

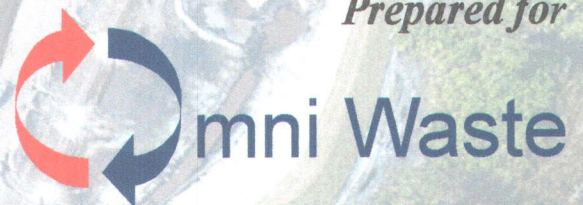


Submitted to:

**Florida Department of
Environmental Protection**

**CERTIFICATION REPORT
CONSTRUCTION OF CELL 1A
AND LEACHATE STORAGE AREA
Oak Hammock Disposal Facility
Osceola County, Florida**

Prepared for

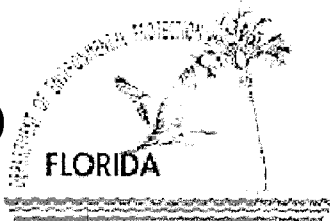


**Omni Waste of Osceola County, LLC
1501 Omni Way
Holopaw, Florida**

GEOSYNTEC CONSULTANTS

**Meredge Drive, Suite 300
Florida 33637**

**Number FX0521
004**



Department of Environmental Protection

Jeb Bush
Governor

Central District
3319 Maguire Boulevard, Suite 232
Orlando, Florida 32803-3767

David B. Struhs
Secretary

ELECTRONIC MAIL

January 21, 2004

Mr. Timothy J. Salopek, President
(tjsomni@aol.com)
Omni Waste of Seminole County, LLC
1501 Omni Way
Holopaw, Florida 34773

OCD-SW-04-0035

Osceola County SW
Oak Hammock Disposal Facility
Certification of Construction Completion

Dear Mr. Salopek:

This will acknowledge receipt of the *Certification of Construction Completion of a Solid Waste Management Facility*, dated, January 18, 2004 by Kenneth W. Cargill, P.E. of GeoSyntec Consultants, addressing Cell 1A.

Based upon my inspection on January 20, 2004, construction of the Cell 1A, as certified by the professional engineer of record, has been completed and is substantially consistent with plans and specifications approved under DEP Permit No. SC49-0199726-001. Accordingly, solid waste may be placed in Cell 1A for disposal upon the Department's acceptance of the certification of construction completion for applicable features of the surface water management system referenced in DEP Permit No. ERP49-199752-001-EI.

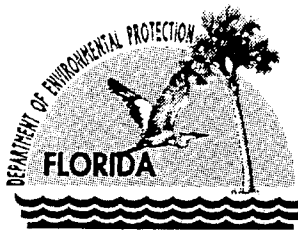
Please contact me at 407-893-3329 if you have questions or need further information.

Sincerely,

James N. Bradner, P.E., Manager
Solid and Hazardous Waste Program

/jnb

cc: David S. Dee, Landers and Parsons (ddee@landersandparsons.com)
Kenneth Cargill, P.E., GeoSyntec Consultants (kcargill@geosyntec.com)
Debra Laisure, P.E., FDEP Environmental Resource Permitting



Jeb Bush
Governor

Department of Environmental Protection

Central District
3319 Maguire Boulevard, Suite 232
Orlando, Florida 32803-3767

David B. Struhs
Secretary

January 23, 2004

Omni Waste of Osceola County, LLC
C/o Mr. Kenneth Cargill, P.E.
GeoSyntec Consultants
14055 Riveredge Drive, Suite 300
Tampa, FL 33637

OCD-ERP-04-0076

Osceola County ERP
Oak Hammock Disposal Facility, ERP49-199752-001-EI
Certification of Construction Completion

Dear Mr. Cargill:

This will acknowledge receipt of the *Environmental Resource Permit As-Built Certification by a Registered Professional*, dated January 19, 2004, addressing construction of the initial phase of the storm water management system at the Oak Hammock Disposal Facility in Osceola County.

Based upon my inspection on January 20, 2004, and as certified by the professional engineer of record, construction of the initial phase of the storm water management system for the facility has been completed and is substantially consistent with plans and specifications approved under DEP Permit No. ERP49-199752-001-EI. This initial phase consists of the storm water management berm along the site access road, around the landfill footprint and around Borrow Area A, the storm water culvert crossings and structures along the site access road, and the emergency spillway on the east side of the landfill footprint.

As discussed during our inspection, as-built drawings will be submitted once the wetlands improvements and installation of the check dams along Bull Creek have been completed.

Please contact me at 407-893-7874 if you have questions or need further information.

Sincerely,

Debra Laisure, P.E.
Engineering Support
Submerged Lands and Environmental
Resources Program

/dl

cc: Lenny Marion, Omni Waste of Osceola County
James N. Bradner, P.E., FDEP Solid and Hazardous Waste Program



GEOSYNTEC CONSULTANTS

14055 Riveredge Drive, Suite 300
Tampa, Florida 33637 • USA
Telephone (813) 558-0990 • Fax (813) 558-9726

19 January 2004

Mr. James N. Bradner, P.E.
Program Manager, Solid/Hazardous Waste
Florida Department of Environmental Protection, Central District
3319 Maguire Boulevard, Suite 232
Orlando, Florida 32803-3767

RECEIVED

JAN 20 2004

**FDEP - CENTRAL DISTRICT
SOLID WASTE**

Re: Certification Report
Oak Hammock Disposal Facility
Omni waste of Osceola County, LLC
Permit Application Nos. SC49-0199726-001

Dear Mr. Bradner:

Submitted herewith are two (2) copies of the certification report (including Record Drawings) for Cell 1A and the initial leachate storage area at the Oak Hammock Disposal Facility in Osceola County, Florida. FDEP form #62-701.900(2) titled *Certification of Construction Completion of a Solid Waste Management Facility*, duly completed and signed, is also attached.

If you have any questions or need additional information, please do not hesitate to contact the undersigned.

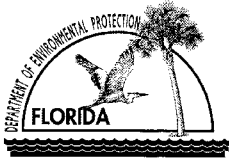
Sincerely,

Kenneth W. Cargill, P.E.
Principal

Attachments

Copy: Mr. Lenny Marion, Omni Waste of Osceola County





Florida Department of Environmental Protection
Twin Towers Office Bldg. • 2600 Blair Stone Road • Tallahassee, FL 32399-2400

DEP Form # 62-701.900(2)
Form Title Certification of Construction Completion
Effective Date May 19, 1994

DEP Application No. _____
(Filled by DEP)

Certification of Construction Completion of a Solid Waste Management Facility

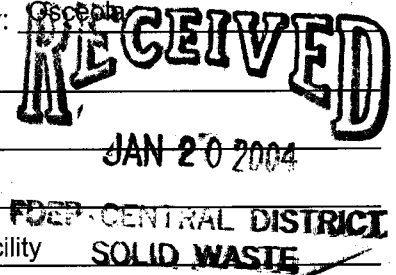
DEP Construction Permit No: SC-49-0199726-001 County: Osceola

Name of Project: Oak Hammock Disposal Facility

Name of Owner: Omni Waste of Osceola County, L.L.C.

Name of Engineer: GeoSyntec Consultants

Type of Project: Initial Construction of a Class I Solid Waste Landfill Facility



Cost: Estimate \$ 6.44 million

Actual \$ 7.75 million

Site Design: Quantity: 1,700 ton/day Site Acreage: Cell 1A, Approximately 12 Acres

Deviations from Plans and Application Approved by DEP: _____

No significant deviations from the approved plans and application.

Address and Telephone No. of Site: 1501 Omni Way, Holopaw, Florida 34773, Phone: (407)891-3720

Name(s) of Site Supervisor: Leonard Marion (Omni)

Date Site inspection is requested: 20 January 2004

This is to certify that, with the exception of any deviation noted above, the construction of the project has been completed in substantial accordance with the plans authorized by Construction

Permit No. SC-49-0199726-001 :Dated: 18 October 2002

Date: 17 January 2004

[Signature]
Signature of Professional Engineer

Page 1 of 1

Submitted to:

**Florida Department of
Environmental Protection**

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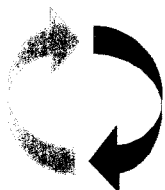
JAN 20 2004

FDEP-CENTRAL DISTRICT
SOLID WASTE



**CERTIFICATION REPORT
CONSTRUCTION OF CELL 1A
AND LEACHATE STORAGE AREA
Oak Hammock Disposal Facility
Osceola County, Florida**

Prepared for



Omni Waste

Omni Waste of Osceola County, LLC
1501 Omni Way
Holopaw, Florida

Prepared by



GEOSYNTEC CONSULTANTS

14055 Riveredge Drive, Suite 300
Tampa, Florida 33637

Project Number FX0521
January 2004

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1. INTRODUCTION

1.1 Overview

This certification report summarizes the Construction Quality Assurance (CQA) activities performed by GeoSyntec Consultants (GeoSyntec) of Tampa, Florida during construction of Cell 1A and ancillary facilities at the Oak Hammock Disposal Facility (OHDF), a Class I landfill, in Osceola County, Florida. The CQA activities performed by GeoSyntec include monitoring of:

- (i) earthwork construction;
- (ii) geosynthetics installation;
- (iii) leachate collection, transmission, and storage systems construction; and
- (iv) ancillary construction associated with development of the landfill.

The CQA activities were performed to confirm that the construction materials and procedures were in compliance with the Construction Permit SC49-0199726-001 issued by the Florida Department of Environmental Protection (FDEP) Central District in accordance with Chapter 62-701, Solid Waste Management Facilities, Florida Administrative Code (FAC).

This certification report was prepared for Messrs. Timothy Salopek and Leonard Marion of Omni Waste of Osceola County, LLC (Omni). It was prepared by Mr. Ayushman Gupta P.E., Mr. Kirk Wills, Dr. Steven Dapp P.E., and Ms. Aidee Cira, and was reviewed by Mr. Ken Cargill, P.E., all of GeoSyntec.

It should be noted that this report covers only Cell 1A and the initial leachate management systems. Cell 1B is currently under construction. The liner system of Cell 1A is continuous through Cell 1B. The liner system in Cell 1 will terminate in an anchor trench in the intercell berms between Cells 1 and 2 on the east side and Cells 1 and 4 on the south side. Cell 1A is separated from Cell 1B by a geomembrane rain flap in order to manage storm water runoff and divert clean storm water from Cell 1B area prior to placing Cell 1B in service for waste deposition.

1.2 Report Organization

This certification report is organized as described below.

- A brief description of the project is provided in Section 2.
- A summary of the CQA program is presented in Section 3.

- A description of the CQA monitoring and testing activities performed during the earthwork related construction activities is provided in Section 4.
- A description of the CQA monitoring and testing activities performed during the geosynthetics installation is provided in Section 5 (including the geomembrane utilized for construction of the flexible leachate storage containers).
- A description of the CQA monitoring and testing activities performed during construction of the leachate management system (i.e., leachate collection, transmission, and storage systems) is provided in Section 6.
- A description of the CQA monitoring and testing performed during the construction of ancillary facilities associated with development of the landfill (i.e., culvert crossings, storm water structures, emergency spillway, utilities, etc.) is provided in Section 7.
- A summary of the observations resulting from the CQA monitoring and testing activities performed by GeoSyntec and a certification statement signed and sealed by a professional engineer registered in the State of Florida are presented in Section 8.

Record drawings presenting as-built surveys for select earthwork activities, primary and secondary liners, and leachate transmission pipe are included in Appendix A. A photographic log of major construction activities is included in Appendix B of this report.

Results of CQA monitoring and testing that are critical with respect to the adequate performance of the OHDF landfill and protection of the surrounding environment have been summarized in a tabular form and are included in this certification report. CQA monitoring and testing results for non-critical activities are not included in this certification report but will be made available upon request.

2. PROJECT DESCRIPTION

2.1 General

The OHDF is located in eastern Osceola County, Florida, west of highway U.S. 441, approximately 6.5 miles south of Holopaw. The landfill facility is connected to highway U.S. 441 by way of a 2.86-mile improved access road, constructed as part of the overall project site development.

The OHDF site comprises a total of approximately 2,179 acres. The landfill footprint at build-out is approximately 264 acres and consists of a total of 21 landfill cells that provide available waste capacity for a period of approximately 30 years. FDEP issued a construct and operate permit for the landfill on 18 October 2002. The first five-year construct and operate permit is referenced as Phase 1 and includes up to four landfill cells (Cells 1 through 4), in the northern part of the landfill at build-out and covering approximately 52 acres. Phase 1 will provide available waste capacity for a period greater than five years based on the projected incoming waste rate of approximately 1,700 tons per day.

The construction of Cell 1A, which is the first half of Cell 1 in the Phase 1 development, has been completed. It is noted that the construction of Cell 1 has been split into a northern half (Cell 1A, now complete) and a southern half (Cell 1B, currently under construction). The liner system of Cell 1A is continuous through Cell 1B. For initial operation, Cells 1A and 1B are separated by a temporary berm and a rain flap. After construction of Cell 1B is completed and Cell 1A is filled to the planned height, the berm and rain flap will be removed, the leachate collection systems will be connected, and the cell will then operate as a single unit (Cell 1).

2.2 Construction Activities

This certification report pertains to CQA monitoring and testing activities only for the construction of Cell 1A and associated facilities indicated in the construction drawings prepared for the initial phase of construction. The initial construction of the OHDF, documented herein, included the construction of:

- landfill perimeter berm;
- storm water management system (along site access road and around landfill footprint and Borrow Area A);
- Cell 1A (including earthwork, liner system, and leachate collection system);
- Cell 1 sump area;
- low-permeability soil layer in sump area of Cells 1 through 6;
- Phase 1 leachate transmission system;
- leachate storage area for Phases 1 and 2;

- site access road, landfill haul road, and cell access road;
- groundwater monitoring wells;
- administration and future use areas;
- site utilities; and
- other ancillary facilities discussed in Section 7.

The construction of the Cell 1A liner system and the leachate management system was the primary focus of the CQA activity, as this was considered critical with respect to the adequate performance of the OHDF landfill and protection of the surrounding environment. The landfill design incorporates a double-composite liner system and other engineering controls that meet or exceed the requirements of Chapter 62-701, FAC. The Cell 1A liner system consists of the following components (from top to bottom):

- minimum 24-in. (610-mm) thick liner protective layer;
- primary geocomposite drainage layer, consisting of a high-density polyethylene (HDPE) geonet with a needle-punched, non-woven geotextile heat bonded to each side, hereafter referred to as primary geocomposite;
- primary liner, consisting of a 60-mil (1.5-mm) thick textured HDPE geomembrane;
- primary geosynthetic clay liner (GCL) consisting of needle-punched reinforced composite composed of granular sodium bentonite encapsulated between non-woven geotextiles (with needle-punched fibers thermally fused to the bottom scrim reinforced non-woven geotextile);
- secondary geocomposite drainage layer, consisting of a HDPE geonet with a needle-punched, non-woven geotextile heat bonded to each side, hereafter referred to as secondary geocomposite;
- secondary liner, consisting of a 60-mil (1.5-mm) thick textured HDPE geomembrane;
- secondary GCL consisting of needle-punched reinforced composite composed of granular sodium bentonite encapsulated between non-woven geotextiles (with needle-punched fibers thermally fused to the bottom scrim reinforced non-woven geotextile); and
- minimum 6-in (152-mm) thick prepared subbase.

The leachate management system is composed of the leachate collection, transmission, and storage systems. The construction of the leachate management system

for Phase 1 development of the OHDF landfill included construction of leachate collection system in Cell 1A, Cell 1 sump, leachate transmission system for Phase 1, and the initial leachate storage area as described below.

Cell 1A Leachate Collection System:

- a 6-in. diameter perforated leachate collection pipe surrounded by gravel aggregate and non-woven geotextile filter fabric, as part of the primary leachate collection system;
- a 6-ft wide secondary geocomposite layer as part of the secondary leachate collection system;

Cell 1 Sump:

- sump gravel beds covered with geotextile separator fabric;
- three 48-in diameter sump risers (2 primary and 1 secondary sump risers);

Phase 1 Leachate Transmission System:

- a 8-inch diameter leachate transmission pipe and manholes from Cell 5 sump area (south of Cell 4) to the leachate storage area (south of Cell 3);

Leachate Storage Area:

- containment area liner system included (from top to bottom) a primary 60-mil thick textured HDPE geomembrane, minimum 2-ft thick liner protective layer, a secondary 60-mil thick textured HDPE geomembrane, and a GCL);
- leak detection and toe drains;
- four flexible storage containers constructed using 60-mil (1.5-mm) thick HDPE geomembrane, which are contained within the double-lined containment area; and
- a leachate truck load out pad with associated leachate transfer pumps, instrumentation, and pipe system.

3. CONSTRUCTION QUALITY ASSURANCE PROGRAM

3.1 General

The scope of CQA monitoring, testing, and documentation services performed by GeoSyntec during the OHDF construction included review of documents, field CQA operations, and preparation of a final certification report and record drawings for the liner system. These activities are described in the following sections of this report.

GeoSyntec provided the CQA monitoring, testing, and documentation as well as the original design and construction drawings. A list of personnel involved in construction of the OHDF is included in Section 3.2 of this report.

GeoSyntec arrived on site to observe the initial earthwork construction associated with the site access road and storm water management berms on 26 June 2003. The earthwork for construction of the landfill perimeter berm and Cell 1A commenced on 18 August 2003. The construction of the leachate storage area started on 24 October 2003. The installation of the liner system in Cell 1A commenced on 16 October 2003. The placement of the liner protective layer in Cell 1A commenced on 17 November 2003. Construction of the Cell 1A and the associated facilities described in this certification report was substantially completed on 19 January 2004.

3.2 Related Documents

As previously noted, this certification report summarizes the CQA activities performed by GeoSyntec during construction of the OHDF. The CQA activities conducted by GeoSyntec were intended to satisfy the requirements of the following documents:

- permit application entitled "*Application for a Permit to Construct and Operate a Class I Landfill*", prepared and submitted by GeoSyntec of Tampa, Florida on 24 May 2002 and approved by the FDEP Central District on 18 October 2002;
- "*Construction Quality Assurance (CQA) Plan*", Appendix Q of the OHDF Permit Application, dated May 2002;
- "*Technical Specifications*", Appendix P of the OHDF Permit Application, dated May 2002;
- permit drawings entitled "*Oak Hammock Disposal, A Solid Waste Facility*", dated May 2002;

- construction drawings entitled “*OHDF Site Access and Storm Water Management*” and Project Manual containing Technical Specifications, dated May 2003, prepared by GeoSyntec of Tampa, Florida; and
- construction drawings entitled “*OHDF Cell 1 and Leachate Systems*” and Project Manuals for Earthwork and Geosynthetics, dated August 2003, prepared by GeoSyntec of Tampa, Florida.

All of the above documents are collectively referred to as the CQA Documents hereafter in this certification report. During construction, minor modifications to include clarifications to the intent of the design and changes to accommodate existing site conditions or preferred construction techniques were made to these documents. However, no substantial changes were made to the CQA Documents.

3.3 Field CQA Operations

The following activities were performed as part of GeoSyntec’s on-site CQA services:

Earthwork:

- collecting samples of soils used as general fill to construct the subgrade, liner subbase, landfill perimeter berms, leachate storage area, access and haul roads, and administration and future uses area building pads for testing at either an on-site or an off-site geotechnical laboratory;
- collecting samples of soils used in the liner protective and low-permeability soil layers for testing at either an on-site or an off-site geotechnical laboratory;
- reviewing and evaluating geotechnical laboratory test results to ensure compliance with the requirements of the CQA Documents;
- monitoring placement, grading, and compaction of earthwork related construction activities (including storm water management berms and related structures);
- testing in-situ density, moisture content, and/or percent compaction of earthwork related construction activities to ensure compliance with the requirements of the CQA Documents;
- notifying Contractor of areas that need additional compaction based on failing in-situ tests and re-testing these areas to ensure compliance with the requirements of the CQA Documents; and

- monitoring anchorage of the geosynthetics in the perimeter anchor trenches.

Geosynthetics:

- monitoring delivery and storage and tracking the inventory of geosynthetic materials delivered for the project;
- coordinating the collection of geosynthetic conformance samples from in-plant sources or delivered rolls and forwarding samples to an off-site geosynthetics testing laboratory;
- collecting and reviewing geosynthetic manufacturers' quality control (MQC) and certification documents and geosynthetic laboratory conformance test results to verify compliance with the requirements of the CQA Documents;
- monitoring installation of geosynthetic materials in Cell 1A and Phase 1 leachate storage area including trial seams, destructive and nondestructive tests, and repair operations; and
- performing destructive testing of geomembrane seams at the minimum frequency required by the CQA Documents.

Leachate Management System:

- collecting samples of aggregate used in the leachate collection system for testing at an off-site geotechnical laboratory;
- reviewing quality control (QC) documents of materials used in the leachate management system, geotechnical laboratory conformance test results on samples of aggregate, and geosynthetic laboratory conformance test results on samples of geotextile filter/separator fabric to verify compliance with the requirements of the CQA Documents;
- monitoring construction of leachate collection system in Cell 1A, Cell 1 sump, leachate transmission system for Phase 1, and leachate storage area; and
- monitoring pressure testing of the leachate transmission line.

Other Construction:

- reviewing QC documents of materials used in the construction of the access and haul roads base course, groundwater monitoring wells, culvert crossings, storm water structures along site access road, drainage storm water structures along Phase 1 perimeter berm, site utilities, and facility fencing;

- monitoring placement, grading, and compaction of access and haul roads base course (limerock) and testing in-situ density and percent compaction to ensure compliance with the requirements of the CQA Documents;
- continuously monitoring installation of the groundwater monitoring wells at the perimeter of Phase 1 development;
- developing and sampling the groundwater monitoring wells and forwarding samples to an analytical testing laboratory;
- periodically monitoring construction of the culvert crossings, storm water structures along site access road, drainage storm water structures along Phase 1 perimeter berm, site utilities, and facility fencing; and
- collecting samples of limerock used in the access and haul road base-course for testing at an off-site geotechnical laboratory.

During construction activities involving monitoring and/or testing, the observations made and results obtained by GeoSyntec CQA personnel were compared with the requirements of the CQA Documents. The construction manager and the appropriate contractor were notified of deficiencies in construction practices and/or materials to ensure appropriate corrective actions are taken. The corrective actions were monitored and/or tested by CQA personnel to ensure compliance with the requirements of the CQA Documents.

3.4 Certification Report and Record Drawings

Record drawings for the Cell 1A earthwork and liner systems and this CQA certification report were prepared as the final task of the CQA program for construction of Cell 1A and associated facilities. This certification report summarizes the CQA monitoring, testing, and documentation activities performed by GeoSyntec.

During construction, CQA monitoring and testing activities were documented by CQA personnel in Daily Field Reports (DFRs) and various other forms. In addition, QC certificates for the geosynthetic and other materials and surveyor's data were provided to GeoSyntec for review. These and other documents are maintained by GeoSyntec and will be made available for FDEP review upon request. Results of CQA monitoring and testing activities that are critical with respect to the adequate performance of the OHDF landfill and protection of the surrounding environment have been summarized in a tabular form and are included in this certification report.

3.5 Project Personnel

Senior personnel or representatives of the firms involved in the project are as follows:

Owner: Omni Waste of Osceola County, LLC

- Tim Salopeck, President
- Lenny Marion, Operations Manager

CQA Consultant: GeoSyntec Consultants - Tampa, Florida

- Ken Cargill, P.E., Project Manager and Certification Engineer
- Ayushman Gupta, P.E., Project Engineer
- Kirk Wills, Field Services Manager
- Richard Chocolas, Site CQA Manager

Earthwork Contractor: Lundquist Excavating, Inc., Kissimmee, Florida

- William Lundquist, Project Manager
- Fred Brunson, Project Manager
- Kenny Jones, Site Superintendent/Surveyor

Earthwork Subcontractor: Bul-Hed, Mulberry, Florida

- Jerry Martin, Project Manager
- Ruben Brock, Site Superintendent

Geosynthetics Installer: Comanco Environmental Corporation, Tampa, Florida

- Jerry Pryor, Project Manager
- Prisciliano Segura, Site Superintendent

Leachate System Contractor Comanco Environmental Corporation, Tampa, Florida

- Steve Kitzmiller, Project Manager
- Mike D'Orazio, Site Superintendent

Surveyor:

Atlantic Land Design, Orlando, Florida

- Gary Burden, P.L.E., Professional Surveyor

Geotechnical Laboratories:

Excel Geotechnical Testing, Roswell, Georgia

- Nader Rad, Ph.D., P.E., Project Manager

Atlantic Testing Laboratories, Inc., Melbourne, Florida

- Donald Tucker Jr., P.E., Project Manager

Geosynthetics Laboratory:

Texas Research Institute, Austin, Texas

- Sam Allen, Project Manager

SGL Testing Services, LLC, Norcross, Georgia

- Rob Swan, Project Manager

4. CONSTRUCTION QUALITY ASSURANCE - EARTHWORK

4.1 General

GeoSyntec monitored the construction of the earthwork related to various components of the OHDF landfill including the landfill perimeter berm; Cell 1A including the intercell berm (between Cells 1A and 2); leachate storage area berms; low-permeability soil layer in sump area of six cells; liner protective layer; building pads; site access road; landfill haul roads (including haul road to leachate storage area and cell access road); storm water management berms (along access road, around the landfill footprint, and around the borrow area); and anchorage of the geosynthetic components of the double-composite liner system. Different materials were used to construct the various components of the landfill. These materials included general fill, low-permeability soil, liner protective layer soil, and stripped materials.

General fill was used to construct the landfill perimeter berm; Cell 1A base, intercell berm, and 6-inch liner subbase; leachate storage area berms; building pads; site access and landfill haul roads embankment and 12-inch subbase; and anchorage of the geosynthetics. The low-permeability soil was used to construct the 2-ft thick compacted layer in sump area of Cells 1 through 6. The liner protective layer soil was used to cover the geosynthetics components of the liner system, construct the drainage soil layer between the primary and secondary liners in the leachate storage area, and to construct the temporary inner cell berm for a rain flap between Cells 1A and 1B. Stripped materials (relatively free of roots) were used to construct the storm water management berms along the site access road, around the landfill footprint, and around Borrow Area A.

