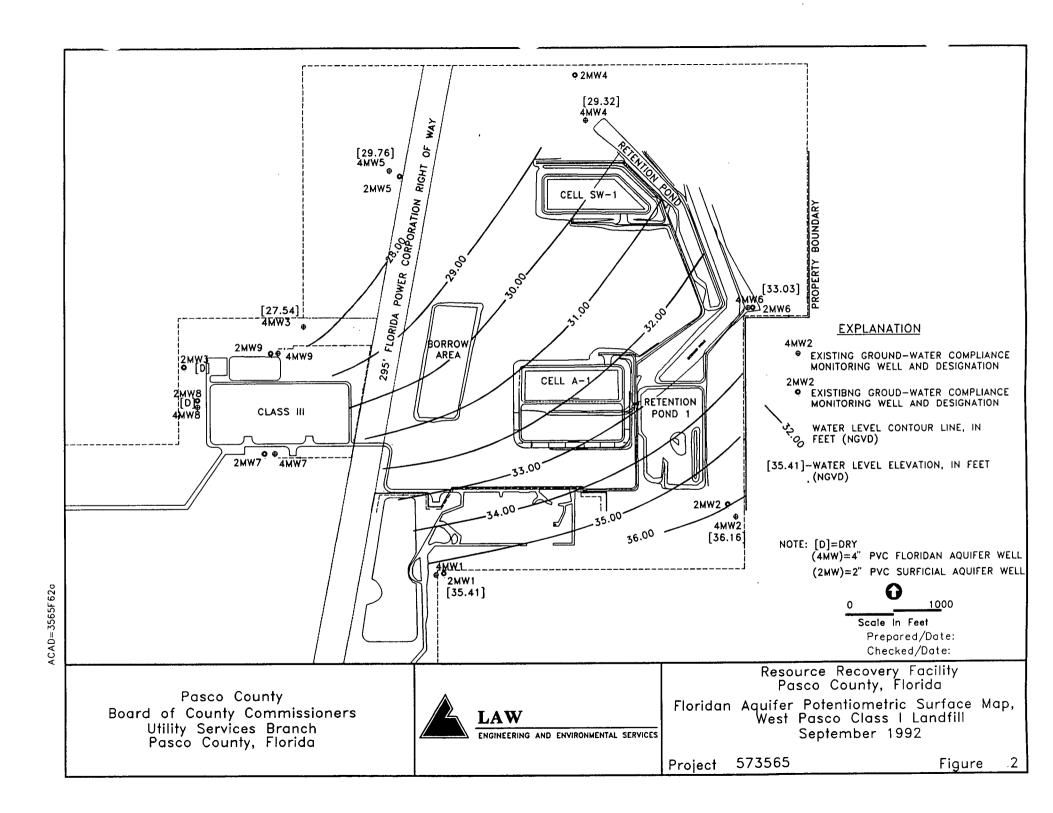


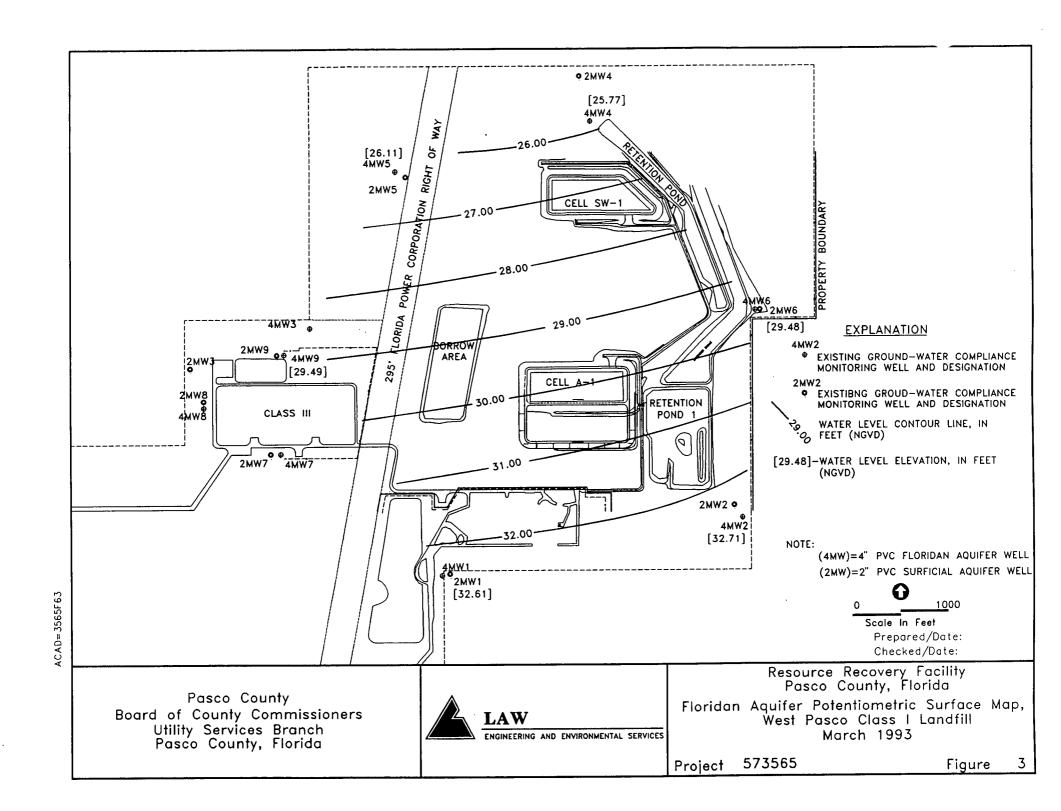
Geologic Cross—Section, West Pasco Class | Landfill

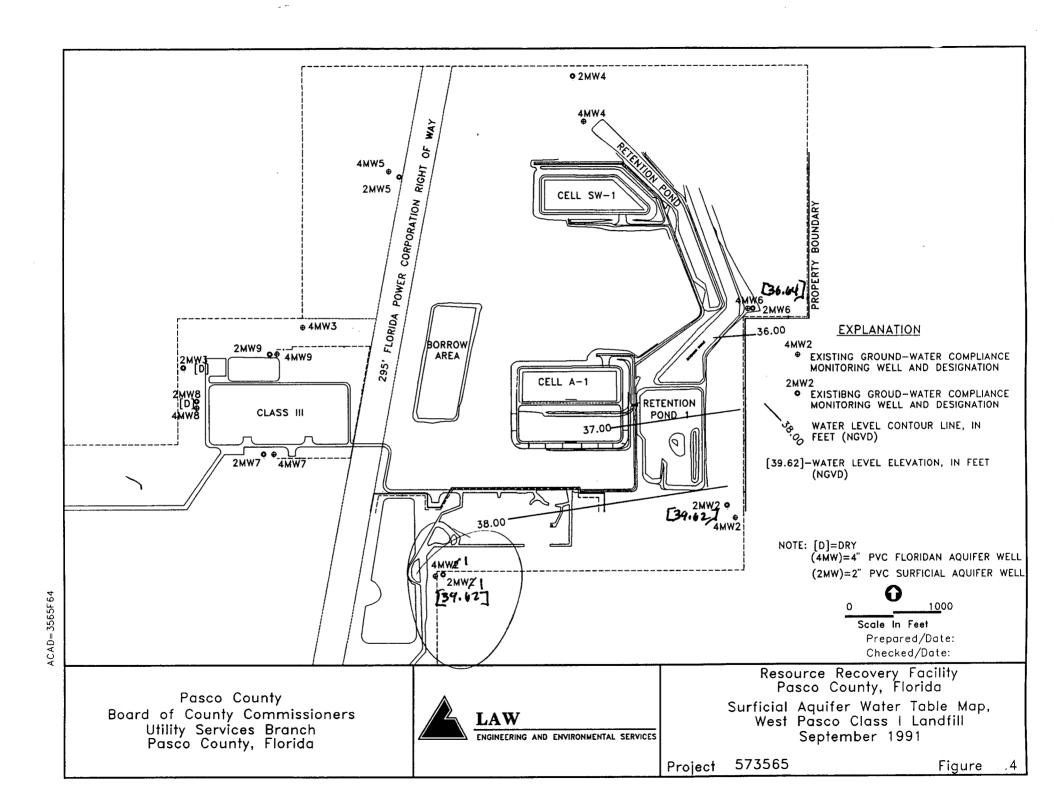
Project 573565

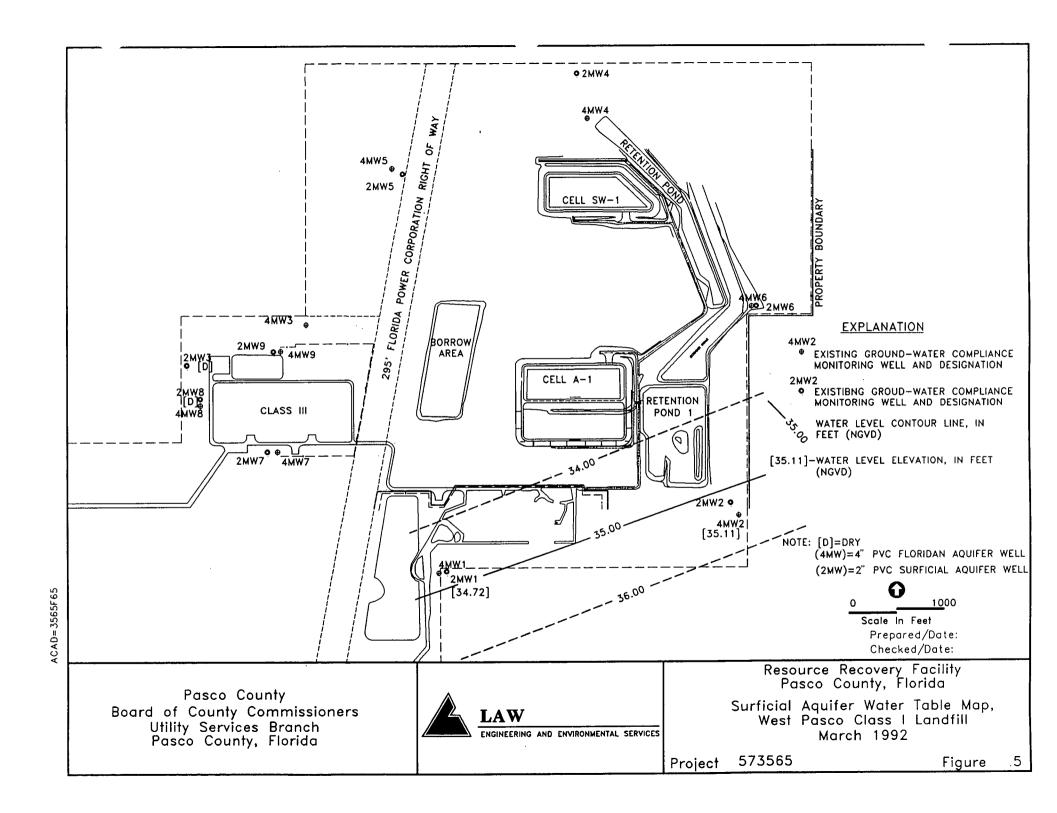
Figure

^e













Sheet 1 of 1

Project: Resource Recovery, Class I Landfill	
Client: Pasco County	Boring: LB-1
Date: 11/22/93	Geologist: O. Thornberry
Location:	Land Surf. Elev.:

			SAMPLE				STP BI	ows/6	ıı .
ELEV.	DEPTH	LITHOLOGIC DESCRIPTION	NO.	DEPTH	LENGTH	1st	2nd	3rd	4th
	0.0'	Grass and topsoil.							
	0.5′	<u>SAND</u> : Light tan, fine-grained, well sorted.						:	
	8.0′	<u>SAND</u> : Light brown, fine-grained, well sorted.							
	11.0'	<u>SAND</u> : White, fine-grained, well sorted.							
	12.0'	<u>SAND</u> : Tan, fine-grained, well sorted.							
U `	13.0'	<u>CLAY</u> : Pale blue-green and red, firm, sandy.							
						ı			

NOTES:								
Total Depth: 14.0' Time Boring Began: 1300 Water Levels: N/A Circulation Loss(es): N/A	Grout Amo							
Casing Installed/Removed: <u>Piezometer (2'</u> Vater Used: <u>N/A - Hollow Stem Auger</u> Drilling Mud Used: <u>N/A</u> Sampling Method, Split Spoon/Shelby Tube:		stalled	to 14'	Proj Date	ect N : <u>1</u>	urface 0.: <u>5</u> 2/7/93 y: <u>T</u>	73565.	01



Sheet <u>1</u> of <u>2</u>

Jose	Recovery, Class I Landfill		
Client: Pasco Cou	nty I	Boring: LB-2	

Date: __11/23/93 Geologist: 0. Thornberry

Location: Land Surf. Elev.:

				SAMPL	3		STP BI	ows/6	11
ELEV.	DEPTH	LITHOLOGIC DESCRIPTION	NO.	DEPTH	LENGTH	1st	2nd	3rd	4th
	0.0'	Grass and topsoil.							
	0.5′	<u>SAND</u> : Light tan, fine-grained, well sorted.							
	9.0'	<u>SAND</u> : Light gray-brown, fine-grained, well sorted.	:						
	10.0′	<u>SAND</u> : Gray (10YR6/2), fine- to medium-grained, rounded to subrounded, moderately sorted.	1	10.0-	18"	6	3	5	4
1	12.0'	SAND: Same as above, with occasional accessory grains (black, fine-grained, rounded to well rounded).	2	12.0- 14.0'	18"	11	27	WOR	3
	14.0′	SAND: Brown (10YR6/2), fine- to medium-grained, rounded to subrounded, well sorted.	3	14.0- 16.0'	18"	4	3	4	4
	16.0′	<u>SAND</u> : Gray (10YR6/2), fine- grained, round to subrounded, very well sorted.	4	16.0- 18.0'	18"	10	WOR	4	5
	18.0′	SAND: Gray (10YR6/2), fine- to medium-grained, rounded to subrounded, moderately sorted.	5	18.0- 20.0'	18"	4	5	6	7
	20.0'	<u>SAND</u> : White (N9), fine-grained, rounded to subrounded, very well sorted.	6	20.0- 22.0'	24"	4	5	8	10
					:				

NOTES: WOR = Weight of Rod Total Depth: 26.0' Grout Amount/Time: N/A
Time Boring Began: 1045 Time Boring Completed: 1130 Total Depth: 26.0'

Water Levels: Approximately 18' below land surface

Circulation Loss(es): N/A

Casing Installed/Removed: N/A

Water Used: N/A - Hollow Stem Auger Drilling Mud Used: N/A

Sampling Method, Split Spoon/Shelby Tube: Split Spoon 10-26' Checked By: TCH

_ Project No.: <u>573565.01</u>

Date: <u>12/7/93</u>



Sheet <u>2</u> of <u>2</u>

Project: Resource Recovery, Class I Landfill	
Client: Pasco County	Boring: LB-2
Date: 11/23/93	Geologist: 0. Thornberry
Location:	Land Surf. Elev.:

				SAMPL	S		STP BI	ows/6	11
ELEV.	DEPTH	LITHOLOGIC DESCRIPTION	NO.	DEPTH	LENGTH	1st	2nd	3rd	4th
	22.0′	SAND: Same as above., with occasional accessory grains (black, fine-grained, rounded).	7	22.0- 24.0'	12"	2	2	4	6
	24.0'	SAND: Alternately red (10R4/2), very fine- to fine-grained, rounded to subrounded, moderately sorted; and brown (10YR7/4), very fine- to medium-grained, rounded to subrounded, poorly sorted, silty to slightly clayey.		24.0- 26.0'	24"	4	6	10	13

NOTES:								
Total Depth:								
Time Boring Began:	Time Boring Completed:							
Water Levels:								
Circulation Loss(es):								
Casing Installed/Removed:								
Water Used:				Proj	ect No	o.: <u>5</u>	73565.	01
Drilling Mud Used:					: <u>1</u> :	2/7/93		
Sampling Method, Split Spoon/Shelby Tube	:			Chec	ked B	y: <u>T</u>	CH	



Sheet <u>1</u> of <u>1</u>

Project: Resource Recovery, Class I Landfill	
Client: Pasco County	Boring: LB-3
Date: 11/23/93	Geologist: O. Thornberry
Location:	Land Surf. Elev.:

			SAMPLE			STP BLOWS/6"			
ELEV.	DEPTH	LITHOLOGIC DESCRIPTION	NO.	DEPTH	LENGTH	1st	2nd	3rd	4th
	0.0'	Grass and topsoil.							
	0.5′	<u>SAND</u> : Light tan, fine-grained, well sorted.							ı B
	9.0′	<u>SAND</u> : Brown, fine-grained, well sorted.							
	20.0'	<u>SAND</u> : Brown, fine-grained, well sorted, clayey.							
	28.0′	CLAY: Pale green, soft, sandy.							
IĮ	:								

NOTES:			
Total Depth: 29.0' Time Boring Began: 1300 Water Levels: N/A	Grout Amount/Time: N/A Time Boring Completed:		
Circulation Loss(es): N/A			
Casing Installed/Removed: Piezometer (2)	' PVC) installed to 29'	below land	surface
Water Used: N/A - Hollow Stem Auger		Project	No.: <u>573565.01</u>
Drilling Mud Used: N/A		Date:	12/7/93
Sampling Method, Split Spoon/Shelby Tube	: N/A	Checked	By: <u>TCH</u>



Sheet <u>1</u> of <u>2</u>

Project: Resource Recovery, Class I Landfill	
Client: Pasco County	Boring: LB-4
Date: 11/23/93	Geologist: O. Thornberry
Location:	Land Surf. Elev.:

				SAMPL	E	STP BLOWS/6"			
ELEV.	DEPTH	LITHOLOGIC DESCRIPTION	NO.	DEPTH	LENGTH	lst	2nd	3rd	4th
	0.0'	Grass and topsoil. SAND: Gray (5Y7/2), fine- to medium-grained, rounded to subrounded, moderately sorted,	1	0.0-2.0'	18"	<u>WOH</u> 12"	WOH 12"	1	1
	2.0′	with black organic material. SAND: Gray (5Y8/2), fine- to medium-grained, rounded to subrounded, moderately sorted, with fine black organic material and occasional accessory grains (black, fine-grained, well rounded, pitted).	2	2.0- 4.0'	18"	1	2	3	3
	4.0'	<u>SAND</u> : Same as above.	3	4.0- 6.0'	18"	4	2	2	2
	5.5′	<pre>CLAY: Pale green (5G8/2), very sandy (fine- to medium-grained, rounded to subrounded), firm.</pre>							
	6.0′	CLAY: Shelby tube.	4	6.0- 8.0'	24"				
:	8.0′	<pre>CLAY: Pale green (5G8/2), slightly sandy (very fine- grained), very firm.</pre>	5	8.0- 10.0'	18"	3	4	6	8
	10.0'	<u>CLAY</u> : Same as above.	6	10.0- 12.0'	18"	4	3	4	6
	12.0'	CLAY: Same as above.	7	12.0- 14.0'	12"	8	11	16	<u></u>

NOTES: WOH = Weight of Hammer

Total Depth: 13.5' Grout Amount/Time: N/A

Time Boring Began: 1400 Time Boring Completed: 1445

Water Levels: N/A

Circulation Loss(es): N/A

Casing Installed/Removed: N/A

Water Used: N/A - Hollow Stem Auger Project No.: 573565.01

Drilling Mud Used: N/A

Sampling Method, Split Spoon/Shelby Tube: Split spoon; shelby 6-8' Checked By: TCH



Sheet <u>2</u> of <u>2</u>

4										
Projec	t: <u>Resc</u>	ource Recovery, Class I Landfill								
Client	: <u>Pasco</u>	County								
Date:_	11/23/9	93	Geologist: 0. Thornberry							
Locati	on:		_ Land S	Surf. Ele	ev.:					
				SAMPL	E		STP BLOWS/6"			
ELEV.	DEPTH	LITHOLOGIC DESCRIPTION	NO.	DEPTH	LENGTH	1st	2nd	3rd	4th	
	13.0′	<u>CLAY</u> : Pale green (5G8/2), soft with abundant weathered limestone.							•	
	13.5′	LIMESTONE: White (N9), hard.								
	,									
:										
:										
						:		-		
NOTES:				7						
Total Time B	Depth: oring Re	gan: Grout Time B	Amount/1	Time:					<u>_</u>	
Water	Levels:		-		·					
Circul	ation Lo	oss(es):ed/Removed:								
Water	Used:				Proj	ect No).: <u>5</u>	73565.	01	
Drilli: Sampli:	ng Mud U	Jsed:			Date	: <u>12</u> ked By	2/7/93	СН		
<u> </u>							·	~		



Sheet <u>1</u> of <u>3</u>

Project: Resource Recovery, Class I Landfill

Client: Pasco County Boring: LB-5

Date: 11/22/93 Geologist: 0. Thornberry

Location: Land Surf. Elev.:

			_	SAMPL	2	STP BLOWS/6"			
ELEV.	DEPTH	LITHOLOGIC DESCRIPTION	NO.	DEPTH	LENGTH	1st	2nd	3rd	4th
	0.0'	Grass and topsoil.	1	0.0-	6"	WOH	1	1	2
	0.5′	SAND: Tan (10YR7/4), fine- to medium-grained, rounded to subrounded, moderately sorted, with organic material.							
	2.0'	<u>SAND</u> : Same as above.	2	2.0- 4.0'	6"	1	2	2	3
	4.0′	<pre>SAND: Brown (10YR6/2), fine- to medium-grained, rounded to subrounded, moderately sorted, with abundant organic material.</pre>	3	4.0- 6.0'	12"	3	4	3	3
	6.0′	SAND: Tan (10YR7/4), fine- to medium-grained, rounded to subrounded, moderately sorted.	4	6.0- 8.0′	18"	4	3	3	7
	8.0′	SAND: Brown (5YR3/4), very fine- to medium-grained, rounded to subrounded, moderately sorted, silty.	5	8.0- 10.0'	12"	5	2	3	4
	10.0′	SAND: Brown (5YR3/4), fine- to coarse-grained, rounded to subrounded, moderately sorted, slightly silty.	6	10.0- 12.0'	12"	2	1	2	2
	12.0'	SAND: Brown (5YR3/4), fine- to medium-grained, rounded to subrounded, well sorted.	7	12.0- 14.0'	24"	2	2	4	26
	14.0'	SAND: Alternately white (N9), fine- to medium-grained, rounded to subrounded, well sorted; and brown (10YR5/4), fine-grained, rounded to subrounded, well sorted.	8	14.0- 16.0'	18"	2	3	8	8

NOTES: WOH = Weight of Hammer WOR = Weight of Rod

Total Depth: 38.0' Grout Amount/Time: N/A

Time Boring Began: 1345 Time Boring Completed: 1600

Water Levels: Approximately 34' below land surface

Circulation Loss(es): N/A

Casing Installed/Removed: Piezometer (2" PVC) Installed to 22' below land surface

Water Used: N/A - Hollow Stem Auger Project No.: 573565.01

Drilling Mud Used: N/A Date: 12/7/93
Sampling Method, Split Spoon/Shelby Tube: Split spoon/Shelby 22-24' Checked By: TO



Sheet 2 of 3

Project: Resource Recovery, Class I Landfill	
Client: Pasco County	Boring: LB-5
Date: 11/22/93	Geologist: O. Thornberry
Location:	Land Surf. Elev.:

				SAMPLE			STP BLOWS/6"			
ELEV.	DEPTH	LITHOLOGIC DESCRIPTION	NO.	DEPTH	LENGTH	1st	2nd	3rd	4th	
	16.0′	SAND: Brown (5YR2/2), very fine- to fine-grained, rounded to subrounded, well sorted, silty.	9	16.0- 17.5'	9"	4	8	8		
	16.5′	SAND: White (N9), fine-grained, rounded to subrounded, well sorted.								
	18.0'	SAND: White (N9), very fine- to medium-grained, rounded to subrounded, moderately sorted.	10	18.0- 20.0'	18"	4	8	14	27	
	20.0'	<u>SAND</u> : Same as above.	11	20.0- 22.0'	12"	6	10	9	9	
	21.5'	<pre>CLAY: Pale green (5G8/2), sandy (very fine- to fine-grained, rounded to subrounded), firm.</pre>								
	22.0'	<u>CLAY</u> : Shelby tube.	12	22.0- 24.0'	24"					
	24.0'	<pre>CLAY: Pale green (5G8/2), sandy (very fine- to fine-grained, rounded to subrounded), very firm.</pre>	13	24.0- 26.0'	12"	3	3	5	5	
	26.0′	<pre>CLAY: Pale green (5G8/2) and rust (10YR6/6), slightly sandy (very fine- to fine-grained, rounded to subrounded), firm.</pre>	14	26.0- 28.0'	18"	3	5	8	11	
	26.5′	SAND: Tan (10YR8/2), fine- to medium-grained, rounded to subrounded, well sorted, clayey.								

	26.5'	SAND: Tan (10YR8/2), fine-medium-grained, rounded to subrounded, well sorted, c								
NOTES:										
Time B	Total Depth: Grout Amount/Time: Time Boring Began: Time Boring Completed: Water Levels:									
Circul	ation Lo	oss(es):								
Drilli	ng Mud U	Jsed:				Date	:1	2/7/93		
Sampli	ng Metho	od, Split Spoon/Shelby Tube:				Cned	кеа в	y: <u>1</u>	CH	
Circul Casing Water Drilli	ation Lo Install Used: ng Mud U	oss(es): ed/Removed:					:1	2/7/93		



Sheet <u>3</u> of <u>3</u>

Project: Resource Recovery, Class I Landfill	
Client: Pasco County	Boring: LB-5
Date: 11/22/93	Geologist: O. Thornberry
Location:	Land Surf. Elev.:

				SAMPL	E	STP BLOWS/6"			
ELEV.	DEPTH	LITHOLOGIC DESCRIPTION	ŅО.	DEPTH	LENGTH	lst	2nd	3rd	4th
	27.5′	<pre>CLAY: Pale green (5G8/2) and rust (10YR6/6), slightly sandy (very fine- to fine-grained, rounded to subrounded), firm.</pre>	·						
	28.0′	<u>CLAY</u> : Pale green (5G8/2) to rust (10R4/6), very firm.	15	28.0- 30.0'	24"	9	9	12	10
	30.0′	CLAY: Pale green (5G8/2) and rust (10YR6/6), slightly sandy (very fine-grained, rounded to subrounded), very firm.	16	30.0- 32.0'	24"	5	5	9	9
 	32.0′	CLAY: Pale green (5G8/2), slightly sandy (very fine- to fine-grained, rounded to subrounded), very firm.	17	32.0- 34.0'	24"	11	9	12	12
	34.0′	<pre>CLAY: Pale green (5G8/2), slightly sandy (very fine- to fine-grained, rounded to subrounded), soft.</pre>	18	34.0- 36.0'	24"	<u>WOR</u> 12"		6	7
	36.0′	SAND: White (5YR8/2), fine- to medium-grained, rounded to subrounded, moderately sorted, slightly clayey.	19	36.0- 38.0'	18"	7	4	5	9

NOTES:						
Total Depth: Time Boring Began: Water Levels:	Grout Amount/ Time Boring C	Time:ompleted	•			
Circulation Loss(es):Casing Installed/Removed:						
Water Used: Drilling Mud Used: Sampling Method, Split Spoon/Shelby Tube	:		Date	ect No.: : <u>12/7/</u> ked By: _	93	.01



Sheet <u>1</u> of <u>1</u>

Project: Resource Recovery, Class I Landfill	
Client: Pasco County	Boring: LB-6
Date: 11/23/93	Geologist: O. Thornberry
Location:	Land Surf. Elev.:

				SAMPL	E	STP BLOWS/6"			
ELEV.	DEPTH	LITHOLOGIC DESCRIPTION	NO.	DEPTH	LENGTH	lst	2nd	3rd	4th
	0.0′	Grass and topsoil.							
	0.5′	<u>SAND</u> : Light tan, fine-grained, well sorted.							
	12.0′	<u>SAND</u> : Brown, fine-grained, well sorted.		3					
	15.0′	<u>SAND</u> : Light brown, fine-grained, well sorted.							
	17.0′	<u>SAND</u> : Light brown, fine-grained, well sorted, clayey.							
at :	20.0'	CLAY: Pale green, soft.							
<u> </u>									
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NOTES:		1424 - 153	***************************************					
Total Depth: 21.0'	'A							
Time Boring Began: 1020	Time Boring Completed: 1045							
Water Levels: N/A								
Circulation Loss(es): N/A								
Casing Installed/Removed: N/A								
Water Used: N/A - Hollow Stem Auger			Proj	ect No.	: 573565	.01		
Drilling Mud Used: N/A				:12/				
Sampling Method, Split Spoon/Shelby Tub	e: N/A				TCH			
				•				



Sheet <u>1</u> of <u>1</u>

Project: Resource Recovery, Class I Landfill	
Client: Pasco County	Boring: LB-7
Date: 11/22/93	Geologist: O. Thornberry
Location:	Land Surf. Elev.:

				SAMPL	<u> </u>		STP BLOWS/6"			
ELEV.	DEPTH	LITHOLOGIC DESCRIPTION	NO.	DEPTH	LENGTH	1st	2nd	3rd	4th	
	0.0'	Grass and topsoil.								
	0.5′	<u>SAND</u> : Light tan, fine-grained, well sorted.								
	8.0′	<u>SAND</u> : Light brown, fine-grained, well sorted.								
	11.0'	<u>SAND</u> : White, fine-grained, well sorted.								
	12.0'	<u>SAND</u> : Light tan, fine-grained, well sorted.								
•	13.0′	CLAY: Pale green, sandy, firm.								
					,			:		
							;			

NOTES:								
Total Depth: 14.0'	Grout Am	ount/T	ime: N	/A				
Time Boring Began: 1100	Time Bor	ing Co	mpleted	: 1130	***			
Water Levels: N/A								
Circulation Loss(es): N/A								
Casing Installed/Removed: Piezometer (2	" PVC) in	stalle	d to 14	.0' below	land s	surface	e	
Water Used: N/A - Hollow Stem Auger				Proj	ect No.	: 57:	3565.	01
Drilling Mud Used: N/A					:12/			
Sampling Method, Split Spoon/Shelby Tube	: <u>N/A</u>				ked By:		Н	



Sheet <u>1</u> of <u>1</u>

Project: Resource Recovery, Class I Landfill	
Client: Pasco County	Boring: LB-8
Date: 11/23/93	Geologist: O. Thornberry
Location:	Land Surf. Elev.:

				SAMPL	E	STP BLOWS/6"			"
ELEV.	DEPTH	LITHOLOGIC DESCRIPTION	NO.	DEPTH	LENGTH	lst	2nd	3rd	4th
	0.0′	Grass and topsoil.	}						
	0.5′	<u>SAND</u> : Light tan, fine-grained, well sorted.							
	7.0′	<u>SAND</u> : Gray-white, fine-grained, well sorted.							
	8.0′	<u>SAND</u> : Gray-brown, fine-grained, well sorted.							
	10.0'	<u>CLAY</u> : Pale green (5G8/2) and rust (10yr6/6), firm.	1	10.0- 12.0'	18"	3	4	7	8
1	12.0'	No recovery.	2	12.0- 14.0'	0"				
	14.0'	CLAY: Shelby tube.	3	14.0- 16.0'	24"				
		•				;			
									,

NOTES:

Total Depth: 16.0' Grout Amount/Time: N/A Time Boring Began: 1530 Time Boring Completed: 1600				
Water Levels: N/A				
Circulation Loss(es): N/A				
Casing Installed/Removed: Piezometer (2)	' PVC) installed to 11.0' below land surface			
Water Used: N/A - Hollow Stem Auger	Project No.: 573565.01			
Drilling Mud Used: N/A	Date: <u>12/7/93</u>			
Sampling Method, Split Spoon/Shelby Tube	: Split spoon/Shelby 12-16' Checked By:TCH			



Sheet 1 of 1

Project: Resource Recovery, Class I Landfill	
Client: Pasco County	Boring: LB-9
Date: 11/23/93	Geologist: O. Thornberry
Location:	Land Surf. Elev.:

				SAMPL	3	STP BLOWS/6"			
ELEV.	DEPTH	LITHOLOGIC DESCRIPTION	NO.	DEPTH	LENGTH	1st	2nd	3rd	4th
	0.0'	Grass and topsoil.							
	0.5′	<u>SAND</u> : Light tan, fine-grained, well sorted.							
	9.0′	<u>SAND</u> : Brown, fine-grained, well sorted.							
	12.0'	<u>SAND</u> : Light brown, fine-grained, well sorted.							
	22.0′	CLAY: Pale green, firm.							
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NOTES:							
Total Depth: 23.0' Time Boring Began: 1445 Water Levels: N/A	Grout Amo Time Bori						
Circulation Loss(es): N/A							
Casing Installed/Removed: N/A							
Vater Used: N/A - Hollow Stem Auger			Proje	ct No	. : 5	73565.	01
Orilling Mud Used: N/A				12,			
Sampling Method, Split Spoon/Shelby Tube	: <u>N/A</u>			ed By			
		-					



Sheet <u>1</u> of <u>1</u>

Project:	Resource	Recovery,	Class	Ι	Landfill

Boring: LB-10 Client: <u>Pasco County</u>

Date: 11/22/93 Geologist: 0. Thornberry

_____Land Surf. Elev.:____ Location:____

			SAMPLE			STP BI	.OWS/6	11	
ELEV.	DEPTH	LITHOLOGIC DESCRIPTION	NO.	DEPTH	LENGTH	lst	2nd	3rd	4th
	10.0′	SAND: White (10YR8/2), fine- to medium-grained, rounded to subrounded, well sorted.	1	10.0-	12"	5	8	12	13
	12.0′	CLAY: Pale green (5G8/2) and rust (10YR6/6), sandy to very sandy (fine- to medium-grained, rounded to subrounded), soft to firm, with some organic material.	2	12.0- 14.0'	12"	4	6	8	6
	14.0'	<pre>CLAY: Pale green (5G8/2) and rust (10YR6/6), firm to very firm.</pre>	3	14.0- 16.0'	18"	2	4	6	6
1	16.0′	<u>LIMESTONE</u> : White (N9), slightly clayey, friable.	4	16.0- 18.0'	18"	9	12	17	16
	18.0'	LIMESTONE: White (N9), hard.	5	18.0-	4"	31 4"			

NOTES	•
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Total Depth: 18.3' _ Grout Amount/Time: <u>N/A</u> Time Boring Began: 0945 ____ Time Boring Completed: 1040

Water Levels: N/A

Circulation Loss(es): 100% at 18'

Casing Installed/Removed: N/A

Water Used: N/A Project No.: <u>573565.01</u> Drilling Mud Used: Bentonite gel, wash boring to 10' Date: <u>12/7/93</u>

TCH Sampling Method, Split Spoon/Shelby Tube: Split spoon Checked By:



Sheet <u>1</u> of <u>1</u>

Project: Resource Recovery, Class I Landfill	
Client: Pasco County	Boring: LB-11
Date: 11/22/93	Geologist: O. Thornberry
Location:	Land Surf. Elev.:

				SAMPL	3		STP BI	ows/6	11
ELEV.	DEPTH	LITHOLOGIC DESCRIPTION	NO.	DEPTH	LENGTH	lst	2nd	3rd	4th
	0.0'	Grass and topsoil.							
	0.5′	<u>SAND</u> : Light tan, fine-grained, well sorted.							
	10.0′	CLAY: Pale green, sandy.							
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NOTES:							
Total Depth: 12.0'	Frout Amount	t/Time:	N/A				
Time Boring Began: 1040	Time Boring	Complet	ted: 110	0	-		
Water Levels: N/A		-					
Circulation Loss(es): N/A							
Casing Installed/Removed: Piezometer (2"	PVC) insta	lled to	12' belo	w land	surface		
Water Used: N/A - Hollow Stem Auger		-			No.: _		01
Drilling Mud Used: N/A					12/7/9		
Sampling Method, Split Spoon/Shelby Tube:_	N/A			hecked	By:	ГСН	



Sheet	1	of	1

Project: Resource Recovery, Class I Landfill	
Client: Pasco County	Boring: LB-12
Date: 11/23/93	Geologist: O. Thornberry
Location:	Land Surf. Elev.:

			SAMPLE			STP BI	ows/6	n
DEPTH	LITHOLOGIC DESCRIPTION	NO.	DEPTH	LENGTH	1st	2nd	3rd	4th
0.0'	Grass and topsoil.							
0.5′	<u>SAND</u> : Light gray, fine-grained, well sorted.							
6.0′	CLAY: Pale green, firm.							
:								
			;					
							!	
:								
	0.0'	0.0' Grass and topsoil. 0.5' SAND: Light gray, fine-grained, well sorted.	0.0' Grass and topsoil. 0.5' SAND: Light gray, fine-grained, well sorted.	DEPTH LITHOLOGIC DESCRIPTION NO. DEPTH 0.0' Grass and topsoil. 0.5' SAND: Light gray, fine-grained, well sorted.	DEPTH LITHOLOGIC DESCRIPTION NO. DEPTH LENGTH 0.0' Grass and topsoil. 0.5' SAND: Light gray, fine-grained, well sorted.	DEPTH LITHOLOGIC DESCRIPTION NO. DEPTH LENGTH 1st 0.0' Grass and topsoil. 0.5' SAND: Light gray, fine-grained, well sorted.	DEPTH LITHOLOGIC DESCRIPTION NO. DEPTH LENGTH 1st 2nd 0.0' Grass and topsoil. 0.5' SAND: Light gray, fine-grained, well sorted.	DEPTH LITHOLOGIC DESCRIPTION NO. DEPTH LENGTH 1st 2nd 3rd 0.0' Grass and topsoil. 0.5' SAND: Light gray, fine-grained, well sorted.

NOTES:			
Total Depth: 7.0'	Grout Amount/Time: N/A		
Time Boring Began: 1345	Time Boring Completed: 1	400	
Water Levels: N/A			
Circulation Loss(es): N/A			
Casing Installed/Removed: N/A			
Water Used: N/A - Hollow Stem Auger		Project No	573565.01
Drilling Mud Used: N/A		Date:12	/7/93
Sampling Method, Split Spoon/Shelby Tube	:_N/A	Checked By	

PAGE 10	J .
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		WELL PERMITS ISSUED REPORT		
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NUMBER 10	COUNTY	1 A		
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35 31100-01 07978 T CANNELLA ADDRESS: NO ADDRESS CITY/STATE: NO CITY, FL 37 31128-01 077000 C-M-GARTON 38 CITY/STATE: NO CITY, FL 39 99998 125 CITY/STATE: NO CITY, FL 30 311305-01 077183 M LINDSAY ADDRESS: NO ADDRESS CITY/STATE: NO CITY, FL 30 311305-01 077183 M LINDSAY ADDRESS: NO ADDRESS CITY/STATE: NO CITY, FL 31 311734-01 077612 DAVID SAYLO ADDRESS: NO ADDRESS CITY/STATE: NO CITY, FL 31 311734-01 077612 DAVID SAYLO ADDRESS: NO ADDRESS CITY/STATE: NO CITY, FL 31 311734-01 077612 DAVID SAYLO ADDRESS: NO ADDRESS CITY/STATE: NO CITY, FL 31 300 18-24-18 DOMESTIC 999998 125 PHONE: (000) 000-0000 53 ADDRESS: NO ADDRESS CITY/STATE: NO CITY, FL 31 311734-01 077612 DAVID SAYLO ADDRESS: NO ADDRESS CITY/STATE: NO CITY, FL 31 300 18-24-18 DOMESTIC 999998 08 100	ADDRESS: NO ADDRESS	3.00 18-24-18 DOMESTIC	999998 70	103 # 43
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ADDRESS: NO ADDRESS ADDRESS: NO ADDRESS CITY/STATE: NO CITY FL ZIP: PHONE: (000) 000-0000 53 311734-01 077612 DAVID SAYLO ADDRESS: NO ADDRESS CITY/STATE: NO CITY FL ZIP: PHONE: (000) 000-0000 53 311734-01 077612 DAVID SAYLO ADDRESS: NO ADDRESS CITY/STATE: NO CITY FL 299998 68 100	ADDRESSE NO ADDRESS	CLITYSTATES NO CITY, PL	99996 125	130 49
47 311/34-01 077612 DAVID SAYLO 3-00 18-24-18 DOMESTIC 999998 68 100	ADDRESS! NO ADDRESS	TAGO 18-24-18 DOMESTIC CITY/STATE: NO CITY. FL	000008 40	
	ADDRESS: NO ADDRESS	3.00 18-24-18 DOMESTIC CLTYZSTATE: NO CITY. FL	999998 68	100

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MCR050-01 21:54:35	SOUTHHEST FLORIDA WATER MANAGEMENT DISTRICT MELL CONSTRUCTION PERMITTING	04-22-94
)	WELL PERMITS ISSUED REPORT	PAGE 3
2 COUNTY! PASCO	ISSUE DATE RANGE: 01/01/76 THRU 04/22/94	
A NUMBER ID INFORMATION	MELL LOCATION CONTRACTOR PRIMARY TELESCOPE DIAMETER S-T-R USE CD DESCRIPTION ID DEPTH FROM TO F	LINER MELL ROM TO DEPTH
311757-C1 977635 A J LAMPEL ADDRESS: NO ADDRESS	3.00 18-24-18 DOMESTIC 999998 42 CITY/STATE; NO CITY, FL 71P: - PHONE.	(000) 000=0000
312186.01 078064 RAMSEY ADDRESS: NO ADDRESS	3-00 18-24-18 DOMESTIC 999998 40	(000) 000-0000
10 312237-01 078115 B ONEAL ADDRESS: NO ADDRESS	3.00 18-24-18 DOMESTIC 999998 50	98
314511.01 080388 B CASTLEBER ADDRESS: NO ADDRESS	3.00 18-24-18 DOMESTIC 999998 40	105
317265:01 083141 A BRANNEN ADDRESS: ND ADDRESS	3-00 18-24-18 DOMESTIC 99998 90	(000) 000-0000
317315.01 083191 T THOMAS ADDRESS: NO ADDRESS	3.00 18-24-18 DOMESTIC 999998 110	(000) 000-0000 7
19 318460.01 084333 J PAINES ADDRESS: NO ADDRESS	3.00 18-24-18 DONESTIC 999908 60	(000) 000-0000 2
21 319349.01 085222 E KRAFT 22 Address: No Address	3:00 18-24-19 DOMESTIC 999998 84	105
23 24 319936.01 085809 R STRINRICK ADDRESS: NO ADDRESS	3.00 18-24-18 DOMESTIC 999998 71	000) 000~0000 2 125 2
25 76 70 70 70 70 70 70 70 70 70 70	3.00 18-24-18 DOMESTIC 001037 40	000) 000-0000 (2000) 31 32
28 323971.01 089839 S F KENNEDY ADDRESS: NO ADDRESS	3.00 18-24-18 DOMESTIC CO1121 74	95 32
30 324313.01 090181 T THOMPSON ADDRESS NO ADDRESS	3.00 18-24-18 DOMESTIC 999998 73	000) 000-0000 39
32 33 324398.01 090266 T MCDANIELS 33 ADDRESS: NO ADDRESS	3.00 (303) DOWNERS	000) 000-0000
34 34 34 36 37 38 38 38 38 38 38 38 38 38 38 38 38 38	J.00 18-24-18 DOMESTIC	doo) 000-4000
324972.01 090838 HARRY RIEGE ADDRESS! NO ADDRESS	CITY/STATE: NO CITY, FL ZIP: - PHONE: (000) 000-0000 47
326071.01 091937 S SORIA 40 ADDRESS! NO ADDRESS	ZIP: - PHONE: (000) 000-0000 90 01
41 326072.01 091938 N HELLS	CITY/STATES NO CITY. FL ZIP: - PHONE: (000) 000-0000 53
ADDRESS! NO ADDRESS	CITY/STATE: NO CITY. FL ZIE: - PHONE: (1	58 20 58 20 000-000, (000

A COMMENT OF THE PROPERTY.