CQA personnel observed these earthwork related construction activities and tested the soils to confirm that the material properties conformed to the CQA Documents, specific maximum lift thicknesses were not exceeded, and compaction requirements were met. GeoSyntec personnel also performed geotechnical soil tests during construction. The testing was performed in-situ, in the on-site geotechnical laboratory, or at off-site geotechnical laboratories. The off-site geotechnical laboratories included Atlantic Testing Laboratories, Inc. (ATL) in Melbourne, Florida and Excel Geotechnical Testing (EGT) in Roswell, Georgia.

4.2 Soil Source Sampling Activities

The general fill and liner protective layer soil were obtained from on-site borrow area (Borrow Area A). The low-permeability soil was obtained from an off-site source north of Haines City, Florida. Clayey soils from this off-site source were tested and approved by GeoSyntec prior to borrowing materials for use in the sump areas.

Representative samples of general fill, low-permeability soil, and liner protective layer soil were obtained from their respective sources and tested to verify conformance with specified material requirements in the CQA Documents. The geotechnical tests were performed to confirm that the following requirements were met for the different materials used in construction of the OHDF landfill.

- *General Fill:* classified as SW, SP, or SM in accordance with the Unified Soil Classification System (USCS) per ASTM D 2487 and was relatively free of debris, foreign objects, large rock fragments, organic matter, and other deleterious materials. In addition, general fill used as liner subbase in Cell 1A and the leachate storage area was free of sharp materials or materials larger than 0.5 inches.
- *Low-Permeability Soil:* was fine-grained soil (i.e., at least 50 percent by weight passing U.S. Standard sieve #200 per ASTM D 422); classified as CL or CH in accordance with the USCS per ASTM D 2487; had maximum particle size of 0.75 inches; and was relatively homogeneous soil free of deleterious materials. Regardless of the classification, the compacted low-permeability soil (compacted to at least 95 percent of the standard Proctor maximum dry unit weight) was required to exhibit a maximum hydraulic conductivity of 1.0×10^{-7} centimeters per second (cm/sec) at a confining stress of 20 pounds per square inch (psi) when tested in accordance with ASTM D 5084.
- *Liner Protective Layer Soil:* classified as SW or SP in accordance with the USCS; had maximum particle size of 0.75 inches; had an average fines content of less than 5 percent per ASTM D 1140; had carbonate content of less than 5 percent per ASTM D 4373; and was relatively homogeneous soil free of deleterious materials. Regardless of the classification, liner protective layer soil was required to exhibit a hydraulic conductivity no less than 1.0×10^{-3} cm/sec in accordance with ASTM D 2434.
- *Stripped Materials:* consisted of sandy soils that were relatively free of roots larger than 2 inches in diameter or 24 inches in length.

A description of the geotechnical tests performed on placed materials and results of these tests are presented in the following sections of this report.

4.3 CQA Monitoring and Testing

GeoSyntec's CQA personnel monitored the placement and/or compaction of soils as described in Section 3. At times, several earthwork construction operations were conducted simultaneously. When this occurred, the on-site personnel monitored the operations considered most critical to the performance of the landfill liner system or leachate storage area. Potentially nonconforming or questionable practices observed by CQA personnel were brought to the attention of the concerned parties for review and correction.

As part of CQA activities, geotechnical testing was performed on each of the soils used in the construction of the OHDF landfill. Depending on the specific test, testing was performed in-situ, at the on-site laboratory, or at an off-site laboratory (ATL or EGT).

The following geotechnical tests were performed:

- in-situ nuclear moisture/density tests on compacted lifts of general fill and low-permeability soils (the tests were performed in accordance with ASTM D 2922 for density and ASTM D 3017 for moisture content);
- in-situ density tests using the sand cone method (ASTM D 1556) to compare to the density tests results obtained using the nuclear gauge;
- moisture content tests on general fill and low-permeability soils in accordance with ASTM D 2216;
- standard Proctor compaction tests on general fill and low-permeability soils in accordance with ASTM D 698;
- modified Proctor compaction tests on general fill and limerock (used as base course for the roads) in accordance with ASTM D 1557;
- grain-size analysis or fines content determination in accordance with ASTM D 422, ASTM C 136, or ASTM D 1140, respectively;
- Atterberg limits tests on the low-permeability soil in accordance with ASTM D 4318;
- hydraulic conductivity tests on the liner protective layer soils in accordance with ASTM D 2434 and on low-permeability soil in accordance with ASTM D 5084; and

- interface friction tests for the interfaces between general fill and GCL and between liner protective layer soil and primary geocomposite, as discussed in Section 5.

GeoSyntec supplied two nuclear gauges (Troxler Model #3440 Serial #15334 and Troxler Model #3430 Serial #25657) that were used to perform the moisture/density tests. The gauges were calibrated daily prior to use by the "standard count" method. These counts were recorded on standard count logs, which are not included in the certification report but are available for review upon request. The in-situ density tests using the sand cone method (ASTM D 1556) were performed periodically to compare to the density tests results obtained using the nuclear gauge.

4.4 General Fill

CQA personnel monitored the excavation (from the Borrow Area A), placement, and compaction of general fill, which was used to construct the landfill perimeter berm; Cell 1A base, intercell berm, and 6-inch liner subbase; leachate storage area berms; building pads; site access and landfill haul roads embankment and 12-inch subbase; and anchorage of the geosynthetics. Earthwork using general fill consisted of following activities:

- clearing, grubbing, and/or stripping areas receiving general fill, as needed;
- monitoring existing subgrade by CQA personnel to confirm that unsuitable materials were removed;
- proof rolling of subgrade by the contractor to detect soft or loose zones using scraper pans, articulated dump trucks, or tires of heavy equipment;
- excavating and hauling general fill from on-site source, Borrow Area A, using scraper pans or excavators and articulated dump trucks;
- placing and spreading general fill in relatively thin lifts using scraper pans and bulldozers or articulated dump trucks and bulldozers;
- compacting general fill using scraper pans or smooth drum rollers;
- scarifying surface of each compacted lift using tracks of a bulldozer or tires of heavy equipment, prior to placement and compaction of subsequent lifts; and
- surveying the limits and elevations of the compacted general fill (Record Drawing for compacted general fill in Cell 1A is included in Appendix A).

General fill was typically required to be compacted to at least 95 percent of the corresponding standard Proctor (ASTM D 698) maximum dry unit weight. However, general fill used to construct the embankments for access and haul roads was required to be compacted to at least 95 percent of the corresponding modified Proctor (ASTM D 1557) maximum dry unit weight. The 12-inch subbase for access and haul roads was required to be compacted to at least 100 percent of the corresponding modified Proctor (ASTM D 1557) maximum dry unit weight.

The geotechnical tests performed on compacted general fill materials are discussed below. It is noted that only earthwork related construction activities that are critical with respect to the adequate performance of the OHDF landfill and protection of the surrounding environment (i.e., construction the landfill perimeter berm; Cell 1A base, intercell berm, and 6-inch liner subbase; leachate storage area berms; and anchorage of the geosynthetics) are discussed in detail. These construction activities are hereafter referred to as *critical construction activities*. Construction activities not critical with respect to the adequate performance of the OHDF landfill and protection of the surrounding environment (i.e., construction of building pads, site access road, and landfill haul roads) are discussed only briefly. These construction activities are hereafter referred to as *non-critical construction activities*.

4.4.1 Standard Proctor Tests

Standard Proctor tests were performed to evaluate the percent compaction from the measured in-situ densities of compacted general fill. Standard Proctor tests were required to be performed at a minimum frequency of 1 test per 25,000 cubic yards (cyd) of compacted general fill.

Nineteen standard Proctor tests were performed during construction for approximately 413,000 cyd of compacted general fill placed for critical construction activities. The CQA test frequency of 1 test per 22,000 cyd (approx.) of compacted general fill exceeded the minimum testing frequency required by the CQA Documents. The standard Proctor tests performed during construction are summarized in Table 4-1 and are presented in Figure 4-1. As noted, the maximum dry unit weight varied from 100.1 to 107.5 pounds per cubic foot (pcf) and the optimum moisture content varied from 13.0 to 17.0 percent.

4.4.2 Density and Percent Compaction

In-situ nuclear moisture/density tests were required to be performed at a frequency of 1 test per 200 lineal feet (lf) per lift per lane for the access and haul roads and at a frequency of 5 tests per acre per lift or 1 test per 250 lf per lift for all other earthwork using general fill. A minimum of two in-situ nuclear moisture/density tests were

performed on each day of active earthwork related construction activities. If the density test failed to meet the minimum compaction requirements, the contractor reworked and recompacted the area surrounding the failure and the area was retested by CQA personnel. The procedure was repeated until satisfactory moisture/density test results were obtained at each test location.

A total of 274,000 cyd (approx.) of general fill was placed to construct the 5,800 lf of perimeter berm around Phase 1 of the OHDF landfill. The in-situ nuclear moisture/density tests performed to evaluate the compaction of the perimeter berm are presented in Table 4-2. A total of 1,024 in-situ nuclear moisture/density tests were performed, which correspond to a CQA test frequency of 1 test per 270 cyd (approx.) of compacted general fill. As noted, areas corresponding to the failing test were reworked and recompacted by the contractor and retested by the CQA personnel.

A total of 103,000 cyd (approx.) of general fill was placed to construct the Cell 1A (Cell 1A base, intercell berm between Cells 1A and 2, and the 6-inch liner subbase). The in-situ nuclear moisture/density tests performed to evaluate the compaction of the Cell 1A are presented in Table 4-3. A total of 220 nuclear moisture/density tests were performed, which correspond to a CQA test frequency of 1 test per 470 cyd (approx.) of compacted general fill. As noted, areas corresponding to the failing test were reworked and recompacted by the contractor and retested by the CQA personnel.

A total of 36,000 cyd of general fill was placed to construct the leachate storage area berms. The in-situ nuclear moisture/density tests performed to evaluate the compaction of the leachate storage area berms are presented in Table 4-4. A total of 62 in-situ nuclear moisture/density tests were performed, which correspond to a CQA test frequency of 1 test per 580 cyd (approx.) of compacted general fill.

For the non-critical construction activities, in-situ nuclear moisture/density tests were performed by CQA personnel at the required minimum frequency. The areas corresponding to the failing tests were reworked and recompacted by the contractor and retested by CQA personnel until the minimum compaction requirements were met. As discussed earlier, the moisture/density tests for non-critical construction activities are not included in this certification report but are available for review upon request.

4.4.3 Grain Size Analyses and USCS Classification

Grain-size distribution analyses (ASTM D 422 and ASTM C 136) and fines content determinations (ASTM D 1140) were performed to evaluate the USCS classification (ASTM D 2487) of general fill materials excavated from Borrow Area A. The grain size distribution analyses and USCS classification were required to be performed at a minimum frequency of 1 test per 10,000 cyd of compacted general fill.

Forty two grain-size distribution analyses or fines content determinations and USCS classification were performed during construction for approximately 413,000 cyd of compacted general fill placed for critical construction activities. The CQA test frequency of 1 test per 9,800 cyd (approx.) of compacted general fill exceeded the minimum testing frequency required by the CQA Documents. The grain-size distribution analyses or fines content determinations and USCS classification performed during construction are summarized in Table 4-5. As noted, the general fill materials excavated from Borrow Area A classified as SP, SP-SM, or SM in accordance with the USCS classification.

4.4.4 Sand Cone Tests

In-situ moisture/densities were measured using the sand cone method (ASTM D 1556) periodically to verify the moisture/density tests results obtained using the nuclear gauge. A total of 21 moisture/densities were measured during construction using the sand cone method and the nuclear density gauge and are summarized in Table 4-6. As noted, the densities measured using the two methods were in general agreement.

4.4.5 Anchorage of Geosynthetics

GeoSyntec CQA personnel periodically monitored the method of anchorage for the geosynthetic materials along the perimeter berm (on north and west sides of Cell 1A) and the intercell berm between Cells 1A and 2 (on east side of Cell 1A). Along the south side of Cell 1A, the layers of geosynthetics were extended into the future Cell 1B area. In the anchor trench, geosynthetics were initially ballasted by sand bags placed over top of the last geosynthetic layer deployed. Soil was subsequently placed and compacted in the trench to provide permanent anchorage for the liner system. The construction sequence for the anchor trenches was as follows:

- a 2-ft deep by 2-ft wide (minimum) trench was excavated approximately 2 feet from the inside crest of perimeter berm and 6 feet from the inside crest of intercell berm;
- the geosynthetic components were then placed in and across the bottom of the anchor trench and ballasted with sandbags; and
- lifts of backfill were placed over the geosynthetic materials and compacted.

4.5 Low-Permeability Soil

As part of this construction project, a low-permeability soil layer was constructed in the sump area of Cells 1 through 6. The low-permeability soil layer in the sump areas was over-built and sloped to prevent ponding of water in the sump areas and within the

perimeter berm embankment. As additional cells are constructed during future development of the OHDF landfill, the sumps will be graded to the required shape and thickness. Since Cell 1 was constructed as part of this project, only Cell 1 sump area was graded to the required shape and thickness indicated in the CQA Documents. The construction of the low-permeability soil layer in the sump areas consisted of the following activities:

- low-permeability soils were trucked from the off-site borrow area to the site;
- soils were stockpiled on-site and subsequently hauled to the sump area using articulated dump trucks;
- low-permeability soil was placed in two approximately 1-foot thick lifts using a bulldozer and moisture conditioned, as needed;
- each lift of soil was compacted using a sheepsfoot roller;
- the surface of each compacted lift was typically scarified with the tracks of a bulldozer prior to placement of the subsequent lift;
- a bulldozer was used to grade/shape the over-built low-permeability soil layer;
- the final lift was rolled with a smooth drum roller to seal the top surface; and
- the limits and elevations of the low-permeability soil layer in the sump areas were surveyed.

When the contractor was ready to install the geosynthetics in Cell 1 sump area, the sump area was excavated and graded to the required shape and thickness using excavator, gradall, and hand tools. The final shape and thickness of the low-permeability soil layer were surveyed and are presented in a record drawing in Appendix A. Prior to deployment of the geosynthetics, the compacted low-permeability soil layer surface was visually observed by CQA personnel for surface cracks and oversized particles. If drying or cracking of the surface was observed, the contractor moisture conditioned and reworked the affected area. Any observed oversized particles were manually removed.

The low-permeability soil layer was required to be compacted to at least 95 percent of the corresponding standard Proctor (ASTM D 698) maximum dry unit weight at a moisture content no more than 4 percent wet of the optimum moisture content. However, the primary acceptance criterion was that the compacted low-permeability soil layer exhibit a maximum hydraulic conductivity of 1.0×10^{-7} cm/sec at a confining stress of 20 psi when tested in accordance with ASTM D 5084.

A series of tests were performed on the low-permeability soil layer. These included nuclear moisture/density, fines content, Atterberg limits, hydraulic conductivity, and standard Proctor tests. The nuclear moisture/density tests performed for the low-permeability soil layer in Cells 1 through 6 are summarized in Table 4-7. The percent

compaction indicated in Table 4-7 were computed using the standard Proctor test results. Two standard Proctor tests were performed by EGT for the low-permeability soils and are presented in Figure 4-2. As noted, the test results for moisture content, density, and percent compaction met or exceeded the requirements of the CQA Documents.

Off-site geotechnical laboratory EGT performed the fines content, Atterberg limits, and hydraulic conductivity tests on thin-walled Shelby tube samples of the compacted low-permeability soil layer. One Shelby tube sample was collected and tested from each cell sump area. The laboratory test results for the Shelby tube samples of the low-permeability soils are summarized in Table 4-8. As noted, the measured hydraulic conductivities of low-permeability soil layer ranged from 8.6×10^{-9} cm/sec to 9.2×10^{-8} cm/sec, exceeding the requirements of the CQA Documents.

Fine-grained soils used to construct the 2-ft thick compacted low-permeability soil layer in the sump area of the cells classified as CH, except for the soils used to construct Cell 5 sump area. The low-permeability soils in Cell 5 sump area had a fines content of 49 percent (i.e., slightly less than the required 50 percent) and thus classified as SC. However, these soils were accepted since the measured hydraulic conductivity of the compacted low-permeability soil layer was less than 1.0×10^{-7} cm/sec.

Nuclear density test holes and holes at the Shelby tube sample locations were filled with 10 percent soil-bentonite mixture. The soil-bentonite mixture was placed in the test holes in thin lifts and was manually compacted.

4.6 Liner Protective Layer Soils

Liner protective layer soil was used to cover the geosynthetic components of the liner system in Cell 1A, to construct the soil layer between the primary and secondary liners in the leachate storage area, and to construct the temporary inner cell berm for a rain flap between Cells 1A and 1B. The minimum thickness of the liner protective layer atop the geosynthetic components of the liner system in Cell 1A was 2 feet. The thickness of the soil layer between the primary and secondary liners in the leachate storage area varied from 1 to 3 feet.

Sandy soils from on-site source, Borrow Area A, were used as liner protective layer soil. CQA personnel periodically monitored the placement of the liner protective layer soil in Cell 1A and leachate storage area. The construction sequence of liner protective layer was as follows:

- articulated dump trucks hauled the sandy soils from Borrow Area A to Cell 1A and leachate storage area; and

- the sandy soils were spread and graded using relatively low ground pressure equipment.

During placement of the liner protective layer soils, CQA personnel periodically monitored the contractor's activities to assure that the risk of damage to the underlying geosynthetics was minimized. CQA personnel also confirmed that at least a 2-ft thick layer of sandy soils was maintained over the geosynthetics where the contractor operated the equipment. A minimum of 3-ft thick layer of sandy soils was maintained over the geosynthetics in heavily trafficked areas.

Off-site geotechnical laboratory EGT performed fines content (ASTM D 1140), soil classification in accordance with USCS (ASTM D 2487), and hydraulic conductivity (ASTM D 2434) tests on samples of the liner protective layer soils. Fines content (ASTM D 1140) and soil classification in accordance with USCS (ASTM D 2487) were required at a minimum frequency of 1 test per 2,000 cyd of in-place liner protective layer soils. Hydraulic conductivity tests were required at a minimum frequency of 1 test per 3,000 cyd of in-place liner protective layer soils.

A total of 41,000 cyd (approx.) of liner protective layer soil was placed in Cell 1A. Twenty three fines content (and USCS classification) and 16 hydraulic conductivity tests were performed on the liner protective layer soils from Cell 1A and are presented in Table 4-9. This corresponds to CQA test frequencies of 1 test per 1,800 cyd (approx.) for fines content (and USCS classification) and 1 test per 2,600 cyd (approx.) for hydraulic conductivity, which exceed the minimum testing frequencies required by the CQA Documents.

A total of 3,000 cyd (approx.) of liner protective layer soil was placed in the leachate storage area. Four fines content (and USCS classification) and 2 hydraulic conductivity tests were performed on the liner protective layer soils from the leachate storage area and are also presented in Table 4-9. This corresponds to CQA test frequencies of 1 test per 750 cyd (approx.) for fines content (and USCS classification) and 1 test per 1,500 cyd (approx.) for hydraulic conductivity, which exceed the minimum testing frequencies required by the CQA Documents.

As noted, the measured hydraulic conductivities of liner protective layer soils exceeded the minimum hydraulic conductivity of 1.0×10^{-3} cm/sec required by the CQA Documents. Some of the liner protective layer soil samples had a fines content greater than the required 5 percent and thus classified as SP-SM. However, these soils were accepted since the measured hydraulic conductivity of the liner protective layer soil samples was at least 4 times the required minimum hydraulic conductivity of 1.0×10^{-3} cm/sec.

4.7 Storm Water Berms

Stripped materials (relatively free of roots) were used to construct the storm water management berms along the site access road, around the landfill footprint, and around the borrow area (Borrow Area A). CQA personal periodically monitored the construction of the storm water management berms to ensure that the stripped materials were relatively free of roots and the placed materials were being tracked and reasonably compacted by construction traffic.

Table 4-1

STANDARD PROCTOR TEST RESULTS FOR GENERAL FILL

SAMPLE ID	SOIL DESCRIPTION	OPTIMUM MOISTURE CONTENT (%)	MAXIMUM DRY DENSITY (pcf)
GF-1S	Brown & gray sand with trace of silt and clay	14.0	107.5
GF-2S	Brown & gray sand with trace of silt and clay	13.0	105.8
GF-3S	Dark brown and gray sand with trace of silt and clay	16.2	100.1
GF-4S	Dark brown and gray sand with trace of silt and clay	16.5	101.0
GF-6S	Brown & gray sand with trace of silt	14.3	105.9
GF-7S	Dark brown & gray sand with trace of silt	14.9	103.7
GF-8S	Dark brown & gray sand with silt	15.9	102.9
GF-9S	Dark brownish gray sand with trace of silt	15.8	103.1
GF-10S	Dark brown gray sand with trace of silt	15.7	103.4
GF-11S	Dark brownish gray sand with trace of silt	16.0	102.9
GF-12S	Dark brownish gray sand with trace of silt	17.0	102.4
GF-13S	Dark grayish brown	15.9	101.3
GF-15S	Dark grayish brown	15.5	102.4
GF-16S	Brown sand with trace of silt	16.2	104.7
GF-17S	Brown sand with trace of silt	15.7	103.2
GF-18S	Brown sand with trace of silt	15.1	105.6
GF-19S	Brown sand with trace of silt	15.6	104.5
GF-20S	Dark brown sand with trace of silt	15.1	105.3
GF-21S	Dark brown sand with trace of silt	14.8	106.6

Table 4-2

COMPACTION TEST RESULTS FOR GENERAL FILL USED TO CONSTRUCT PERIMETER BERM

	NUCLEAR GAUGE			APPLICABLE STANDARD PROCTOR				
TEST STANDARD	ASTM D 3017	ASTM D 2922		ASTM D 698				
TESTING FREQUENCY	5 test per acre per lift or 1 test per 250 lf per lift			1 test per 25,000 yd³				
TEST RESULTS								
CQA Test No.¹	Moisture Content (%)	Dry Unit Weight (pcf)	Percent Compaction² (%)	Sample ID	Optimum Moisture Content (%)	Maximum Dry Unit Weight (pcf)	Retest No.³	Pass/Fail (P/F)
LF-204	14.5	103.4	97.6	GF-6S	14.3	105.9		P
LF-205	9.0	106.0	100.1	GF-6S	14.3	105.9		P
LF-206	14.3	101.6	95.9	GF-6S	14.3	105.9		P
LF-207	12.1	103.4	97.6	GF-6S	14.3	105.9		P
LF-208	10.4	104.1	98.3	GF-6S	14.3	105.9		P
LF-209	12.8	101.8	96.1	GF-6S	14.3	105.9		P
LF-210	12.1	103.3	97.5	GF-6S	14.3	105.9		P
LF-211	10.3	101.3	95.7	GF-6S	14.3	105.9		P
LF-212	9.7	101.4	95.8	GF-6S	14.3	105.9		P
LF-223	8.8	107.8	104.8	GF-8S	15.9	102.9		P
LF-224	7.2	102.0	99.1	GF-8S	15.9	102.9		P
LF-225	9.3	102.6	99.7	GF-8S	15.9	102.9		P
LF-226	8.6	103.1	100.2	GF-8S	15.9	102.9		P
LF-227	11.1	102.9	100.0	GF-8S	15.9	102.9		P
LF-228	8.1	103.9	101.0	GF-8S	15.9	102.9		P
LF-229	10.4	103.9	101.0	GF-8S	15.9	102.9		P
LF-230	11.0	103.5	100.6	GF-8S	15.9	102.9		P
LF-231	10.5	104.5	101.6	GF-8S	15.9	102.9		P
LF-232	11.6	104.8	101.8	GF-8S	15.9	102.9		P
LF-233	10.6	105.3	102.3	GF-8S	15.9	102.9		P
LF-234	11.9	103.6	100.7	GF-8S	15.9	102.9		P
LF-235	9.6	104.5	101.6	GF-8S	15.9	102.9		P
LF-236	10.2	104.2	101.3	GF-8S	15.9	102.9		P
LF-237	11.3	103.8	100.9	GF-8S	15.9	102.9		P
LF-238	14.1	100.6	97.8	GF-8S	15.9	102.9		P
LF-239	12.4	104.1	101.2	GF-8S	15.9	102.9		P
LF-240	14.8	103.7	100.8	GF-8S	15.9	102.9		P
LF-241	10.5	103.6	100.7	GF-8S	15.9	102.9		P
LF-242	12.2	104.4	101.5	GF-8S	15.9	102.9		P
LF-243	13.0	105.1	102.1	GF-8S	15.9	102.9		P
LF-244	10.8	101.2	98.3	GF-8S	15.9	102.9		P
LF-245	12.6	101.6	98.7	GF-8S	15.9	102.9		P
LF-246	13.2	100.2	97.4	GF-8S	15.9	102.9		P
LF-247	9.2	100.0	97.2	GF-8S	15.9	102.9		P
LF-248	10.9	101.2	98.3	GF-8S	15.9	102.9		P
LF-249	9.4	103.3	100.4	GF-8S	15.9	102.9		P
LF-250	12.9	102.0	99.1	GF-8S	15.9	102.9		P
LF-251	12.6	100.6	97.8	GF-8S	15.9	102.9		P
LF-252	13.8	100.0	97.2	GF-8S	15.9	102.9		P
LF-265	11.2	102.4	99.5	GF-8S	15.9	102.9		P
Notes:								
¹ Missing test numbers correspond to non-critical construction activities.								
² Required minimum percent compaction was 95% of Standard Proctor (ASTM D 698) maximum dry unit weight.								
³ Retest number for failing density tests.								

Table 4-2 (continued)

COMPACTION TEST RESULTS FOR GENERAL FILL USED TO CONSTRUCT PERIMETER BERM

	NUCLEAR GAUGE			APPLICABLE STANDARD PROCTOR				
TEST STANDARD	ASTM D 3017	ASTM D 2922		ASTM D 698				
TESTING FREQUENCY	5 test per acre per lift or 1 test per 250 lf per lift			1 test per 25,000 yd ³				
TEST RESULTS								
CQA Test No. ¹	Moisture Content (%)	Dry Unit Weight (pcf)	Percent Compaction ² (%)	Sample ID	Optimum Moisture Content (%)	Maximum Dry Unit Weight (pcf)	Retest No. ³	Pass/Fail (P/F)
LF-266	12.9	103.6	100.7	GF-8S	15.9	102.9		P
LF-267	11.2	100.6	97.8	GF-8S	15.9	102.9		P
LF-268	12.4	103.6	100.7	GF-8S	15.9	102.9		P
LF-269	15.7	101.3	98.4	GF-8S	15.9	102.9		P
LF-270	11.6	101.5	98.6	GF-8S	15.9	102.9		P
LF-271	11.9	102.8	99.9	GF-8S	15.9	102.9		P
LF-272	12.3	103.9	101.0	GF-8S	15.9	102.9		P
LF-273	13.0	104.1	101.2	GF-8S	15.9	102.9		P
LF-274	12.4	102.0	99.1	GF-8S	15.9	102.9		P
LF-275	9.6	104.7	101.7	GF-8S	15.9	102.9		P
LF-276	10.3	105.7	102.7	GF-8S	15.9	102.9		P
LF-277	12.6	103.5	100.6	GF-8S	15.9	102.9		P
LF-278	11.8	101.7	98.8	GF-8S	15.9	102.9		P
LF-279	12.3	102.3	99.4	GF-8S	15.9	102.9		P
LF-280	10.1	106.3	103.3	GF-8S	15.9	102.9		P
LF-281	9.6	105.0	102.0	GF-8S	15.9	102.9		P
LF-282	15.1	101.5	98.6	GF-8S	15.9	102.9		P
LF-283	10.1	105.9	102.9	GF-8S	15.9	102.9		P
LF-284	10.6	105.0	102.0	GF-8S	15.9	102.9		P
LF-285	11.7	103.9	101.0	GF-8S	15.9	102.9		P
LF-286	13.7	103.5	100.6	GF-8S	15.9	102.9		P
LF-287	16.0	101.3	98.4	GF-8S	15.9	102.9		P
LF-288	12.2	104.9	101.9	GF-8S	15.9	102.9		P
LF-289	11.8	103.2	100.3	GF-8S	15.9	102.9		P
LF-290	14.7	99.5	96.7	GF-8S	15.9	102.9		P
LF-291	16.6	100.3	97.5	GF-8S	15.9	102.9		P
LF-292	16.9	100.4	97.6	GF-8S	15.9	102.9		P
LF-293	18.5	99.2	96.4	GF-8S	15.9	102.9		P
LF-294	13.5	103.9	101.0	GF-8S	15.9	102.9		P
LF-295	10.5	103.5	100.6	GF-8S	15.9	102.9		P
LF-316	11.7	103.2	100.3	GF-8S	15.9	102.9		P
LF-317	12.5	105.6	102.6	GF-8S	15.9	102.9		P
LF-318	9.1	101.8	98.9	GF-8S	15.9	102.9		P
LF-319	10.7	104.4	101.5	GF-8S	15.9	102.9		P
LF-320	8.6	104.2	101.3	GF-8S	15.9	102.9		P
LF-321	12.3	100.9	98.1	GF-8S	15.9	102.9		P
LF-322	9.0	107.6	104.6	GF-8S	15.9	102.9		P
LF-323	10.5	103.8	100.9	GF-8S	15.9	102.9		P
LF-324	12.1	101.7	98.8	GF-8S	15.9	102.9		P
LF-325	10.3	106.9	103.9	GF-8S	15.9	102.9		P
Notes:								
1 Missing test numbers correspond to non-critical construction activities.								
2 Required minimum percent compaction was 95% of Standard Proctor (ASTM D 698) maximum dry unit weight.								
3 Retest number for failing density tests.								

Table 4-2 (continued)

COMPACTION TEST RESULTS FOR GENERAL FILL USED TO CONSTRUCT PERIMETER BERM

	NUCLEAR GAUGE			APPLICABLE STANDARD PROCTOR				
TEST STANDARD	ASTM D 3017	ASTM D 2922	ASTM D 698					
TESTING FREQUENCY	5 test per acre per lift or 1 test per 250 lf per lift			1 test per 25,000 yd³				
TEST RESULTS								
CQA Test No. ¹	Moisture Content (%)	Dry Unit Weight (pcf)	Percent Compaction ² (%)	Sample ID	Optimum Moisture Content (%)	Maximum Dry Unit Weight (pcf)	Retest No. ³	Pass/Fail (P/F)
LF-326	11.6	104.8	101.8	GF-8S	15.9	102.9		P
LF-327	10.0	103.9	101.0	GF-8S	15.9	102.9		P
LF-328	10.9	104.3	101.4	GF-8S	15.9	102.9		P
LF-329	11.0	104.0	101.1	GF-8S	15.9	102.9		P
LF-330	12.3	103.3	100.4	GF-8S	15.9	102.9		P
LF-331	10.7	103.7	100.8	GF-8S	15.9	102.9		P
LF-332	11.2	104.1	101.2	GF-8S	15.9	102.9		P
LF-333	12.4	105.6	102.6	GF-8S	15.9	102.9		P
LF-334	11.5	103.6	100.7	GF-8S	15.9	102.9		P
LF-335	11.3	104.2	101.3	GF-8S	15.9	102.9		P
LF-346	10.5	106.6	100.7	GF-6S	14.3	105.9		P
LF-347	13.5	105.6	99.7	GF-6S	14.3	105.9		P
LF-348	12.1	107.6	101.6	GF-6S	14.3	105.9		P
LF-349	11.8	105.7	99.8	GF-6S	14.3	105.9		P
LF-350	9.2	102.5	96.8	GF-6S	14.3	105.9		P
LF-351	11.3	103.4	97.6	GF-6S	14.3	105.9		P
LF-352	11.8	106.1	100.2	GF-6S	14.3	105.9		P
LF-353	16.8	100.7	95.1	GF-6S	14.3	105.9		P
LF-354	16.8	100.7	95.1	GF-6S	14.3	105.9		P
LF-355	13.2	103.3	97.5	GF-6S	14.3	105.9		P
LF-356	12.0	103.4	97.6	GF-6S	14.3	105.9		P
LF-357	10.7	100.8	95.2	GF-6S	14.3	105.9		P
LF-358	9.0	105.7	99.8	GF-6S	14.3	105.9		P
LF-359	10.1	103.7	97.9	GF-6S	14.3	105.9		P
LF-360	10.3	103.8	98.0	GF-6S	14.3	105.9		P
LF-361	13.6	104.3	98.5	GF-6S	14.3	105.9		P
LF-362	11.2	103.5	97.7	GF-6S	14.3	105.9		P
LF-363	11.4	103.5	97.7	GF-6S	14.3	105.9		P
LF-364	12.9	102.5	96.8	GF-6S	14.3	105.9		P
LF-365	14.3	102.8	97.1	GF-6S	14.3	105.9		P
LF-372	12.9	99.6	96.0	GF-7S	14.9	103.7		P
LF-373	9.8	101.8	98.2	GF-7S	14.9	103.7		P
LF-374	10.3	103.9	100.2	GF-7S	14.9	103.7		P
LF-375	18.4	99.0	93.5	GF-6S	14.3	105.9	LF-501	F
LF-376	19.9	97.4	92.0	GF-6S	14.3	105.9	LF-502	F
LF-377	13.7	104.0	98.2	GF-6S	14.3	105.9		P
LF-378	19.9	98.2	92.7	GF-6S	14.3	105.9	LF-504	F
LF-379	14.1	103.4	97.6	GF-6S	14.3	105.9		P
LF-380	15.1	100.8	95.2	GF-6S	14.3	105.9		P
LF-381	11.1	105.2	99.3	GF-6S	14.3	105.9		P

Notes:

¹ Missing test numbers correspond to non-critical construction activities.² Required minimum percent compaction was 95% of Standard Proctor (ASTM D 698) maximum dry unit weight.³ Retest number for failing density tests.

Table 4-2 (continued)

COMPACTION TEST RESULTS FOR GENERAL FILL USED TO CONSTRUCT PERIMETER BERM

	NUCLEAR GAUGE			APPLICABLE STANDARD PROCTOR				
TEST STANDARD	ASTM D 3017	ASTM D 2922		ASTM D 698				
TESTING FREQUENCY	5 test per acre per lift or 1 test per 250 lf per lift			1 test per 25,000 yd³				
TEST RESULTS								
CQA Test No. ¹	Moisture Content (%)	Dry Unit Weight (pcf)	Percent Compaction ² (%)	Sample ID	Optimum Moisture Content (%)	Maximum Dry Unit Weight (pcf)	Retest No. ³	Pass/Fail (P/F)
LF-382	12.3	104.1	98.3	GF-6S	14.3	105.9		P
LF-383	12.3	104.7	98.9	GF-6S	14.3	105.9		P
LF-384	16.5	101.4	95.8	GF-6S	14.3	105.9		P
LF-385	14.1	103.0	97.3	GF-6S	14.3	105.9		P
LF-386	12.8	104.7	98.9	GF-6S	14.3	105.9		P
LF-387	10.4	106.6	100.7	GF-6S	14.3	105.9		P
LF-388	14.5	103.1	97.4	GF-6S	14.3	105.9		P
LF-389	11.8	105.8	99.9	GF-6S	14.3	105.9		P
LF-390	9.0	105.8	99.9	GF-6S	14.3	105.9		P
LF-391	16.5	99.8	94.2	GF-6S	14.3	105.9	LF-506	F
LF-392	13.5	103.7	97.9	GF-6S	14.3	105.9		P
LF-393	14.8	102.5	96.8	GF-6S	14.3	105.9		P
LF-394	13.3	104.1	98.3	GF-6S	14.3	105.9		P
LF-455	7.5	104.9	101.9	GF-11S	16.0	102.9		P
LF-456	10.8	105.4	102.4	GF-11S	16.0	102.9		P
LF-457	11.6	104.2	101.3	GF-11S	16.0	102.9		P
LF-458	10.6	106.4	103.4	GF-11S	16.0	102.9		P
LF-459	6.7	101.2	98.3	GF-11S	16.0	102.9		P
LF-460	18.4	100.0	97.2	GF-11S	16.0	102.9		P
LF-461	8.5	104.7	101.7	GF-11S	16.0	102.9		P
LF-462	10.7	102.2	99.3	GF-11S	16.0	102.9		P
LF-463	11.6	103.6	100.7	GF-11S	16.0	102.9		P
LF-464	12.1	104.1	101.2	GF-11S	16.0	102.9		P
LF-465	13.3	101.5	98.6	GF-11S	16.0	102.9		P
LF-466	10.9	103.4	100.5	GF-11S	16.0	102.9		P
LF-467	12.6	98.2	95.4	GF-11S	16.0	102.9		P
LF-468	16.1	100.4	97.6	GF-11S	16.0	102.9		P
LF-469	11.7	98.0	95.2	GF-11S	16.0	102.9		P
LF-470	12.8	98.4	95.6	GF-11S	16.0	102.9		P
LF-471	15.5	101.9	99.0	GF-11S	16.0	102.9		P
LF-472	13.6	102.8	99.9	GF-11S	16.0	102.9		P
LF-473	15.8	100.9	98.1	GF-11S	16.0	102.9		P
LF-474	16.5	100.8	98.0	GF-11S	16.0	102.9		P
LF-475	15.0	101.1	98.3	GF-11S	16.0	102.9		P
LF-476	11.3	103.4	100.5	GF-11S	16.0	102.9		P
LF-477	16.1	99.2	96.4	GF-11S	16.0	102.9		P
LF-478	14.4	101.3	98.4	GF-11S	16.0	102.9		P
LF-479	15.0	102.2	99.3	GF-11S	16.0	102.9		P
LF-480	16.2	99.0	96.2	GF-11S	16.0	102.9		P
LF-481	15.8	100.6	97.8	GF-11S	16.0	102.9		P
Notes:								
1 Missing test numbers correspond to non-critical construction activities.								
2 Required minimum percent compaction was 95% of Standard Proctor (ASTM D 698) maximum dry unit weight.								
3 Retest number for failing density tests.								

Table 4-2 (continued)

COMPACTION TEST RESULTS FOR GENERAL FILL USED TO CONSTRUCT PERIMETER BERM

	NUCLEAR GAUGE			APPLICABLE STANDARD PROCTOR				
TEST STANDARD	ASTM D 3017	ASTM D 2922	ASTM D 698					
TESTING FREQUENCY	5 test per acre per lift or 1 test per 250 lf per lift			1 test per 25,000 yd³				
TEST RESULTS								
CQA Test No. ¹	Moisture Content (%)	Dry Unit Weight (pcf)	Percent Compaction ² (%)	Sample ID	Optimum Moisture Content (%)	Maximum Dry Unit Weight (pcf)	Retest No. ³	Pass/Fail (P/F)
LF-483	7.9	100.8	98.0	GF-11S	16.0	102.9		P
LF-484	11.2	105.0	102.0	GF-11S	16.0	102.9		P
LF-485	9.8	107.3	104.3	GF-11S	16.0	102.9		P
LF-486	12.0	106.6	103.6	GF-11S	16.0	102.9		P
LF-487	7.2	104.6	101.7	GF-11S	16.0	102.9		P
LF-488	15.0	101.8	98.2	GF-7S	14.9	103.7		P
LF-489	12.3	101.9	98.3	GF-7S	14.9	103.7		P
LF-490	12.8	101.3	97.7	GF-7S	14.9	103.7		P
LF-491	11.6	103.5	99.8	GF-7S	14.9	103.7		P
LF-492	11.2	100.6	97.0	GF-7S	14.9	103.7		P
LF-501	13.9	102.2	96.5	GF-6S	14.3	105.9		P
LF-502	6.8	104.9	99.1	GF-6S	14.3	105.9		P
LF-503	7.5	106.0	100.1	GF-6S	14.3	105.9		P
LF-504	8.6	106.2	100.3	GF-6S	14.3	105.9		P
LF-505	10.1	107.4	101.4	GF-6S	14.3	105.9		P
LF-506	8.0	103.0	97.3	GF-6S	14.3	105.9		P
LF-516	14.6	104.5	98.7	GF-6S	14.3	105.9		P
LF-517	10.7	107.6	101.6	GF-6S	14.3	105.9		P
LF-518	8.8	111.1	104.9	GF-6S	14.3	105.9		P
LF-519	10.0	109.3	103.2	GF-6S	14.3	105.9		P
LF-520	10.4	108.5	102.5	GF-6S	14.3	105.9		P
LF-521	10.8	107.9	101.9	GF-6S	14.3	105.9		P
LF-522	10.4	106.9	100.9	GF-6S	14.3	105.9		P
LF-523	10.3	109.1	103.0	GF-6S	14.3	105.9		P
LF-524	7.4	106.8	100.8	GF-6S	14.3	105.9		P
LF-525	11.4	103.7	97.9	GF-6S	14.3	105.9		P
LF-526	11.2	108.7	102.6	GF-6S	14.3	105.9		P
LF-527	10.8	108.7	102.6	GF-6S	14.3	105.9		P
LF-528	11.7	104.3	98.5	GF-6S	14.3	105.9		P
LF-541	18.9	101.7	96.0	GF-6S	14.3	105.9		P
LF-542	14.8	101.5	95.8	GF-6S	14.3	105.9		P
LF-543	9.5	101.4	95.8	GF-6S	14.3	105.9		P
LF-544	13.2	102.9	97.2	GF-6S	14.3	105.9		P
LF-545	14.7	103.6	97.8	GF-6S	14.3	105.9		P
LF-546	13.7	105.9	100.0	GF-6S	14.3	105.9		P
LF-547	16.6	100.8	95.2	GF-6S	14.3	105.9		P
LF-548	13.5	105.5	99.6	GF-6S	14.3	105.9		P
LF-549	11.5	106.8	100.8	GF-6S	14.3	105.9		P
LF-550	11.0	105.9	100.0	GF-6S	14.3	105.9		P
LF-551	10.8	107.9	101.9	GF-6S	14.3	105.9		P
Notes:								
1 Missing test numbers correspond to non-critical construction activities.								
2 Required minimum percent compaction was 95% of Standard Proctor (ASTM D 698) maximum dry unit weight.								
3 Retest number for failing density tests.								

Table 4-2 (continued)

COMPACTION TEST RESULTS FOR GENERAL FILL USED TO CONSTRUCT PERIMETER BERM

	NUCLEAR GAUGE			APPLICABLE STANDARD PROCTOR				
TEST STANDARD	ASTM D 3017	ASTM D 2922		ASTM D 698				
TESTING FREQUENCY	5 test per acre per lift or 1 test per 250 lf per lift			1 test per 25,000 yd ³				
TEST RESULTS								
CQA Test No. ¹	Moisture Content (%)	Dry Unit Weight (pcf)	Percent Compaction ² (%)	Sample ID	Optimum Moisture Content (%)	Maximum Dry Unit Weight (pcf)	Retest No. ³	Pass/Fail (P/F)
LF-560	16.2	97.6	95.3	GF-12S	17.0	102.4		P
LF-561	14.5	97.2	94.9	GF-12S	17.0	102.4		P
LF-562	13.7	97.6	95.3	GF-12S	17.0	102.4		P
LF-536	13.6	105.0	102.5	GF-12S	17.0	102.4		P
LF-564	18.4	101.0	98.6	GF-12S	17.0	102.4		P
LF-565	13.7	101.1	98.7	GF-12S	17.0	102.4		P
LF-566	14.5	104.6	102.1	GF-12S	17.0	102.4		P
LF-567	14.6	102.8	100.4	GF-12S	17.0	102.4		P
LF-568	15.0	100.0	97.7	GF-12S	17.0	102.4		P
LF-569	16.1	101.2	98.8	GF-12S	17.0	102.4		P
LF-570	10.8	98.7	96.4	GF-12S	17.0	102.4		P
LF-571	12.6	102.9	100.5	GF-12S	17.0	102.4		P
LF-572	7.6	106.7	104.2	GF-12S	17.0	102.4		P
LF-573	10.4	104.3	101.9	GF-12S	17.0	102.4		P
LF-574	11.3	105.4	101.6	GF-7S	14.9	103.7		P
LF-575	10.1	102.2	98.6	GF-7S	14.9	103.7		P
LF-576	10.1	102.1	98.5	GF-7S	14.9	103.7		P
LF-577	12.2	103.8	100.1	GF-7S	14.9	103.7		P
LF-578	13.0	102.8	97.1	GF-6S	14.3	105.9		P
LF-579	11.7	108.7	102.6	GF-6S	14.3	105.9		P
LF-581	13.4	102.5	96.8	GF-6S	14.3	105.9		P
LF-582	13.0	102.7	97.0	GF-6S	14.3	105.9		P
LF-583	12.6	104.4	98.6	GF-6S	14.3	105.9		P
LF-584	13.8	103.4	97.6	GF-6S	14.3	105.9		P
LF-585	11.2	101.6	95.9	GF-6S	14.3	105.9		P
LF-586	12.2	101.5	95.8	GF-6S	14.3	105.9		P
LF-587	13.6	102.3	96.6	GF-6S	14.3	105.9		P
LF-588	12.7	102.8	97.1	GF-6S	14.3	105.9		P
LF-589	11.8	102.1	96.4	GF-6S	14.3	105.9		P
LF-590	13.0	101.5	95.8	GF-6S	14.3	105.9		P
LF-604	10.8	104.3	98.5	GF-6S	14.3	105.9		P
LF-605	8.6	105.9	100.0	GF-6S	14.3	105.9		P
LF-606	10.3	105.9	100.0	GF-6S	14.3	105.9		P
LF-607	8.1	100.7	95.1	GF-6S	14.3	105.9		P
LF-608	8.3	101.6	95.9	GF-6S	14.3	105.9		P
LF-609	12.9	100.7	95.1	GF-6S	14.3	105.9		P
LF-610	15.0	101.7	96.0	GF-6S	14.3	105.9		P
LF-611	9.9	102.8	97.1	GF-6S	14.3	105.9		P
LF-612	11.4	106.8	100.8	GF-6S	14.3	105.9		P
LF-613	18.0	97.9	92.4	GF-6S	14.3	105.9	LF-650	F
Notes:								
1 Missing test numbers correspond to non-critical construction activities.								
2 Required minimum percent compaction was 95% of Standard Proctor (ASTM D 698) maximum dry unit weight.								
3 Retest number for failing density tests.								

Table 4-2 (continued)

COMPACTION TEST RESULTS FOR GENERAL FILL USED TO CONSTRUCT PERIMETER BERM

	NUCLEAR GAUGE			APPLICABLE STANDARD PROCTOR				
TEST STANDARD	ASTM D 3017	ASTM D 2922	ASTM D 698					
TESTING FREQUENCY	5 test per acre per lift or 1 test per 250 lf per lift			1 test per 25,000 yd³				
TEST RESULTS								
CQA Test No.¹	Moisture Content (%)	Dry Unit Weight (pcf)	Percent Compaction² (%)	Sample ID	Optimum Moisture Content (%)	Maximum Dry Unit Weight (pcf)	Retest No.³	Pass/Fail (P/F)
LF-614	13.7	105.5	99.6	GF-6S	14.3	105.9		P
LF-615	10.2	100.1	94.5	GF-6S	14.3	105.9	LF-652	F
LF-616	14.2	100.3	94.7	GF-6S	14.3	105.9		P
LF-617	12.3	102.3	96.6	GF-6S	14.3	105.9		P
LF-618	13.2	102.7	97.0	GF-6S	14.3	105.9		P
LF-619	11.2	103.4	97.6	GF-6S	14.3	105.9		P
LF-620	12.9	103.1	97.4	GF-6S	14.3	105.9		P
LF-621	13.5	102.4	96.7	GF-6S	14.3	105.9		P
LF-622	12.1	105.3	99.4	GF-6S	14.3	105.9		P
LF-623	24.6	79.5	75.1	GF-6S	14.3	105.9	LF-654	F
LF-624	11.8	107.2	101.2	GF-6S	14.3	105.9		P
LF-625	12.2	103.7	97.9	GF-6S	14.3	105.9		P
LF-626	11.2	102.0	96.3	GF-6S	14.3	105.9		P
LF-627	9.6	103.3	97.5	GF-6S	14.3	105.9		P
LF-628	7.4	103.7	97.9	GF-6S	14.3	105.9		P
LF-629	11.9	103.8	98.0	GF-6S	14.3	105.9		P
LF-630	9.2	104.6	98.8	GF-6S	14.3	105.9		P
LF-631	11.7	104.6	98.8	GF-6S	14.3	105.9		P
LF-632	9.7	103.7	97.9	GF-6S	14.3	105.9		P
LF-633	12.1	101.6	95.9	GF-6S	14.3	105.9		P
LF-634	10.8	104.3	98.5	GF-6S	14.3	105.9		P
LF-635	9.0	107.2	101.2	GF-6S	14.3	105.9		P
LF-636	13.2	105.5	99.6	GF-6S	14.3	105.9		P
LF-637	10.1	107.9	101.9	GF-6S	14.3	105.9		P
LF-638	8.6	101.2	95.6	GF-6S	14.3	105.9		P
LF-639	8.9	105.3	99.4	GF-6S	14.3	105.9		P
LF-640	7.8	104.3	98.5	GF-6S	14.3	105.9		P
LF-641	7.5	106.7	100.8	GF-6S	14.3	105.9		P
LF-642	8.3	104.2	98.4	GF-6S	14.3	105.9		P
LF-643	6.8	101.2	95.6	GF-6S	14.3	105.9		P
LF-644	13.0	101.2	95.6	GF-6S	14.3	105.9		P
LF-645	10.3	100.9	95.3	GF-6S	14.3	105.9		P
LF-646	9.7	98.5	95.0	GF-7S	14.9	103.7		P
LF-647	12.7	98.7	95.2	GF-7S	14.9	103.7		P
LF-648	14.9	98.9	95.4	GF-7S	14.9	103.7		P
LF-649	9.7	106.8	103.0	GF-7S	14.9	103.7		P
LF-650	7.7	102.2	98.6	GF-7S	14.9	103.7		P
LF-651	16.9	99.9	96.3	GF-7S	14.9	103.7		P
LF-652	17.3	98.8	95.3	GF-7S	14.9	103.7		P
LF-653	15.2	100.4	96.8	GF-7S	14.9	103.7		P
Notes:								
¹ Missing test numbers correspond to non-critical construction activities.								
² Required minimum percent compaction was 95% of Standard Proctor (ASTM D 698) maximum dry unit weight.								
³ Retest number for failing density tests.								

Table 4-2 (continued)

COMPACTION TEST RESULTS FOR GENERAL FILL USED TO CONSTRUCT PERIMETER BERM

	NUCLEAR GAUGE			APPLICABLE STANDARD PROCTOR				
TEST STANDARD	ASTM D 3017	ASTM D 2922		ASTM D 698				
TESTING FREQUENCY	5 test per acre per lift or 1 test per 250 lf per lift			1 test per 25,000 yd³				
TEST RESULTS								
CQA Test No.¹	Moisture Content (%)	Dry Unit Weight (pcf)	Percent Compaction² (%)	Sample ID	Optimum Moisture Content (%)	Maximum Dry Unit Weight (pcf)	Retest No.³	Pass/Fail (P/F)
LF-654	16.3	100.6	95.6 97.0	GF-7S 65	14.9 14.3	103.7 105.4		P
LF-655	9.7	99.8	96.2	GF-7S	14.9	103.7		P
LF-656	14.7	101.7	98.1	GF-7S	14.9	103.7		P
LF-657	7.2	101.2	97.6	GF-7S	14.9	103.7		P
LF-658	15.0	104.2	100.5	GF-7S	14.9	103.7		P
LF-659	10.1	106.1	102.3	GF-7S	14.9	103.7		P
LF-660	8.6	103.5	99.8	GF-7S	14.9	103.7		P
LF-661	12.4	105.7	101.9	GF-7S	14.9	103.7		P
LF-662	11.4	110.0	106.1	GF-7S	14.9	103.7		P
LF-663	9.0	109.1	105.2	GF-7S	14.9	103.7		P
LF-664	8.7	111.3	107.3	GF-7S	14.9	103.7		P
LF-665	11.7	105.0	101.3	GF-7S	14.9	103.7		P
LF-666	23.0	96.6	93.2	GF-7S	14.9	103.7	LF-739	F
LF-667	20.2	94.5	91.1	GF-7S	14.9	103.7	LF-738	F
LF-668	19.2	101.2	97.6	GF-7S	14.9	103.7		P
LF-669	21.9	96.7	93.2	GF-7S	14.9	103.7	LF-737	F
LF-670	23.3	93.8	90.5	GF-7S	14.9	103.7	LF-736	F
LF-671	21.2	96.3	92.9	GF-7S	14.9	103.7	LF-735	F
LF-672	21.3	98.5	95.0	GF-7S	14.9	103.7		P
LF-673	18.5	99.0	95.5	GF-7S	14.9	103.7		P
LF-674	18.5	101.5	97.9	GF-7S	14.9	103.7		P
LF-675	11.1	104.8	101.1	GF-7S	14.9	103.7		P
LF-676	9.4	108.4	104.5	GF-7S	14.9	103.7		P
LF-677	8.8	102.0	98.4	GF-7S	14.9	103.7		P
LF-678	10.4	107.4	103.6	GF-7S	14.9	103.7		P
LF-679	11.7	105.3	101.5	GF-7S	14.9	103.7		P
LF-680	9.3	102.1	98.5	GF-7S	14.9	103.7		P
LF-681	18.0	102.5	98.8	GF-7S	14.9	103.7		P
LF-682	15.3	104.0	100.3	GF-7S	14.9	103.7		P
LF-683	14.0	104.5	100.8	GF-7S	14.9	103.7		P
LF-684	15.9	102.6	98.9	GF-7S	14.9	103.7		P
LF-685	17.5	102.4	98.7	GF-7S	14.9	103.7		P
LF-686	16.3	103.3	99.6	GF-7S	14.9	103.7		P
LF-687	15.8	104.8	101.1	GF-7S	14.9	103.7		P
LF-688	13.0	99.3	95.8	GF-7S	14.9	103.7		P
LF-689	13.0	105.1	101.4	GF-7S	14.9	103.7		P
LF-690	13.7	99.3	95.8	GF-7S	14.9	103.7		P
LF-691	16.0	99.0	95.5	GF-7S	14.9	103.7		P
LF-692	15.5	99.6	96.0	GF-7S	14.9	103.7		P
LF-693	14.5	106.0	102.2	GF-7S	14.9	103.7		P
Notes:								
¹ Missing test numbers correspond to non-critical construction activities.								
² Required minimum percent compaction was 95% of Standard Proctor (ASTM D 698) maximum dry unit weight.								
³ Retest number for failing density tests.								