HCR050-01 21:54:35	SOUTHWEST FLORIDA WATER MANAGEMENT DISTRICT MELL CONSTRUCTION PERMITTING	T	04-22-94 PAGE 4
COUNTY: PASCO ISSUE DATE R	MELL PERMITS ISSUED REPORT ANGE: 01/01/70 THRU: 06/22/90	and the second	1 1 2
NUMBER ID INFORMATION	MELL LOCATION DIAMETER S-T-R USE CD DESCRIPTION	CONTRACTOR PRIMARY TELE	SCORE LINES MELL 4 TO FROM TO DEPTH 5
5 326213-01 092079 M HOWARD)6 ADDRESS: NO ADDRESS	3.00 18-24-18 DOMESTIC CITY/STATE: ND CITY. FL	001121 87	PHONE: (000) 000=0000
327113.01 092979 MASILONSKI ADDRESS: NO ADDRESS	3.00 18-24-18 DOMESTIC CITY/STATE: NO CITY: FE	999908 63 - 71P1 -	PHONE: (000) 000-0000
110 329132.01 094995 L E AASENG ADDRESS: NO ADDRESS	3.00 18-24-18 DOMESTIC CITY/STATE: NO CITY, FL	999998 42	66 13
312 342700.01 108369 GILLETTE, B ADDRESSI NO ADDRESS	CITY/STATES NO CITY FIL	201609	PHONE: (000) 000-0000 14
343137-01 108789 TRUNDLE F ADDRESS: NO ADDRESS	4.00 18-24-18 DOMESTIC	001841 41	PHONE: (000) 000-0000 17 18 19
348262-01 011268 JOHN P. HILLYER ADDRESS: 138 SHIRLA RAE	4.00 18-24-18 DOMESTIC CITY/STATE: BROOKSVILLE, FL	001056	PHONE 2 (000) 000-0000 20
000 000 000 000 000 000 000 000 000 00	4.00 18-24-18 DOMESTIC CITY/STATES BROOKSVILLE, AFL	001040	PHONE: (000) 000-0000 73 74 75
71 353529.01 012289 DAVID E. DAVIS 22 ADDRESS: 70 NANCY DR	4:00 18-24-16 DOMESTIC CITY/STATE: BROOKSVILLE, FL	001096	PHONE: (000)*000-0000 26
23 369866.01 027617 ROYCE, KEVIN 24 Address: Lot 164 Shirla Rae	4.00 18-24-18 DOMESTIC CITY/STATE: BROOKSVILLE, FL	002160 110	PHONE: (000) 000-0000 [79]
25 372311.01 029402 AIRAL ADORESS: 215 RDYAL DAK	CITY/STATE: BROKSVILLE, FL	002132	PHONE: (000) 000-0000 37
27 375524.01 032458 PETTY, PAULINE ADDRESS: 177 NANCY DR	4.00 18-24-18 DOMESTIC CITY/STATE: BROOKSVILLE, FL	001056 55	PHONE: (000) 000-0000 33 PHONE: (000) 000-0000 38 39
30 390451.01 043685 HRUSKA, MARK	4.00 18-24-18 DOUSCTIC	001056	PHONE: (000) 000-0000 15
37 390660.01 043704 FERANDES, MIRIAM ADDRESS: 6974 ANGELA PRIVE	CITYZŠYATEJ BROOKSVILLE, PL.	21P: 33692- 41 002169 122	PHONE: (000) 000-0000 1
34 390858.01 043874 BLAZEVIE, ANTHONY R ADDRESS: 250 HELEN K	4-00 IA-24-IA DOUESTIC		PHONE 8 (000) 000-0000 4
30 37 391717.01.064635 CHANDLER, DURAND ADDRESS: 345 SHIRLEY RAE DR	CITY/STATE: SPRING HILL, FL	ZIP1 33526~ F	PHONE: (000) 000~0000
198 391718-01 044435 CHANDLER, DURAND 40 ADDRESS: 345 SHIRLEY RAE DR	4/00% 18-24-18 DOMESTIC	ZIP1 33512-1 F	PHONE: (000) dda-d000
392973.01 045409 JOHNSON, RICHARD	4.00 18-34-19 DOUEST-6	ZIP1 33512- F	PHONE: (000) 000-0000 13
1)41 ADDRESS: 171 HELEN K DRIVE	CITYSTATES SPRING MILLS EL	001056 63 ZIP: 33526- F	HONE: (000) 000-0000 50

	WCR050-01 21:54:35		SOUTHMEST FLORIDA MATER MANAGEMENT DISTRICT WELL CONSTRUCTION PERMITTING		04-22-94 PAGE 5
) <u>></u>		206-77 J. M. 363 . V 23	MELL PERMITS ISSUED REPORT		
2	COUNTY: PASCO	ISSUE DATE RAN	NGE 1 01/01/70 THRU: 04/22/94	100	
1/3	HCP OHNER	OHNER			1 3
1	NUMBER ID	INFORMATION	DIAMETER S-T-R USE CD DESCRIPTION	CONTRACTOR PRIMARY TELESCO ID DEPTH FROM	TO FROM TO DEPTH 5
) 2	396807.01 048481 ADDRESS:	LARSEN, RONALD LOT 163 SHIRLA RAE	4.00 18-24-18 DOMESTIC Cliy/State: Brooksville. Fi	001841 95	NE: (000) 000-0000 8
8		1837 MOCKING BIRD LANE	4.00 18-20-18 DOMESTIC CITY/STATE: SPRING HILL, FL	002092 58	70 III
1 1	ADDRESS:	HELTON, MADILENE 117 LAURA LEE DRIVE	4.00 18-24-18 DOMESTIC CITY/STATE: BROOKSVILLE, FL	001533 70	90 13 NE: (000) 000-0000 14
	407082.01 055789 ADDRESS:	HOUSE, DUANE LOT 28 MOCKINGBIRD LANE	2.00 18-24-18 DOMESTIC	002501 104	130
14	1			ZIP1 33526- PHO	NE1 (000) 000-0000
) <u>[</u>	ADDRESS:	LOT 19 CHRISTOPHER RD	2.00 18-24-18 DOMESTIC	002882 99 2181 33526= 880	99
1 :	411678.01 058752	GREER. JOSEPH			HES (000) 000-0000 21
1 2	ADDRESS:	50 EAST HELEN K DRIVE	4.00 18-24-18 DOMESTIC CITY/STATE: SPRING HILL, FL	001056 77 ZIP: 33526⇒ PHO	NE: (000) 000-0000 22
) 19	411779.01 058762	KLINE, D	4.00 18-24-18 DOMESTIC		(24)
20	1	608 HIGHLAND	CITY/STATES HUDSON, FL	001533 105 ZIPI 33568- PHO	NE1 (000) 000-0000 25
1 21	412263-01 059031	TEAGUE KEVIN P	A.00 18-24-18 DOMESTIC		90 27
23	Ī		CITY/STATE: SPRING HILL, FL		NE: (000) 000-0000 29
24	414483.01 060340 ADDRESS:	CREATIVE DESIGN 2941 OLIVE DAK STREET	4.00 18-24-18 PUBLIC SUPPLY CITY/STATE: HUDSON, FL	002392 68	80 31
25					NE: (000) 000-0000 37
27	ADDRESS:	GOBLE, DALE 47 HELEN K DRIVE	4.00 18-24-18 DOMESTIC CITY/STATE: BROOKSVILLE, FL	002405 74 ZIP: 33512- PHO	175
1 29	419375.01 062916	ASTORE, RON	4.00 18-24-18 DOMESTIC		NE: (000) 000-0000 33
29	ADDRESS:	LOT 16 HIGHLAND BLVD	CITY/STATE: SPRING HILL, FL	001841 52 ZIP: 33526- PHOP	NE: (000) 000-0000 37
30	420335.01 052085	SCHMITT ENTERPRISES 9276 CHILTON STREET	4.00 18-24-18 DOMESTIC	00212081	33
37	77340 773 373633		CITY/STATES SPRING HILL, FL		v€3: (000): 000=0000;
33	426515.01 065776 ADDRESS:	ARIOLA, CHARLES 216 SHADY HILLS ROAD	4.00 18-24-18 DOMESTIC	002451 53	47
34	Į		CLIVETATES BROOKSVILLE FL		451 (000) 000-0000 ····
25	428984.01 066812 ADDRESS:	125 SUNCOAST BLVD.	4.00 18-24-18 DOMESTIC CITY/STATE: SPRING HILL, FL	002092 92	114
137	432713.01.068557	ANDERSON MOBILE HOME			NE: (000) 000-0000
38	ADDRESS:	12155 CORTEZ BLVD	CITY/STATE: BROOKSVILLE, FL	002836 47 ZIPI 33573- PHON	ies (000) opo-0085
39	442659-01 115959	CLINE DEABRA LYNN	2.00 IS-24-18 DOMESTIC		E. (400, 400-4000)
40		28 HIGHLAND BLVD	CITY/STATE: SPRING HILL, FL	00200 160 ZIP: 34610- PHON	(E: (000) 000-0000 51
4 2	447989.01 119303 ADDRESS:	KRYSHER, HALT 1478 AUBRY	3.00 18-24-18 DOMESTIC	001436 52	85 D3
4.2	1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1		CLIVESTATE SPRING HILL FL		E : (000) 000-0000 .
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MELL PERMITS ISSUED REPORT	ł
COUNTY: PASCO ISSUE DATE RANGE: 01/01/70 THRU 04/22/94	1
	R MELL 4 TO DEPTH 5
3 464562-01 131614 HARMON, HARRY 6 ADDRESS: 32 HIGHLAND BLVD CITY/STATE; SPRING HILL: FL ZIP: 34610- PHONE: (000)	100 67
ADDRESS: 32 HIGHEAND BLVD CLTY/STATE: SPRING HILL; FL ZIP: 34610- PHONE: (000)	100
10 464566-01 131614 HARMON, HARRY 4-00 18-24-18 DOMESTIC 002871 70 11 ADDRESS: 32 HIGHLAND BLVD CITY/STATE: SPRING HILL, FL ZIP: 34610- PHONE: (000)	100 13
)12 466087-01 132306 TAYLOR, PATRICK R 2.00 18-24-18 DDMESTIC 002501 57 13 ADDRESS LOT 36 CARDINAL LANE CXTY/STATE: SPRING HILL; FL ZIP: 34606- PHONE: (000)	70 16 000-0000 17
14 489322-01 144985 MCDANIEL/TOLER 4-00 18-24-18 DOMESTIC 001318 105 ADDRESS: 267 HELEN K OR CITY/STATE: SPRING HILL: FL 71P: 34610- PHONE: (008)	1 30 19 000 0000 20
496766-01 148741 MEDLEN GARRY 2.00 18-24-18 DOMESTIC 002800 54 CITY/STATE: SPRING HILL. FL ZIP: 34610- PHONE: (000)	75 22 000-0000 23
10 497398-DI 149884 HINCHER, JOE 4.00 18-24-18 DOMESTIC 901699 84 20 ADDRESS: 69 NANCY DRIVE CITY/STATE: SHADY HIELS, FL ZIP: - PHONE: (000)	
306200-01 193763 SANDERSON-STEVE	
73 507515.01 154437 BURNEY, LOIS 74 ADDRESS: 15691 PEACE ROAD 75 S17765.01 146500 PANOM 76 S17765.01 146500 PANOM 77 S17765.01 146500 PANOM 78 PRINCE	135 31 000=0000 37
ADDRESS: 229 SHIRLEY RAE CHANDLER CITY/STATE: SPRING HILL, FL 279: 34610- PHONE: (000)	90 31 000-0000 33
ADDRESS: 181 PLATNUM ST. CITY/STATE: SPRING HILL, FL ZIP: 34610- PHONE: (000)	85 000-000 35
30 523907.01 172475 CHARLES JENSEN 4.00 18-24-18 DOMESTIC 009048 84 CITY/STATE: SPRING HILL.FL 2193.34610- PHONE: (813)	100 40
37 524880.01 173432 ROBERT ROM 4.00 16-24-18 DOMESTIC 002800 97 CITY/STATE! SPRING HILL FL 21P: 34610- PHONE: (813)	110 [0]
35 531532.01 1784.95 DAVID DAVIS 4.00 18-24-18 DOMESTIC 002364 84 CITY/STATE; SHADY HILLS, FL ZIP: 34610- PHONE; (813)	101 46
37 B36360.DI 188510 MARGREY STEFANSKI 2.00 18-24-18 DOMESTIC 002207 42 38 ADDRESS: 15606 HELEN K DR CLITY/STATE: SPRING HILL, FL 21P: 34610- PHONE: (000)-	48
37 547856.01 204421 SANDRA PAULSON 4.00 18-24-18 DOMESTIC 002805 146 CITY/STATE: HUDSON, FL ZIP: 34667- PHONE: (000)	250 27
551906.01 206863 ALFRED W SCHIAVONE 4.00 18-24-18 DOMESTIC 001232 ADDRESS: 16824 LAURA LEE DRIVE CITY/STATE: SPRING HILL. FL ZIP: 23610- PHONE: (813)	856=33 42 00 1

APPROPRIEST AND ARTHUR PROPERTY

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	HCR050-01 21:54:35		SOUTHWEST FLORIDA MATER HELL CONSTRUCTI	MANAGEMENT DISTRICT ON PERMITTING				04-22-94)
> >			MELL PERMITS I	SSUED REPORT				PAGE 7	1
2	COUNTY: PASCO	ISSUE DATE RAN	GET 01/01/70 THRU 04/22/	94					ζ.
3	MCP OMNER OM	NER	MELL LOCATION					- 2	1
4		FORMATION	DIAMETER S-T-R	USE CD DESCRIPTION	ID	DEPTH FE	ELESCOPE LINE	TO DEPTH 5	10
6 7	A_SYYY	NERAL DELIVERY	3.00 19-24-18 CITY/STATE: SPRIN	DOMESTIC G_HILLFL	001232	4C 33526-	PHONE: (000)	86 7	1
8	305845.01 048465 GI ADDRESS: RT	5	3.00 19-24-18 CITY/STATE: BROOK	DOMESTIC Sville, Fu	001436		PHONE: (000)	9 10	
2 =	307439.01 073320 W ADDRESS: NO	G CHANDL ADDRESS	3.00 19-24-18 CITY/STATE: NO CI	DOMESTIC TY. FL	999998 ZIP:	48		86 13)
)12	30744C.01 073321 E	K CHANDL	3,00 19-24-18	DONESTIC			PHONE: (000)	000-0000	}
14	307902.01 073783 A L		CLITY/STATE: NO CI	TYS IFL S NO.	ZIPT	71	PHONE: (000)	000-0000 - 17)
15	ADDRESS: NO	ADDRESS	J#80 19-24-18 CLTY/STATE: NO CL	DOMESTIC TY. FL	999998 7181	106	PHONE: (000)	18	
17	307971.01 C73852 E P ADDRESS: NO	ADDRESS	3.00 19-24-18 CITY/STATE: NO CIT	DOMESTIC TV. FL	999998 ZIP:	43 _	PHONE: (000)	102)
19	308008.01 073889 L E Address1 No	E RYALS Address	3.00 19-24-18 CITY/STATE: NO.CIT	DOMESTIC	99998			24	\rightarrow
21	308438.01 071286 NO	NAME	4,00 19-24-18		ZIPI		PHONE: (000)	000-0000 - 2	ı
22	ADDRESS: NO		CITY/STATE: NO CIT	IV. FL	QQQQQA ZIP:	- 6A _	PHONE: (000)	221 78)
24	308439.01 074320 HEL ADDRESS: NO	ADDRESS	3.00 19-24-18 CITY/STATE: NO CIT	DOMESTIC	999998 ZIP:	60		1 1 1 30	
26	308550.01 074431 L C ADDRESS: NO	HOUSELL Address	3.00 19-24-18 CITY/STATE1 NO CIT	DOMESTIC	999998	50	PHONE: (000)	116 37	Ì
28	309080.01 074960 P C	HITT			ZIPI	-	PHONE: (DOO)	000-0000	
29 30	ADDRESS: NO 310342.01 076221 D M		3.0G 19-24-18 CITY/STATE: NO CIT	Y. FL	999998 ZIP:	50 _	PHONE: (000)	100 37 000-0000 38	ì
31	ADDRESS: NO	ADDRESS	CITYNSTATES NO CIT	DOMESTIC	99999	49		96 40	- 1
33	311605.01 076393 C S ADDRESS: NO	TRICKLAN Address	3.06 19-24-18 CITY/STATE1 NO CIT	DOMESTIC	ZIP;	61	PHONE: (000)	000-0000	,
34	311606.01 977484 H C ADDRESS: NO	HILES	3.00 19-24-18	DOMESTIC	71P1		PHONE: (000)	000-0000 4	,
36	311607.01 077485 H.L		CLIANSTATE: NO CIT	Y, FL	ZIP:	72 -	PHONE: (000)	000-0000	
36	ADDRESS! NO	ADDRESS	CITY/STATEL NO CIT	DOMESTIC. V. FL	999998 ZIP1	24	PHONE: (000)	000-0000 49)
40	312729-01 078607 1 D	ALOERMA ADDRESS	JANO 10-24-18 CITY/STATE: NO CIT	DOMESTIC	999998	<u> </u>		100	
41	313262.01 079140 J C ADDRESS: NO	BEAVER	3.00 19-24-18	DOMESTIC	ZIP:	-	PHONE: (000)		,
43		ADDRESS		Y. FL	7151	60 _	RHONE : (000)	000-0000 63	,

Principal Andrews

MCR050-01 21:54:35	SOUTHMEST FLORIDA WATER MANAGEMENT DISTRICT MELL CONSTRUCTION PERMITTING		04-22-94 PAGE 8
19306	DATE RANGE: 01/01/70 THRU: 04/22/94		
NUMBER ID INFORMATION	MELL LOCATION STATEMENT OF THE PROPERTY OF THE	CONTRACTOR PRIMARY TELESCOPE IO DEPTH FROM TO FR	INER MELL
313580.01 079458 L DAVIS ADDRESS: NO ADDRESS	4.00 19-24-18 DOMESTIC CITY/STATE: NO CITY, FI	999998 51	72
314688.01 080565 J POST Address: No Address	4.00 19-24-18 DOMESTIC CITY/STATE: NO CITY, FL	99998 120	900) 000-0060 167
314786.01 08C663 E REMON ADDRESS: NO ADDRESS	3.00 19-24-18 DOMESTIC CITY/STATE: NO CITY, FL	999998 53	00) 600-0000 A7
314920.01 080797 T HENDERSON ADDRESSE NO ADDRESS	GAGO 19-24-14 DOMESTIC CLTY/STATE: NO CITY: NEU	099008 40	000) 000-0000
316530-01 082406 E USHERHOOD	4.00 19-24-10 DATESTIC	999998 50	1001, 000-0000
316685.01 082561 W HOSEY ADDRESS: NO ADDRESS	3.00 19-24-18 DOMESTIC		100) 000=0000 <u> </u>
18117-01 083991 D LONES ADDRESS NO ADDRESS	CITY/STATE: NO CITY, FL	ZIP: - PHONE: (0	00) 000-0000
ADDRESS: NO ADDRESS	Japo 19-24-18 DOMESTIC	ZIP: - PHONE: (0	00) 000-000
318435.01 084308 P E WHISLER	CITY/STATE: NO CITY, FL	ZIP: - PHONE: (0	00) 000-0000
ADDRESS: NO ADDRESS	CLIVESTATE: NO CITY. FL	999998 100 - PHONES (0	90) 909-0000
ADDRESS: NO ADDRESS 322693.01 088563 E MYERS	CITY/STATE: NO CITY; FL	999908 41 - PHONE: (0	00) 000-0000
ADDRESS: NO ADDRESS 126963-01 092829 M HOULE	4.00 19-24-18 DOMESTIC CITY/STATE: NO CITY, FL	999998 72 ZIP: - PHONE: (0	72 00) 000-0000
ADDRESS NO ADDRESS	3,00 19-24-1A DOMESTIC CITY/STATEL NO CITY FL	999998 42	00):000-0000
28555.01 004419 W C CONNER ADDRESSI NO ADDRESS	J.00 19-24-18 DOMESTIC	999998 60	AO.
30661.01 096518 R HINKLE Address: No Address	4.00 19-24-18 DOMESTIC CITY/STATE: NO CITY, FL	001388 133	80) 000-0000 200
34240.01 100077 VERLIN E	CITY/STATE NO CITY, FU	001696 49	00) 000-0000 105
39283.01 100120 KIEGEL D. ADDRESS! NO ADDRESS	CITY/STATE: NO CITY, FL	FIP1 - PHONE; (6)	00):000-0000
37484.01 103314 MCNUTT ADDRESS: NO ADDRESS	4=00 19=24=19 DOUESTA	ZIP: - PHONE: (00	00) 000-0000
	CITY STATE NO CITY FL		001-000-0000

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WCR050-01	SOUTH	MEST FLORIDA MATER MANAGEMENT DISTRICT			
21:54:35		WELL CONSTRUCTION PERMITTING		04-22-94 PAGE 9	
COUNTY: PASCO	ISSUE DATE RANGEI 01	MELL PERMITS ISSUED REPORT /01/70 THRU 04/22/94			
MCP OWNER OWNER NUMBER ID INFORMA		MELL LOCATION DIAMETER S-T-R USE CD DESCRIPTION	ONTRACTOR PRIMARY I	FLESCORE LINER MELL ROM TO FROM TO DEPTH	
338354.01 104176 CERJES, ADDRESS: NO ADDR	R RESS	4.00 19-24-18 DOMESTIC CITY/STATE: NO CITY, FL	001498 66 _	PHONE: (000) 000-0000	
340346.01 106144 EADY,R ADDRESS: NO ADDR	V PESS:	4.08 19-24-18 DOMESTIC CITY/STATE: NO CITY'S FL	001056 107 ZIP1 -	-170 PHONE: (000) 000-0000	· m·
350731.G1 013224 HILLIAN ADDRESS: RT 2	C. HAYES	4.00 19-24-18 DOMESTIC CITY/STATE: EDWARDS, MO	001696 105 ZIP: 65326-	PHONE: (000) 000-0000	-
353630.01 015414 ROACH, ADDRESS: 2307 PA	ARK DRIVE	4.00 19-24-18 DOMESTIC	001696 71 ZIP1:33552-	PHONE: (000) 000-0000	
354835.01 016311 SPAULDI ADDRESS: 1218 SU	INCOASE BLVD	4.00 19-24-18 DOMESTIC CITY/STATE: NEW PORT RICHEY. FL	001533 63 ZIP: 33553-	PHONE: (000) 000-0000	
359063.C1 019516 DRAIN, ADDRESS: 3902 LE	ILA AVE	4.00 19-24-18 DOMESTIC CITY/STATE: TAMPA, FL	001699 79 ZIP: 33616-	PHONE: (000) 000-0000	
362955.01 022442 HINKLE, Address: RT 5, B	IOX 1731	4.00 19-24-18 DOMESTIC CITY/STATE! DROCKSVILLE, FL	001056 50 ZIP: 33512=	PHONE: (000) 900-0000	
365840.01 024643 ALLEN. Address: 685 Pay	51	2.00 19-24-19 DOMESTIC CITY/STATE: NEW PORT RICHEY, FL	002177 88 ZIP: 33553-	PHONE: (000) 000-0000	
367750.01 025997 HAHLE, ADDRESS: LOT 30	BLUEBIRD LANE	4.00 19-24-18 DOMESTIC CITY/STATE: BROOKSVILLE, FL	002159 67 ZIP: 33512-	PHONE: (000) 000-0000	
369860.01 027611 COLSTON Address: Cardina	L LANE SHADY HILLS	4.00 19-24-18 DOMESTIC CITY/STATE: BROOKSVILLE, FL	001817 80 ZIP: 33512-	PHONE: (000) 000-0000	14
379193.01 036013 KELLEHA Address: Lot A C	ARDINAL LANE	2.00 19-24-18 DOMESTIC CITY/STATE: HUDSON, FL	002177 ZIP: 33567-	PHONE: (000) 000-0000	
384752.01 040038 KELLY, ADDRESS: PO BOX	11043 ROBIN LANE	CITOS TATES BROOKS VILLE . FL	001056 102 ZIP1 33612-	PHONE: (000) 000-0000	
ADDRESS: CARDINA	L LANE	4.00 19-24-18 DOMESTIC CITY/STATE: SHADY HILLS FL	001056 44 21P1 33526=	PHONE: (000) 000-0000	***
390531.01 043644 NICHOLS ADDRESS: 1760 BL	UEBIRD LANE	4.00 19-24-18 DOMESTIC CITY/STATE: SPRING HILL, FL	002263 105 ZIP: 33526-	PHONE: (000) 000-0000	
395304.01 047366 CARTER. Address: General	DELIVERY	3.00 19-24-18 DOMESTIC CITY/STATE: HUDSON, FL	001056 ZIP2 33568-	PHONE ((000) 000-0000	
395652.01 047289 BARRETT. Address: General	DELIVERY	3200 10-24-18 DOMESTIC CITY/STATE: SPRINGHILL, FL	000012 ZIP: 33526-	PHONE: (000) 000-0000	++-
399298.01 050631 MULLIN, ADDRESS: 1717 CA	PAUL A ROINAL LANE	2.00 19-24-18 DOMESTIC CITY/STATE: SPRING HILL: EL	002800 42 '	PHONE: (000) 000-0000	

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	21:54:35		SOUTHWEST FLORIDA MATER MANAGEMENT DISTRICT WELL CONSTRUCTION PERMITTING		04-22-94 PAGE 10
{(`			HELL PERMITS ISSUED REPORT		
2			EI 01/01/70 THRU 04/22/94		7-1
) [3	NUMBER ID	OMNER INFORMATION	MELL LOCATION DIAMETER S-T-R USE CD DESCRIPTION	CONTRACTOR PRIMARY TELE	SCOPE LINER MELL 4 TO FROM TO DEPTH 5
) 1	ADDRESS	HOUSE: HILLARD D 28 MOCKINGBIRD LANE	2.00 19-24-18 DOMESTIC CITY/STATE: HUDSON. FL	999999	PHONE: (000) 000-0000 6
8 9	36.6	PEAL, HEBERT 148 SECOND STREET	4.00 19-24-18 DONESTIC CITY/STATE: SPRING HILL, FL	001533 84	PHONE: (000) 000-0000
10 11	•	LOT 20 JAY	4.00 19-24-18 DOMESTIC CITY/STATE: BROOKSVILLE, FL	002092 96	PHONE: (000) 000-0000 14
5) 12 13		390 AMY LANE	2400 19-24-18 DOMESTIC CLTY/STATE: HUDSON: FE	001036 61	PHONE 1 (000) 000-0000 17
15		CREMS LAKE	6.00 19-24-18 DOMESTIC CITY/STATE: SHADY HILLS, FL	002364 63	2HONE 2 (200) 000-0000 70
17	ADDRESS:	BILLINGS, JOHNNIE L 15330 DENNIS DRIVE	2.00 19-24-18 DOMESTIC CITY/STATE: HUDSON. FL	002800 62	PHONE: (000) 000-0000 23
20	ADDRESS:	RT.5 BOX 1752 CARDNELL LN.	4.00 19-24-18 DOMESTIC CITY/STATE: LAND-D-LAKES, FL	ZIP: -	PHONE: (000) 000-0000 . 26
23			2:00 19-24-18 DOMESTIC CITY/STATE: HUDSON, FL		27 120 28 PHONE: (000) 000-0000 29
) ²⁴ 25	ADDRESS:	JOERGENS, LAWRENCE LOT 84 SPARROW TRAIL	2.00 19-24-18 DOMESTIC CITY/STATE: SPRING HILL: FL	002800 63 ZIP: 34610-	PHONE: (000) 000-0000 37
26 27	* * * * * * * * * * * * * * * * * * *	1709 BLUEBIRD LANE	2.00 19-24-18 DOMESTIC CITY/STATE: SPRING HILL, FL	990000 ZIP3 34610-	
29		1709 BLUEBIRD LANE	4.00 19-24-18 DOMESTIC CITY/STATE: SPRING HILL, FL	002364 73 ZIP: 34610-	26 85 27 PHONE: (000) 000-0000 38
调	ADDRE221	CASTLEBERRY, BILLY 123 RAVEN ST	CITY STATE I SPRING HILL . I FL	002800 0557#21P1634610-#1	29 40 PHONE 3% (000) 000-0000 %4 4 4 1
)33	ADDRESS1	PASCO COUNTY PARKS ENREC 16735 CREWS LAKE DRIVE	A.00 19-24-18 PUBLIC SUPPLY	002805 103	PHONE: (000) 000-0000
35 36	ADDRESS:	LORENSTEN. RICHARD 1729-D BLUE BIRD LANE	4.00 19-24-18 DOMESTIC CITY/STATE: SHADY HILLS, FL	002364 44	PHONE: (000) 000-0000 47
		BARTON, MIKE LOT 31 CARDINAL LANE	CITY/STATE; SPRING HILL, FL	0020076	PHONE 1 (000) 000-0000
曾	ADDRESS:	TIMMONS JAMES	2.00 10-24-18 DOMESTIC CITY/STATE: SPRING HILL, FL	002800 84	94 000 000-0000 53
)43				***************************************	33

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WCR050-01

HCR050-01 21:54:58	SOUTHWEST FLORIDA MATER MANAGEMENT DISTRICT MELL CONSTRUCTION PERMITTING	7			04-22-94 PAGE 2
COUNTY: PASCO	WELL PERMITS ISSUED REPORT UP DATE RANGE: 01/01/70 THRU: 04/22/94				
NUMBER ID INFORMATION	MELL LOCATION DIAMETER S-T-R USE CD DESCRIPTION	CONTRACTOR	PRIMARY T	FLESCOPE LINE	9 MELL
307105.01 071286 NO NAME ADDRESS: NO ADDRESS	4.00 30-24-18 DOMESTIC CITY/STATE: NO CITY FL	000065	102	ROM TO FROM	TO DEPTH
307106.01 071285 NO NAME ADDRESS: NO ADDRESS	0.00 30-24-18 DOMESTIC CITY/STATES NO CITY FE	ZIP: 000065	63	PHONE: (000)	-000-0000
308067.01 073948 D E LAMOTTE ADDRESS: NO ADDRESS	3-00 30=24=18 00055776	21P: 000065	40	PHONE: (000)	000-0000
308094.01 073975 C B CRONAN	CITY/STATE: NO CITY, FL	ZIP:		PHONE: (000)	000-0000
308154-01 074035 C A HINES ADDRESS: NO ADDRESS	CLTY/STATET NO CITY, FU ()	ZIP: 000065		PHONE: (000)	000-0000
314989.01 080866 R L VELEZ ADDRESS: NO ADDRESS	4.00 30-24-18 DOMESTIC	71P:		PHONE: (000)	00=0000
331262.01 097118 S STOKES ADDRESS: NO ADDRESS	CITY/STATE: NO CITY, FL	ZIP:		PHONE: (000)	000-0000
358711.01 019199 FELICIANO. AL RE	CHIVASIALES NO CHIV, FL		-	PHONE; (600)	000-0000
ADDRESS: 1045 RIGGINS RO 359346.01 019777 HEAVILIAN, HALT	FR A.		33512-	PHONE: (000)	000-0000
377911.01 034983 FIRST MESI FVAN	MCBRIDE ST CITY/STATE: BROOKSVILLE, FL	2	33512-		000=0000
ADDRESS: SHADY HILLS RO	CITY/STATE: HUDSON, FL	001525 ZIP:	140 33568-	PHONE : (000)	000-0000
ADDRESS: 7809 BLUE BIRD 310719-01 076597 H D JONES	4.00 3C-24-18 DOMESTIC CITY/STATE: BROOKSVILLE, FL	001286 ZIP:	148 -	PHONE: (000)	000-0000
ADDRESSI NO ADDRESS	CITY/STATES NO CITY, FL	ZIPI	- 30 -	PHONE : (000)	000-0000
ADDRESS: NO ADDRESS	4.00 31-24-18 DOMESTIC CLITYSTATE NO CITY FL	000065 ZIP:	50 _	RHONE 1 (000)	147
312718.01 078596 J STALTER ADDRESS: NO ADDRESS	3.00 31-24-18 DOMESTIC CITY/STATE; NO CITY, FL	999998 ZIP:	60 _	PHONE: (000)	150
ADDRESS: NO ADDRESS	3.00 31-24-18 DOMESTIC CITY/STATE! NO CITY, FL	999998 ZIP1	120	PHONE: (000)	1.00
ADDRESS: NO ADDRESS	3:00 31-24-18 DOMESTIC CITY/STATE: NO CITY, FL	999998 ZIP1	<u> </u>		01
14454.01 080331 DR WHITEHUR ADDRESS: NO ADDRESS	J.OO J1-24-18 DOMESTIC CITY/STATE 1 NO CITY, FL	999998 ZIP:	60	PHONE: (000)	108

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MCR050-01 21:54:58		SOUTHWEST FLORIDA MATER MANAGEMENT DISTRICT MELL CONSTRUCTION PERMITTING		04-22-94 PAGE 3
		WELL PERMITS ISSUED REPORT		
COUNTY: PASCO		SE: 01/01/70 THRU 04/22/94	Part of	
NUMBER ID	INFORMATION	DIAMETER S-T-R USE CO DESCRIPTION	CONTRACTOR PRIMARY TELESCO ID DEPTH FROM	DE LINEO MELL To from to depth
314529.01 080406 ADDRESS:	M C JACOBSE NO ADDRESS	4.00 31-24-18 DOMESTIC CITY/STATE: NO CITY. FL	999998 62	120
322762.01 088631		3,00 31-24-18 DOMESTIC	990008 68	NE: (000) 000-0000
323532.01 089400	NO ADDRESS	CITY/STATES ND CITY, FL. 3.00 31-24-18 DOMESTIC	21P3 - PHO	NE: (000) 000-0000
ADDRESS:	NO ADDRESS	CITY/STATE: NO CITY, FL	001056 70 ZIP: - PHO	NE: (000) 000-0000
025097•01 090963 ADDRESSI	S HILL GARA NO ADDRESS	3.00 31-24-14 DOMESTIC	001056 70 ZIP: - PHD	NE; (000) 000-0000
537935.01 103763 Address:		6.00 31-24-18 DOMESTIC CITYSTATE: NO CITY FI	001056 52	100
383620.01 039152	JENNINGS, CURTIS & CHARLISS	4.00 31-24-18 DOMESTIC		4E; (000) 000-0000
	PRICE, GARY	CITY/STATE: LAND O*LAKES. FL		NE: (000) 000-0000
ADDRESS:	LOT 28 MOCKINGBIRD RD	4.00 31-24-18 DOMESTIC CITY/STATE: HUDSON, FL	002871 83 - PHO	180 181 (000) 000-p000
541521.01 199124 ADDRESS:	TERRI BARNICK 17444 CALDWELL DR	4.00 31-24-14 DOMESTIC CITY/STATE: SPRINGHILL, FL		260
			2171 34010- PHUI	NE: (000) 000-0000
			A	
		The Control of the Co		
	<u> </u>			
		18 18 18 18 18 18 18 18 18 18 18 18 18 1		

21:53:44		UTHHEST FLORIDA MATER MANAGEMENT DISTRICT MELL CONSTRUCTION PERMITTING HELL PERMITS ISSUED REPORT				04-22-94 PAGE 2
COUNTYS PASCO	ISSUE DATE RANGE	91/01/70 THRU 04/22/94				
	ER ORMATION		CONTRACTOR	PRIMARY DEPTH	TELESCOPE LINE	A HELL
004913-01 046297 SAM ADDRESS: PO	80X 187	4.00 13-24-17 DOMESTIC CITY/STATE: NEW PORT RICHEY, FL	001232	31 31 31553=		07
05402-01 047831 LOM ADDRESS: RT	RY, JERRY W 1 SHADY HILLS RD	J.00 13-24-17 DOMESTIC CITY/STATE: BROOKSVILLE: FL	001056	hide i deed		100
	ERAL DELIVERY	3.00 13-24-17 DOMESTIC CITY/STATE: HUDSON, FL	001076	127 33568-	PHONE: (000)	132
06367.01 072266 G PI ADDRESS: NO	ADDRESS	CLIVASTATEL NO CLITY FE	000008 ZIP:	A2	PHONE: (000)	115_
06981.01 071286 NO ADDRESS: NO	ADDRESS	1.00 13-24-17 DOMESTIC	999998 21P:	73	PHONE: (000)	13.
07943.01 C73824 C E ADDRESS: NO	ADDRESS	3.00 13-24-17 DOMESTIC CITY/STATE: NO CITY, FL	999998 ZIP:	81 _	PHONE: (000)	107
11208-01 077086 S AC	ADDRESS	CITY/STATE: NO CITY, FL	999998 21P:			
11771.01 077649 A ST ADDRESS: NO	ADDRESS -	CITY/STATE: NO CITY, FL	ODGGGA ZIP:		PHONE: (000)	105
12567-C1 078445 M E ADDRESS: NO A	NDDRESS	4.00 13-24-17 DOMESTIC CLTY/STATE: NO CLTY. FL	999998 719:	60 _	PHONE: (000)	90
13934.01 079811 LARA ADDRESS: NO A	IDDRESS	4.00 13-24-17 DOMESTIC CITY/STATE: NO CITY, FL	999998 ZIP:	91	PHONE: (000)	121
16086-01 081963 P BO ADDRESS: NO A	DDRESS	3.00 13-24-17 DOMESTIC CITY/STATE: NO CITY, FL	999998 ZIP:	84		104
ADDRESS! NO	DORESS	CITY/STATES NO CITY, EL	QQQQQA ZIP1		PHONE: (000)	110
7274-01 083150 R BU ADDRESSI NO A	DDRESS	3.00 13-24-17 DDMESTIC	999998	92	- 112112. (000)	130
20135.01 C74516 P JE ADDRESS: NO A	DDRESS	CITY/STATE: NO CITY, FL	999998 ZIP:	63	PHONE 1 (000)	90
ADDRESS: NO A	DDRESS	3.00 13-24-17 DOMESTIC CITY/STATE! NO CITY, FL	999998 ZIP1	84	PHONE: (000)	110
1200-01 080347 L MC ADDRESS: NO A	DORESS	3.00 13=24=17 DOMESTIC CITY/STATE: NO CITY, FL	999998		PHONE: (000)	
1908.01 087778 C 80 ADDRESS: NO A	HON DDRESS	3.00 13-24-17 DOMESTIC	001037	42	PHONE: (000)	90

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CR050-01 1:53:44	SOUTHWEST FLORIDA WATER MANAGEMENT DISTRIC WELL CONSTRUCTION PERMITTING	T	04-22-94 PAGE 4
	MELL PERMITS ISSUED REPORT		PAUC 4
OUNTY: PASCO ISSUE (DATE RANGE: 01/01/70 THRU 04/22/94		
NOMBER TO INFORMATION	MELL LOCATION DIAMETER S-Y-R USE CD DESCRIPTION	CONTRACTOR PRIMARY TELESCORE ID DEPTH FROM TO FE	LINES MELL
66935.01 025460 GONZALEZ ADDRESS: 2-A SHADEY HILLS RO	STITISTICE BROOKSVILLE FL	001525 52	000 000-000
68740.01 026734 HUMBERTSON, HARVIN Address: 183 Moodlann	CITY/STATE: BROOKSVILLES FO	002159 42	000) 000-0000
70655.01 028163 MORAGUES, CAROLYN ADDRESS: 529 MEMORY LANE	3.00 13-24-17 DOMESTIC CITY/STATE: BROOKSVILLE, FL	001436 63	000) 000-0000
ADDRESS: COT 9 UNIT I HOLIDA		001605 36	
78558.01 035631 LENMON. CHUCK Address: Lot*1087 Hoods Ave	4.00 13-24-17 DOMESTIC	002092 100	000) 000-0000 105
84539.01 019149 HOOBLER, PAUL ADDRESS: 5932 MEADOM DR	4.00 13-24-17 DOMESTIC CITY/STATE: HUDSON, FL	001841 115	<u>000) 000-0000</u>
0501.01 043627 KEPPEL, L E ADDRESS: 116 NICKS DRIVE	4.00 13-24-17 DOMESTIC CITY/STATE: BROOKSVILLE, FL	002364 63	000) 000-0000
2084-01 045236 KERR, KENNETH	CITY/STATE: SPRING HILL, FL	001533 33	000) 000-0000
0944.01 058215 BARADLAY, TIBOR ADDRESS: 295 LITTLE RANCH RO	TO THE ORDER STILLE & FL	002364 84	110
1239.01 067900 C. ALFONSO-J. RODRI Address: 45 Little Ranch Roa	CITY/STATE: BROOKSVILLE, FL	002405 21	000) 000-000
2054-01 068240 JACKSON, BELINDA M. ADDRESS: 1709 NORTH CLUB	4.00 13-24-17 DOMESTIC CITY/STATE: TAMPA, FL	002887 63	200) 000-000
5256-01 039055 RISSER OIL COMPANY ADDRESS: 16021 WALSHINGHAN R	D CITY/STATES INDIAN ROCKS BEACH, FL	001351 86	0001 000-0000
9772-01 114656 FORTNER, RICHARD ADDRESS; 1815 SHADY HILLS RD		002800 63	900 - 000-000 900-000
0329.01 114940 HILDITCH: DEAN ADDRESS: 41 LITTLE RANCH RD		002800 46	000) 000-0000
1058.01 118328 SYONE. CHARLES ADDRESS: 108 GARY RD	CITY/STATE: SPRING HILL, FL	001151 63	00) 000-0000
1016-01 007770 VENICE GARDENS UTIL	ITY 9.00 L3-24-L7 DOMESTIC	002887 77	
ADDRESS: P.O. DRAWER 208	CITY/STATE: VENICE, FL		13) 485-4851