Table 4-2 (continued)

COMPACTION TEST RESULTS FOR GENERAL FILL USED TO CONSTRUCT PERIMETER BERM

	NUCLEAR GAUGE			APPLICABLE STANDARD PROCTOR				
TEST STANDARD	ASTM D 3017	ASTM D 2922	ASTM D 698					
TESTING FREQUENCY	5 test per acre per lift or 1 test per 250 lf per lift			1 test per 25,000 yd³				
TEST RESULTS								
CQA Test No.¹	Moisture Content (%)	Dry Unit Weight (pcf)	Percent Compaction²	Sample ID	Optimum Moisture Content (%)	Maximum Dry Unit Weight (pcf)	Retest No.³	Pass/Fail (P/F)
LF-694	11.9	108.7	104.8	GF-7S	14.9	103.7		P
LF-695	11.6	105.5	101.7	GF-7S	14.9	103.7		P
LF-696	12.6	110.3	106.4	GF-7S	14.9	103.7		P
LF-697	10.7	108.7	104.8	GF-7S	14.9	103.7		P
LF-698	11.5	103.2	98.6	GF-16S	16.2	104.7		P
LF-699	10.5	104.8	100.1	GF-16S	16.2	104.7		P
LF-700	10.5	99.7	95.2	GF-16S	16.2	104.7		P
LF-701	11.7	103.8	99.1	GF-16S	16.2	104.7		P
LF-702	10.6	103.3	98.7	GF-16S	16.2	104.7		P
LF-703	9.7	100.7	96.2	GF-16S	16.2	104.7		P
LF-704	11.5	106.5	101.7	GF-16S	16.2	104.7		P
LF-705	12.3	107.8	103.0	GF-16S	16.2	104.7		P
LF-706	12.1	106.1	101.3	GF-16S	16.2	104.7		P
LF-707	11.5	105.1	100.4	GF-16S	16.2	104.7		P
LF-716	12.6	103.7	100.0	GF-7S	14.9	103.7		P
LF-717	15.9	104.1	100.4	GF-7S	14.9	103.7		P
LF-718	14.2	99.5	95.9	GF-7S	14.9	103.7		P
LF-719	21.3	99.4	95.9	GF-7S	14.9	103.7		P
LF-720	20.8	100.3	96.7	GF-7S	14.9	103.7		P
LF-721	25.0	95.9	92.5	GF-7S	14.9	103.7	LF-734	F
LF-722	16.1	102.1	98.5	GF-7S	14.9	103.7		P
LF-723	18.1	99.9	96.3	GF-7S	14.9	103.7		P
LF-724	15.8	101.4	97.8	GF-7S	14.9	103.7		P
LF-725	18.8	98.1	94.6	GF-7S	14.9	103.7		P
LF-726	13.6	101.8	98.2	GF-7S	14.9	103.7		P
LF-727	14.1	98.4	94.9	GF-7S	14.9	103.7		P
LF-728	15.6	100.3	96.7	GF-7S	14.9	103.7		P
LF-729	17.7	99.0	95.5	GF-7S	14.9	103.7		P
LF-730	19.3	98.6	95.1	GF-7S	14.9	103.7		P
LF-731	13.9	99.9	96.3	GF-7S	14.9	103.7		P
LF-732	15.8	100.9	97.3	GF-7S	14.9	103.7		P
LF-733	21.3	95.5	92.1	GF-7S	14.9	103.7	LF-741	F
LF-734	20.7	100.0	96.4	GF-7S	14.9	103.7		P
LF-735	18.6	100.4	96.8	GF-7S	14.9	103.7		P
LF-736	13.2	103.6	99.9	GF-7S	14.9	103.7		P
LF-737	11.8	108.8	104.9	GF-7S	14.9	103.7		P
LF-738	14.7	101.4	97.8	GF-7S	14.9	103.7		P
LF-739	17.0	100.5	96.9	GF-7S	14.9	103.7		P
LF-740	15.9	99.8	96.2	GF-7S	14.9	103.7		P
LF-741	14.5	101.9	98.3	GF-7S	14.9	103.7		P
Notes:								
1 Missing test numbers correspond to non-critical construction activities.								
2 Required minimum percent compaction was 95% of Standard Proctor (ASTM D 698) maximum dry unit weight.								
3 Retest number for failing density tests.								

Table 4-2 (continued)

COMPACTION TEST RESULTS FOR GENERAL FILL USED TO CONSTRUCT PERIMETER BERM

	NUCLEAR GAUGE			APPLICABLE STANDARD PROCTOR				
TEST STANDARD	ASTM D 3017	ASTM D 2922	ASTM D 698					
TESTING FREQUENCY	5 test per acre per lift or 1 test per 250 lf per lift			1 test per 25,000 yd³				
TEST RESULTS								
CQA Test No. ¹	Moisture Content (%)	Dry Unit Weight (pcf)	Percent Compaction ² (%)	Sample ID	Optimum Moisture Content (%)	Maximum Dry Unit Weight (pcf)	Retest No. ³	Pass/Fail (P/F)
LF-742	15.8	101.1	97.5	GF-7S	14.9	103.7		P
LF-743	11.3	104.4	100.7	GF-7S	14.9	103.7		P
LF-744	9.8	104.0	100.3	GF-7S	14.9	103.7		P
LF-745	8.3	109.4	104.5	GF-16S	16.2	104.7		P
LF-746	9.2	105.7	101.0	GF-16S	16.2	104.7		P
LF-747	8.9	105.6	100.9	GF-16S	16.2	104.7		P
LF-748	9.1	104.0	99.3	GF-16S	16.2	104.7		P
LF-749	8.9	107.5	102.7	GF-16S	16.2	104.7		P
LF-750	16.4	103.6	98.9	GF-16S	16.2	104.7		P
LF-751	12.1	101.5	96.9	GF-16S	16.2	104.7		P
LF-752	11.2	100.6	96.1	GF-16S	16.2	104.7		P
LF-753	10.1	103.7	99.0	GF-16S	16.2	104.7		P
LF-754	9.9	103.4	98.8	GF-16S	16.2	104.7		P
LF-755	9.5	99.8	95.3	GF-16S	16.2	104.7		P
LF-756	8.6	102.4	97.8	GF-16S	16.2	104.7		P
LF-757	8.3	104.4	99.7	GF-16S	16.2	104.7		P
LF-758	7.0	105.6	100.9	GF-16S	16.2	104.7		P
LF-759	9.1	102.9	98.3	GF-16S	16.2	104.7		P
LF-760	10.0	102.4	97.8	GF-16S	16.2	104.7		P
LF-761	10.2	102.5	97.9	GF-16S	16.2	104.7		P
LF-762	10.4	106.3	101.5	GF-16S	16.2	104.7		P
LF-763	10.4	104.8	100.1	GF-16S	16.2	104.7		P
LF-764	12.9	104.1	99.4	GF-16S	16.2	104.7		P
LF-765	10.9	101.1	96.6	GF-16S	16.2	104.7		P
LF-766	11.0	100.2	95.7	GF-16S	16.2	104.7		P
LF-767	12.9	102.1	98.5	GF-7S	14.9	103.7		P
LF-768	12.6	100.2	96.6	GF-7S	14.9	103.7		P
LF-769	16.1	101.1	97.5	GF-7S	14.9	103.7		P
LF-770	17.9	99.9	96.3	GF-7S	14.9	103.7		P
LF-771	13.4	101.3	97.7	GF-7S	14.9	103.7		P
LF-772	15.5	100.3	96.7	GF-7S	14.9	103.7		P
LF-773	5.6	107.6	103.8	GF-7S	14.9	103.7		P
LF-774	13.4	106.4	102.6	GF-7S	14.9	103.7		P
LF-775	11.8	103.9	100.2	GF-7S	14.9	103.7		P
LF-776	12.6	104.6	100.9	GF-7S	14.9	103.7		P
LF-777	15.7	100.8	97.2	GF-7S	14.9	103.7		P
LF-778	13.1	100.8	97.2	GF-7S	14.9	103.7		P
LF-779	15.5	98.7	95.2	GF-7S	14.9	103.7		P
LF-780	13.6	99.2	95.7	GF-7S	14.9	103.7		P
LF-781	13.9	98.1	94.6	GF-7S	14.9	103.7		P
Notes:								
1 Missing test numbers correspond to non-critical construction activities.								
2 Required minimum percent compaction was 95% of Standard Proctor (ASTM D 698) maximum dry unit weight.								
3 Retest number for failing density tests.								

Table 4-2 (continued)

COMPACTION TEST RESULTS FOR GENERAL FILL USED TO CONSTRUCT PERIMETER BERM

	NUCLEAR GAUGE			APPLICABLE STANDARD PROCTOR				
TEST STANDARD	ASTM D 3017	ASTM D 2922	ASTM D 698					
TESTING FREQUENCY	5 test per acre per lift or 1 test per 250 lf per lift			1 test per 25,000 yd³				
TEST RESULTS								
CQA Test No. ¹	Moisture Content (%)	Dry Unit Weight (pcf)	Percent Compaction ² (%)	Sample ID	Optimum Moisture Content (%)	Maximum Dry Unit Weight (pcf)	Retest No. ³	Pass/Fail (P/F)
LF-782	17.3	101.6	98.0	GF-7S	14.9	103.7		P
LF-783	17.3	99.3	95.8	GF-7S	14.9	103.7		P
LF-784	17.2	99.8	96.2	GF-7S	14.9	103.7		P
LF-785	18.2	104.2	100.5	GF-7S	14.9	103.7		P
LF-786	17.7	101.1	97.5	GF-7S	14.9	103.7		P
LF-787	11.0	105.1	101.4	GF-7S	14.9	103.7		P
LF-788	18.2	101.1	97.5	GF-7S	14.9	103.7		P
LF-789	20.1	98.5	95.0	GF-7S	14.9	103.7		P
LF-790	17.9	101.7	98.1	GF-7S	14.9	103.7		P
LF-791	14.3	102.8	99.1	GF-7S	14.9	103.7		P
LF-972	14.8	103.4	99.7	GF-7S	14.9	103.7		P
LF-793	13.0	101.6	98.0	GF-7S	14.9	103.7		P
LF-794	7.1	104.8	101.1	GF-7S	14.9	103.7		P
LF-795	8.4	106.6	102.8	GF-7S	14.9	103.7		P
LF-796	19.8	99.8	95.3	GF-16S	16.2	104.7		P
LF-797	15.9	100.1	95.6	GF-16S	16.2	104.7		P
LF-798	19.0	100.2	95.7	GF-16S	16.2	104.7		P
LF-799	17.4	102.5	97.9	GF-16S	16.2	104.7		P
LF-800	12.8	101.7	97.1	GF-16S	16.2	104.7		P
LF-801	19.2	100.8	96.3	GF-16S	16.2	104.7		P
LF-802	16.7	100.2	95.7	GF-16S	16.2	104.7		P
LF-803	16.0	100.5	96.0	GF-16S	16.2	104.7		P
LF-804	15.6	100.8	96.3	GF-16S	16.2	104.7		P
LF-805	20.5	99.7	95.2	GF-16S	16.2	104.7		P
LF-806	13.2	102.2	97.6	GF-16S	16.2	104.7		P
LF-807	14.4	101.2	96.7	GF-16S	16.2	104.7		P
LF-808	17.5	102.5	97.9	GF-16S	16.2	104.7		P
LF-809	14.6	100.8	96.3	GF-16S	16.2	104.7		P
LF-810	15.3	101.0	96.5	GF-16S	16.2	104.7		P
LF-811	19.1	100.2	95.7	GF-16S	16.2	104.7		P
LF-812	16.3	100.9	96.4	GF-16S	16.2	104.7		P
LF-813	11.3	105.3	100.6	GF-16S	16.2	104.7		P
LF-814	10.6	105.1	100.4	GF-16S	16.2	104.7		P
LF-815	9.6	103.2	98.6	GF-16S	16.2	104.7		P
LF-816	12.1	101.9	97.3	GF-16S	16.2	104.7		P
LF-817	11.8	101.6	97.0	GF-16S	16.2	104.7		P
LF-818	9.7	103.8	99.1	GF-16S	16.2	104.7		P
LF-819	10.2	106.0	101.2	GF-16S	16.2	104.7		P
LF-820	10.6	103.5	98.9	GF-16S	16.2	104.7		P
LF-821	10.7	102.5	97.9	GF-16S	16.2	104.7		P
Notes:								
1 Missing test numbers correspond to non-critical construction activities.								
2 Required minimum percent compaction was 95% of Standard Proctor (ASTM D 698) maximum dry unit weight.								
3 Retest number for failing density tests.								

Table 4-2 (continued)

COMPACTION TEST RESULTS FOR GENERAL FILL USED TO CONSTRUCT PERIMETER BERM

	NUCLEAR GAUGE			APPLICABLE STANDARD PROCTOR				
TEST STANDARD	ASTM D 3017	ASTM D 2922		ASTM D 698				
TESTING FREQUENCY	5 test per acre per lift or 1 test per 250 lf per lift			1 test per 25,000 yd ³				
TEST RESULTS								
CQA Test No. ¹	Moisture Content (%)	Dry Unit Weight (pcf)	Percent Compaction ² (%)	Sample ID	Optimum Moisture Content (%)	Maximum Dry Unit Weight (pcf)	Retest No. ³	Pass/Fail (P/F)
LF-822	9.8	100.8	96.3	GF-16S	16.2	104.7		P
LF-823	9.4	100.0	95.5	GF-16S	16.2	104.7		P
LF-824	12.5	101.2	96.7	GF-16S	16.2	104.7		P
LF-825	10.1	101.1	96.6	GF-16S	16.2	104.7		P
LF-826	10.3	102.6	98.0	GF-16S	16.2	104.7		P
LF-827	12.4	103.3	98.7	GF-16S	16.2	104.7		P
LF-828	15.8	102.4	97.8	GF-16S	16.2	104.7		P
LF-829	13.4	101.5	96.9	GF-16S	16.2	104.7		P
LF-830	11.6	103.9	99.2	GF-16S	16.2	104.7		P
LF-831	18.9	99.7	95.2	GF-16S	16.2	104.7		P
LF-832	9.8	102.6	98.0	GF-16S	16.2	104.7		P
LF-833	11.9	109.3	104.4	GF-16S	16.2	104.7		P
LF-834	9.2	108.0	103.2	GF-16S	16.2	104.7		P
LF-835	9.6	103.2	98.6	GF-16S	16.2	104.7		P
LF-836	8.3	99.7	95.2	GF-16S	16.2	104.7		P
LF-837	10.2	103.4	98.8	GF-16S	16.2	104.7		P
LF-838	13.5	103.3	98.7	GF-16S	16.2	104.7		P
LF-839	12.5	104.5	99.8	GF-16S	16.2	104.7		P
LF-840	15.1	100.0	95.5	GF-16S	16.2	104.7		P
LF-841	17.3	99.5	95.0	GF-16S	16.2	104.7		P
LF-842	7.9	101.6	97.0	GF-16S	16.2	104.7		P
LF-843	8.5	106.0	101.2	GF-16S	16.2	104.7		P
LF-866	20.5	100.0	95.5	GF-16S	16.2	104.7		P
LF-867	16.8	100.9	96.4	GF-16S	16.2	104.7		P
LF-868	19.8	99.8	95.3	GF-16S	16.2	104.7		P
LF-869	21.1	99.5	95.0	GF-16S	16.2	104.7		P
LF-870	18.3	100.3	95.8	GF-16S	16.2	104.7		P
LF-871	17.5	100.1	95.6	GF-16S	16.2	104.7		P
LF-872	18.6	100.5	96.0	GF-16S	16.2	104.7		P
LF-873	16.8	100.3	95.8	GF-16S	16.2	104.7		P
LF-874	20.1	100.6	96.1	GF-16S	16.2	104.7		P
LF-875	17.5	101.1	96.6	GF-16S	16.2	104.7		P
LF-876	22.8	100.2	95.7	GF-16S	16.2	104.7		P
LF-877	17.7	101.9	97.3	GF-16S	16.2	104.7		P
LF-878	13.9	100.5	96.0	GF-16S	16.2	104.7		P
LF-879	12.8	106.5	101.7	GF-16S	16.2	104.7		P
LF-880	13.8	106.9	102.1	GF-16S	16.2	104.7		P
LF-881	15.0	107.6	102.8	GF-16S	16.2	104.7		P
LF-888	13.6	103.6	100.4	GF-17S	15.7	103.2		P
LF-889	12.2	102.5	99.3	GF-17S	15.7	103.2		P
Notes:								
1 Missing test numbers correspond to non-critical construction activities.								
2 Required minimum percent compaction was 95% of Standard Proctor (ASTM D 698) maximum dry unit weight.								
3 Retest number for failing density tests.								

Table 4-2 (continued)

COMPACTION TEST RESULTS FOR GENERAL FILL USED TO CONSTRUCT PERIMETER BERM

	NUCLEAR GAUGE			APPLICABLE STANDARD PROCTOR				
TEST STANDARD	ASTM D 3017	ASTM D 2922		ASTM D 698				
TESTING FREQUENCY	5 test per acre per lift or 1 test per 250 lf per lift			1 test per 25,000 yd³				
TEST RESULTS								
CQA Test No.¹	Moisture Content (%)	Dry Unit Weight (pcf)	Percent Compaction² (%)	Sample ID	Optimum Moisture Content (%)	Maximum Dry Unit Weight (pcf)	Retest No.³	Pass/Fail (P/F)
LF-890	11.8	100.3	97.2	GF-17S	15.7	103.2		P
LF-891	10.2	99.7	96.6	GF-17S	15.7	103.2		P
LF-892	13.0	103.4	100.2	GF-17S	15.7	103.2		P
LF-893	12.4	102.0	98.8	GF-17S	15.7	103.2		P
LF-907	14.5	103.3	100.1	GF-17S	15.7	103.2		P
LF-908	13.5	104.1	100.9	GF-17S	15.7	103.2		P
LF-909	8.9	107.9	103.1	GF-16S	16.2	104.7		P
LF-910	7.6	109.2	104.3	GF-16S	16.2	104.7		P
LF-911	8.6	107.3	102.5	GF-16S	16.2	104.7		P
LF-912	11.7	107.3	102.5	GF-16S	16.2	104.7		P
LF-913	9.3	111.7	106.7	GF-16S	16.2	104.7		P
LF-914	14.8	103.5	98.9	GF-16S	16.2	104.7		P
LF-915	13.4	105.1	100.4	GF-16S	16.2	104.7		P
LF-916	8.7	102.2	97.6	GF-16S	16.2	104.7		P
LF-917	12.0	100.9	96.4	GF-16S	16.2	104.7		P
LF-918	11.3	104.6	99.9	GF-16S	16.2	104.7		P
LF-919	12.1	100.5	96.0	GF-16S	16.2	104.7		P
LF-920	11.5	103.4	98.8	GF-16S	16.2	104.7		P
LF-921	15.0	100.6	96.1	GF-16S	16.2	104.7		P
LF-922	6.4	99.7	95.2	GF-16S	16.2	104.7		P
LF-923	12.3	105.1	100.4	GF-16S	16.2	104.7		P
LF-924	16.0	99.5	95.0	GF-16S	16.2	104.7		P
LF-925	13.7	99.7	95.2	GF-16S	16.2	104.7		P
LF-926	14.4	103.3	98.7	GF-16S	16.2	104.7		P
LF-927	7.4	104.1	99.4	GF-16S	16.2	104.7		P
LF-928	10.4	99.7	95.2	GF-16S	16.2	104.7		P
LF-929	12.9	99.3	94.8	GF-16S	16.2	104.7		P
LF-930	15.0	102.4	97.8	GF-16S	16.2	104.7		P
LF-931	14.3	99.9	95.4	GF-16S	16.2	104.7		P
LF-932	14.3	104.1	100.9	GF-17S	15.7	103.2		P
LF-933	13.6	102.9	99.7	GF-17S	15.7	103.2		P
LF-934	19.0	101.1	98.0	GF-17S	15.7	103.2		P
LF-935	18.0	99.2	96.1	GF-17S	15.7	103.2		P
LF-936	18.9	101.1	98.0	GF-17S	15.7	103.2		P
LF-937	16.1	103.9	100.7	GF-17S	15.7	103.2		P
LF-938	15.9	103.3	100.1	GF-17S	15.7	103.2		P
LF-939	11.9	106.4	103.1	GF-17S	15.7	103.2		P
LF-940	15.3	107.0	103.7	GF-17S	15.7	103.2		P
LF-941	16.1	104.8	101.6	GF-17S	15.7	103.2		P
LF-942	18.4	102.0	98.8	GF-17S	15.7	103.2		P
Notes:								
¹ Missing test numbers correspond to non-critical construction activities.								
² Required minimum percent compaction was 95% of Standard Proctor (ASTM D 698) maximum dry unit weight.								
³ Retest number for failing density tests.								

Table 4-2 (continued)

COMPACTION TEST RESULTS FOR GENERAL FILL USED TO CONSTRUCT PERIMETER BERM

	NUCLEAR GAUGE			APPLICABLE STANDARD PROCTOR				
TEST STANDARD	ASTM D 3017	ASTM D 2922		ASTM D 698				
TESTING FREQUENCY	5 test per acre per lift or 1 test per 250 lf per lift			1 test per 25,000 yd ³				
TEST RESULTS								
CQA Test No. ¹	Moisture Content (%)	Dry Unit Weight (pcf)	Percent Compaction ² (%)	Sample ID	Optimum Moisture Content (%)	Maximum Dry Unit Weight (pcf)	Retest No. ³	Pass/Fail (P/F)
LF-943	15.5	107.0	103.7	GF-17S	15.7	103.2		P
LF-944	14.0	104.3	101.1	GF-17S	15.7	103.2		P
LF-945	14.7	103.2	100.0	GF-17S	15.7	103.2		P
LF-946	14.6	100.3	97.2	GF-17S	15.7	103.2		P
LF-949	6.1	108.3	104.9	GF-17S	15.7	103.2		P
LF-950	11.8	102.5	99.3	GF-17S	15.7	103.2		P
LF-951	10.1	103.8	100.6	GF-17S	15.7	103.2		P
LF-952	17.7	99.4	96.3	GF-17S	15.7	103.2		P
LF-953	16.1	102.6	99.4	GF-17S	15.7	103.2		P
LF-954	15.9	100.9	97.8	GF-17S	15.7	103.2		P
LF-955	12.5	102.9	99.7	GF-17S	15.7	103.2		P
LF-956	17.9	99.4	96.3	GF-17S	15.7	103.2		P
LF-957	17.7	98.0	95.0	GF-17S	15.7	103.2		P
LF-958	15.3	104.0	100.8	GF-17S	15.7	103.2		P
LF-959	19.2	98.3	95.3	GF-17S	15.7	103.2		P
LF-960	14.7	101.2	98.1	GF-17S	15.7	103.2		P
LF-961	12.9	101.8	98.6	GF-17S	15.7	103.2		P
LF-962	15.2	101.4	98.3	GF-17S	15.7	103.2		P
LF-963	13.8	103.4	100.2	GF-17S	15.7	103.2		P
LF-964	14.4	102.0	98.8	GF-17S	15.7	103.2		P
LF-965	13.8	105.4	102.1	GF-17S	15.7	103.2		P
LF-966	17.6	103.0	99.8	GF-17S	15.7	103.2		P
LF-967	15.9	102.6	99.4	GF-17S	15.7	103.2		P
LF-968	16.8	100.7	97.6	GF-17S	15.7	103.2		P
LF-969	18.5	101.4	98.3	GF-17S	15.7	103.2		P
LF-970	20.5	98.2	95.2	GF-17S	15.7	103.2		P
LF-971	20.9	98.0	95.0	GF-17S	15.7	103.2		P
LF-972	13.1	102.3	99.1	GF-17S	15.7	103.2		P
LF-973	17.0	100.8	97.7	GF-17S	15.7	103.2		P
LF-974	19.6	98.3	95.3	GF-17S	15.7	103.2		P
LF-975	17.6	99.0	95.9	GF-17S	15.7	103.2		P
LF-976	15.3	100.4	97.3	GF-17S	15.7	103.2		P
LF-977	19.0	98.4	95.3	GF-17S	15.7	103.2		P
LF-978	18.4	99.6	96.5	GF-17S	15.7	103.2		P
LF-979	13.0	102.5	97.9	GF-16S	16.2	104.7		P
LF-980	11.5	105.0	100.3	GF-16S	16.2	104.7		P
LF-981	13.5	101.2	96.7	GF-16S	16.2	104.7		P
LF-982	15.9	104.4	99.7	GF-16S	16.2	104.7		P
LF-983	14.8	104.8	100.1	GF-16S	16.2	104.7		P
LF-984	13.1	105.1	100.4	GF-16S	16.2	104.7		P
Notes:								
1 Missing test numbers correspond to non-critical construction activities.								
2 Required minimum percent compaction was 95% of Standard Proctor (ASTM D 698) maximum dry unit weight.								
3 Retest number for failing density tests.								

Table 4-2 (continued)

COMPACTION TEST RESULTS FOR GENERAL FILL USED TO CONSTRUCT PERIMETER BERM

	NUCLEAR GAUGE			APPLICABLE STANDARD PROCTOR				
TEST STANDARD	ASTM D 3017	ASTM D 2922	ASTM D 698					
TESTING FREQUENCY	5 test per acre per lift or 1 test per 250 lf per lift			1 test per 25,000 yd³				
TEST RESULTS								
CQA Test No.¹	Moisture Content (%)	Dry Unit Weight (pcf)	Percent Compaction² (%)	Sample ID	Optimum Moisture Content (%)	Maximum Dry Unit Weight (pcf)	Retest No.³	Pass/Fail (P/F)
LF-985	13.1	106.5	101.7	GF-16S	16.2	104.7		P
LF-986	14.1	105.8	101.1	GF-16S	16.2	104.7		P
LF-987	11.6	102.7	99.5	GF-17S	15.7	103.2		P
LF-988	8.1	98.9	95.8	GF-17S	15.7	103.2		P
LF-989	10.0	98.7	95.6	GF-17S	15.7	103.2		P
LF-990	15.7	99.7	96.6	GF-17S	15.7	103.2		P
LF-991	11.3	107.3	102.5	GF-16S	16.2	104.7		P
LF-992	8.3	105.3	100.6	GF-16S	16.2	104.7		P
LF-993	10.9	107.2	102.4	GF-16S	16.2	104.7		P
LF-994	12.2	105.6	100.9	GF-16S	16.2	104.7		P
LF-995	11.4	102.6	98.0	GF-16S	16.2	104.7		P
LF-996	17.4	100.3	95.8	GF-16S	16.2	104.7		P
LF-997	15.9	102.8	99.6	GF-17S	15.7	103.2		P
LF-998	16.6	103.3	100.1	GF-17S	15.7	103.2		P
LF-999	14.9	100.3	97.2	GF-17S	15.7	103.2		P
LF-1000	16.3	99.4	96.3	GF-17S	15.7	103.2		P
LF-1001	17.2	99.6	96.5	GF-17S	15.7	103.2		P
LF-1002	16.1	99.1	96.0	GF-17S	15.7	103.2		P
LF-1003	14.3	100.4	97.3	GF-17S	15.7	103.2		P
LF-1004	15.5	99.0	95.9	GF-17S	15.7	103.2		P
LF-1005	16.0	99.3	96.2	GF-17S	15.7	103.2		P
LF-1006	15.7	101.4	98.3	GF-17S	15.7	103.2		P
LF-1007	15.1	98.9	95.8	GF-17S	15.7	103.2		P
LF-1008	14.7	99.4	96.3	GF-17S	15.7	103.2		P
LF-1009	16.3	100.3	97.2	GF-17S	15.7	103.2		P
LF-1010	15.8	99.1	96.0	GF-17S	15.7	103.2		P
LF-1011	14.2	103.4	98.8	GF-16S	16.2	104.7		P
LF-1012	14.5	106.3	101.5	GF-16S	16.2	104.7		P
LF-1013	14.2	103.0	98.4	GF-16S	16.2	104.7		P
LF-1014	13.3	99.9	95.4	GF-16S	16.2	104.7		P
LF-1015	12.0	104.1	99.4	GF-16S	16.2	104.7		P
LF-1016	14.8	105.4	100.7	GF-16S	16.2	104.7		P
LF-1017	19.0	99.5	95.0	GF-16S	16.2	104.7		P
LF-1018	14.4	107.1	102.3	GF-16S	16.2	104.7		P
LF-1019	14.7	104.9	100.2	GF-16S	16.2	104.7		P
LF-1020	23.9	94.6	90.4	GF-16S	16.2	104.7	LF-1351	F
LF-1021	14.9	103.5	98.9	GF-16S	16.2	104.7		P
LF-1022	11.8	103.3	100.1	GF-17S	15.7	103.2		P
LF-1023	10.9	102.8	99.6	GF-17S	15.7	103.2		P
LF-1024	18.9	98.1	95.1	GF-17S	15.7	103.2		P
Notes:								
¹ Missing test numbers correspond to non-critical construction activities.								
² Required minimum percent compaction was 95% of Standard Proctor (ASTM D 698) maximum dry unit weight.								
³ Retest number for failing density tests.								

Table 4-2 (continued)

COMPACTION TEST RESULTS FOR GENERAL FILL USED TO CONSTRUCT PERIMETER BERM

	NUCLEAR GAUGE			APPLICABLE STANDARD PROCTOR				
TEST STANDARD	ASTM D 3017	ASTM D 2922	ASTM D 698					
TESTING FREQUENCY	5 test per acre per lift or 1 test per 250 lf per lift			1 test per 25,000 yd³				
TEST RESULTS								
CQA Test No. ¹	Moisture Content (%)	Dry Unit Weight (pcf)	Percent Compaction ² (%)	Sample ID	Optimum Moisture Content (%)	Maximum Dry Unit Weight (pcf)	Retest No. ³	Pass/Fail (P/F)
LF-1025	17.6	100.0	96.9	GF-17S	15.7	103.2		P
LF-1026	16.8	99.1	96.0	GF-17S	15.7	103.2		P
LF-1027	15.3	101.0	97.9	GF-17S	15.7	103.2		P
LF-1028	14.6	105.6	102.3	GF-17S	15.7	103.2		P
LF-1029	15.2	103.5	100.3	GF-17S	15.7	103.2		P
LF-1030	12.3	103.7	100.5	GF-17S	15.7	103.2		P
LF-1031	16.4	99.0	95.9	GF-17S	15.7	103.2		P
LF-1032	18.9	98.3	95.3	GF-17S	15.7	103.2		P
LF-1033	15.3	97.8	94.8	GF-17S	15.7	103.2		P
LF-1034	10.7	104.7	101.5	GF-17S	15.7	103.2		P
LF-1035	18.6	101.5	98.4	GF-17S	15.7	103.2		P
LF-1036	16.8	101.5	98.4	GF-17S	15.7	103.2		P
LF-1037	14.6	103.7	100.5	GF-17S	15.7	103.2		P
LF-1038	16.5	99.5	96.4	GF-17S	15.7	103.2		P
LF-1039	20.1	99.7	96.6	GF-17S	15.7	103.2		P
LF-1040	18.1	101.8	98.6	GF-17S	15.7	103.2		P
LF-1041	20.1	98.9	95.8	GF-17S	15.7	103.2		P
LF-1042	16.1	105.5	102.2	GF-17S	15.7	103.2		P
LF-1043	9.2	104.2	101.0	GF-17S	15.7	103.2		P
LF-1044	15.6	98.5	95.4	GF-17S	15.7	103.2		P
LF-1045	20.3	98.9	95.8	GF-17S	15.7	103.2		P
LF-1046	18.4	99.5	96.4	GF-17S	15.7	103.2		P
LF-1047	22.0	97.9	94.9	GF-17S	15.7	103.2		P
LF-1048	18.3	96.1	93.1	GF-17S	15.7	103.2	LF-1050	F
LF-1049	13.4	99.2	96.1	GF-17S	15.7	103.2		P
LF-1050	21.2	97.7	94.7	GF-17S	15.7	103.2		P
LF-1051	17.5	98.9	95.8	GF-17S	15.7	103.2		P
LF-1052	18.3	102.3	99.1	GF-17S	15.7	103.2		P
LF-1053	21.6	97.9	94.9	GF-17S	15.7	103.2		P
LF-1054	22.8	97.0	94.0	GF-17S	15.7	103.2	LF-1071	F
LF-1055	18.9	99.6	96.5	GF-17S	15.7	103.2		P
LF-1056	19.2	101.3	98.2	GF-17S	15.7	103.2		P
LF-1057	9.3	103.8	100.6	GF-17S	15.7	103.2		P
LF-1058	14.7	101.5	98.4	GF-17S	15.7	103.2		P
LF-1059	7.5	103.9	100.7	GF-17S	15.7	103.2		P
LF-1060	5.5	100.5	97.4	GF-17S	15.7	103.2		P
LF-1061	9.3	104.0	100.8	GF-17S	15.7	103.2		P
LF-1062	7.1	105.3	102.0	GF-17S	15.7	103.2		P
LF-1063	9.1	102.7	99.5	GF-17S	15.7	103.2		P
LF-1064	11.9	104.1	100.9	GF-17S	15.7	103.2		P

Notes:

¹ Missing test numbers correspond to non-critical construction activities.² Required minimum percent compaction was 95% of Standard Proctor (ASTM D 698) maximum dry unit weight.³ Retest number for failing density tests.

Table 4-2 (continued)

COMPACTION TEST RESULTS FOR GENERAL FILL USED TO CONSTRUCT PERIMETER BERM

	NUCLEAR GAUGE			APPLICABLE STANDARD PROCTOR				
TEST STANDARD	ASTM D 3017	ASTM D 2922		ASTM D 698				
TESTING FREQUENCY	5 test per acre per lift or 1 test per 250 lf per lift			1 test per 25,000 yd³				
TEST RESULTS								
CQA Test No. ¹	Moisture Content (%)	Dry Unit Weight (pcf)	Percent Compaction ² (%)	Sample ID	Optimum Moisture Content (%)	Maximum Dry Unit Weight (pcf)	Retest No. ³	Pass/Fail (P/F)
LF-1065	11.6	99.2	96.1	GF-17S	15.7	103.2		P
LF-1066	10.9	98.0	95.0	GF-17S	15.7	103.2		P
LF-1067	13.0	98.4	95.3	GF-17S	15.7	103.2		P
LF-1068	14.8	97.9	94.9	GF-17S	15.7	103.2		P
LF-1069	10.4	101.3	98.2	GF-17S	15.7	103.2		P
LF-1070	5.1	102.9	99.7	GF-17S	15.7	103.2		P
LF-1071	18.6	103.0	99.8	GF-17S	15.7	103.2		P
LF-1072	20.0	100.1	97.0	GF-17S	15.7	103.2		P
LF-1073	18.1	100.0	96.9	GF-17S	15.7	103.2		P
LF-1074	18.6	99.3	96.2	GF-17S	15.7	103.2		P
LF-1075	21.1	97.6	94.6	GF-17S	15.7	103.2		P
LF-1076	19.3	98.3	95.3	GF-17S	15.7	103.2		P
LF-1077	13.6	104.1	100.9	GF-17S	15.7	103.2		P
LF-1078	19.9	99.7	96.6	GF-17S	15.7	103.2		P
LF-1079	18.3	100.5	97.4	GF-17S	15.7	103.2		P
LF-1080	15.7	103.8	100.6	GF-17S	15.7	103.2		P
LF-1081	13.4	105.8	102.5	GF-17S	15.7	103.2		P
LF-1082	11.3	100.9	97.8	GF-17S	15.7	103.2		P
LF-1083	9.9	109.0	105.6	GF-17S	15.7	103.2		P
LF-1084	13.8	100.0	96.9	GF-17S	15.7	103.2		P
LF-1085	9.9	102.7	99.5	GF-17S	15.7	103.2		P
LF-1086	9.5	98.7	95.6	GF-17S	15.7	103.2		P
LF-1087	17.6	98.6	95.5	GF-17S	15.7	103.2		P
LF-1088	13.4	100.6	97.5	GF-17S	15.7	103.2		P
LF-1089	14.7	99.2	96.1	GF-17S	15.7	103.2		P
LF-1090	8.6	103.0	99.8	GF-17S	15.7	103.2		P
LF-1091	9.5	106.2	102.9	GF-17S	15.7	103.2		P
LF-1092	15.9	103.7	100.5	GF-17S	15.7	103.2		P
LF-1093	13.9	104.6	101.4	GF-17S	15.7	103.2		P
LF-1094	11.3	103.9	100.7	GF-17S	15.7	103.2		P
LF-1095	12.1	105.2	101.9	GF-17S	15.7	103.2		P
LF-1096	8.8	102.5	99.3	GF-17S	15.7	103.2		P
LF-1097	10.3	106.9	103.6	GF-17S	15.7	103.2		P
LF-1098	7.5	104.0	100.8	GF-17S	15.7	103.2		P
LF-1099	11.5	99.8	96.7	GF-17S	15.7	103.2		P
LF-1100	12.2	97.8	94.8	GF-17S	15.7	103.2		P
LF-1101	10.6	103.3	100.1	GF-17S	15.7	103.2		P
LF-1102	11.5	103.0	99.8	GF-17S	15.7	103.2		P
LF-1103	8.1	106.1	102.8	GF-17S	15.7	103.2		P
LF-1104	17.7	98.7	95.6	GF-17S	15.7	103.2		P
Notes:								
1 Missing test numbers correspond to non-critical construction activities.								
2 Required minimum percent compaction was 95% of Standard Proctor (ASTM D 698) maximum dry unit weight.								
3 Retest number for failing density tests.								

Table 4-2 (continued)

COMPACTION TEST RESULTS FOR GENERAL FILL USED TO CONSTRUCT PERIMETER BERM

		NUCLEAR GAUGE		APPLICABLE STANDARD PROCTOR				
TEST STANDARD	ASTM D 3017	ASTM D 2922		ASTM D 698				
TESTING FREQUENCY	5 test per acre per lift or 1 test per 250 lf per lift			1 test per 25,000 yd ³				
TEST RESULTS								
CQA Test No. ¹	Moisture Content (%)	Dry Unit Weight (pcf)	Percent Compaction ² (%)	Sample ID	Optimum Moisture Content (%)	Maximum Dry Unit Weight (pcf)	Retest No. ³	Pass/Fail (P/F)
LF-1105	12.1	106.0	102.7	GF-17S	15.7	103.2		P
LF-1106	17.2	103.1	99.9	GF-17S	15.7	103.2		P
LF-1107	9.2	105.0	101.7	GF-17S	15.7	103.2		P
LF-1108	8.9	107.9	104.6	GF-17S	15.7	103.2		P
LF-1109	11.7	105.7	102.4	GF-17S	15.7	103.2		P
LF-1110	16.3	105.2	101.9	GF-17S	15.7	103.2		P
LF-1111	9.2	102.4	99.2	GF-17S	15.7	103.2		P
LF-1112	5.8	100.0	96.9	GF-17S	15.7	103.2		P
LF-1113	17.1	102.9	99.7	GF-17S	15.7	103.2		P
LF-1114	9.6	107.3	104.0	GF-17S	15.7	103.2		P
LF-1115	10.7	107.3	104.0	GF-17S	15.7	103.2		P
LF-1116	8.7	107.6	104.3	GF-17S	15.7	103.2		P
LF-1117	11.9	100.6	97.5	GF-17S	15.7	103.2		P
LF-1118	11.6	103.5	100.3	GF-17S	15.7	103.2		P
LF-1119	10.0	107.5	104.2	GF-17S	15.7	103.2		P
LF-1120	8.9	100.1	97.0	GF-17S	15.7	103.2		P
LF-1121	10.2	104.4	101.2	GF-17S	15.7	103.2		P
LF-1122	6.4	102.7	99.5	GF-17S	15.7	103.2		P
LF-1123	9.0	105.0	101.7	GF-17S	15.7	103.2		P
LF-1124	15.7	97.9	94.9	GF-17S	15.7	103.2		P
LF-1125	8.0	107.1	103.8	GF-17S	15.7	103.2		P
LF-1126	15.1	104.3	101.1	GF-17S	15.7	103.2		P
LF-1127	17.1	104.5	101.3	GF-17S	15.7	103.2		P
LF-1128	13.7	104.9	101.6	GF-17S	15.7	103.2		P
LF-1129	17.8	102.9	99.7	GF-17S	15.7	103.2		P
LF-1130	15.3	105.3	102.0	GF-17S	15.7	103.2		P
LF-1131	19.1	100.4	97.3	GF-17S	15.7	103.2		P
LF-1132	11.7	99.7	96.6	GF-17S	15.7	103.2		P
LF-1133	16.6	104.8	101.6	GF-17S	15.7	103.2		P
LF-1134	16.5	100.1	97.0	GF-17S	15.7	103.2		P
LF-1135	19.0	100.4	97.3	GF-17S	15.7	103.2		P
LF-1136	19.1	99.1	96.0	GF-17S	15.7	103.2		P
LF-1137	15.3	98.8	95.7	GF-17S	15.7	103.2		P
LF-1138	12.7	103.0	99.8	GF-17S	15.7	103.2		P
LF-1139	12.1	101.3	98.2	GF-17S	15.7	103.2		P
LF-1140	17.3	101.9	98.7	GF-17S	15.7	103.2		P
LF-1141	13.0	103.9	100.7	GF-17S	15.7	103.2		P
LF-1142	16.0	100.6	97.5	GF-17S	15.7	103.2		P
LF-1143	21.1	98.0	95.0	GF-17S	15.7	103.2		P
LF-1144	15.9	102.8	99.6	GF-17S	15.7	103.2		P
Notes:								
1 Missing test numbers correspond to non-critical construction activities.								
2 Required minimum percent compaction was 95% of Standard Proctor (ASTM D 698) maximum dry unit weight.								
3 Retest number for failing density tests.								

Table 4-2 (continued)

COMPACTION TEST RESULTS FOR GENERAL FILL USED TO CONSTRUCT PERIMETER BERM

	NUCLEAR GAUGE			APPLICABLE STANDARD PROCTOR				
TEST STANDARD	ASTM D 3017	ASTM D 2922		ASTM D 698				
TESTING FREQUENCY	5 test per acre per lift or 1 test per 250 lf per lift			1 test per 25,000 yd³				
TEST RESULTS								
CQA Test No. ¹	Moisture Content (%)	Dry Unit Weight (pcf)	Percent Compaction ² (%)	Sample ID	Optimum Moisture Content (%)	Maximum Dry Unit Weight (pcf)	Retest No. ³	Pass/Fail (P/F)
LF-1145	15.9	101.4	98.3	GF-17S	15.7	103.2		P
LF-1146	18.9	100.0	96.9	GF-17S	15.7	103.2		P
LF-1147	11.9	101.8	98.6	GF-17S	15.7	103.2		P
LF-1148	16.4	103.5	100.3	GF-17S	15.7	103.2		P
LF-1149	17.7	101.5	98.4	GF-17S	15.7	103.2		P
LF-1150	17.7	101.6	98.4	GF-17S	15.7	103.2		P
LF-1151	16.6	100.4	97.3	GF-17S	15.7	103.2		P
LF-1152	14.8	100.9	97.8	GF-17S	15.7	103.2		P
LF-1153	16.8	104.3	101.1	GF-17S	15.7	103.2		P
LF-1154	18.6	102.9	99.7	GF-17S	15.7	103.2		P
LF-1155	15.5	100.1	97.0	GF-17S	15.7	103.2		P
LF-1156	18.5	98.3	95.3	GF-17S	15.7	103.2		P
LF-1157	17.5	99.8	96.7	GF-17S	15.7	103.2		P
LF-1158	20.3	99.3	96.2	GF-17S	15.7	103.2		P
LF-1159	19.5	98.2	95.2	GF-17S	15.7	103.2		P
LF-1160	19.5	100.7	97.6	GF-17S	15.7	103.2		P
LF-1161	14.3	100.9	97.8	GF-17S	15.7	103.2		P
LF-1162	18.5	99.1	96.0	GF-17S	15.7	103.2		P
LF-1163	16.7	101.1	98.0	GF-17S	15.7	103.2		P
LF-1164	19.6	97.8	94.8	GF-17S	15.7	103.2		P
LF-1165	19.3	98.8	95.7	GF-17S	15.7	103.2		P
LF-1166	18.5	99.0	95.9	GF-17S	15.7	103.2		P
LF-1167	18.3	99.6	96.5	GF-17S	15.7	103.2		P
LF-1168	18.0	98.9	95.8	GF-17S	15.7	103.2		P
LF-1169	18.3	97.9	94.9	GF-17S	15.7	103.2		P
LF-1170	17.1	100.9	97.8	GF-17S	15.7	103.2		P
LF-1171	16.0	103.0	99.8	GF-17S	15.7	103.2		P
LF-1172	8.4	99.5	96.4	GF-17S	15.7	103.2		P
LF-1173	13.2	101.1	98.0	GF-17S	15.7	103.2		P
LF-1174	14.4	101.5	98.4	GF-17S	15.7	103.2		P
LF-1175	15.1	99.8	96.7	GF-17S	15.7	103.2		P
LF-1176	18.5	101.0	97.9	GF-17S	15.7	103.2		P
LF-1177	18.9	99.3	96.2	GF-17S	15.7	103.2		P
LF-1178	16.7	98.1	95.1	GF-17S	15.7	103.2		P
LF-1179	19.1	98.7	95.6	GF-17S	15.7	103.2		P
LF-1180	18.3	99.8	96.7	GF-17S	15.7	103.2		P
Notes:								
1 Missing test numbers correspond to non-critical construction activities.								
2 Required minimum percent compaction was 95% of Standard Proctor (ASTM D 698) maximum dry unit weight.								
3 Retest number for failing density tests.								

Table 4-2 (continued)

COMPACTION TEST RESULTS FOR GENERAL FILL USED TO CONSTRUCT PERIMETER BERM

	NUCLEAR GAUGE			APPLICABLE STANDARD PROCTOR				
TEST STANDARD	ASTM D 3017	ASTM D 2922	ASTM D 698					
TESTING FREQUENCY	5 test per acre per lift or 1 test per 250 lf per lift			1 test per 25,000 yd ³				
TEST RESULTS								
CQA Test No. ¹	Moisture Content (%)	Dry Unit Weight (pcf)	Percent Compaction ² (%)	Sample ID	Optimum Moisture Content (%)	Maximum Dry Unit Weight (pcf)	Retest No. ³	Pass/Fail (P/F)
LF-1181	17.8	99.1	96.0	GF-17S	15.7	103.2		P
LF-1182	12.3	99.2	96.1	GF-17S	15.7	103.2		P
LF-1183	12.1	98.0	95.0	GF-17S	15.7	103.2		P
LF-1184	15.6	100.9	97.8	GF-17S	15.7	103.2		P
LF-1185	14.1	104.0	100.8	GF-17S	15.7	103.2		P
LF-1186	13.7	101.2	98.1	GF-17S	15.7	103.2		P
LF-1187	13.9	101.7	98.5	GF-17S	15.7	103.2		P
LF-1188	17.4	98.4	95.3	GF-17S	15.7	103.2		P
LF-1189	18.2	98.7	95.6	GF-17S	15.7	103.2		P
LF-1190	16.6	100.9	97.8	GF-17S	15.7	103.2		P
LF-1191	18.0	100.0	96.9	GF-17S	15.7	103.2		P
LF-1192	18.1	99.1	96.0	GF-17S	15.7	103.2		P
LF-1193	20.6	97.6	94.6	GF-17S	15.7	103.2		P
LF-1194	16.5	98.2	95.2	GF-17S	15.7	103.2		P
LF-1195	16.4	101.8	98.6	GF-17S	15.7	103.2		P
LF-1196	19.8	98.1	95.1	GF-17S	15.7	103.2		P
LF-1197	16.3	98.3	95.3	GF-17S	15.7	103.2		P
LF-1198	14.4	102.2	99.0	GF-17S	15.7	103.2		P
LF-1199	11.4	100.8	97.7	GF-17S	15.7	103.2		P
LF-1200	18.4	99.8	96.7	GF-17S	15.7	103.2		P
LF-1201	14.4	99.6	96.5	GF-17S	15.7	103.2		P
LF-1202	14.5	103.2	100.0	GF-17S	15.7	103.2		P
LF-1203	12.4	97.9	94.9	GF-17S	15.7	103.2		P
LF-1204	20.1	98.2	95.2	GF-17S	15.7	103.2		P
LF-1205	12.8	105.8	102.5	GF-17S	15.7	103.2		P
LF-1206	19.4	101.4	98.3	GF-17S	15.7	103.2		P
LF-1207	18.5	100.5	97.4	GF-17S	15.7	103.2		P
LF-1208	17.5	101.3	98.2	GF-17S	15.7	103.2		P
LF-1209	16.4	101.0	97.9	GF-17S	15.7	103.2		P
LF-1210	12.7	107.8	104.5	GF-17S	15.7	103.2		P
LF-1211	19.2	97.8	94.8	GF-17S	15.7	103.2		P
LF-1212	18.6	101.2	98.1	GF-17S	15.7	103.2		P
LF-1213	19.5	98.0	95.0	GF-17S	15.7	103.2		P
LF-1214	18.5	101.7	98.5	GF-17S	15.7	103.2		P
LF-1215	15.6	105.8	102.5	GF-17S	15.7	103.2		P
LF-1216	15.1	100.3	97.2	GF-17S	15.7	103.2		P
LF-1217	18.3	99.4	96.3	GF-17S	15.7	103.2		P
LF-1218	16.8	101.1	98.0	GF-17S	15.7	103.2		P
LF-1219	20.0	98.7	95.6	GF-17S	15.7	103.2		P
LF-1220	15.6	98.6	95.5	GF-17S	15.7	103.2		P

Notes:

¹ Missing test numbers correspond to non-critical construction activities.

² Required minimum percent compaction was 95% of Standard Proctor (ASTM D 698) maximum dry unit weight.

³ Retest number for failing density tests.