MCR050=01 21:53:44	SOUTHWEST FLORIDA MATER MANAGEMENT DISTRIC MELL CONSTRUCTION PERMITTING	T			04-22-94
	WELL PERMITS ISSUED REPORT				PAGE 3
COUNTY: PASCO ISSUE DAT	E RANGE: 01/01/70 THRU 04/22/94				
NUMBER ID INFORMATION	MELL LOCATION DIAMETER S-T-R USE CO DESCRIPTION	CONTRACTOR ID	PRIMARY DEPTH	/ TELESCOPE IN	FR MELL TO DEPTH
322380.01 088250 ACCENT BLDS ADDRESS: NO ADDRESS	4.00 13-24-17 DOMESTIC CITY/STATE: NO CITY. FL	001056 ZIP:	50	PHONE: (000	120
23280.01 089149 H DAVIDSON ADDRESS: NO ADDRESS	3.00 13-24-17 DOMESTIC CITY/STATE: NO CITY, FL	001094 ZIP:	57		100
29989.01 095851 J SANDIFER ADDRESS: NO ADDRESS	4.00 13-24-17 DOMESTIC CITY/STATE: NO CITY, FL	001696 ZIP:	44		110
30259.01 096120 D E LOCKE ADDRESS: NO ADDRESS	3.00 13-24-17 DOMESTIC CITY/STATE: NO CITY: FL	001121	45	PHONE: (000	68
B31444.01 097300 RUIY ADDRESS: NO ADDRESS	3.00 13-24-17 DOMESTIC CITY/STATE: NO CITY FL	ZIP1 001251 ZIP1			100
35157.01 100993 MCTEAGUE E ADDRESS: NO ADDRESS	4.00 13-24-17 DOMESTIC CITY/STATE: NO CITY, FL	001696	105	1100) 000=0000 135
36473.01 102306 HUGHES ADDRESS: NO ADDRESS	CITY/STATE: NO CITY, FL	21P: 001696	70	PHONE: (000)	100
36474.01 102307 SNOMDEN L ADDRESS: NO ADDRESS	4:00 13-24-17 DOMESTIC CITY/STATE: NO CITY, FL	ZIP:	63	1000	100
38033-01 103861 LANDIG. ROGER Address: 44 Oakdale RD.	4.00 13-24-17 DOMESTIC CITY/STATE: BROOKSVILLE, FL	ZIP: 001699	65	PHONE: (000)	120
39355.01 131429 HUTCHINSON Address: 5316 9th ave drive He		71P: 001696	33512= 52	PHONE: (000)	. 000=0000
40205.01 106007 GARAREK.N ADDRESS: NO ADDRESS	4.00 13-24-17 DOMESTIC	001056	34205-	PHONE: (000)	
44283-01 109822 GRABIL,J	4.00 13-24-17 DOUGGE	ZIP:		PHONE: (000)	000-0000
45002.01 110482 DEHOFF;M A ADDRESS: NO ADDRESS	CITYZETATE: NO CYTY, FE	ZIP:	-	PHONE: (000)	000-0000
45008.01 11C488 MENCHEY, MRS ADDRESS: NO ADDRESS	4.00 13=24=17 DOMESTIC	001696 7191	63	PHONE: (000)	000=0000
48176.01 011202 DOREST UTUE	CITY/STATE: NO CITY, FL	001696 ZIP:	-	PHONE: (000)	000-0000
ADDRESS: 1335 MAGGIE LN 54093-01 015779 LASKER, GENE	CITYSTATET BROOKSVILLE, FL	001696 ZIP:	33512-	PHONE: (000)	000-0000
ADDRESS: 33 LITTLE RANCH SHADY (4313-01 023459 ANTHONY 9. ACLIANO	THE PLANT OF THE P	001037 ZIP:	33512-	PHONE: (000)	000-0000
ADDRESS: SHADY HILLS-LOT 224	4.00 13-24-17 DOMESTIC CITY/STATE: BROOKSVILLE, FL	001817 ZIP:	42 33512-		108

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	SOUTHWEST FLORIDA MATER MANAGEMENT DISTRICT MELL CONSTRUCTION PERMITTING		04-22-94 PAGE 5
DUNTY: PASCO ISSUE DATE RANGE			
	MELL LOCATION DIAMETER S-T-R USE CD DESCRIPTION	CONTRACTOR PRIMARY TELESCOPE ID DEPTH FROM TO FR	LINER HELL
52803.01 123197 RODRIGUEZ, JOSEPH ADDRESS: 45 LITTLE RANCH RD	4.00 13-24-17 DOMESTIC CITY/STATE: REPORKSYILLE, FL	002357 42	75
57047-01 054864 CHRISTOPHER HOMES ADDRESS: 10392 SPRING HIEL DRIVE	2.00 13-24-17 DONESTIC CITY/STATE: SPRING HILL, FL	001036 53	000) 000-0000 000) 000-0000
70993.01 134915 POWERS, JAMES Address: Lot 40 Helen K	4.00 13-24-17 DOMESTIC CITY/STATE: SPRING HILL, FL	002800 63	78 000) 000-0000
75813-01 137421 CORRIVEAU, RAY ADDRESS: LOT 38 HELEN K	CLITY/STATE: SPRING HILL . FLORE	60.280063	000) 000-0000
78376.01 138888 LARSH, ERNIE ADDRESS: LOT 37 HELENK	4.00 13-24-17 DOMESTIC CLIY/STATE: SPRING HILLS FI	002800 84	000) 000=0000 000) 000=0000
84730.01 142657 AKINS, MAYNE ADDRESS: 233 SHADEYHILLS ROAD	4.00 13-24-17 PUBLIC SUPPLY CITY/STATE: SPRING HILL, FL	001699 220	000) 000-0000
ADDRESS: LOT 5 PEACE BLVD	2.00 13-24-17 DOMESTIC CITY/STATE: SPRING HILL, FL	002207 42	000) 000-0000
08087.01 154741 ASSOCIATION OF RETARDED CITE ADDRESS: 39 LITTLE RANCH ROAD	ZEN 4-40 13-24-17 PUBLIC SUPPLY CITY/STATE: SPRING HILL, FL	002207 42 37 43	000) 000-0000
18736-01 167019 ROBERT KAZER ADDRESS: 16421 TIGER TRAIL	2.00 13-24-17 DOMESTIC	009090 47	73
39789-01 197674 STEVE LONG ADDRESS: LOT DO SHADY HILLS RD	A.00 I3-24-17 DOMESTIC CITY/STATE: SPRING HILL, FL	002800 42	990) 000-0000 65
ADDRESS: 15100 HELEN K DRIVE	4.00 13-24-17 DOMESTIC CITY/STATE: SPRING HILL, FL	002263 105	000) 000-0000 155
28413.C1 074294 M RAULERSON ADDRESSI NO ADDRESS	GITY/STATE INO CITY, FE	000012 84	120
9300-01 075180 R E LANE SR ADDRESS: NO ADDRESS	4-00 14-24-17 DOMESTIC	99998 60	100) 000-0000 100
1289.01 077167 A STATLER ADDRESS: NO ADDRESS	3.00 14-24-17 DOMESTIC CITY/STATE: NO CITY, FL	999998 94	10e
2239.01 078117 A FABRIZI ADDRESS: ND ADDRESS	CITYS TATE TAND CITY OF THE	999998 50	00) 000-0000
J096-01 079573 D E ESTOCAD ADDRESS: NO ADDRESS	3.00 1a-2a-17 DOMESTIC CITY/STATE: NO CITY, FL	999998 70	100) 000-0000
4687.01 080564 R JANDZINIS ADDRESS: NO ADDRESS	3.00 14-24-17 DOMESTIC	71P: - PHONE: (0	00) 000-0000

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	CR050-01 1:53;44		SUUTHWEST FLORIDA WATER WELL CONSTRUCTI	R MANAGEMENT DISTRICT				04-22- PAGE	* '
)			HELL PERMITS I	ISSUED REPORT				PAGE	6
<u>14</u> 5	. 330000373 3300300 500000000000000000000	ISSUE DATE: RAN	Company of the Compan						1
)3 N	NCP OMNER ID	OWNER INFORMATION	MELL LOCATION DIAMETER S-T-R	USE CO DESCRIPTION	CONTRACTOR	PRIMARY DEPTH	TELESCORE LI	MED MEL	3
) 6 31	16169.01 076201 ADDRESS:	R HOORE NO ADDRESS	4.00 14-24-17 CITY/STATE: NO CI	DOMESTIC	999998 ZIP:	40		10	00 7
8 31	7355.01 083231 ADDRESS:	H HUTCHINSO NO ADORESS	3.00 14-24-17 City/State: No ci	DOMESTIC	999908 ZIP:	62	PHONE: (00		9
		NO ADDRESS	3.00 14-24-17 CITY/STATE: NO CI	DOMESTIC TY, FL	999998 ZIP:	51	PHONE: (00	1.2	20 13
13		NO ADDRESS	3.00 14-24-17 CITY/STATES NO CI	DOMESTIC TY+ FL	001037 ZIP:	40	PHONE: (00		15
)15 32	1532.01 087404 ADDRESS:	M TRACER ND ADDRESS	3±00 14-24-17 CITY/STATER NO CI	DOMESTIC TYS FL	001037 71P1	44	PHONE: (60)		
18		NO ADDRESS	4.00 14-24-17 CITY/STATE: NO CI	DOMESTIC TY, FL	001436 ZIP:	56	PHONE: (00	•	121
20		NO ADDRESS	3.00 14-24-17 CITY/STATES NO CI	DOMESTIC TY. FL	001037 ZIP:	42 _		9	24
22		NO ADDRESS	4.00 14-24-17 CITY/STATE: NO CI	DOMESTIC TV, FL	001056 ZIP:	61 _		17	27
) 24	17.73 (27.55%) (2.66%) (2.66%)	NO ADDRESS	3.00 14-24-17 CITY/STATE: NO CI	DOMESTIC TY: FL	031037 ZIP:	45 _	PHONE: (000	5	10
27	6933.01 092799 ADDRESS1	NO ADDRESS	3.00 14-24-17 City/States No Ci	DOMESTIC TY: FL	001271 ZIP:	76 _	PHONE: (000	11	33 P\$ 34
29		NO ADDRESS	4.00 14-24-17 CITY/STATE: NO CI	DOMESTIC TV, FL	001056 ZIP:	46 -	PHONE: (000	12	0 36
31		NO ADDRESS	CRTVZETATE: NO.CI	DOMESTIC TV. FU.	001696 ZIP:	A4			39
) ³³ 341	1890.01 107606 ADDRESS1	RILEY.O NO ADDRESS	4.00 14-24-17 CITY/STATES NO CI	DOMESTIC	001696	66	PHONE: (000	10	<u>.</u>
36		NO ADDRESS	2+00 14-24-17 CITY/STATE: NO CI	DOMESTIC TY. FL	001986 ZIP:		PHONE: (000		45
37 38	7171.01 010358 ADDRESS1	LIFFORD ALLEY	CITY/STATE: TAMPA.	DOMESTIC FL	001817	33604-	PHONE: (000	13	18
) 39 34E	8688-01 011637	ATCHAEL TOOHEY		004555	001000	50 33512-		7	<u> </u>
) 42 350	0314.C1 012879 ADDRESS: 1	TICKLE LITTLE RANCH RD	4.00 14-24-17		*****		PHONE: (000	10:	5 5

创作的情况从基础的

21:53:44	SOUTHWEST FLORIDA WATER MANAGEMENT DISTRIC MELL CONSTRUCTION PERMITTING	:T	04-22-94 PAGE 7
COUNTY: PASCO ISSUE DATE RA	MELL PERMITS ISSUED REPORT NEE: 01/01/70 THRU 04/22/94		
	NELL LOCATION DIAMETER S-Y-R USE CD DESCRIPTION	CONTRACTOR PRIMARY IFLESCOPE	LINER MELL
357440.01 018338 JORDAN, ROBERT Address: 546 N Peace Blvd	2.00 14-24-17 DOMESTIC CITY/STATE: BROOKSVILLE, FL	001533 53	FROM TO DEPTH
363007.01 022425 MORLEY, ARNOLD ADDRESS: BOX 24 SOUTH GARY ROAD	4-00 14-24-17 DOMESTIC CITY/STATE: BROOKSVILLE, FL	002159	: (000) 000-0000
368018.01 026158 COTGREAVE, WALTER ADDRESS: 300 TIGER TRAIL	2.00 14-24-17 DOMESTIC CITY/STATE: NEW PORT RICHEY, FL	002177	: (000) 000-0060
368577.01 026587 HANSON, MRS. ADDRESS: PO BOX 164	CITY/STATE: ARIPEKA SECTION	002159 52	: (000) 000-0000
370019.01 027761 BONCZKIENIZ, STAN ADDRESS: JAMES RD NORTH	GITY/STATES NEW PORT RICHEYS FL	001533 64	: (000) 000-0000 71 : (000) 000-0000
375414.01 032447 GELTCH, KEN Address: Tiger Trail	4.00 14-24-17 DOMESTIC CITY/STATE: BROOKSVILLE, FL	001056 76	: (000) 000-0000
377364.01 034272 HENLEY, EDWARD Address: 518 Tiger Trail	4.00 14-24-17 DOMESTIC: CITY/STATE: NEW PORT RICHEY, FL	002177 100	: (000) 008-0000
377365.01 034273 REDDING, HELEN ADDRESS: LOT 1281 PENNY LANE	4.00 14-24-17 DOMESTIC CITY/STATE: NEW PORT RICHEY, FL	992177 55	: (000) 000-0000 : (000) 000-0000
377470.01 034650 TOOLE, DAYLON ADDRESS: LOT 452 AKINS RD	4.00 14-24-17 DDMESTIC CITY/STATE: BROOKSVILLE: FL	001056 52	: (000) 000-0000
TOPMAN ADDRESS: GARY RO	4.00 14-24-17 DOMESTIC CITY/STATE: HUDSON, FL	092075 84	(000) 000-0000
379685-01 036348 MCCOV. EDNA ADDRESS: 421 SOUTH PEACE	4.00 14-24-17 DOMESTIC CITY/STATE: SHADY HILLS. FL	002075 45	(000) 000-0000
381395-01 037590 DEVINCENT, HILLIAM ADDRESS: LOT 481 LITTLE RANCH RD	CITYZSTATE: HUOSON, FL	001525 46	(000) 000-0000
381396.01 037590 DEVINCENT, HILLIAM Address: Lot 481 Little Ranch RD		001525 46	(000) 000=0000 (000) 000=0000
381696.01 037789 MICKLE, JAMES & NORMA ADDRESS: 246 LITTLE RANCH RD	4.00 14-24-17 DOMESTIC CITY/STATE: SPRING HILL, FL	002075 45	(000) 000-0000
388208.01 042116 TIBERII, JOSEPH ADDRESS: 414 SO PEACE BLVO	4.00 14-24-17 DOMESTIC CLTY/STATE: SPRING HILL, FL	002075 45	(000) 000-0000
ADDRESS: LOT 38 MOULTEN ROAD	4:00 14-24-17 DOMESTIC CITY/STATE: HUDSON, FL	001841 122	196
05421-01 054589 TAYLOR, BETTY ADDRESS: PO BOX 434	4.00 14-24-17 DOMESTIC	C01040 98	135

21:53:44	SOUTHWEST FLORIDA MATER MANAGEMENT DISTRICT WELL CONSTRUCTION PERMITTING		04-22-94 PAGE 8
COUNTY: PASCO ISSUE DATE	HELL PERMITS ISSUED REPORT		
MCP OWNER OWNER			
TO THE THE TANK THE	DIAMETER S-T-R USE CD DESCRIPTION	CONTRACTOR PRIMARY TELESCOPE LI ID DEPTH FROM TO FROM	TO DEPTH
406248.01 055333 ANGELSON, JOE ADDRESS: LOT 24 S GARY ROAD	2.00 14-24-17 DOMESTIC CLIY/STATE: SPRING HILL. FL	002092 37 ZIP: 33526=RHDNE: {00	44 2)
407275.01 136461 ALLSTATE HOMES ADDRESS: POST OFFICE BOX 280158	4.00 14-24-17 DOMESTIC CITY/STATE: TAMPA, FL	001056 63 ZIP: 33682-0158 PHONE: (00	170
409345.01 057220 VIRGILIO, VICTOR E ADDRESS: LOT 4860 PEACE BLVD	4.00 14-24-17 DOMESTIC CITY/STATE: BROOKSVILLE, FL	002801 46	0) 000-0000
414674-01 060903 THOMAS. E ADDRESS: LOT 238 GARDY ROAD	4.00 14-24-17 DOMESTIC CITY/STATE: BROOKSVILLE, FL)) 000-0000
415856.01 055333 ANGELSON, JOE ADDRESSI LOT 24 S GARY ROAD	2.00 14-24-17 DOMESTIC	002800 42 ZIPI 33526- PHONE: (000	50
420578.01 C63403 CAMBPELL, PATTY Address: Lot 488 James Street	2.00 14-24-17 DOMESTIC CITY/STATE: HUDSON, FL	001151 47 ZIP: 33568- PHONE: (000	82
A22949-01.064569 HAMKINS, G ADDRESS: 150 JAMES ST	A.00 14-24-17 DOMESTIC CITY/STATES SPRING HILL FL	002836 67 ZIP: 33526- PHONE: (000	AA.
427947-01 066331 JABLONSKI, ZIGGY ADDRESS: 422 AKINS DRIVE	A.00 14-24-17 DOMESTIC CITY/STATE: SPRINGHILL, FL	002302 52 ZIP: 33526- PHONE: (000	
435354.01 069706 VIRGILIO, VICTOR ADDRESS: 538 PEACE BLVD NORTH	2.00 14-24-17 DONESTIC CITY/STATE: SPRINGHILL, FL	002814 71 71P: 33526- PHONE: (000	aı
NAMHON (SNINMAN 000E11 10.01E8E# ST SHAL 851 1823HDDA	4.00 14-24-17 DOMESTIC CITY/STATE: SPRING HILL, FL	002836 45 ZIP: 34610- PHONE: (000	90
442623-01 115942 MCCOY, DENNIS ADDRESS: 421 S PEACE BLVD	4.00 14-24-17 DOMESTIC CITY/STATE: SPRING HILL, FL	002871 78 ZIP: 34610- PHONE: (000	115
473178-01 135976 ARMSTRONG: 5ARL	4400 14-24-17 DOMESTIC CITY/STATES SPRINGCHILL; FLO	002364 42	50
800867.01 140386 PARRISH. JOE ADDRESS: 619 NORTH PEACE	4.00 14-24-17 DOMESTIC	002263 63	145
86615.01 143743 MOORES, DON Address: 150 n gary RD	4.00 14-24-17 DOMESTIC CITY/STATE: SPRING HILL, FL	002800 51	75
91046.01 058215 BARADLAY, TIBOR ADDRESS: 295 LITTLE RANCH ROAD			
192509.01 151853 HUBBELL, HENRY ADDRESS: LOT 31 TIGER TRAIL	CITY/STATE: SPRING HILL, FL	002002 60	
003121.01 152191 BOWDEN, KAREN ADDRESS: 515 BRADY AVE	4.00 14-24-17 DOMESTIC	ZIP: - PHONE: (000 002800 63) 000-0000

21:53:44		THMEST FLORIDA MATER MANAGEMENT DISTRICT MELL CONSTRUCTION PERMITTING			04-22-94 PAGE 9
COUNTY: PASCO	ISSUE DATE RANGE!	MELL PERMITS ISSUED REPORT 01/01/70 THRU:04/22/94			
NUMBER ID I	MNER NFORMATION	MELL LOCATION DIAMETER S-T-R USE CD DESCRIPTION	CONTRACTOR PRIMARY	TELESCORE LINE	D HELL
509882.01 155674 F ADDRESS: 1	LEET FINANCE 4529 PEACE BLVD	4.00 14-24-17 DOMESTIC CITY/STATE: SPRING HILL. FI	002364 42 ZIP: 34610-	FRUM TO FROM	TO DEPTH
ADORESS: 6	41 TOD TRAIL	4.00 14-24-17. DOMESTIC CITY/STATE: GROOKSVILLE, FL	001436 84 ZIP: 34610-		126
	4910 GARSON LP	4.00 14-24-17 DOMESTIC CITY/STATE: SPRING HILL, FL	002800 42 ZIP: 34610-		60
	OT 595 JAMES ST	CLIVASTATES HUDSON SEL	000048 01 ZIP1 34669-	(000)	
WDDME221	ARIA HARKELKA 4510 GLODDNER LANE	2:00 14-24-17 DOMESTIC CITY/STATE1 SPRING HILL: FL	002207 21 ZIP: 14610-		
	4730 GARSON LOOP MRD.	4.00 14-24-17 DOMESTIC CITY/STATE: SHADY HILLS, FL	001817 80 ZIP: 34610-		222
	ANIEL LITTLE SR	2.00 14-24-17 DOMESTIC CITY/STATE! SPRING HILLS FL	002800 42 ZIP: 34610-		50
AUDRESS: 9	DY HARRIS 207 GROUSE WAY	4:00 14-24-17 DOMESTIC CITY/STATE: HUDSON, FL	009090 43 ZIP: 34668-		40
46269.01 203194 J ADDRESS: L	ACK SANDIFFR OT #245 LITTLE RANCH ROAD	4.00 14-24-17 DOMESTIC CITY/STATE: HUDSON, FL	002392 42 ZIP: 34667-	(000)	50
				Leave to the second	
		A CONTRACTOR OF THE PROPERTY O			
1 91 5					

21:54:08	SOUTHWEST FLORIDA MATER MANAGEMENT DISTRICT MELL CONSTRUCTION PERMITTING		-		04-22-94 PAGE 2
COUNTY: PASCO	ISSUE DATE RANGE: 01/01/70 THRU 04/22/04				
MCP OWNER OWNER					
THE STATE OF THE S	DN DIAMETER S-T-R USE CD DESCRIPTION	CONTRACTOR ID	PRIMAR DEPTH	Y TELESCOPE LIN FROM TO FROM	ER MELL
308989.01 071286 NO NAME ADDRESS: NO ADDRES	S 4.00 23-24-17 DOMESTIC CITY/STATE: NO CITY, FL	999998	61		112
310354-01 076233 E POTTER Address: No Addres	4.00 23-24-17 DOMESTIC	999998	31	- PHONE: (000) 000-0000
312338.01 078216 J C FARME	CLITYSTATES NO CITY, FL	ZIP		- PHONE: (000) 000-0000
ADDRESS: NO ADDRES	3.00 23-24-17 DOMESTIC CITY/STATE: NO CITY, FL	999998 ZIP:	40	- PHONE: (000	98
312468.01 078346 R A LONG ADDRESS: NO ADDRES	CITY/STATE: NO CITY: FL	900008	60		113
312598.01 078476 R HARRIS ADDRESS: NO ADDRES	4.00 23-24-17 nomestre	ZIP		PHONEI (000) 000-00ō
313579.01 079457 M J EDWAR	CATTYS TATE: NO. CLTY. FT	999998 710		PHONE: (000	133
ADDRESS: NO ADDRES	CITY/STATE: NO CITY, FL	999998 ZIP:	61 -	PHONE: (000)	97
314619.01 080496 D D CANAL ADDRESS: NO ADDRES	3.00 23-24-17 DOMESTIC CITY/STATE: NO CITY, FE	999998	60		***
315245.01 081122 HILEY KIN ADDRESS: NO ADDRES	4400 23-24-17 DOWESTER	21P;		PHONE: (000)	000-0000
315290.01 081167 OHELAHI	CITY/STATE: NO CITY, FL	ZIP:		PHONE: (000)	210
ADDRESS: NO ADDRES	3.00 23-24-17 DOMESTIC CLIY/STATE: NO CLIY, FL	999998	40 _	PHONE: (000)	74
315971.01 081848 M HOPPER ADDRESS: NO ADDRES	4.00 23-24-17 DDMESTIC CITY/STATE: NO CITY, FL	999998	90		90
317052-01 082928 M G BROCK ADDRESS: NO ADDRES	3.00 23-24-17 DOMESTIC	2101		PHONE: (000)	000-0000
	CLITYSTATE: NO CITY, FL	999998 ZIP:	42 -	PHONE: (000)	000-0000
317431.01 083306 C FREEMAN ADDRES		000002 ZIP1	- 31 _	PHONE: (000)	85
318583.01 084456 M J VESTU ADDRESS: NO ADDRES	0.	999998	28		36
318909-01 084782 J T BICKE		ZIP	-	PHONE : (000)	
ADDRESS: NO ADDRES	CTITYSTATE! NO CITY, FL	999998 ZIP:	60 -	PHONE: (000)	000-0000
ADDRESS I NO ADDRES	CLTY/STATES NO CLTY. FL	999998 ZIP1	81	Dubuga (Assa)	1150
321076.01 086949 J GRANBERS	Y AA00 23-24-17 DOUTETIA	999998		PHONE: (000)	***************************************
321483.01 087356 RAYMON HE	T	ZIPI		PHONE: (000)	000-0000
ADDRESS: NO ADDRESS	4.00 23-24-17 DOMESTIC CLIYYSTATES NO CITY. FL	001037	52	PHONE:(000)	84

ICR050-01 11:54:08	SOUTHWEST FLORIDA WAYER MANAGEMENT DISTRICT MELL CONSTRUCTION PERMITTING	T		04-22-94 PAGE 3
	WELL PERMITS ISSUED REPORT			
OUNTY: PASCO ISSUE DATE	RANGE: 01/01/70 THRU:04/22/94			
MCP OMNER OWNER NUMBER ID INFORMATION	MELL LOCATION	CONTRACTOR DO		
THE CHARLEDIN	DIAMETER S-T-R USE CO DESCRIPTION	ID D	EPTH FROM TO	TIMER MELL FROM TO DEPTH
22070.01 087940 J SEUFERT ADDRESS: NO ADDRESS	4.00 23-24-17 DOMESTIC CITY/STATE: NO CITY. FL	001056 ZIP:	65 - PHONE	: (000) 000-0000
22187.01 075833 J HEMITT ADDRESS: NO ADDRESS	3.00 23-24-17 DOMESTIC	999998	81	
23152.01 089021 D HILL	CITY/STATE: NO CLTY. FL	ZIP:	- PHONE	: (000) 000-0000
ADDRESS: NO ADDRESS	3.00 23-24-17 DOMESTIC CITY/STATE: NO CITY, FL	001056 ZIP:	40	110
23534.01 Q89402 T DIMASCO	4.00 23-24-17 DOMESTIC			: (000) 000-0000
ADDRESS! NO ADDRESS	CLTY/STATES NO CITY, FL.	21P1	AA PHONE	: (000) 000-0000
24297-01 090165 R M KNEES Address: No Address	4.00 23-24-17 DOMESTIC CITY/STATE: NO CITY: FL	001056 ZIP:	92	IAO
24843.01 090710 L C KENNEDY	3.00 23-24-17 DOMESTIC			<u>: (000) 000-0000</u>
ADDRESS: NO ADDRESS	CITY/STATE: NO CITY, FL	001121 ZIP:	100 - PHONE	: (000) 000-0000
24944.DI 0908II H TAYLOR ADDRESSI NO ADDRESS	3.00 23-24-17 DOMESTIC	001037		96
27906.01 093770 K R FRENCH	CITY STATES NO CITY, FL	ZIP:	- PHONE	: (000) 000-0000
ADDRESS: NO ADDRESS	3.00 23-24-17 DOMESTIC CITY/STATE: NO CITY, FL	001121		70
29437.01 095299 C SINGLETON		ZIP:	- PHONE	: (000) 000-0000
ADDRESS: NO ADDRESS	4.00 23-24-17 DOMESTIC CITY/STATE: NO CITY, FL	001696 ZIP:	46	101
30316.01 096177 R STAFFORD ADDRESS: ND ADDRESS	3.00 23-24-17 DOMESTIC			: (000) 000-0000
그는 그 사람들은 사람들은 사람들은 사람들은 기계를 받는 것이 없다.	CITY/STATE: NO CITY, FL	001056 ZIP:		(000) 000-0000
30611.01 096468 C VANCUYK ADDRESS: NO ADDRESS	3.00 23-24-17 DOMESTIC	001056	88	
38982.01 104792 PHILLIPS.J	CITY/STATE: NO CITY, FL	ZIP:		: (000) 000-0000
ADDRESSE NO ADDRESS	CITON 23-24-17 DOMESTIC	001056	40	107
3330.01 030069 LOGAN, R. L.		ZIPI	PHONE	(000) 000-0000
ADDRESS: 11 DAKHOOD DR	4.00 23-24-17 DOMESTIC CITY/STATE: NEW PORT RICHEY, FL	001841 ZIP: 33	63	(000) 000=0000
OPPORT THE TRANSPORT CHARLES STREET ADDRESS: LOT #33 CHARLES STREET	2.00 23m24m17 Douggest		Marie Company	
	CITY/STATE: NEWPORT RICHEY, FL	001904 ZIP: 33	552- PHONE:	(000) 000-0000
ADDRESSI 158 PARKHOOD ST	2.00 23-24-17 DOMESTIC	001094		302
•	CATTOSIAIRE NEW PORT RICHEY, FL	ZIP1 33		(000) 000-0000
ADDRESS! LOT 1661 LARRY RD	2.00 23-24-17 DOMESTIC CITY/STATE: BROOKSVILLE, FL	001904		110
55099-01 016572 CASSER. KEN		ZIP: 33	512- PHONE 1	(000) 000-0000
ADDRESS: PO BOX 10176	2.00 23-24-17 DOMESTIC CITY/STATE: TAMPA, FL	001533	46	(222) 224

MCR050-01 21:54:08	sou	THMEST FLORIDA WATER WELL CONSTRUCTI	MANAGEMENT DISTRICT			•	04-22-94
		MELL PERMITS I					PAGE 4
COUNTY: PASCO	ISSUE DATE RANGEI	11/01/70 THRU 04/22/	94				
J MCP OMNER OHNER		MELL LOCATION DIAMETER S-T-R		CONTRACTOR	PRIMARY T	ELESCOPE LINE	3
5 361535.01 021338 HENDERSC 6 ADDRESS: 89 HOODL	N. JERRY A.	2.00 23-24-17	DOUECTES	ID 001904	DEPIH FI	ROM TO FROM	TO DEPTH 5
361571.01 021357 GOLANDS.	MICK	CITY/STATE: BROOK	SVILLE. FL		50 33512-	PHONE: (000)	-000-0000 F
ADDRESS: 511 E MO	INTANA	2.00 23-24-17 CITY/STATE: NEM P	DOMESTIC ORT RICHEY, FL	001587 ZIP:	33552-	PHONE: (000)	700-2000
IC 362645.01 022143 WOOLIMES ADDRESS: LOT 12 N	ELSON RD	4.00 23-24-17 CITY/STATE: HUDSO	DOMESTIC N. FL	002100	40 33568-		50 13
12 363463.01 022766 CASSUL, 13 ADDRESS: 521 HAYS	KEN RD E BRD AVE	CITY/STATE: PORT	DOMESTIC	001587		PHONE: (000)	000-0000
14 368152.01 026268 DRANKHAL ADDRESS: 4TH AVE	770	4.00 23-24-17	DOMESTICA	ZIP:		PHONE: (000)	000-0000 17
369677.01 027478 KEMMIS.	TIMOTHY 1	CI IVSTATE I NEW P	ORT RICHEY FI	001841 7191	33552 	PHONE: (006)	102 19 000-0000 20
ADDRESS: 21 G DEN	NIS OR	2.00 23-24-17 CITY/STATE: NEW PO	DOMESTIC DRT RICHEY, FL	002100 ZIP:	40 33552-	PHONE: (000)	45 22
19 369678.01 027479 BAUER, G ADDRESS: LOT 6 DE	NNIS DR	2-00 23-24-17 CITY/STATE: HUDSON	DOMESTIC	002100			24
71 372217-01 029347 MEL ACTT	ERHAUF SA BOULEVARD SOUTH	CITY/STATE: HUDSON		ZIP: 		PHONE: (000)	000-0000 26
378212.01 035474 SPARKS.	DAVID C	4.00 23-24-17	DOUGGES		33562-	PHONE: (000)	000-0000 29
ADDRESS: 520 HAYS	1.8.7	CITY/STATE: SPRING	HILL. FL	001056 ZIP:	70 13526-	PHONE: (000)	130 31
DDNESSI 049 PIEC	E BLVD	4.00 23-24-17 CITY/STATE: SHADY	DOMESTIC HILLS, FL	002035 7101	94 33512-	Buours /sss	145
3 390021.01 043470 GLDVER. ADDRESS: 949 JACK	MILDRED IE DRIVE	4.00 23-24-17 CITY/STATE: HUDSON	DOMESTIC	001056	113	PHONE: (000)	36
O 394533.01 046313 MCDANIELS	S. THOMAS			ZIP: _001170	33562-	PHONE: (000)	000-0000
395092.01 047121 11771 #	IAMES S		DOMESTIC RT RICHEY. FL	ZIPI	33553-	PHONE 1: (000)	000-0000 40
ADDRESS: GENERAL C	DELIVERY	J.00 23-24-17 CITYZSTATE: HUDSON	a FL	001649 ZIRI	33568**	PHONE: (000)	99999 43 899999
ADDRESSE LOT 944 F	PETER MAY BOULEVARD	4.00 23-24-17 CITY/STATE: HUDSON	DOMESTIC • FL	002364	72		A1 46
403666.01 053208 DOLCE, AN	ITHONY ROAD	4400 23-24-17	DOMESTIC	002800	33568-	PHONE: (000)	000-0000
9 407482.01 056001 CILIENTO.	777	CITY/STATE: SPRING	Mille FL	ZIPS	33512-	PHONE: (000)	000-0000
407483.01 056001 CH IENTO	MARIO	CTITYSTATE: SPRING	HILL, FL	002800 ZIP:	33526-	PHONE: (000)	000-0000 52)
ADDRESS: 628 HAYS	ROAD	CITY/STATE: SPRING	DOMESTIC HILL FI	002800	84		100

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WCR050-01
                                                            SOUTHWEST FLORIDA MATER MANAGEMENT DISTRICT
 21:54:08
                                                                                                                                                                      04-22-94
                                                                       WELL CONSTRUCTION PERMITTING
                                                                                                                                                                     PAGE
                                                                        MELL PERMITS ISSUED REPORT
 COUNTY: PASCO
                                       ISSUE DATE RANGE: 01/01/70 THRU 04/22/94
  MCP OMNER ONNER NUMBER ID INFORMATION
                                                                  MELL LOCATION
DIAMETER S-T-R
                                                                                                                     CONTRACTOR PRIMARY TELESCORE LINER MELL
ID DEPTH FROM TO FROM TO DEPTH
                                                                                          USE CD DESCRIPTION
 431069.C1 067714 JOHNSTON, GAY L.
ADDRESS: PETERMAX ROAD
                                                                    4.00 23-24-17 DONESTIC
CITY/STATE: HUDSON. FL
                                                                                                                        002836 70
ZIP: 33567-
                                                                                                                                                    PHONE: (000) 000-0000
 434109.01 069151 L.C.KENNEDY
ADDRESS: 14310 TIMOTHY LANE
                                                                   2:00 23-24-17 DOMESTIC
CITY/STATE: NEW PORT RICHEY, FL
                                                                                                                        001151 52
ZIP1 33652-
                                                                                                                                                    PHONE: (000) 000-0000
 438328.01 113957 CILIENTO, MARIO
ADDRESS: 512 HAYS RD
                                                                   2.00 23-24-17 DOMESTIC
CITY/STATE: SPRING HILL FL
                                                                                                                        002800 63
ZIP: 34610-
                                                                                                                                                    PHONE: (000) 000-0000
 438574.01 114087 CALDMELL, IRIS
                                                                   4.00 23-24-17 DOMESTIC
CITY/STATE: BROOKSVILLE, FL
                                                                                                                       ZIP1 33526-
                                                                                                                                                   PHONE: (000) 000-0000
439300:01 114439 BUBENZER, DONNA
ADDRESS: LOT 24 3RD AVE
                                                                     2.00 23-24-17 DOMESTIC
                                                                                                                       001986 81
7191 33552
                                                                                                                                                    PHONE: (000) 000=0000
467803.01 133078 BARNETT.EVERETT ADDRESS: 654 N HAYES RD
                                                                   2.00 23-24-17 DOMESTIC CITY/STATE: SPRING HILL. FL
                                                                                                                       002800 46
ZIP: 34610-
                                                                                                                                                   PHONE: (000) 000-0000
470105.01 134575 BIEGANOWSKI RICHARD
                                                                   2.00 23-24-17 DOMESTIC
                                                                                                                       001151 62
                                                                                                                            ZIP: 34626-
                                                                                                                                                   PHONE: (000) 000-0000
478010-01 138712 UTTER BERNARD N
                                                                   4.00 23-24-17 DOMESTIC
CITY/STATE: SPRING HILL, FL
                                                                                                                        009061_
                                                                                                                              ZIP: 34610-
                                                                                                                                                   PHONE: (000) 000-0000
478011.01 138712 UTTER.BERNARD N
ADDRESS: 523 HAYS RD
                                                                   4.00 23-24-17 DOMESTIC
CITY/STATE: SPRING HILL. FI
                                                                                                                       1009001
                                                                                                                             51 56
21P: 34610-
                                                                                                                                                   PHONE: (000) 000-0000
480835.01 140369 LORENTSEN, K
ADDRESS: TRACT 48 HAYES RD
                                                                   4.00 23-24-17 DOMESTIC
CITY/STATE: SPRING HILL, FL
                                                                                                                       002800 42
ZIP: 34010-
                                                                                                                                                   PHONE: (000) 000-0000
486583.C1 143614 FLYNN.ED
ADDRESS: 15723 JACKIE DR
                                                                   4.00 23-24-17 DOMESTIC
CITY/STATE: HUDSON, FL
                                                                                                                       002263 63
ZIP: 34667-
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7 8 523460.01 171840 ARTHUR JOHNSO 9 ADDRESS: LOT 34 BATTENHODO	N	009048	00) (000-0000
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APPENDIX 7 GEOTECHNICAL INVESTIGATION REPORT

APPENDIX 7

GEOTECHNICAL INVESTIGATION REPORT



March 30, 1994

Mr. Richard Mayer Law Environmental, Inc. 1715 N. Westshore Blvd., Suite 875 Tampa, Florida 33607

Subject:

Results of Geotechnical Exploration

Pasco County Resource Recovery Pasco County, Florida

Law Engineering Project No. 461-06530.27

Dear Mr. Mayer:

Law Engineering is pleased to submit the results of our geotechnical exploration for the subject project. Our services were provided in accordance with the Intercompany Subconsulting Agreement which was signed by Mr. Richard Mayer of Law Environmental and Mr. John Phillips of Law Engineering.

The scope of work for this project was provided by Law Environmental and consisted of the following:

- Set 5 piezometers;
- Drill 5 soil test borings;
- Take 3 undisturbed samples:
- Perform 3 consolidation tests:
- Perform 5 grain size tests.

The geotechnical exploration was performed at the Pasco County Resource Recovery site immediately south of the existing cell A-1.

The borings performed encountered a relatively uniform subsurface profile consisting of 3 strata. Beneath the ground surface, the borings encountered clean fine sands to depths ranging from 6 feet to 20 feet below grade. In boring LB-2, the fine sand was encountered to the termination depth of 26 feet. The sandy soil encountered was very loose to firm with standard penetration resistance values (N-values) ranging from 1 to 22 blows per foot.

The borings then encountered a more cohesive clayey soil ranging in thickness from 2 to 16 feet in thickness. The clay layer was firm to very stiff with N-values ranging from 6 to 23 blows per foot. Three undisturbed samples were obtained from this clay layer and consolidation tests were performed.

In two of the five borings, the limestone formation was encountered at depths of 13.5 and 16 feet below existing grade. The formation consisted of calcareous silty clay with limestone fragments. The limestone encountered was very stiff to very hard with N-values ranging from 29 blows per foot to 50 blows for 3 inches of penetration.

The subsurface conditions encountered are shown on the Test Boring Records in the Appendix. These records represent our interpretation of the subsurface conditions based on the field logs, visual examination of field samples by an engineer, and tests of the field samples. The lines designating the interface between various strata on the Test Boring Records represent the approximate interface location. In addition, the transition between strata may be gradual.

In addition to the soil test borings performed, the following 5 piezometers were installed to the following depths:

Piezometer	Depth
LB-1	14 feet
LB-3	29 feet
LB-5	22 feet
LB-7	14 feet
LB-11	12 feet

At test locations LB-6, LB-9, and LB-12 an auger boring was drilled to 21, 21, and 6 feet, respectively, however, no piezometers were set at these locations.

The results of the three consolidation tests are presented in the Appendix. The samples tested were classified as slightly sandy clay soils. Also, five grain size tests were performed on samples obtained from each soil test boring. As requested, the grain size samples tested were from the surficial sandy stratum. The grain size curves are also presented in the Appendix.

We were informed that the landfill cell would likely be 75 to 100 feet in height. We were also informed that the landfill material weighs about 85 pcf. This corresponds to a stress in the range of 6.4 to 8.5 ksf. Using this stress range together with the soil conditions found in the soil test borings together with the results of our laboratory testing, we calculated that settlement should range from 1.5 to 2.0 feet. Ultimate bearing capacities over 15 ksf should be available at this site.

We wish to remind you that our exploration services include storing the samples collected and making them available for inspection for 60 days. The samples are then discarded unless you request otherwise.

We appreciate the opportunity to work on this project. If you have any questions, please give us a call.

Sincerely,

LAW ENGINEERING, INC.

John C. Phillips, P.E.

Senior Geotechnical Engineer

Registered Florida No. 47586

Curtis J. Roos, P.E.