Table 4-2 (continued)

COMPACTION TEST RESULTS FOR GENERAL FILL USED TO CONSTRUCT PERIMETER BERM

	NUCLEAR GAUGE			APPLICABLE STANDARD PROCTOR				
TEST STANDARD	ASTM D 3017	ASTM D 2922	ASTM D 698					
TESTING FREQUENCY	5 test per acre per lift or 1 test per 250 lf per lift			1 test per 25,000 yd ³				
TEST RESULTS								
CQA Test No. ¹	Moisture Content (%)	Dry Unit Weight (pcf)	Percent Compaction ² (%)	Sample ID	Optimum Moisture Content (%)	Maximum Dry Unit Weight (pcf)	Retest No. ³	Pass/Fail (P/F)
LF-1221	20.9	97.8	94.8	GF-17S	15.7	103.2		P
LF-1222	17.3	98.8	95.7	GF-17S	15.7	103.2		P
LF-1223	20.4	97.7	94.7	GF-17S	15.7	103.2		P
LF-1224	16.6	100.6	97.5	GF-17S	15.7	103.2		P
LF-1225	10.4	110.1	105.2	GF-16S	16.8	104.7		P
LF-1226	9.5	101.6	97.0	GF-16S	16.8	104.7		P
LF-1227	12.3	107.5	102.7	GF-16S	16.8	104.7		P
LF-1228	12.8	107.0	102.2	GF-16S	16.8	104.7		P
LF-1229	14.8	105.7	101.0	GF-16S	16.8	104.7		P
LF-1230	10.4	106.5	101.7	GF-16S	16.8	104.7		P
LF-1231	14.2	105.3	100.6	GF-16S	16.8	104.7		P
LF-1232	15.4	103.3	98.7	GF-16S	16.8	104.7		P
LF-1233	9.4	104.7	100.0	GF-16S	16.8	104.7		P
LF-1234	12.4	109.5	104.6	GF-16S	16.8	104.7		P
LF-1235	11.0	106.6	101.8	GF-16S	16.8	104.7		P
LF-1236	9.1	105.5	100.8	GF-16S	16.8	104.7		P
LF-1237	9.2	108.8	103.9	GF-16S	16.8	104.7		P
LF-1238	6.9	109.0	104.1	GF-16S	16.8	104.7		P
LF-1239	8.0	108.4	103.5	GF-16S	16.8	104.7		P
LF-1240	11.8	110.0	105.1	GF-16S	16.8	104.7		P
LF-1241	8.5	107.3	102.5	GF-16S	16.8	104.7		P
LF-1242	7.4	101.9	97.3	GF-16S	16.8	104.7		P
LF-1243	15.0	107.9	103.1	GF-16S	16.8	104.7		P
LF-1244	11.5	101.0	96.5	GF-16S	16.8	104.7		P
LF-1245	20.9	98.9	95.8	GF-17S	15.7	103.2		P
LF-1246	22.0	98.3	95.3	GF-17S	15.7	103.2		P
LF-1247	21.2	98.2	95.2	GF-17S	15.7	103.2		P
LF-1248	12.2	105.1	101.8	GF-17S	15.7	103.2		P
LF-1249	19.4	102.6	99.4	GF-17S	15.7	103.2		P
LF-1250	18.4	102.4	99.2	GF-17S	15.7	103.2		P
LF-1251	20.2	99.3	96.2	GF-17S	15.7	103.2		P
LF-1252	20.0	98.9	95.8	GF-17S	15.7	103.2		P
LF-1253	16.3	103.2	100.0	GF-17S	15.7	103.2		P
LF-1254	15.3	104.3	101.1	GF-17S	15.7	103.2		P
LF-1255	12.9	105.9	102.6	GF-17S	15.7	103.2		P
LF-1258B	8.4	105.7	101.0	GF-16S	16.8	104.7		P
LF-1259B	9.6	104.4	99.7	GF-16S	16.8	104.7		P
LF-1260B	15.1	107.5	102.7	GF-16S	16.8	104.7		P
LF-1261B	16.4	107.4	102.6	GF-16S	16.8	104.7		P
LF-1262B	14.4	106.0	101.2	GF-16S	16.8	104.7		P
Notes:								
1 Missing test numbers correspond to non-critical construction activities.								
2 Required minimum percent compaction was 95% of Standard Proctor (ASTM D 698) maximum dry unit weight.								
3 Retest number for failing density tests.								

Table 4-2 (continued)

COMPACTION TEST RESULTS FOR GENERAL FILL USED TO CONSTRUCT PERIMETER BERM

	NUCLEAR GAUGE			APPLICABLE STANDARD PROCTOR				
TEST STANDARD	ASTM D 3017	ASTM D 2922	ASTM D 698					
TESTING FREQUENCY	5 test per acre per lift or 1 test per 250 lf per lift			1 test per 25,000 yd³				
TEST RESULTS								
CQA Test No. ¹	Moisture Content (%)	Dry Unit Weight (pcf)	Percent Compaction ² (%)	Sample ID	Optimum Moisture Content (%)	Maximum Dry Unit Weight (pcf)	Retest No. ³	Pass/Fail (P/F)
LF-1263B	15.8	107.4	102.6	GF-16S	16.8	104.7		P
LF-1264B	12.3	105.0	100.3	GF-16S	16.8	104.7		P
LF-1265B	13.0	109.8	104.9	GF-16S	16.8	104.7		P
LF-1266B	12.1	107.8	103.0	GF-16S	16.8	104.7		P
LF-1267B	14.3	106.2	101.4	GF-16S	16.8	104.7		P
LF-1268B	12.7	107.9	103.1	GF-16S	16.8	104.7		P
LF-1269B	12.1	109.7	104.8	GF-16S	16.8	104.7		P
LF-1270B	12.6	105.2	100.5	GF-16S	16.8	104.7		P
LF-1271B	13.4	106.8	102.0	GF-16S	16.8	104.7		P
LF-1274	12.0	107.1	103.8	GF-17S	15.7	103.2		P
LF-1275	18.3	98.5	95.4	GF-17S	15.7	103.2		P
LF-1276	14.9	101.9	98.7	GF-17S	15.7	103.2		P
LF-1277	11.3	105.5	102.2	GF-17S	15.7	103.2		P
LF-1278	14.7	100.3	97.2	GF-17S	15.7	103.2		P
LF-1279	15.2	102.7	99.5	GF-17S	15.7	103.2		P
LF-1280	12.5	103.0	99.8	GF-17S	15.7	103.2		P
LF-1281	14.7	103.1	99.9	GF-17S	15.7	103.2		P
LF-1282	18.5	99.9	96.8	GF-17S	15.7	103.2		P
LF-1283	17.4	101.9	98.7	GF-17S	15.7	103.2		P
LF-1284	9.8	102.5	99.3	GF-17S	15.7	103.2		P
LF-1285	14.0	103.0	99.8	GF-17S	15.7	103.2		P
LF-1288	17.0	105.3	100.6	GF-16S	16.8	104.7		P
LF-1289	14.5	106.7	101.9	GF-16S	16.8	104.7		P
LF-1290	21.8	99.3	94.8	GF-16S	16.8	104.7		P
LF-1291	21.2	99.8	95.3	GF-16S	16.8	104.7		P
LF-1292	15.2	103.9	99.2	GF-16S	16.8	104.7		P
LF-1293	15.2	106.5	101.7	GF-16S	16.8	104.7		P
LF-1294	19.4	100.8	96.3	GF-16S	16.8	104.7		P
LF-1295	13.5	108.4	103.5	GF-16S	16.8	104.7		P
LF-1296	10.7	108.2	103.3	GF-16S	16.8	104.7		P
LF-1297	9.9	105.2	100.5	GF-16S	16.8	104.7		P
LF-1298	10.2	106.8	102.0	GF-16S	16.8	104.7		P
LF-1299	10.9	105.9	101.1	GF-16S	16.8	104.7		P
LF-1300	10.3	108.4	103.5	GF-16S	16.8	104.7		P
LF-1301	13.8	102.4	97.8	GF-16S	16.8	104.7		P
LF-1302	12.9	102.0	97.4	GF-16S	16.8	104.7		P
LF-1303	12.2	106.9	102.1	GF-16S	16.8	104.7		P
LF-1304	12.8	99.3	94.8	GF-16S	16.8	104.7		P
LF-1305	12.0	101.7	97.1	GF-16S	16.8	104.7		P
LF-1306	13.3	100.9	96.4	GF-16S	16.8	104.7		P
Notes:								
1 Missing test numbers correspond to non-critical construction activities.								
2 Required minimum percent compaction was 95% of Standard Proctor (ASTM D 698) maximum dry unit weight.								
3 Retest number for failing density tests.								

Table 4-2 (continued)

COMPACTION TEST RESULTS FOR GENERAL FILL USED TO CONSTRUCT PERIMETER BERM

		NUCLEAR GAUGE		APPLICABLE STANDARD PROCTOR				
TEST STANDARD	ASTM D 3017	ASTM D 2922		ASTM D 698				
TESTING FREQUENCY	5 test per acre per lift or 1 test per 250 lf per lift			1 test per 25,000 yd ³				
TEST RESULTS								
CQA Test No. ¹	Moisture Content (%)	Dry Unit Weight (pcf)	Percent Compaction ² (%)	Sample ID	Optimum Moisture Content (%)	Maximum Dry Unit Weight (pcf)	Retest No. ³	Pass/Fail (P/F)
LF-1307	11.3	103.0	98.4	GF-16S	16.8	104.7		P
LF-1308	13.8	100.3	95.8	GF-16S	16.8	104.7		P
LF-1309	12.6	101.2	96.7	GF-16S	16.8	104.7		P
LF-1310	13.0	102.7	98.1	GF-16S	16.8	104.7		P
LF-1331	15.2	103.6	98.2	GF-18S	15.8	105.5		P
LF-1332	13.3	103.3	97.9	GF-18S	15.8	105.5		P
LF-1333	15.2	106.5	100.9	GF-18S	15.8	105.5		P
LF-1334	13.1	102.8	97.4	GF-18S	15.8	105.5		P
LF-1335	14.4	106.6	101.0	GF-18S	15.8	105.5		P
LF-1336	14.1	108.3	102.7	GF-18S	15.8	105.5		P
LF-1337	14.3	106.2	100.7	GF-18S	15.8	105.5		P
LF-1338	11.6	101.5	96.2	GF-18S	15.8	105.5		P
LF-1339	10.3	102.6	97.3	GF-18S	15.8	105.5		P
LF-1340	22.6	94.9	90.0	GF-18S	15.8	105.5	LF-1402	F
LF-1341	23.1	95.7	90.7	GF-18S	15.8	105.5	LF-1401	F
LF-1342	13.6	101.3	96.0	GF-18S	15.8	105.5		P
LF-1343	12.3	105.3	99.8	GF-18S	15.8	105.5		P
LF-1344	13.7	100.0	94.8	GF-18S	15.8	105.5		P
LF-1345	14.2	101.2	95.9	GF-18S	15.8	105.5		P
LF-1346	15.6	102.3	97.0	GF-18S	15.8	105.5		P
LF-1347	14.8	102.5	97.2	GF-18S	15.8	105.5		P
LF-1348	13.3	103.6	98.2	GF-18S	15.8	105.5		P
LF-1349	14.5	105.0	99.5	GF-18S	15.8	105.5		P
LF-1350	15.1	102.3	97.0	GF-18S	15.8	105.5		P
LF-1351	14.1	105.8	100.3	GF-18S	15.8	105.5		P
LF-1352	13.9	101.3	96.0	GF-18S	15.8	105.5		P
LF-1353	15.7	104.1	98.7	GF-18S	15.8	105.5		P
LF-1354	14.5	104.4	99.0	GF-18S	15.8	105.5		P
LF-1355	15.6	102.7	97.3	GF-18S	15.8	105.5		P
LF-1356	12.4	103.5	98.1	GF-18S	15.8	105.5		P
LF-1357	13.0	104.3	98.9	GF-18S	15.8	105.5		P
LF-1358	13.3	103.5	98.1	GF-18S	15.8	105.5		P
LF-1359	16.1	103.2	97.8	GF-18S	15.8	105.5		P
LF-1360b	19.7	99.7	96.6	GF-17S	15.7	103.2		P
LF-1361	14.5	101.3	96.8	GF-19S	15.6	104.6		P
LF-1362	14.1	101.7	97.2	GF-19S	15.6	104.6		P
LF-1363	12.6	102.5	98.0	GF-19S	15.6	104.6		P
LF-1364	13.8	103.4	98.9	GF-19S	15.6	104.6		P
LF-1365	12.8	103.0	98.5	GF-19S	15.6	104.6		P
LF-1390	21.4	99.6	95.2	GF-19S	15.6	104.6		P
Notes:								
1 Missing test numbers correspond to non-critical construction activities.								
2 Required minimum percent compaction was 95% of Standard Proctor (ASTM D 698) maximum dry unit weight.								
3 Retest number for failing density tests.								

Table 4-2 (continued)

COMPACTION TEST RESULTS FOR GENERAL FILL USED TO CONSTRUCT PERIMETER BERM

	NUCLEAR GAUGE			APPLICABLE STANDARD PROCTOR				
TEST STANDARD	ASTM D 3017	ASTM D 2922		ASTM D 698				
TESTING FREQUENCY	5 test per acre per lift or 1 test per 250 lf per lift			1 test per 25,000 yd³				
TEST RESULTS								
CQA Test No. ¹	Moisture Content (%)	Dry Unit Weight (pcf)	Percent Compaction ² (%)	Sample ID	Optimum Moisture Content (%)	Maximum Dry Unit Weight (pcf)	Retest No. ³	Pass/Fail (P/F)
LF-1391	14.5	100.7	96.3	GF-19S	15.6	104.6		P
LF-1392	18.9	99.7	95.3	GF-19S	15.6	104.6		P
LF-1393	20.8	99.6	95.2	GF-19S	15.6	104.6		P
LF-1394	14.3	105.3	100.7	GF-19S	15.6	104.6		P
LF-1395	14.0	102.8	98.3	GF-19S	15.6	104.6		P
LF-1396	14.8	102.9	98.4	GF-19S	15.6	104.6		P
LF-1397	20.6	99.1	94.7	GF-19S	15.6	104.6		P
LF-1398	17.1	105.3	100.7	GF-19S	15.6	104.6		P
LF-1399	19.3	99.7	95.3	GF-19S	15.6	104.6		P
LF-1400	16.7	104.3	99.7	GF-19S	15.6	104.6		P
LF-1401	13.9	106.7	101.1	GF-18S	15.8	105.5		P
LF-1402	11.6	108.1	102.5	GF-18S	15.8	105.5		P
LF-1403	12.3	108.3	102.7	GF-18S	15.8	105.5		P
LF-1404	12.8	106.1	100.6	GF-18S	15.8	105.5		P
LF-1405	17.4	101.8	96.5	GF-18S	15.8	105.5		P
LF-1406	11.7	107.4	101.8	GF-18S	15.8	105.5		P
LF-1407	10.8	107.7	102.1	GF-18S	15.8	105.5		P
LF-1408	11.1	108.5	102.8	GF-18S	15.8	105.5		P
LF-1409	13.4	106.5	100.9	GF-18S	15.8	105.5		P
LF-1410	12.6	109.3	103.6	GF-18S	15.8	105.5		P
LF-1411	14.5	106.2	100.7	GF-18S	15.8	105.5		P
LF-1412	15.5	107.2	101.6	GF-18S	15.8	105.5		P
LF-1413	15.2	105.8	100.3	GF-18S	15.8	105.5		P
LF-1414	14.0	104.0	98.6	GF-18S	15.8	105.5		P
LF-1415	13.7	106.6	101.0	GF-18S	15.8	105.5		P
LF-1416	13.1	107.5	101.9	GF-18S	15.8	105.5		P
LF-1417	15.2	104.7	99.2	GF-18S	15.8	105.5		P
LF-1418	14.3	105.3	99.8	GF-18S	15.8	105.5		P
LF-1421	17.2	100.9	95.6	GF-18S	15.8	105.5		P
LF-1422	19.9	99.8	94.6	GF-18S	15.8	105.5		P
LF-1423	15.7	101.1	95.8	GF-18S	15.8	105.5		P
LF-1424	16.3	101.5	96.2	GF-18S	15.8	105.5		P
LF-1425	12.7	101.5	96.2	GF-18S	15.8	105.5		P
LF-1426	14.5	101.0	95.7	GF-18S	15.8	105.5		P
LF-1427	12.2	101.9	96.6	GF-18S	15.8	105.5		P
LF-1428	15.8	100.0	94.8	GF-18S	15.8	105.5		P
LF-1439	13.1	102.7	97.3	GF-18S	15.8	105.5		P
LF-1140	14.5	102.0	96.7	GF-18S	15.8	105.5		P
LF-1441	13.6	103.5	98.1	GF-18S	15.8	105.5		P
LF-1442	13.0	103.0	97.6	GF-18S	15.8	105.5		P
Notes:								
1 Missing test numbers correspond to non-critical construction activities.								
2 Required minimum percent compaction was 95% of Standard Proctor (ASTM D 698) maximum dry unit weight.								
3 Retest number for failing density tests.								

Table 4-2 (continued)

COMPACTION TEST RESULTS FOR GENERAL FILL USED TO CONSTRUCT PERIMETER BERM

	NUCLEAR GAUGE			APPLICABLE STANDARD PROCTOR				
TEST STANDARD	ASTM D 3017	ASTM D 2922	ASTM D 698					
TESTING FREQUENCY	5 test per acre per lift or 1 test per 250 lf per lift			1 test per 25,000 yd ³				
TEST RESULTS								
CQA Test No. ¹	Moisture Content (%)	Dry Unit Weight (pcf)	Percent Compaction ² (%)	Sample ID	Optimum Moisture Content (%)	Maximum Dry Unit Weight (pcf)	Retest No. ³	Pass/Fail (P/F)
LF-1443	14.2	103.4	98.0	GF-18S	15.8	105.5		P
LF-1444	13.9	102.1	96.8	GF-18S	15.8	105.5		P
LF-1445	14.8	104.6	99.1	GF-18S	15.8	105.5		P
LF-1446	20.0	99.2	94.8	GF-19S	15.6	104.6		P
LF-1447	20.3	99.7	95.3	GF-19S	15.6	104.6		P
LF-1448	20.0	100.7	96.3	GF-19S	15.6	104.6		P
LF-1449	16.7	100.3	95.9	GF-19S	15.6	104.6		P
LF-1450	15.8	99.4	95.0	GF-19S	15.6	104.6		P
LF-1451	18.7	99.6	95.2	GF-19S	15.6	104.6		P
LF-1452	19.8	99.8	95.4	GF-19S	15.6	104.6		P
LF-1453	20.9	99.2	94.8	GF-19S	15.6	104.6		P
LF-1454	15.9	101.1	96.7	GF-19S	15.6	104.6		P
LF-1455	15.6	101.8	97.3	GF-19S	15.6	104.6		P
LF-1456	9.8	102.5	97.2	GF-18S	15.8	105.5		P
LF-1457	10.3	106.2	100.7	GF-18S	15.8	105.5		P
LF-1458	8.5	103.2	97.8	GF-18S	15.8	105.5		P
LF-1459	9.6	101.9	96.6	GF-18S	15.8	105.5		P
LF-1460	10.4	101.4	96.1	GF-18S	15.8	105.5		P
LF-1461	9.2	101.0	95.7	GF-18S	15.8	105.5		P
LF-1462	12.4	103.2	97.8	GF-18S	15.8	105.5		P
LF-1463	10.0	103.6	98.2	GF-18S	15.8	105.5		P
LF-1464	9.7	101.5	96.2	GF-18S	15.8	105.5		P
LF-1465	11.1	106.5	100.9	GF-18S	15.8	105.5		P
LF-1466	10.4	101.1	95.8	GF-18S	15.8	105.5		P
LF-1467	9.6	101.3	96.0	GF-18S	15.8	105.5		P
LF-1499	9.1	109.2	103.5	GF-18S	15.8	105.5		P
LF-1500	12.1	104.7	99.2	GF-18S	15.8	105.5		P
LF-1501	12.1	106.1	100.6	GF-18S	15.8	105.5		P
LF-1502	14.3	102.6	97.3	GF-18S	15.8	105.5		P
LF-1503	11.5	103.8	98.4	GF-18S	15.8	105.5		P
LF-1504	8.0	105.2	99.7	GF-18S	15.8	105.5		P
LF-1505	11.9	102.1	96.8	GF-18S	15.8	105.5		P
LF-1506	10.2	108.1	102.5	GF-18S	15.8	105.5		P
LF-1507	14.8	103.9	98.5	GF-18S	15.8	105.5		P
LF-1508	12.9	103.8	98.4	GF-18S	15.8	105.5		P
LF-1509	16.9	104.5	99.1	GF-18S	15.8	105.5		P
LF-1510	14.7	103.9	98.5	GF-18S	15.8	105.5		P
LF-1511	13.0	106.9	101.3	GF-18S	15.8	105.5		P
LF-1512	7.6	109.6	103.9	GF-18S	15.8	105.5		P
LF-1513	11.2	103.8	98.4	GF-18S	15.8	105.5		P
Notes:								
1 Missing test numbers correspond to non-critical construction activities.								
2 Required minimum percent compaction was 95% of Standard Proctor (ASTM D 698) maximum dry unit weight.								
3 Retest number for failing density tests.								

Table 4-2 (continued)

COMPACTION TEST RESULTS FOR GENERAL FILL USED TO CONSTRUCT PERIMETER BERM

	NUCLEAR GAUGE			APPLICABLE STANDARD PROCTOR				
TEST STANDARD	ASTM D 3017	ASTM D 2922		ASTM D 698				
TESTING FREQUENCY	5 test per acre per lift or 1 test per 250 lf per lift			1 test per 25,000 yd ³				
TEST RESULTS								
CQA Test No. ¹	Moisture Content (%)	Dry Unit Weight (pcf)	Percent Compaction ² (%)	Sample ID	Optimum Moisture Content (%)	Maximum Dry Unit Weight (pcf)	Retest No. ³	Pass/Fail (P/F)
LF-1514	8.8	108.1	102.5	GF-18S	15.8	105.5		P
LF-1531	14.9	100.2	95.8	GF-19S	15.6	104.6		P
LF-1532	14.1	99.4	95.0	GF-19S	15.6	104.6		P
LF-1533	20.1	99.0	94.6	GF-19S	15.6	104.6		P
LF-1534	19.5	99.4	95.0	GF-19S	15.6	104.6		P
LF-1535	20.0	99.8	95.4	GF-19S	15.6	104.6		P
LF-1536	19.3	100.0	95.6	GF-19S	15.6	104.6		P
LF-1537	17.2	100.3	95.9	GF-19S	15.6	104.6		P
LF-1538	19.4	99.6	95.2	GF-19S	15.6	104.6		P
LF-1539	18.7	99.2	94.8	GF-19S	15.6	104.6		P
LF-1540	18.3	99.9	95.5	GF-19S	15.6	104.6		P
LF-1541	19.1	100.2	95.8	GF-19S	15.6	104.6		P
LF-1578	16.1	101.7	95.4	GF-21S	14.8	106.6		P
LF-1579	17.6	100.3	95.2	GF-20S	15.1	105.3		P
LF-1580	15.7	100.2	95.1	GF-20S	15.1	105.3		P
LF-1581	17.3	101.3	96.2	GF-20S	15.1	105.3		P
LF-1582	7.0	107.1	101.7	GF-20S	15.1	105.3		P
LF-1583	12.7	105.4	100.1	GF-20S	15.1	105.3		P
LF-1584	7.4	105.2	99.9	GF-20S	15.1	105.3		P
LF-1585	6.1	106.9	101.5	GF-20S	15.1	105.3		P
LF-1586	9.8	111.7	106.0	GF-20S	15.1	105.3		P
LF-1587	8.6	111.0	105.4	GF-20S	15.1	105.3		P
LF-1588	12.3	101.4	96.3	GF-20S	15.1	105.3		P
LF-1589	7.0	105.3	100.0	GF-20S	15.1	105.3		P
LF-1590	11.7	105.9	100.6	GF-20S	15.1	105.3		P
LF-1591	12.8	106.8	101.4	GF-20S	15.1	105.3		P
LF-1595	12.3	103.8	98.0	GF-20S	15.1	105.3		P
LF-1596	13.1	100.9	95.2	GF-20S	15.1	105.3		P
LF-1597	14.9	100.6	95.5	GF-20S	15.1	105.3		P
LF-1598	23.0	98.5	93.5	GF-20S	15.1	105.3	LF-1621	F
LF-1599	16.0	105.3	100.0	GF-20S	15.1	105.3		P
LF-1600	13.8	101.0	96.0	GF-20S	15.1	105.3		P
LF-1601	15.3	104.5	99.3	GF-20S	15.1	105.3		P
LF-1602	11.8	101.6	96.5	GF-20S	15.1	105.3		P
LF-1603	9.4	108.8	103.3	GF-20S	15.1	105.3		P
LF-1604	13.7	105.2	99.9	GF-20S	15.1	105.3		P
LF-1621	20.1	98.3	93.3	GF-21S	14.8	106.6	LF-1641	F
LF-1622	11.3	104.7	98.2	GF-21S	14.8	106.6		P
LF-1623	13.3	107.3	100.7	GF-21S	14.8	106.6		P
LF-1624	10.1	107.3	100.6	GF-21S	14.8	106.6		P
Notes:								
1 Missing test numbers correspond to non-critical construction activities.								
2 Required minimum percent compaction was 95% of Standard Proctor (ASTM D 698) maximum dry unit weight.								
3 Retest number for failing density tests.								

Table 4-2 (continued)

COMPACTION TEST RESULTS FOR GENERAL FILL USED TO CONSTRUCT PERIMETER BERM

	NUCLEAR GAUGE			APPLICABLE STANDARD PROCTOR				
TEST STANDARD	ASTM D 3017	ASTM D 2922		ASTM D 698				
TESTING FREQUENCY	5 test per acre per lift or 1 test per 250 lf per lift			1 test per 25,000 yd ³				
TEST RESULTS								
CQA Test No. ¹	Moisture Content (%)	Dry Unit Weight (pcf)	Percent Compaction ² (%)	Sample ID	Optimum Moisture Content (%)	Maximum Dry Unit Weight (pcf)	Retest No. ³	Pass/Fail (P/F)
LF-1625	8.7	106.3	99.7	GF-21S	14.8	106.6		P
LF-1626	10.0	103.7	97.3	GF-21S	14.8	106.6		P
LF-1627	17.1	96.8	90.8	GF-21S	14.8	106.6	LF-1666	F
LF-1628	19.2	101.4	95.1	GF-21S	14.8	106.6		P
LF-1629	17.2	103.7	97.3	GF-21S	14.8	106.6		P
LF-1630	17.8	102.8	96.4	GF-21S	14.8	106.6		P
LF-1631	16.6	102.6	96.2	GF-21S	14.8	106.6		P
LF-1632	14.0	105.8	99.3	GF-21S	14.8	106.6		P
LF-1641	17.3	101.1	96.0	GF-20S	15.1	105.3		P
LF-1666	12.4	104.5	98.0	GF-21S	14.8	106.6		P
Notes:								
1 Missing test numbers correspond to non-critical construction activities.								
2 Required minimum percent compaction was 95% of Standard Proctor (ASTM D 698) maximum dry unit weight.								
3 Retest number for failing density tests.								

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Table 4-3

COMPACTION TEST RESULTS FOR GENERAL FILL USED TO CONSTRUCT CELL 1A

	NUCLEAR GAUGE			APPLICABLE STANDARD PROCTOR				
TEST STANDARD	ASTM D 3017	ASTM D 2922	ASTM D 698					
TESTING FREQUENCY	5 test per acre per lift			1 test per 25,000 yd ³				
TEST RESULTS								
CQA Test No. ¹	Moisture Content (%)	Dry Unit Weight (pcf)	Percent of Compaction ² (%)	Sample ID	Optimum Moisture Content (%)	Maximum Dry Unit Weight (pcf)	Retest No. ³	Pass/Fail (P/F)
LF-41	18.0	100.3	93.3	GF-1S	14.0	107.5	LF-56	F
LF-42	17.3	103.9	96.7	GF-1S	14.0	107.5		P
LF-43	18.3	102.2	95.1	GF-1S	14.0	107.5		P
LF-56	15.8	104.7	97.4	GF-1S	14.0	107.5		P
LF-57	20.4	104.5	97.2	GF-1S	14.0	107.5		P
LF-58	18.3	103.9	96.7	GF-1S	14.0	107.5		P
LF-59	17.8	104.2	96.9	GF-1S	14.0	107.5		P
LF-60	16.9	102.5	95.3	GF-1S	14.0	107.5		P
LF-61	17.0	98.3	91.4	GF-1S	14.0	107.5	LF-83	F
LF-62	15.3	99.3	92.4	GF-1S	14.0	107.5	LF-442	F
LF-63	14.2	104.1	96.8	GF-1S	14.0	107.5		P
LF-64	18.5	102.2	95.1	GF-1S	14.0	107.5		P
LF-65	16.9	102.3	95.2	GF-1S	14.0	107.5		P
LF-66	14.5	102.2	95.1	GF-1S	14.0	107.5		P
LF-67	12.2	101.2	94.1	GF-1S	14.0	107.5	LF-442	F
LF-68	17.8	98.1	91.3	GF-1S	14.0	107.5	LF-82	F
LF-69	18.7	98.2	91.3	GF-1S	14.0	107.5	LF-89	F
LF-70	16.9	97.3	90.5	GF-1S	14.0	107.5	LF-104	F
LF-71	12.8	100.9	93.9	GF-1S	14.0	107.5	LF-78	F
LF-72	13.9	105.3	98.0	GF-1S	14.0	107.5		P
LF-73	14.5	100.0	93.0	GF-1S	14.0	107.5	LF-79	F
LF-74	13.3	100.6	93.6	GF-1S	14.0	107.5	LF-80	F
LF-75	12.6	105.0	97.7	GF-1S	14.0	107.5		P
LF-76	14.7	101.3	94.2	GF-1S	14.0	107.5	LF-81	F
LF-77	13.0	100.5	93.5	GF-1S	14.0	107.5	LF-82	F
LF-78	19.0	102.8	95.6	GF-1S	14.0	107.5		P
LF-79	17.9	102.0	94.9	GF-1S	14.0	107.5		P
LF-80	13.5	103.4	96.2	GF-1S	14.0	107.5		P
LF-81	13.7	103.9	96.7	GF-1S	14.0	107.5		P
LF-82	18.2	102.8	95.6	GF-1S	14.0	107.5		P
LF-83	15.3	102.4	95.3	GF-1S	14.0	107.5		P
LF-84	17.8	103.4	96.2	GF-1S	14.0	107.5		P
LF-85	16.3	104.1	96.8	GF-1S	14.0	107.5		P
LF-86	12.1	103.7	96.5	GF-1S	14.0	107.5		P
LF-87	16.6	102.9	95.7	GF-1S	14.0	107.5		P
LF-89	16.0	103.2	96.0	GF-1S	14.0	107.5		P
LF-90	17.2	102.8	95.6	GF-1S	14.0	107.5		P
LF-91	20.1	102.4	95.3	GF-1S	14.0	107.5		P
LF-92	20.5	99.3	92.4	GF-1S	14.0	107.5	LF-158	F
LF-93	17.3	102.2	95.1	GF-1S	14.0	107.5		P

Notes:

1 Missing test numbers correspond to non-critical construction activities.

2 Required minimum percent compaction was 95% of Standard Proctor (ASTM D 698) maximum dry unit weight.

3 Retest number for failing density tests.

Table 4-3 (continued)

COMPACTION TEST RESULTS FOR GENERAL FILL USED TO CONSTRUCT CELL 1A

	NUCLEAR GAUGE			APPLICABLE STANDARD PROCTOR				
TEST STANDARD	ASTM D 3017	ASTM D 2922	ASTM D 698					
TESTING FREQUENCY	5 test per acre per lift			1 test per 25,000 yd ³				
TEST RESULTS								
CQA Test No. ¹	Moisture Content (%)	Dry Unit Weight (pcf)	Percent of Compaction ² (%)	Sample ID	Optimum Moisture Content (%)	Maximum Dry Unit Weight (pcf)	Retest No. ³	Pass/Fail (P/F)
LF-94	14.8	102.2	95.1	GF-1S	14.0	107.5		P
LF-95	19.3	101.8	94.7	GF-1S	14.0	107.5		P
LF-96	18.3	102.4	95.3	GF-1S	14.0	107.5		P
LF-97	15.6	103.1	95.9	GF-1S	14.0	107.5		P
LF-98	14.7	102.2	95.1	GF-1S	14.0	107.5		P
LF-99	13.3	103.3	96.1	GF-1S	14.0	107.5		P
LF-100	13.3	103.9	96.7	GF-1S	14.0	107.5		P
LF-101	15.7	95.9	89.2	GF-1S	14.0	107.5	LF-156	F
LF-102	16.7	95.9	89.2	GF-1S	14.0	107.5	LF-161	F
LF-103	17.3	102.6	95.4	GF-1S	14.0	107.5		P
LF-104	16.8	103.5	96.3	GF-1S	14.0	107.5		P
LF-105	17.3	103.1	95.9	GF-1S	14.0	107.5		P
LF-106	16.0	104.0	96.7	GF-1S	14.0	107.5		P
LF-120	12.5	96.6	96.5	GF-3S	16.2	100.1		P
LF-121	20.2	98.4	98.3	GF-3S	16.2	100.1		P
LF-122	19.5	98.4	98.3	GF-3S	16.2	100.1		P
LF-123	11.8	102.3	102.2	GF-3S	16.2	100.1		P
LF-124	17.8	99.8	99.7	GF-3S	16.2	100.1		P
LF-125	13.8	98.2	98.1	GF-3S	16.2	100.1		P
LF-126	11.0	101.4	101.3	GF-3S	16.2	100.1		P
LF-127	12.2	99.7	99.6	GF-3S	16.2	100.1		P
LF-128	18.4	100.1	100.0	GF-3S	16.2	100.1		P
LF-129	16.6	96.3	96.2	GF-3S	16.2	100.1		P
LF-130	19.3	98.7	98.6	GF-3S	16.2	100.1		P
LF-131	19.1	101.7	101.6	GF-3S	16.2	100.1		P
LF-132	16.8	97.4	97.3	GF-3S	16.2	100.1		P
LF-133	18.2	99.1	99.0	GF-3S	16.2	100.1		P
LF-134	15.3	99.3	99.2	GF-3S	16.2	100.1		P
LF-135	17.4	97.9	97.8	GF-3S	16.2	100.1		P
LF-136	8.6	101.9	101.8	GF-3S	16.2	100.1		P
LF-137	6.6	102.7	102.6	GF-3S	16.2	100.1		P
LF-138	11.5	100.6	100.5	GF-3S	16.2	100.1		P
LF-139	8.1	102.5	102.4	GF-3S	16.2	100.1		P
LF-140	14.1	98.2	98.1	GF-3S	16.2	100.1		P
LF-141	9.4	102.3	102.2	GF-3S	16.2	100.1		P
LF-142	12.1	101.0	100.9	GF-3S	16.2	100.1		P
LF-143	6.0	99.7	99.6	GF-3S	16.2	100.1		P
LF-144	7.3	101.4	101.3	GF-3S	16.2	100.1		P
LF-145	11.0	103.7	103.6	GF-3S	16.2	100.1		P
LF-146	12.8	100.8	100.7	GF-3S	16.2	100.1		P
Notes:								
1 Missing test numbers correspond to non-critical construction activities.								
2 Required minimum percent compaction was 95% of Standard Proctor (ASTM D 698) maximum dry unit weight.								
3 Retest number for failing density tests.								

Table 4-3 (continued)

COMPACTION TEST RESULTS FOR GENERAL FILL USED TO CONSTRUCT CELL 1A

	NUCLEAR GAUGE			APPLICABLE STANDARD PROCTOR				
TEST STANDARD	ASTM D 3017	ASTM D 2922		ASTM D 698				
TESTING FREQUENCY	5 test per acre per lift			1 test per 25,000 yd³				
TEST RESULTS								
CQA Test No. ¹	Moisture Content (%)	Dry Unit Weight (pcf)	Percent of Compaction ²	Sample ID	Optimum Moisture Content (%)	Maximum Dry Unit Weight (pcf)	Retest No. ³	Pass/Fail (P/F)
LF-147	10.9	99.2	99.1	GF-3S	16.2	100.1		P
LF-148	16.6	99.5	99.4	GF-3S	16.2	100.1		P
LF-149	20.8	99.3	99.2	GF-3S	16.2	100.1		P
LF-150	10.6	101.2	101.1	GF-3S	16.2	100.1		P
LF-151	16.6	99.3	99.2	GF-3S	16.2	100.1		P
LF-152	12.9	101.7	101.6	GF-3S	16.2	100.1		P
LF-153	11.6	99.9	99.8	GF-3S	16.2	100.1		P
LF-154	11.1	103.4	97.6	GF-6S	14.3	105.9		P
LF-155	12.3	107.4	101.4	GF-6S	14.3	105.9		P
LF-156	10.7	101.0	95.4	GF-6S	14.3	105.9		P
LF-157	14.2	101.5	95.8	GF-6S	14.3	105.9		P
LF-158	16.7	106.4	100.5	GF-6S	14.3	105.9		P
LF-159	14.7	104.8	99.0	GF-6S	14.3	105.9		P
LF-160	13.8	105.3	99.4	GF-6S	14.3	105.9		P
LF-161	15.1	104.4	98.6	GF-6S	14.3	105.9		P
LF-162	13.2	105.7	99.8	GF-6S	14.3	105.9		P
LF-163	12.0	105.3	99.4	GF-6S	14.3	105.9		P
LF-164	12.6	104.1	98.3	GF-6S	14.3	105.9		P
LF-165	10.8	102.9	97.2	GF-6S	14.3	105.9		P
LF-166	11.3	104.8	99.0	GF-6S	14.3	105.9		P
LF-167	12.9	104.6	98.8	GF-6S	14.3	105.9		P
LF-168	11.6	105.5	99.6	GF-6S	14.3	105.9		P
LF-169	19.5	100.8	95.2	GF-6S	14.3	105.9		P
LF-170	21.7	96.5	91.1	GF-6S	14.3	105.9	LF-175	F
LF-171	19.4	100.8	95.2	GF-6S	14.3	105.9		P
LF-172	18.4	101.0	95.4	GF-6S	14.3	105.9		P
LF-173	15.1	104.6	98.8	GF-6S	14.3	105.9		P
LF-174	11.5	102.0	96.3	GF-6S	14.3	105.9		P
LF-175	15.3	103.1	97.4	GF-6S	14.3	105.9		P
LF-176	15.5	101.8	96.1	GF-6S	14.3	105.9		P
LF-177	15.1	104.5	98.7	GF-6S	14.3	105.9		P
LF-178	15.3	102.6	96.9	GF-6S	14.3	105.9		P
LF-179	14.9	103.3	97.5	GF-6S	14.3	105.9		P
LF-180	14.3	103.7	97.9	GF-6S	14.3	105.9		P
LF-181	14.1	102.8	97.1	GF-6S	14.3	105.9		P
LF-182	11.5	105.2	99.3	GF-6S	14.3	105.9		P
LF-183	15.7	103.2	97.5	GF-6S	14.3	105.9		P
LF-184	16.1	104.9	99.1	GF-6S	14.3	105.9		P
LF-185	15.5	104.5	98.7	GF-6S	14.3	105.9		P
LF-186	15.3	104.8	99.0	GF-6S	14.3	105.9		P

Notes:

¹ Missing test numbers correspond to non-critical construction activities.² Required minimum percent compaction was 95% of Standard Proctor (ASTM D 698) maximum dry unit weight.³ Retest number for failing density tests.

Table 4-3 (continued)

COMPACTION TEST RESULTS FOR GENERAL FILL USED TO CONSTRUCT CELL 1A

	NUCLEAR GAUGE			APPLICABLE STANDARD PROCTOR				
TEST STANDARD	ASTM D 3017	ASTM D 2922		ASTM D 698				
TESTING FREQUENCY	5 test per acre per lift			1 test per 25,000 yd ³				
TEST RESULTS								
CQA Test No. ¹	Moisture Content (%)	Dry Unit Weight (pcf)	Percent of Compaction ² (%)	Sample ID	Optimum Moisture Content (%)	Maximum Dry Unit Weight (pcf)	Retest No. ³	Pass/Fail (P/F)
LF-187	11.7	103.2	97.5	GF-6S	14.3	105.9		P
LF-188	12.5	101.7	96.0	GF-6S	14.3	105.9		P
LF-189	15.8	102.1	96.4	GF-6S	14.3	105.9		P
LF-190	11.6	102.9	97.2	GF-6S	14.3	105.9		P
LF-191	12.3	102.0	96.3	GF-6S	14.3	105.9		P
LF-192	9.9	102.4	96.7	GF-6S	14.3	105.9		P
LF-193	7.7	103.8	98.0	GF-6S	14.3	105.9		P
LF-194	9.9	105.4	99.5	GF-6S	14.3	105.9		P
LF-195	10.8	106.5	100.6	GF-6S	14.3	105.9		P
LF-196	11.9	105.1	99.2	GF-6S	14.3	105.9		P
LF-197	14.0	104.6	98.8	GF-6S	14.3	105.9		P
LF-198	14.4	106.7	100.8	GF-6S	14.3	105.9		P
LF-199	14.7	102.9	97.2	GF-6S	14.3	105.9		P
LF-200	12.7	103.8	98.0	GF-6S	14.3	105.9		P
LF-201	14.8	100.6	95.0	GF-6S	14.3	105.9		P
LF-202	14.7	101.5	95.8	GF-6S	14.3	105.9		P
LF-203	13.9	102.0	96.3	GF-6S	14.3	105.9		P
LF-204B	15.6	100.7	95.1	GF-6S	14.3	105.9		P
LF-205B	15.0	99.1	93.6	GF-6S	14.3	105.9	LF-313	F
LF-206B	13.7	101.6	95.9	GF-6S	14.3	105.9		P
LF-207B	13.8	101.6	95.9	GF-6S	14.3	105.9		P
LF-208B	11.0	97.9	92.4	GF-6S	14.3	105.9	LF-311	F
LF-209B	7.1	104.2	98.4	GF-6S	14.3	105.9		P
LF-210B	11.1	100.7	95.1	GF-6S	14.3	105.9		P
LF-211B	12.5	100.6	95.0	GF-6S	14.3	105.9		P
LF-212B	14.9	100.8	95.2	GF-6S	14.3	105.9		P
LF-213	14.8	100.8	95.2	GF-6S	14.3	105.9		P
LF-214	9.9	105.8	99.9	GF-6S	14.3	105.9		P
LF-253	11.8	105.5	101.7	GF-7S	14.9	103.7		P
LF-254	6.8	102.5	98.8	GF-7S	14.9	103.7		P
LF-255	7.3	93.4	90.1	GF-7S	14.9	103.7	LF-259	F
LF-256	7.9	104.5	100.8	GF-7S	14.9	103.7		P
LF-257	10.1	98.8	95.3	GF-7S	14.9	103.7		P
LF-258	14.9	101.5	97.9	GF-7S	14.9	103.7		P
LF-259	8.7	100.2	96.6	GF-7S	14.9	103.7		P
LF-260	8.3	100.9	97.3	GF-7S	14.9	103.7		P
LF-261	13.1	98.0	95.2	GF-8S	15.9	102.9		P
LF-262	8.6	99.4	96.6	GF-8S	15.9	102.9		P
LF-263	11.4	103.7	100.8	GF-8S	15.9	102.9		P
LF-264	9.2	98.8	96.0	GF-8S	15.9	102.9		P

Notes:

¹ Missing test numbers correspond to non-critical construction activities.² Required minimum percent compaction was 95% of Standard Proctor (ASTM D 698) maximum dry unit weight.³ Retest number for failing density tests.

Table 4-3 (continued)

COMPACTION TEST RESULTS FOR GENERAL FILL USED TO CONSTRUCT CELL 1A

	NUCLEAR GAUGE			APPLICABLE STANDARD PROCTOR				
TEST STANDARD	ASTM D 3017	ASTM D 2922		ASTM D 698				
TESTING FREQUENCY	5 test per acre per lift			1 test per 25,000 yd³				
TEST RESULTS								
CQA Test No. ¹	Moisture Content (%)	Dry Unit Weight (pcf)	Percent of Compaction ² (%)	Sample ID	Optimum Moisture Content (%)	Maximum Dry Unit Weight (pcf)	Retest No. ³	Pass/Fail (P/F)
LF-296	7.4	107.4	101.4	GF-6S	14.3	105.9		P
LF-297	8.0	108.0	102.0	GF-6S	14.3	105.9		P
LF-298	7.2	106.6	100.7	GF-6S	14.3	105.9		P
LF-299	6.6	107.0	101.0	GF-6S	14.3	105.9		P
LF-300	6.8	106.6	100.7	GF-6S	14.3	105.9		P
LF-301	6.1	106.2	100.3	GF-6S	14.3	105.9		P
LF-302	7.9	107.4	101.4	GF-6S	14.3	105.9		P
LF-303	6.9	106.1	100.2	GF-6S	14.3	105.9		P
LF-304	8.2	106.7	100.8	GF-6S	14.3	105.9		P
LF-305	8.7	107.7	101.7	GF-6S	14.3	105.9		P
LF-306	7.7	106.2	100.3	GF-6S	14.3	105.9		P
LF-307	13.0	100.4	96.8	GF-7S	14.9	103.7		P
LF-308	11.6	104.0	100.3	GF-7S	14.9	103.7		P
LF-309	11.2	101.4	97.8	GF-7S	14.9	103.7		P
LF-310	10.9	100.8	97.2	GF-7S	14.9	103.7		P
LF-311	12.8	101.5	97.9	GF-7S	14.9	103.7		P
LF-312	11.3	102.0	98.4	GF-7S	14.9	103.7		P
LF-313	10.6	103.9	100.2	GF-7S	14.9	103.7		P
LF-314	11.7	103.3	99.6	GF-7S	14.9	103.7		P
LF-315	11.0	102.4	98.7	GF-7S	14.9	103.7		P
LF-336	17.1	99.7	96.1	GF-7S	14.9	103.7		P
LF-337	7.5	99.3	95.8	GF-7S	14.9	103.7		P
LF-338	14.8	99.2	95.7	GF-7S	14.9	103.7		P
LF-339	14.2	100.3	96.7	GF-7S	14.9	103.7		P
LF-340	15.9	98.9	95.4	GF-7S	14.9	103.7		P
LF-341	11.7	102.9	99.2	GF-7S	14.9	103.7		P
LF-342	16.9	99.3	95.8	GF-7S	14.9	103.7		P
LF-343	10.1	106.6	102.8	GF-7S	14.9	103.7		P
LF-344	13.6	103.3	99.6	GF-7S	14.9	103.7		P
LF-345	10.3	102.8	99.1	GF-7S	14.9	103.7		P
LF-366	7.4	104.7	101.0	GF-7S	14.9	103.7		P
LF-367	12.4	107.8	104.0	GF-7S	14.9	103.7		P
LF-368	13.6	100.6	97.0	GF-7S	14.9	103.7		P
LF-369	9.4	100.2	96.6	GF-7S	14.9	103.7		P
LF-370	7.3	104.4	100.7	GF-7S	14.9	103.7		P
LF-371	14.6	103.7	100.0	GF-7S	14.9	103.7		P
LF-395	13.0	107.9	104.4	GF-10S	15.7	103.4		P
LF-396	9.6	100.4	97.1	GF-10S	15.7	103.4		P
LF-397	7.8	105.3	101.8	GF-10S	15.7	103.4		P
LF-398	9.5	101.3	98.0	GF-10S	15.7	103.4		P

Notes:

¹ Missing test numbers correspond to non-critical construction activities.² Required minimum percent compaction was 95% of Standard Proctor (ASTM D 698) maximum dry unit weight.³ Retest number for failing density tests.

Table 4-3 (continued)

COMPACTION TEST RESULTS FOR GENERAL FILL USED TO CONSTRUCT CELL 1A

	NUCLEAR GAUGE			APPLICABLE STANDARD PROCTOR				
TEST STANDARD	ASTM D 3017	ASTM D 2922	ASTM D 698					
TESTING FREQUENCY	5 test per acre per lift			1 test per 25,000 yd ³				
TEST RESULTS								
CQA Test No. ¹	Moisture Content (%)	Dry Unit Weight (pcf)	Percent of Compaction ² (%)	Sample ID	Optimum Moisture Content (%)	Maximum Dry Unit Weight (pcf)	Retest No. ³	Pass/Fail (P/F)
LF-399	8.7	105.7	102.2	GF-10S	15.7	103.4		P
LF-400	6.4	101.2	97.9	GF-10S	15.7	103.4		P
LF-401	8.1	101.9	98.5	GF-10S	15.7	103.4		P
LF-402	9.2	102.6	99.2	GF-10S	15.7	103.4		P
LF-403	11.0	105.8	102.3	GF-10S	15.7	103.4		P
LF-404	9.0	105.4	101.9	GF-10S	15.7	103.4		P
LF-405	9.8	104.0	100.6	GF-10S	15.7	103.4		P
LF-406	10.1	105.8	102.3	GF-10S	15.7	103.4		P
LF-407	13.3	99.4	96.1	GF-10S	15.7	103.4		P
LF-408	10.2	102.6	99.2	GF-10S	15.7	103.4		P
LF-409	7.8	103.3	99.9	GF-10S	15.7	103.4		P
LF-410	12.4	100.8	97.5	GF-10S	15.7	103.4		P
LF-411	8.2	105.6	102.1	GF-10S	15.7	103.4		P
LF-412	16.9	100.9	97.6	GF-10S	15.7	103.4		P
LF-413	8.8	99.3	96.0	GF-10S	15.7	103.4		P
LF-414	7.5	100.3	97.0	GF-10S	15.7	103.4		P
LF-415	7.5	107.7	104.2	GF-10S	15.7	103.4		P
LF-416	10.6	100.7	97.4	GF-10S	15.7	103.4		P
LF-417	10.9	103.4	100.0	GF-10S	15.7	103.4		P
LF-418	11.7	106.4	102.9	GF-10S	15.7	103.4		P
LF-419	10.8	105.9	102.4	GF-10S	15.7	103.4		P
LF-420	8.1	98.2	95.0	GF-10S	15.7	103.4		P
LF-421	11.0	102.4	99.0	GF-10S	15.7	103.4		P
LF-422	12.6	100.8	97.5	GF-10S	15.7	103.4		P
LF-423	15.5	99.1	95.8	GF-10S	15.7	103.4		P
LF-424	8.3	103.5	100.1	GF-10S	15.7	103.4		P
LF-425	20.0	98.6	95.4	GF-10S	15.7	103.4		P
LF-426	11.7	104.0	100.6	GF-10S	15.7	103.4		P
LF-427	13.0	103.1	99.7	GF-10S	15.7	103.4		P
LF-428	17.6	98.7	95.5	GF-10S	15.7	103.4		P
LF-429	14.8	99.6	96.3	GF-10S	15.7	103.4		P
LF-430	15.0	101.2	97.9	GF-10S	15.7	103.4		P
LF-431	16.0	100.2	96.9	GF-10S	15.7	103.4		P
LF-432	16.8	100.6	97.3	GF-10S	15.7	103.4		P
LF-433	11.3	107.4	103.9	GF-10S	15.7	103.4		P
LF-434	16.7	98.7	95.5	GF-10S	15.7	103.4		P
LF-435	10.9	99.4	96.1	GF-10S	15.7	103.4		P
LF-436	16.1	97.4	94.2	GF-10S	15.7	103.4		F
LF-442	10.2	105.0	101.5	GF-10S	15.7	103.4		P
Notes:								
1 Missing test numbers correspond to non-critical construction activities.								
2 Required minimum percent compaction was 95% of Standard Proctor (ASTM D 698) maximum dry unit weight.								
3 Retest number for failing density tests.								