Chief Engineer

Registered Florida No. 27570

JCP/CJR:sim/1653027.RGE

Distribution: 2 - A

2 - Addressee

Appendix:

Field Exploration Plan

Key to Classifications and Symbols

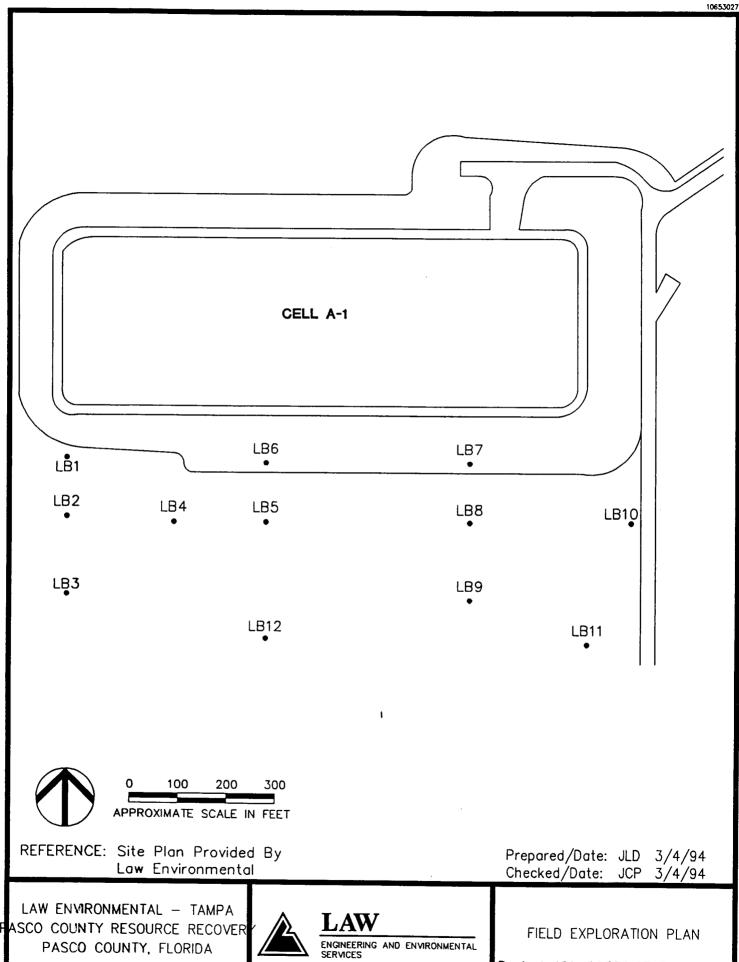
Test Boring Records

Grain Size Curves

Consolidation Test Results Field Testing Procedures

Laboratory Testing Procedures

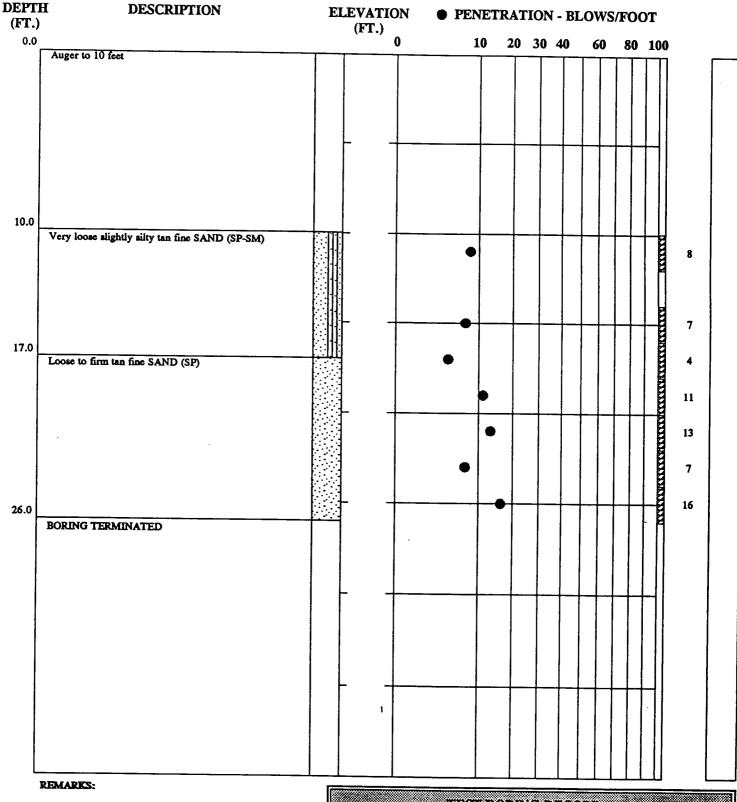
APPENDIX



Project 461-06530.27 Figure

CORRELATION OF PENETRATION RESISTANCE WITH RELATIVE DENSITY AND CONSISTENCY

	NO. OF BLOWS, N	RELATIVE DENS	RELATIVE DENSITY PARTICAL SIZE IDENTIFICATION									
	0-4 5-10	Very Loose	BOULDERS:		LS:	Greater than 300 mm						
SAN	DS: 11-30	Loose Firm		COBBLES: GRAVEL: Coarse - Fine -		75 mm to 300 mm						
1	31-50 OVER 50	Dense Very Dense				19.0 mm (4.75 mm (to 75 mm to 19.0 mm					
		CONSISTENC	Y	SANDS:	Coarse - Medium - Fine -	0.425 mm	to 4.75 mm 1 to 2.00 mm 1 to 0.425 mm					
	0-2	Very Soft		SILTS & C			0.075 mm					
SIL		Šoft Firm										
CLA		Stiff Very stiff										
"-	31-50	Hard										
	OVER 50	Very Hard										
	KEY TO DRILLING SYMBOLS											
. 🔳	Undisturbed Sample	_	Water Table 24	HR.		M=82%	Moisture Content					
₽ 2	Split Spoon Sample	록	Water Table at	Time of Dri	lling	◄	Loss of Drilling Water					
-	KEY TO SOIL CLASSIFICATIONS											
	ASPHALT			MH - High plasticity inorganic silts								
	SP - Poorly graded s		1	ML - Low plasticity inorganic silts								
	SP-SM - Slightly sile	ty sands			OH - High pl	lasticity org	anic silts and clays					
	SM - Silty sands				OL - Low pla	asticity orga	nic silts and clays					
	SP-SC - Slightly clay	yey sands			LIMESTONI imestone	E FORMAT	TON - Weathered					
	SC - Clayey sands			1	LIMESTONI	E FORMAT	TION - Limestone					
	SW - Well graded sa	ands		1 312 312	PEAT - Peat		:					
	CL - Low plasticity	inorganic clays		**	FILL - Sand	fill						
	CH - High plasticity	inorganic clays										



TEST BORING RECORD

BORING NUMBER

LB-2

DATE DRILLED

November 23, 1993

PROJECT NUMBER

461-06530.27

PROJECT

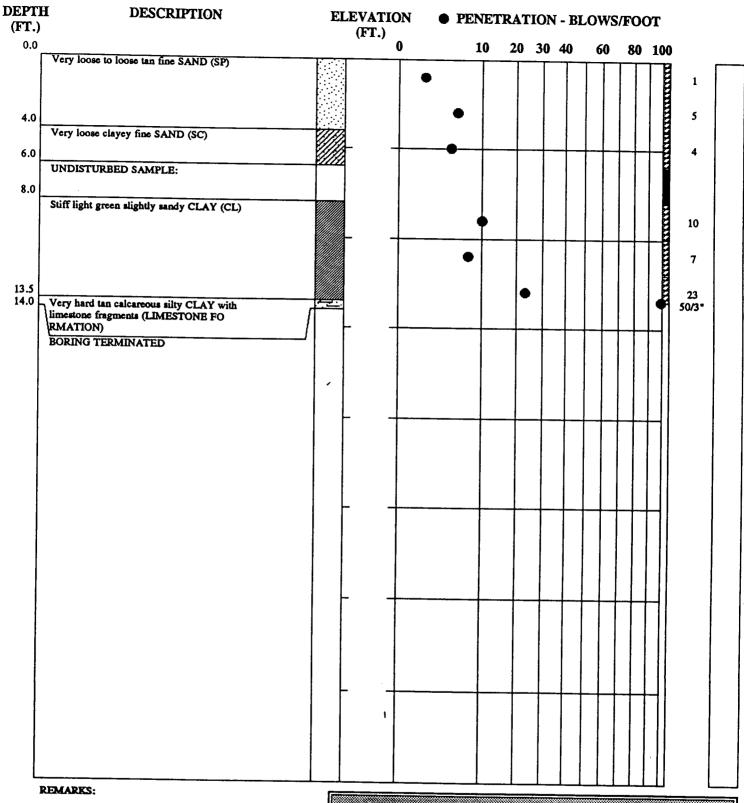
PASCO COUNTY RESOURCE RECOV

PAGE 1 OF 1

SEE KEY SHEET FOR EXPLANATION OF SYMBOLS AND ABBREVIATIONS USED ABOVE



LAW ENGINEERING



TEST BORING RECORD

BORING NUMBER
DATE DRILLED

LB-4

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November 23, 1993

PROJECT NUMBER

461-06530.27

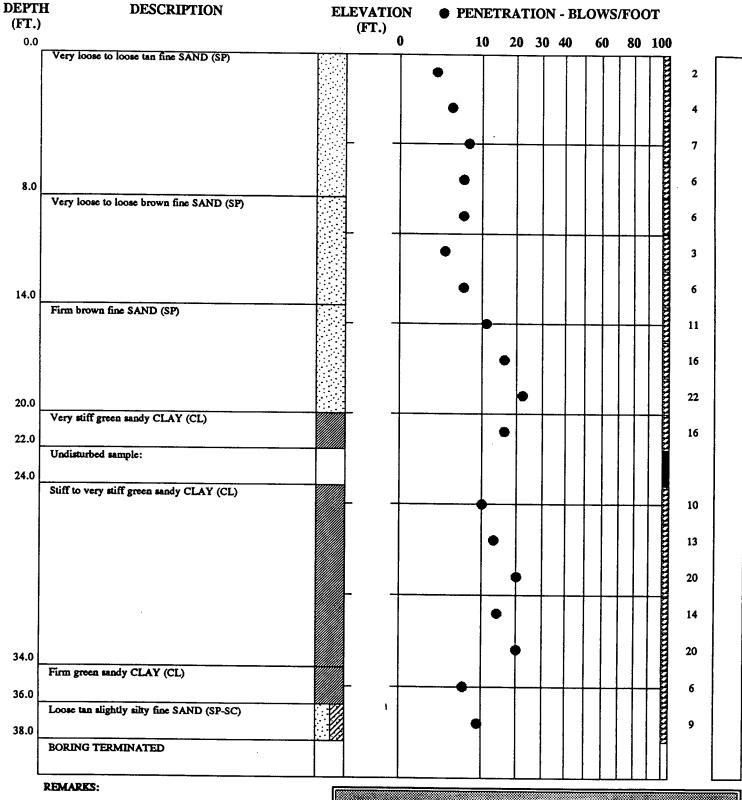
PROJECT
PAGE 1 OF 1

PASCO COUNTY RESOURCE RECOV

SEE KEY SHEET FOR EXPLANATION OF SYMBOLS AND ABBREVIATIONS USED ABOVE



LAW ENGINEERING



SEE KEY SHEET FOR EXPLANATION OF SYMBOLS AND ABBREVIATIONS USED ABOVE

TEST BORING RECORD

LB-5

BORING NUMBER

DATE DRILLED

PROJECT NUMBER

PROJECT

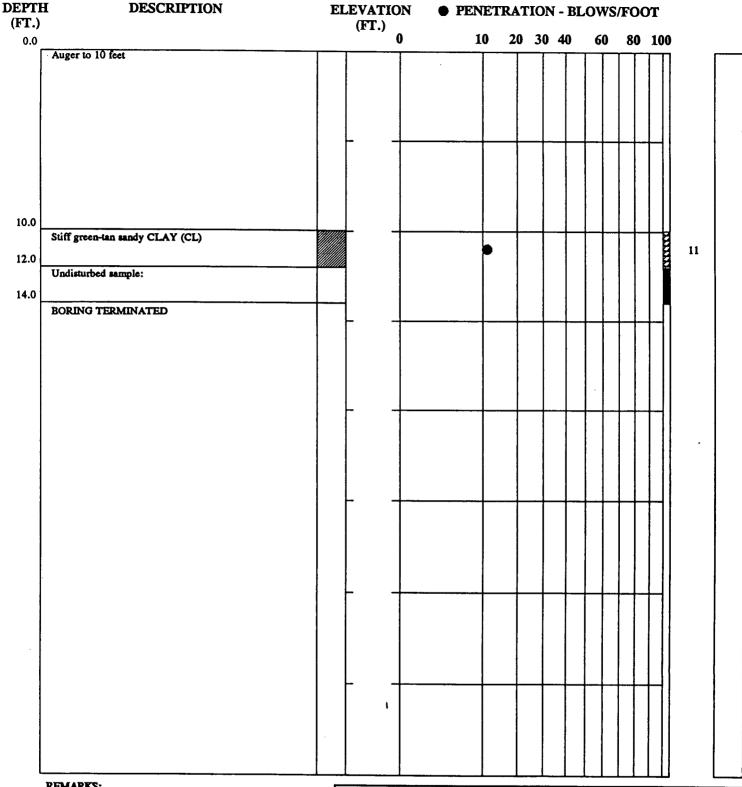
PAGE 1 OF 1

November 22, 1993

461-06530.27

LAW ENGINEERING

PASCO COUNTY RESOURCE RECOV



REMARKS:

TEST BORING RECORD

BORING NUMBER DATE DRILLED

LB-8

November 23, 1993

PROJECT NUMBER

461-06530.27

PROJECT

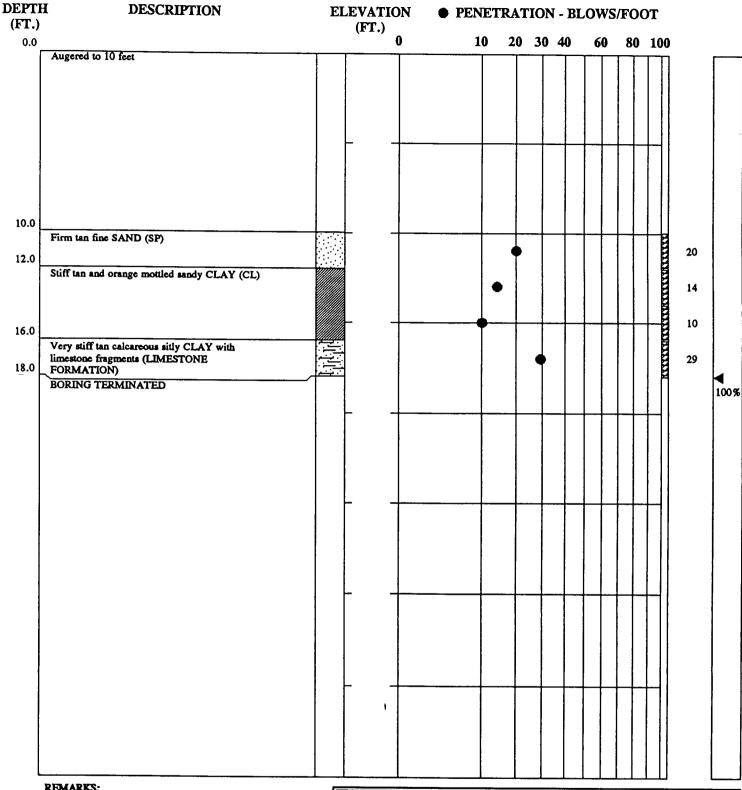
PAGE 1 OF 1

PASCO COUNTY RESOURCE RECOV

SEE KEY SHEET FOR EXPLANATION OF SYMBOLS AND ABBREVIATIONS USED ABOVE



LAW ENGINEERING



REMARKS:

TEST BORING RECORD

BORING NUMBER

LB-10

DATE DRILLED

November 22, 1993

PROJECT NUMBER

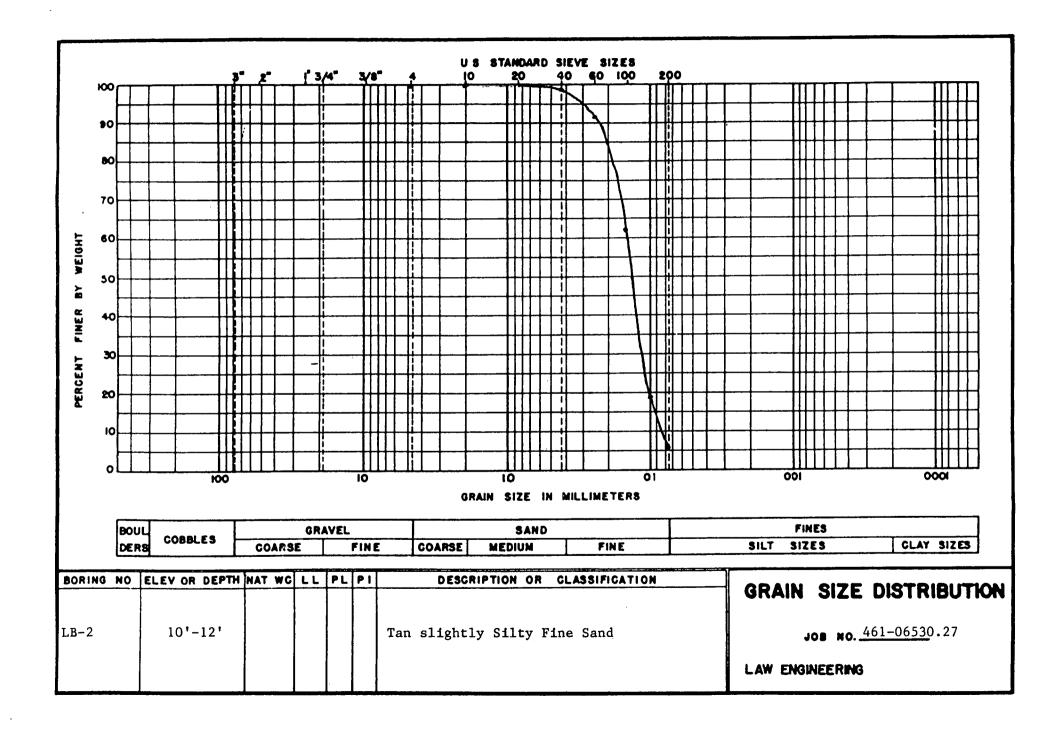
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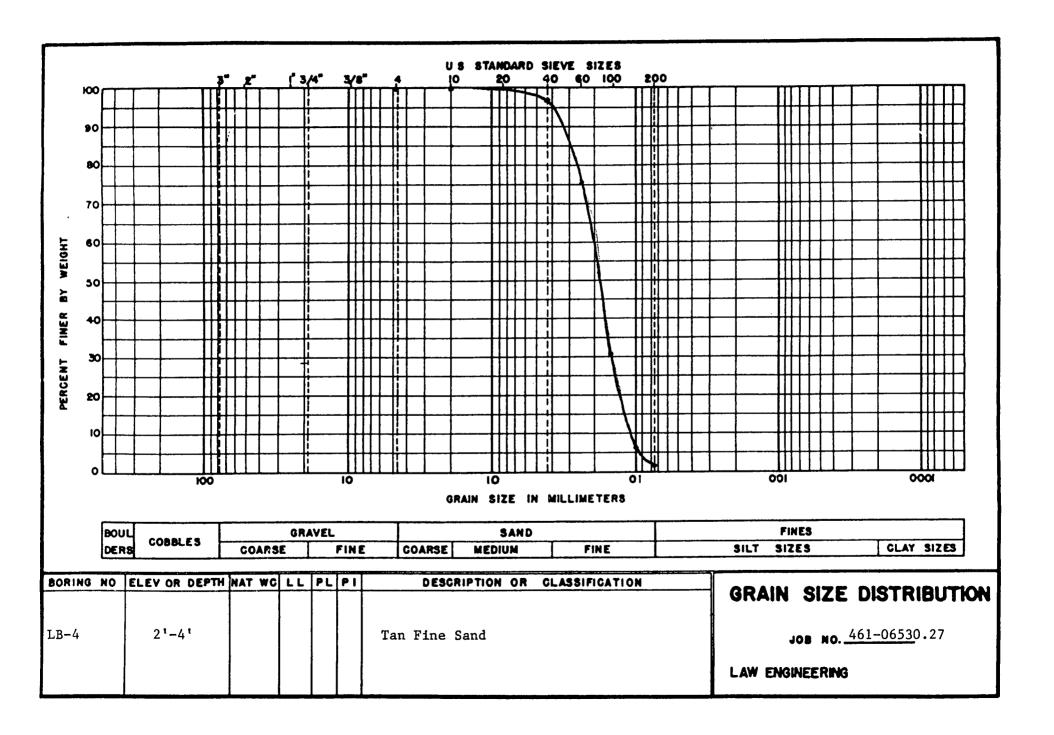
PROJECT PAGE 1 OF 1 PASCO COUNTY RESOURCE RECOV

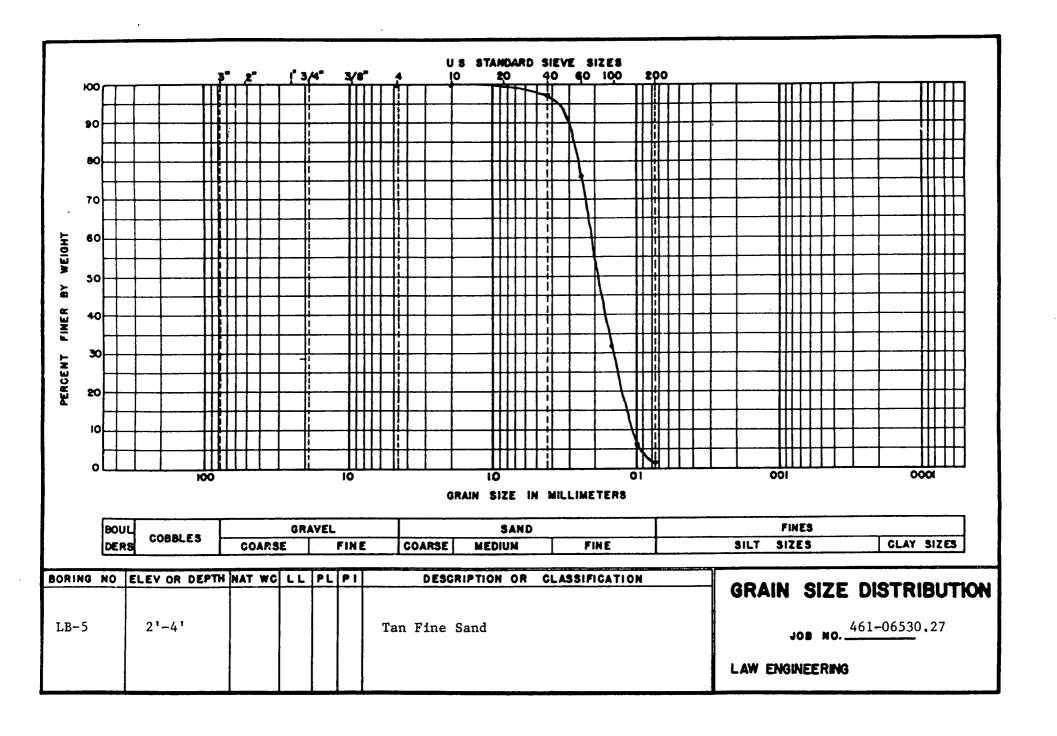
SEE KEY SHEET FOR EXPLANATION OF SYMBOLS AND ABBREVIATIONS USED ABOVE

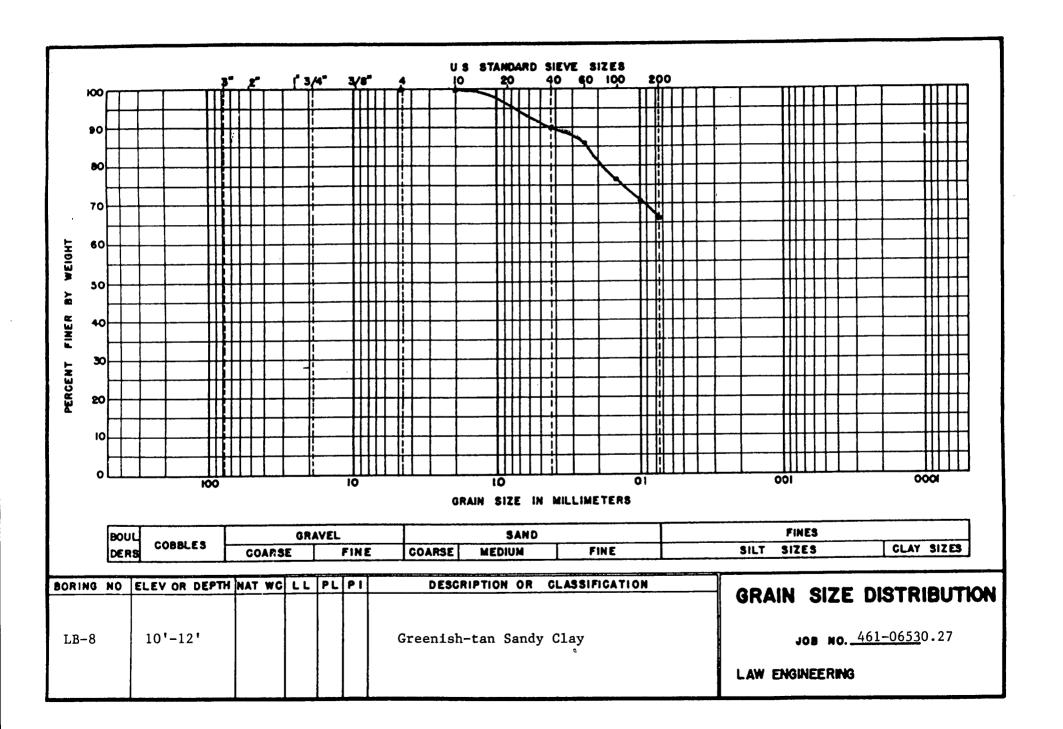


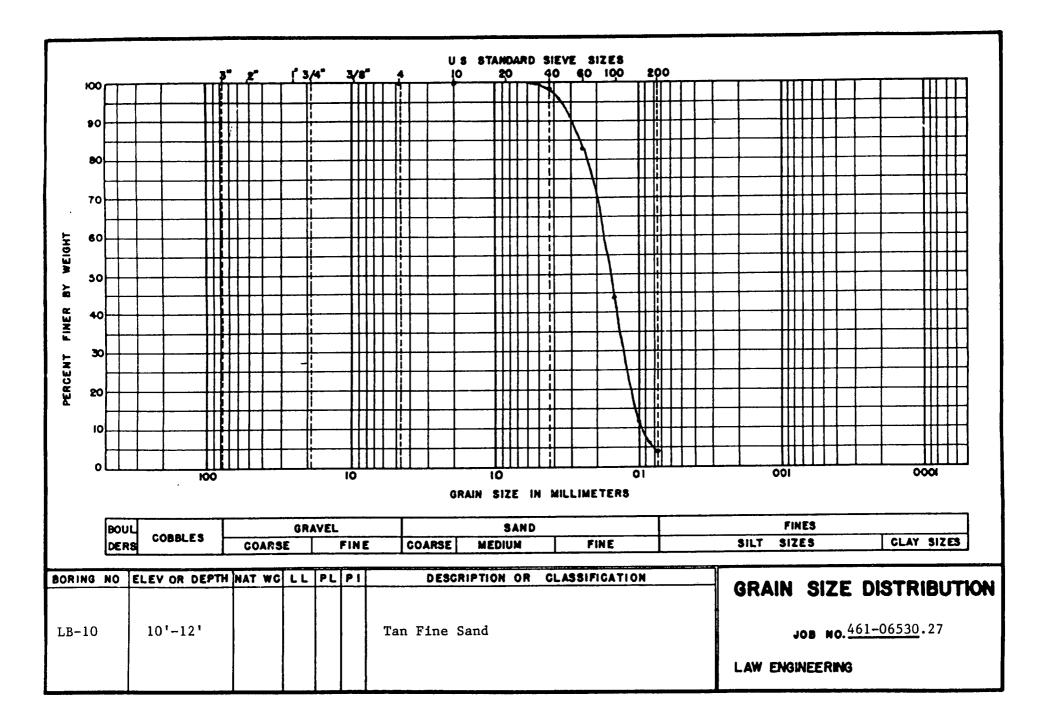
LAW ENGINEERING











LAW ENGINEERING TESTING COMPANY CONSOLIDATION TEST DATA

PROJECT NAME & NO. ARE Misc Testing Tampa Law Engineering BORING NUMBER IS LB-4 SAMPLE IDENTIFICATION IS UD#1 6'- 8'

SPECIFIC GRAVITY = 2.68	INITIAL WET UNIT WT (PCF)	=	113.4
WET SAMPLE WT (GM) = 109.64	INITIAL WATER CONTENT (%)	=	30.5
VOL OF SAMPLE (CC) = 60.33	INITIAL SATURATION (%)	=	88.4
DIA OF SAMPLE (IN) = 2.50	INITIAL VOID RATIO	=	.925
	SAMPLE THICKNESS (IN)	=	.745

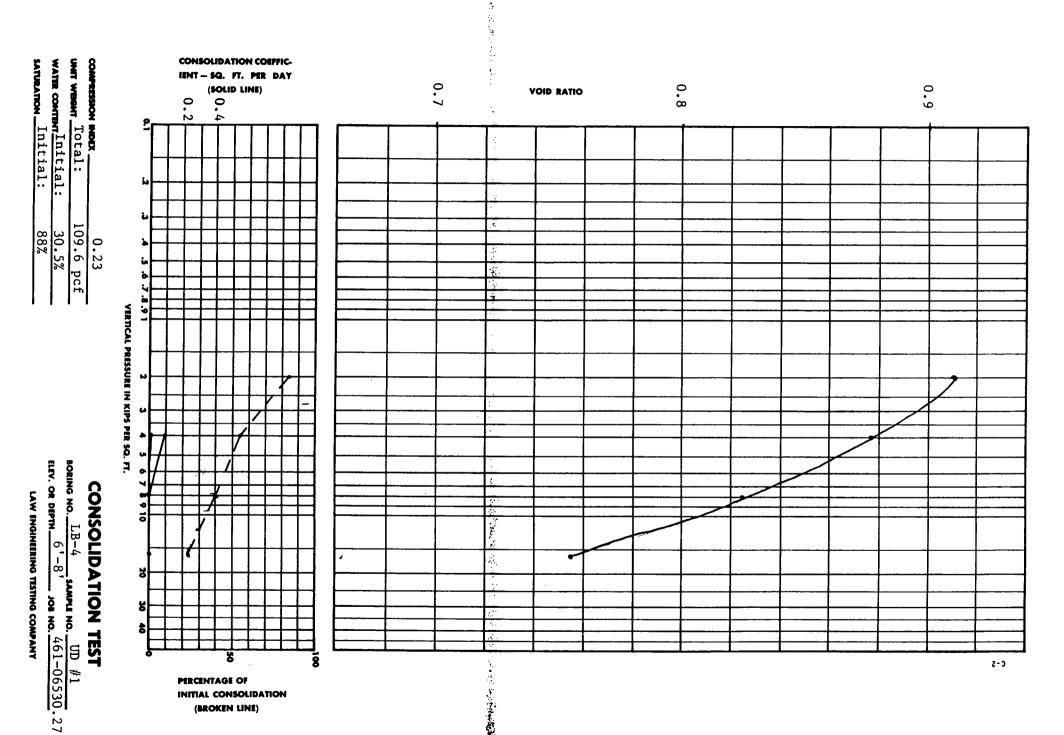
SAMPLE INUNDATED AT 1.0 KSF LOG OF TIME METHOD DOUBLE DRAINAGE NO STONE CORRECTIONS

TEST DATA REDUCTION (INITIAL, PRIMARY & SECONDARY)

					FAU	101	00
16.00	.0468	.0652	.0680	32.00	.6845	22	10
8.00	.0281	.0388	.0415	8.00	.7110	40	12
4.00	.0147	.0202	.0207	3.00	.7318	55	3
2.00	.0067	.0072	.0077	.21	.7448	84	13
KSF	(IN)	(IN)	(IN)	MIN	AT DEND	INT	SEC
STRESS	DO	D100	DEND	T50	SAMPLE HT	×	*

CC
.111
.177
.226

l.



LAW ENGINEERING TESTING COMPANY CONSOLIDATION TEST DATA

PAGE T NAME & NO. ARE Misc Testing Tampa Law Engineering BORING NUMBER IS LB-5

SAMPLE IDENTIFICATION IS UD#1 22'- 24'

SPECIFIC GRAVITY = 2.68 INITIAL WET UNIT WT (PCF) = 106.0

WET SAMPLE WT (GM) = 102.46 INITIAL WATER CONTENT (%) = 41.0

VOL OF SAMPLE (CC) = 60.33 INITIAL SATURATION (%) = 89.7

DIA OF SAMPLE (IN) = 2.50 INITIAL VOID RATIO = 1.225

SAMPLE THICKNESS (IN) = .747

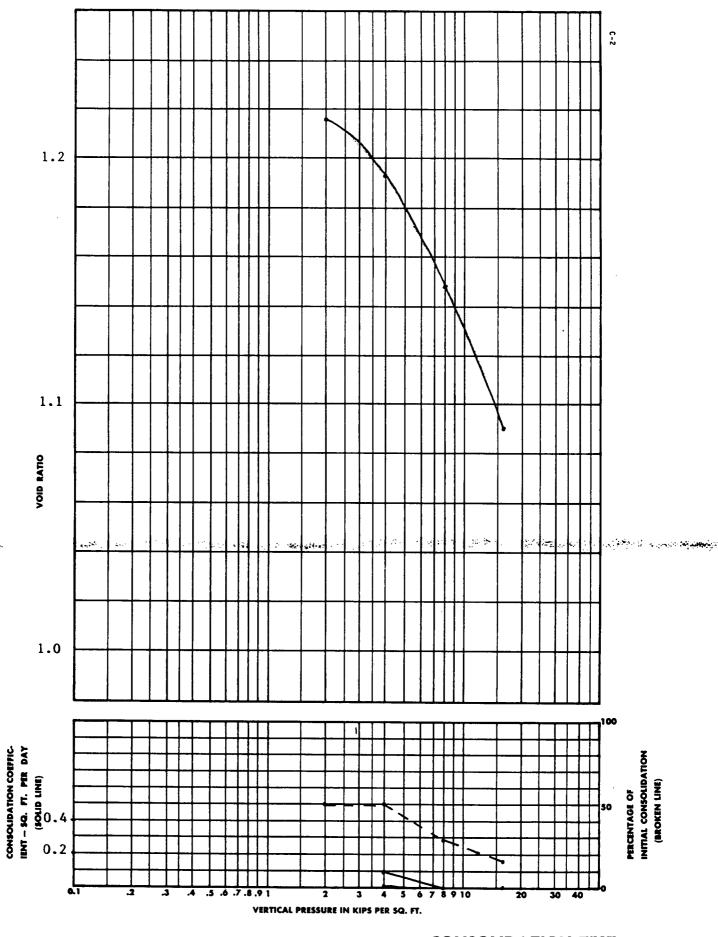
SAMPLE INUNDATED AT 1.0 KSF LOG OF TIME METHOD DOUBLE DRAINAGE

NO STONE CORRECTIONS

TEST DATA REDUCTION (INITIAL, PRIMARY & SECONDARY)

STRESS	DO	D100	DEND	T50	SAMPLE HT	×	*
KSF	(IN)	(IN)	(II)	MIN	AT DEND	INT	SEC
2.00	.0022	.0031	.0036	.60	.7474	50	21 .
4.00	.0077	.0118	.0122	5.00	.7388	50	4
8.00	.0161	.0256	.0269	8.00	.7241	29	8
16.00	.0299	-0450	.0467	16.00	.7043	16	8

STRESS	STRAIN	VOID	* AVG	CV	PERM	ж	CC
kee	X	RATIO	*STRESS	FT2/QAY	FT/DAY	FT2/KIP	
i	.00	1.217	*	•			
4	1.15	1.192	* 3.0	.1	2.09E-05	5.89E-03	.085
8.00	3.12	1.148	* 6.0	.0	1.04E-05	4.94E-03	.145
16.00	5.77	1.090	* 12.0	.0	3.40E-06	3.30E-03	.195



COMPRESSION INDEX	0.20
UNIT WESHT Total:	102.5 pcf
WATER CONTENTINITIAL:	41.0%
SATURATION Initial:	89.7%

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CONSOLIDATION TEST

BORING NO. LB-5 SAMPLE NO. UD #1
ELEV. OR DEPTH 22'-24' JOS NO.461-06530, 27
LAW ENGINEERING TESTING COMPANY

LAW ENGINEERING TESTING COMPANY CONSOLIDATION TEST DATA

PROJECT NAME & NO. ARE Misc Testing Tampa Law Engineering BORING NUMBER IS LB-8 SAMPLE IDENTIFICATION IS UD#1 14'-16'

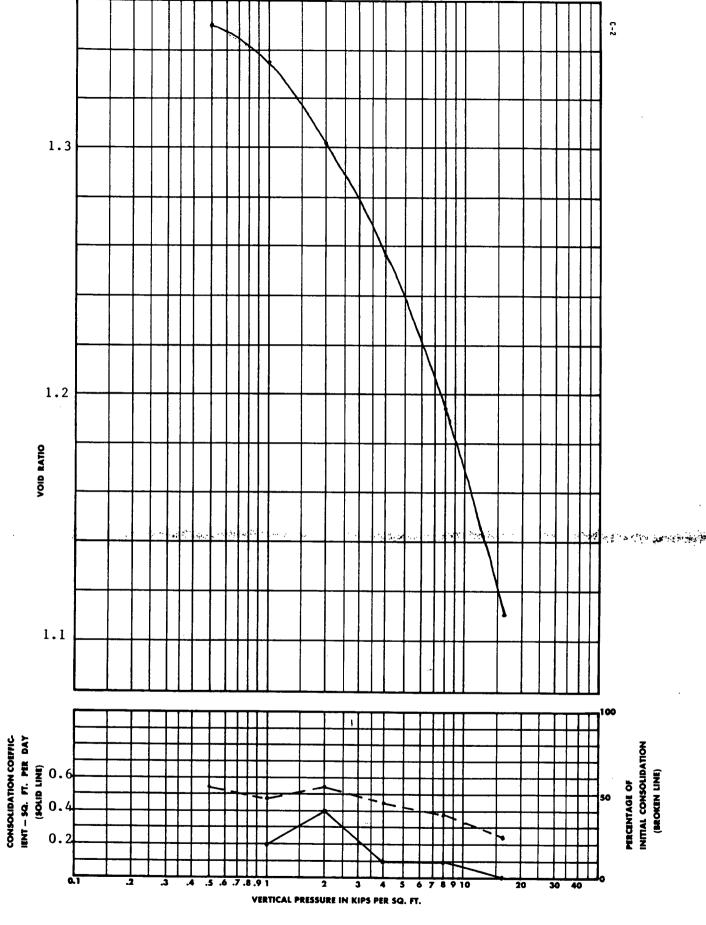
SPECIFIC GRAVITY	= 2.6	INITIAL WET UNIT WT (PCF)	=	103.6
WET SAMPLE WT (GM)	= 100.2	I INITIAL WATER CONTENT (%)	=	46.0
VOL OF SAMPLE (CC)	= 60.3	INITIAL SATURATION (%)	=	90.9
DIA OF SAMPLE (IN)	= 2.5	INITIAL VOID RATIO	=	1.356
		SAMPLE THICKNESS (IN)	=	.747

SAMPLE INUNDATED AT .3 KSF LOG OF TIME METHOD DOUBLE DRAINAGE NO STONE CORRECTIONS

TEST DATA REDUCTION (INITIAL, PRIMARY & SECONDARY)

STRESS	DO	D100	DEND	T50	SAMPLE HT	*	×
KSF	(IN)	(IN)	(IN)	MIN	AT DEND	INT	SEC
.50	.0022	-0036	.0038	.30	.7466	54	6
1.00	.0051	.0065	.0068	1.80	.7436	48	9
2.00	.0116	.0156	.0177	.70	.7327	54	19
4.00	.0237	.0308	.0318	1.80	.7186	45	7
8.00	.0391	.0510	.0520	4.00	.6984	38	. 4
16.00	.0576	.0762	.0782	10.00	.6722	23	7

STRESS	STRAIN	VOID	* AVG	CV	PERM	MV	CC
KSF	X	RATIO	*STRESS	FT2/DAY	FT/DAY	FT2/KIP	
.50	.00	1.345	•				
1.00	.40	1.336	* .8	.2	7.63E-05	8.03E-03	.031
2.00	1.86	1.301	* 1.5	.4	3.50E-04	1.50E-02	.114
4.00	3.75	1.257	* 3.0	.1	8.75E-05	9.45E-03	.147
8.00	6.46	1.194	• 6.0	.1	2.79E-05	6.89E-03	.211
16.00	9.97	1.111	* 12.0	.0	6.90E-06	4.40E-03	.273



COMPRESSION INDEX	0.27
UNIT WEEKT Total:	103.6 pcf
WATER CONTENT_Initial:	46,0%
SATURATION Initial:	90.9%

a the difference

CONSOLIDATION TEST

BORING NO. LB-8	SAMPLE NO. UD #1	
ELEV. OR DEPTH $\frac{14'-1}{}$	6' JOS NO. 461-06530.2	7
LAW ENGINEERING	TESTING COMPANY	

FIELD TESTING PROCEDURES

<u>AUGER DRILLING</u> - The soil test borings were made in general accordance with ASTM Designation D-1586, "Penetration Test and Split-Barrel Sampling of Soils". A hollow stem auger drilling process was used to advance the borings.

At regular intervals, soil samples were obtained with a standard 1.4-inch I.D., 2.0-inch O.D., splittube sampler. The sampler was first seated six inches and then driven an additional foot with blows of a 140 pound hammer falling 30 inches. The number of hammer blows required to drive the sampler the final foot is designated the "Penetration Resistance". The penetration resistance, when properly interpreted, is an index to the soil strength and density.

Representative portions of the soil samples, obtained from the sampler, were placed in glass jars and transported to our laboratory. The samples were then examined by an engineer in order to verify the field classifications.

LABORATORY TESTING PROCEDURES

GRAIN SIZE - The grain size tests were performed to determine the particle size and distribution of the sample tested. The sample was dried, weighed, and washed over a No. 200 mesh sieve. The dried sample was then passed through a standard set of nested sieves to determine the grain size distribution of the soil particles coarser than the No. 200 sieve. This test is similar to that described by ASTM Designation D-422.

<u>CONSOLIDATION TEST</u> - A section of the undisturbed sample was extruded from the sampling tube for consolidation testing. The section was trimmed into a disc 2.4 inches in diameter and 0.7 inches thick. The disc was confined in a stainless steel ring and placed between porous plates. It was then subjected to incrementally increasing vertical loads and resulting deformations measured with a micrometer dial gauge. The test results are presented in the form of a pressure versus void ratio curve on the accompanying Consolidation Test sheet.

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