Table 4-4

**COMPACTION TEST RESULTS FOR GENERAL FILL USED TO CONSTRUCT
LEACHATE STORAGE AREA BERMS**

	NUCLEAR GAUGE			APPLICABLE STANDARD PROCTOR				
TEST STANDARD	ASTM D 3017	ASTM D 2922	ASTM D 698					
TESTING FREQUENCY	5 test per acre per lift or 1 test per 250 lf per lift			1 test per 25,000 yd³				
TEST RESULTS								
CQA Test No. ¹	Moisture Content (%)	Dry Unit Weight (pcf)	Percent of Compaction ² (%)	Sample ID	Optimum Moisture Content (%)	Maximum Dry Unit Weight (pcf)	Retest No. ³	Pass/Fail (P/F)
LF-493	15.7	101.6	98.0	GF-7S	14.9	103.7		P
LF-494	12.3	104.3	100.6	GF-7S	14.9	103.7		P
LF-495	11.5	100.8	97.2	GF-7S	14.9	103.7		P
LF-496	19.2	98.5	95.0	GF-7S	14.9	103.7		P
LF-497	13.7	99.0	95.5	GF-7S	14.9	103.7		P
LF-498	16.8	98.5	95.0	GF-7S	14.9	103.7		P
LF-499	17.7	101.3	97.7	GF-7S	14.9	103.7		P
LF-500	12.4	105.8	102.0	GF-7S	14.9	103.7		P
LF-508B	10.1	102.3	99.9	GF-15S	15.5	102.4		P
LF-509	13.6	102.3	99.9	GF-15S	15.5	102.4		P
LF-510	11.1	103.4	101.0	GF-15S	15.5	102.4		P
LF-511	15.1	100.6	98.2	GF-15S	15.5	102.4		P
LF-512	17.4	98.3	96.0	GF-15S	15.5	102.4		P
LF-513	11.2	99.6	97.3	GF-15S	15.5	102.4		P
LF-514	10.8	99.0	96.7	GF-15S	15.5	102.4		P
LF-515	7.5	105.6	103.1	GF-15S	15.5	102.4		P
LF-529	17.0	104.3	101.9	GF-15S	15.5	102.4		P
LF-530	10.6	104.0	101.6	GF-15S	15.5	102.4		P
LF-531	9.8	102.6	100.2	GF-15S	15.5	102.4		P
LF-532	14.0	102.0	99.6	GF-15S	15.5	102.4		P
LF-533	8.9	99.7	97.4	GF-15S	15.5	102.4		P
LF-534	13.1	106.0	103.5	GF-15S	15.5	102.4		P
LF-535	9.8	98.5	96.2	GF-15S	15.5	102.4		P
LF-536	10.8	107.0	104.5	GF-15S	15.5	102.4		P
LF-537	9.7	101.8	99.4	GF-15S	15.5	102.4		P
LF-538	19.2	97.9	95.6	GF-15S	15.5	102.4		P
LF-539	14.3	100.1	97.8	GF-15S	15.5	102.4		P
LF-540	18.5	100.3	97.9	GF-15S	15.5	102.4		P
LF-552	11.0	101.6	99.2	GF-15S	15.5	102.4		P
LF-553	13.6	103.3	100.9	GF-15S	15.5	102.4		P
LF-554	14.9	102.0	99.6	GF-15S	15.5	102.4		P
LF-555	11.2	100.2	97.9	GF-15S	15.5	102.4		P
LF-556	13.6	102.4	100.0	GF-15S	15.5	102.4		P
LF-557	16.5	97.7	95.4	GF-15S	15.5	102.4		P
LF-558	15.6	100.5	98.1	GF-15S	15.5	102.4		P
LF-559	16.6	100.0	97.7	GF-15S	15.5	102.4		P
LF-591	14.7	102.7	100.3	GF-15S	15.5	102.4		P
LF-592	14.9	101.3	98.9	GF-15S	15.5	102.4		P
LF-593	11.0	105.4	102.9	GF-15S	15.5	102.4		P
LF-594	19.1	99.7	97.4	GF-15S	15.5	102.4		P

Notes:

¹ Missing test numbers correspond to non-critical construction activities.² Required minimum percent compaction was 95% of Standard Proctor (ASTM D 698) maximum dry unit weight.³ Retest number for failing density tests.

Table 4-4 (continued)

**COMPACTION TEST RESULTS FOR GENERAL FILL USED TO CONSTRUCT
LEACHATE STORAGE AREA PONDS**

		NUCLEAR GAUGE		APPLICABLE STANDARD PROCTOR				
TEST STANDARD	ASTM D 3017	ASTM D 2922		ASTM D 698				
TESTING FREQUENCY	5 test per acre per lift or 1 test per 250 lf per lift			1 test per 25,000 yd ³				
TEST RESULTS								
CQA Test No. ¹	Moisture Content (%)	Dry Unit Weight (pcf)	Percent of Compaction ² (%)	Sample ID	Optimum Moisture Content (%)	Maximum Dry Unit Weight (pcf)	Retest No. ³	Pass/Fail (P/F)
LF-595	10.0	105.2	102.7	GF-15S	15.5	102.4		P
LF-596	10.8	106.0	103.5	GF-15S	15.5	102.4		P
LF-597	11.0	103.2	100.8	GF-15S	15.5	102.4		P
LF-598	15.5	99.4	97.1	GF-15S	15.5	102.4		P
LF-599	14.7	103.5	101.1	GF-15S	15.5	102.4		P
LF-600	17.4	100.9	98.5	GF-15S	15.5	102.4		P
LF-601	11.4	102.7	100.3	GF-15S	15.5	102.4		P
LF-602	16.7	102.3	99.9	GF-15S	15.5	102.4		P
LF-603	15.9	101.3	98.9	GF-15S	15.5	102.4		P
LF-894	8.5	107.2	103.9	GF-17S	15.7	103.2		P
LF-895	10.4	113.9	110.4	GF-17S	15.7	103.2		P
LF-896	11.6	109.3	105.9	GF-17S	15.7	103.2		P
LF-897	9.7	108.1	104.7	GF-17S	15.7	103.2		P
LF-989	9.9	110.1	106.7	GF-17S	15.7	103.2		P
LF-899	15.9	105.7	102.4	GF-17S	15.7	103.2		P
LF-900	15.0	103.0	99.8	GF-17S	15.7	103.2		P
LF-901	10.1	109.8	106.4	GF-17S	15.7	103.2		P
LF-902	10.8	109.1	105.7	GF-17S	15.7	103.2		P
LF-903	9.4	104.7	101.5	GF-17S	15.7	103.2		P
LF-904	10.5	103.0	99.8	GF-17S	15.7	103.2		P
LF-905	11.6	101.9	98.7	GF-17S	15.7	103.2		P
LF-906	11.5	103.2	100.0	GF-17S	15.7	103.2		P
Notes:								
1 Missing test numbers correspond to non-critical construction activities.								
2 Required minimum percent compaction was 95% of Standard Proctor (ASTM D 698) maximum dry unit weight.								
3 Retest number for failing density tests.								

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Table 4-5

**GRAIN SIZE ANALYSES AND USCS CLASSIFICATION FOR
GENERAL FILL MATERIALS OBTAINED FROM BORROW AREA A**

		PARTICLE SIZE ANALYSIS						SOIL CLASSIFICATION	
TEST STANDARD		ASTM D 422						ASTM D 2847	
TESTING FREQUENCY		1 test per 10,000 yd ³						1 test per 10,000 yd ³	
TEST RESULTS									
Sample No. ²	Soil Description	Percent Passing by Weight Through U.S. Standard Sieve						Classification ¹	Pass/Fail (P/F)
		No. 4	No. 10	No. 40	No. 60	No. 80	No. 200		
ATL-1	Brown & gray sand with trace silt	100.0	99.7	95.5	77.7	44.7	4.8	SP	P
ATL-2	Dark brown & gray sand with trace silt	100.0	99.9	97.7	85.2	50.1	5.1	SP-SM	P
ATL-3	Dark brown & gray sand with trace silt	100.0	99.9	97.1	83.0	46.7	3.6	SP	P
ATL-4	Dark brown & gray sand with trace silt	100.0	99.9	97.4	82.0	45.8	5.0	SP	P
ATL-5	Dark brown & gray sand with trace silt	100.0	99.9	96.5	81.1	44.8	4.5	SP	P
ATL-6	Dark brown & gray sand with trace silt	100.0	100.0	97.4	83.3	47.2	5.0	SP	P
ATL-7	Dark brown & gray sand with trace silt	100.0	99.9	97.5	83.0	45.4	4.7	SP	P
ATL-10	Dark grayish brown	100.0	100.0	97.5	80.8	43.9	3.7	SP	P
ATL-11	Dark brown & gray sand with trace silt and clay	100.0	99.1	97.5	82.5	47.4	2.8	SP	P
PB-03	Medium brown silty sand	100.0	100.0	96.0	75.0	53.0	4.0	SP	P
PB-04	Medium brown silty sand	100.0	99.0	94.0	73.0	51.0	4.0	SP	P
PB-05	Medium brown silty sand	100.0	100.0	95.0	70.0	49.0	4.0	SP	P
PB-06	Medium brown silty sand	100.0	100.0	97.0	79.0	50.0	2.0	SP	P
PB-07	Medium brown silty sand	100.0	100.0	96.0	80.0	50.0	2.0	SP	P
PB-08	Dark brown silty sand	100.0	100.0	97.0	77.0	50.0	3.0	SP	P
PB-09	Dark brown silty sand	100.0	100.0	98.0	80.0	45.0	1.0	SP	P
PB-10	Medium brown silty sand	100.0	100.0	95.0	73.0	45.0	1.0	SP	P
PB-11	Medium brown silty sand	100.0	99.0	91.0	57.0	26.0	1.0	SP	P
Notes:									
1 General fill material was required to classify as SW, SP, or SM.									
2 Samples "ATL" were tested by Atlantic Testing Laboratories, Inc.; Samples "PB" were tested in the on-site laboratory; and Samples "TP" were tested by Excel Geotechnical Testing as part of a soil investigation performed in the Borrow Area A during construction.									

Table 4-5 (continued)

**GRAIN SIZE ANALYSES AND USCS CLASSIFICATION FOR
GENERAL FILL MATERIALS OBTAINED FROM BORROW AREA A**

		PARTICLE SIZE ANALYSIS						SOIL CLASSIFICATION	
TEST STANDARD		ASTM D 422						ASTM D 2847	
TESTING FREQUENCY		1 test per 10,000 yd³						1 test per 10,000 yd³	
TEST RESULTS									
Sample No. ²	Soil Description	Percent Passing by Weight Through U.S. Standard Sieve						Classification ¹	Pass/Fail (P/F)
		No. 4	No. 10	No. 40	No. 60	No. 80	No. 200		
PB-12	Medium brown silty sand	100.0	100.0	96.0	74.0	40.0	5.0	SP	P
PB-13	Medium brown silty sand	100.0	99.0	92.0	66.0	21.0	2.0	SP	P
PB-14	Medium brown silty sand	100.0	100.0	92.5	59.0	30.0	3.0	SP	P
PB-15	Medium brown silty sand	100.0	100.0	93.0	60.0	30.0	3.0	SP	P
PB-16	Dark brown silty sand	100.0	100.0	92.0	58.0	27.0	4.0	SP	P
PB-17	Dark brown silty sand	100.0	100.0	93.0	57.0	25.0	5.0	SP	P
PB-18	Dark brown silty sand	100.0	100.0	93.0	54.0	26.0	3.0	SP	P
PB-19	Dark brown silty sand	100.0	100.0	95.0	71.0	25.0	4.0	SP	P
PB-20	Light brown silty sand	99.0	99.0	95.0	68.0	38.0	2.0	SP	P
PB-21	Light brown silty sand	100.0	99.0	92.0	72.0	6.0	2.0	SP	P
PB-22	Light brown silty sand	100.0	98.0	90.0	70.0	9.0	2.0	SP	P
PB-23	Light brown silty sand	100.0	100.0	96.0	75.0	13.0	2.0	SP	P
TP-1C	Brown silty fine sand	-	-	-	-	-	7.4	SP-SM	P
TP-1E	Light brown silty fine sand	-	-	-	-	-	17.4	SM	P
TP-3E	Dark brown slightly cemented silty fine sand	-	-	-	-	-	15.7	SM	P
TP-4A	Light grey to light brown silty fine sand	-	-	-	-	-	12.7	SM	P
TP-4D	Light brown silty fine sand	-	-	-	-	-	7.9	SP-SM	P
TP-5A	Light grey silty fine sand	-	-	-	-	-	5.9	SP-SM	P
TP-5C	Dark brown to black silty fine sand	-	-	-	-	-	11.9	SP-SM	P

Notes:

¹ General fill material was required to classify as SW, SP, or SM.

² Samples "ATL" were tested by Atlantic Testing Laboratories, Inc.; Samples "PB" were tested in the on-site laboratory; and Samples "TP" were tested by Excel Geotechnical Testing as part of a soil investigation performed in the Borrow Area A during construction.

Table 4-5 (continued)

**GRAIN SIZE ANALYSES AND USCS CLASSIFICATION FOR
GENERAL FILL MATERIALS OBTAINED FROM BORROW AREA A**

		PARTICLE SIZE ANALYSIS						SOIL CLASSIFICATION	
TEST STANDARD		ASTM D 422						ASTM D 2847	
TESTING FREQUENCY		1 test per 10,000 yd ³						1 test per 10,000 yd ³	
TEST RESULTS									
Sample No. ²	Soil Description	Percent Passing by Weight Through U.S. Standard Sieve						Classification ¹	Pass/Fail (P/F)
		No. 4	No. 10	No. 40	No. 60	No. 80	No. 200		
TP-7D	Brown silty fine sand	-	-	-	-	-	8.5	SP-SM	P
TP-7E	Light brown silty fine sand	100.0	100.0	96.0	63.0	40.0	8.3	SP-SM	P
TP-Comp 1	Light grey to light brown silty fine sand	100.0	100.0	98.0	83.0	55.0	3.0	SP	P
TP-Comp 2	Brown silty fine sand	100.0	100.0	98.0	82.0	55.0	6.0	SP-SM	P
TP-Comp 3	Brown silty fine sand	100.0	100.0	97.0	82.0	54.0	11.0	SP-SM	P
Notes:									
1 General fill material was required to classify as SW, SP, or SM.									
2 Samples "ATL" were tested by Atlantic Testing Laboratories, Inc.; Samples "PB" were tested in the on-site laboratory; and Samples "TP" were tested by Excel Geotechnical Testing as part of a soil investigation performed in the Borrow Area A during construction.									

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Table 4-6

**SAND CONE TESTS FOR GENERAL FILL
PERFORMED TO VERIFY NUCLEAR DENSITY TEST RESULTS**

SAND CONE			NUCLEAR GAUGE			DIFFERENCE (Sand Cone - Nuclear)	
Test No.	Dry Unit Weight (pcf)	Moisture Content (%)	Test No.	Dry Unit Weight (pcf)	Moisture Content (%)	Dry Unit Weight (pcf)	Moisture Content (%)
	ASTM D 1556	ASTM D 2216		ASTM D 2922	ASTM D 3017		
SCT-1	104.6	8.4	LF-366	104.7	7.4	-0.1	1.0
SCT-2	101.1	10.4	AR-396	103.1	5.9	-2.0	4.5
SCT-3	97.6	14.8	LF-382	101.2	12.3	-3.6	2.5
SCT-4	96.6	9.2	LF-413	99.3	8.8	-2.7	0.4
SCT-5	98.7	11.8	LF-469	98.0	11.7	0.7	0.1
SCT-6	102.0	8.4	LF-515	105.6	7.5	-3.6	0.9
SCT-7	102.7	14.4	LF-614	105.5	13.7	-2.8	0.7
SCT-8	108.5	14.9	LF-658	104.2	15.0	4.3	-0.1
SCT-9	101.2	15.3	LF-871	100.1	17.5	1.1	-2.2
SCT-10	110.2	11.3	HR-156	107.1	12.6	3.1	-1.3
SCT-11	107.8	10.0	LF-1015	104.1	12.0	3.7	-2.0
SCT-12	102.9	16.0	LF-1133	104.8	16.6	-1.9	-0.6
SCT-13	100.5	14.2	LF-1168	98.9	18.0	1.6	-3.8
SCT-14	101.4	9.4	LF-1203	97.9	12.4	3.5	-3.0
SCT-15	97.3	17.8	LF-1251	99.3	20.2	-2.0	-2.4
SCT-16	109.7	10.3	HR-734	107.6	12.2	2.1	-1.9
SCT-17	98.6	19.4	LF-1498	99.4	20.3	-0.8	-0.9
SCT-18	108.0	11.1	LF-1583	105.4	12.7	2.6	-1.6
SCT-19	110.8	13.2	LF-1603	108.8	9.4	2.0	3.8
SCT-20	107.2	8.3	LF-1625	106.3	8.7	0.9	-0.4
SCT-21	99.1	14.4	LF-1641	101.1	17.3	-2.0	-2.9

Table 4-7

**COMPACTION TEST RESULTS FOR
LOW PERMEABILITY SOIL LAYER IN SUMP AREA OF CELLS**

		NUCLEAR GAUGE			APPLICABLE STANDARD PROCTOR			
		In-Situ Moisture	In-Situ Density					
TEST STANDARD		ASTM D 3017	ASTM D 2922		ASTM D 698			
TESTING FREQUENCY		1 test per lift per cell sump						
TEST RESULTS								
Location	CQA Test No.	Moisture Content (%)	Dry Unit Weight (pcf)	Percent of Compaction ¹ (%)	Sample ID	Optimum Moisture Content (%)	Maximum Dry Unit Weight (pcf)	Pass/Fail (P/F)
Cell 1 Sump	LF-708	30.8	88.8	105.5	CB-04 S	34.8	84.2	P
	LF-709	36.8	83.4	99.0	CB-04 S	34.8	84.2	P
	LF-710	31.4	89.3	106.1	CB-04 S	34.8	84.2	P
	LF-711	32.5	80.1	95.1	CB-04 S	34.8	84.2	P
	LF-712	32.9	80.8	96.0	CB-04 S	34.8	84.2	P
	LF-713	33.3	87.5	103.9	CB-04 S	34.8	84.2	P
	LF-714	35.2	82.4	97.9	CB-04 S	34.8	84.2	P
	LF-715	34.2	81.1	96.3	CB-04 S	34.8	84.2	P
Cell 2 Sump	LF-1419	30.8	90.5	96.4	CB-04N	27.6	93.9	P
	LF-1420	28.2	90.5	96.4	CB-04N	27.6	93.9	P
Cell 3 Sump	LF-1360	28.9	91.5	97.4	CB-04N	27.6	93.9	P
Cell 4 Sump	LF-882	34.2	80.4	95.5	CB-04 S	34.8	84.2	P
	LF-883	30.5	81.4	96.7	CB-04 S	34.8	84.2	P
	LF-884	32.6	82.5	98.0	CB-04 S	34.8	84.2	P
	LF-885	35.0	81.0	96.2	CB-04 S	34.8	84.2	P
	LF-886	30.6	85.6	101.7	CB-04 S	34.8	84.2	P
	LF-887	31.0	89.1	105.8	CB-04 S	34.8	84.2	P
Cell 5 Sump	LF-947	31.3	88.6	105.2	CB-04 S	34.8	84.2	P
	LF-948	29.2	90.6	107.6	CB-04 S	34.8	84.2	P
Cell 6 Sump	LF-1311	31.0	90.4	96.3	CB-04N	27.6	93.9	P
	LF-1312	27.6	91.9	97.9	CB-04N	27.6	93.9	P
	LF-1313	28.6	90.5	96.4	CB-04N	27.6	93.9	P
	LF-1314	27.8	91.5	97.4	CB-04N	27.6	93.9	P
Note:								
1 Required minimum percent compaction was 95 percent of Standard Proctor maximum dry unit weight at a moisture content no more than 4 percent wet of the optimum moisture content.								

Table 4-8

**LABORATORY TEST RESULTS FOR LOW PERMEABILITY SOIL LAYER
IN SUMP AREA OF CELLS 1 THROUGH 6**

	FINES CONTENT DETERMINATION	SOIL CLASSIFICATION			HYDRAULIC CONDUCTIVITY		
TEST STANDARD	ASTM D 1140	ASTM D 2487			ASTM ASTM D 5084		
TEST FREQUENCY	1 test per Cell				1 test per Cell		
TEST RESULTS							
Sample No. ¹	Fines Content (Passing No. 200 Sieve) (%)	Atterberg Limits			Soil Classification ²	Hydraulic Conductivity ³ (cm/s)	Pass/Fail (P/F)
		LL	PL	PI			
ST-1 (Cell 1 Sump)	54.5	81	20	61	CH	8.6x10 ⁻⁹	P
ST-2 (Cell 2 Sump)	51.6	84	18	66	CH	1.5x10 ⁻⁸	P
ST-3 (Cell 3 Sump)	50.6	80	22	58	CH	2.0x10 ⁻⁸	P
ST-4 (Cell 4 Sump)	55.9	-	-	-	CH	2.2x10 ⁻⁸	P
ST-5 (Cell 5 Sump)	48.6	-	-	-	SC	9.2x10 ⁻⁸	P
ST-6 (Cell 6 Sump)	54.7	88	24	64	CH	4.5x10 ⁻⁸	P
CB-04 N ⁴ (Borrow Source)	68.9	93	25	68	CH	1.3x10 ⁻⁸	P
CB-04 S ⁴ (Borrow Source)	74.2	106	29	77	CH	1.1x10 ⁻⁸	P
Notes: 1 Tests performed by Excel Geotechnical Testing (EGT). 2 CL or CH soils in accordance with USCS were required. 3 Required hydraulic conductivity was less than or equal to 1.0x10 ⁻⁷ cm/sec at confining stress of 20 psi. 4 Conformance tests performed during identification of borrow area for low-permeability soils.							

Table 4-9

LABORATORY TEST RESULTS FOR LINER PROTECTIVE LAYER SOIL

	FINES CONTENT	SOIL CLASSIFICATION	HYDRAULIC CONDUCTIVITY	
TEST STANDARD	ASTM D 1140	ASTM D 2487	ASTM D 2434	
TESTING FREQUENCY	1 test per 2,000 yd ³	1 test per 2,000 yd ³	1 test per 3,000 yd ³	
TEST RESULTS				
Sample ID ¹	Fines Content Passing No. 200 Sieve (%)	Soil Classification ²	Hydraulic Conductivity ³ (cm/sec)	Pass/Fail (P/F)
PC-01	-	-	6.9x10 ⁻³	P
PC-02	-	-	4.7x10 ⁻³	P
PC-03	7	SP-SM	8.0x10 ⁻³	P
PC-04	3	SP	7.3x10 ⁻³	P
PC-05	2	SP	8.9x10 ⁻³	P
PC-06	2	SP	6.7x10 ⁻³	P
PC-07	2	SP	7.6x10 ⁻³	P
PC-08	6	SP-SM	5.1x10 ⁻³	P
PC-09	2	SP	6.9x10 ⁻³	P
PC-10	1	SP	7.1x10 ⁻³	P
PC-11	4	SP	4.2x10 ⁻³	P
PC-12	2	SP	7.1x10 ⁻³	P
PC-13	11	SP-SM	4.9x10 ⁻³	P
PC-14	1	SP	5.7x10 ⁻³	P
PC-15	1	SP	-	P
PC-16	0	SP	-	P
PC-17	9	SP-SM	5.3x10 ⁻³	P
PC-18	3	SP	-	P
PC-19	2	SP	-	P
PC-20	4	SP	-	P
PC-21	9	SP-SM	4.3x10 ⁻³	P
PC-22	1	SP	-	P
PC-23	2	SP	-	P
PC-24	1	SP	-	P
PC-25	2	SP	-	P
LP-01	9	SP-SM	8.3x10 ⁻³	P
LP-02	2	SP	7.5x10 ⁻³	P
LP-03	3	SP	-	P
LP-04	4	SP	-	P
Notes: 1 Samples "PC" and "LP" correspond to liner protective layer soils from Cell 1A and leachate storage area, respectively. 2 Liner protective layer material was required to classify as SW or SP. 3 Required hydraulic conductivity was greater than or equal to 1.0x10 ⁻³ cm/sec.				

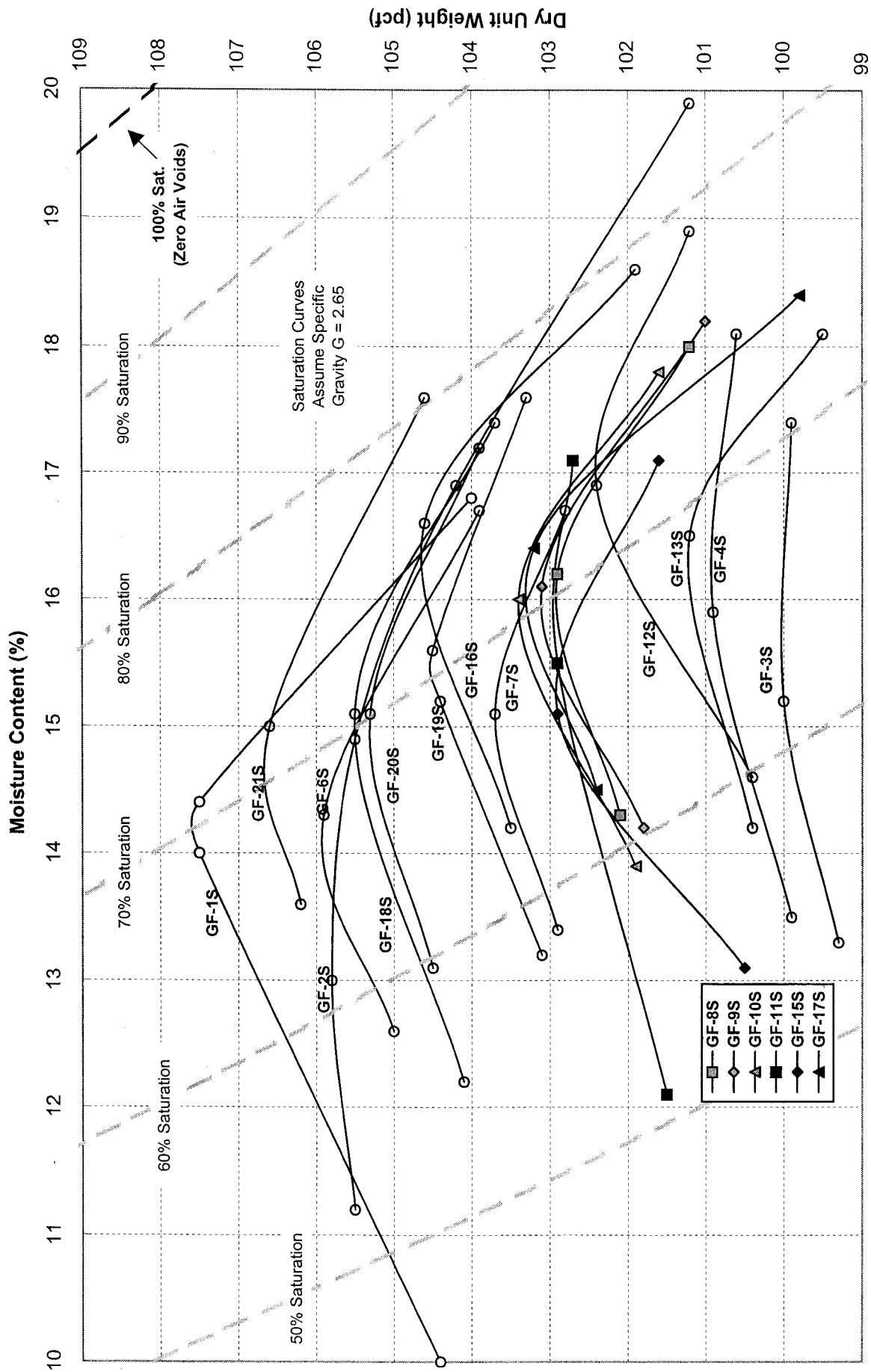


Figure 4-1: Standard Proctor Test Results for General Fill

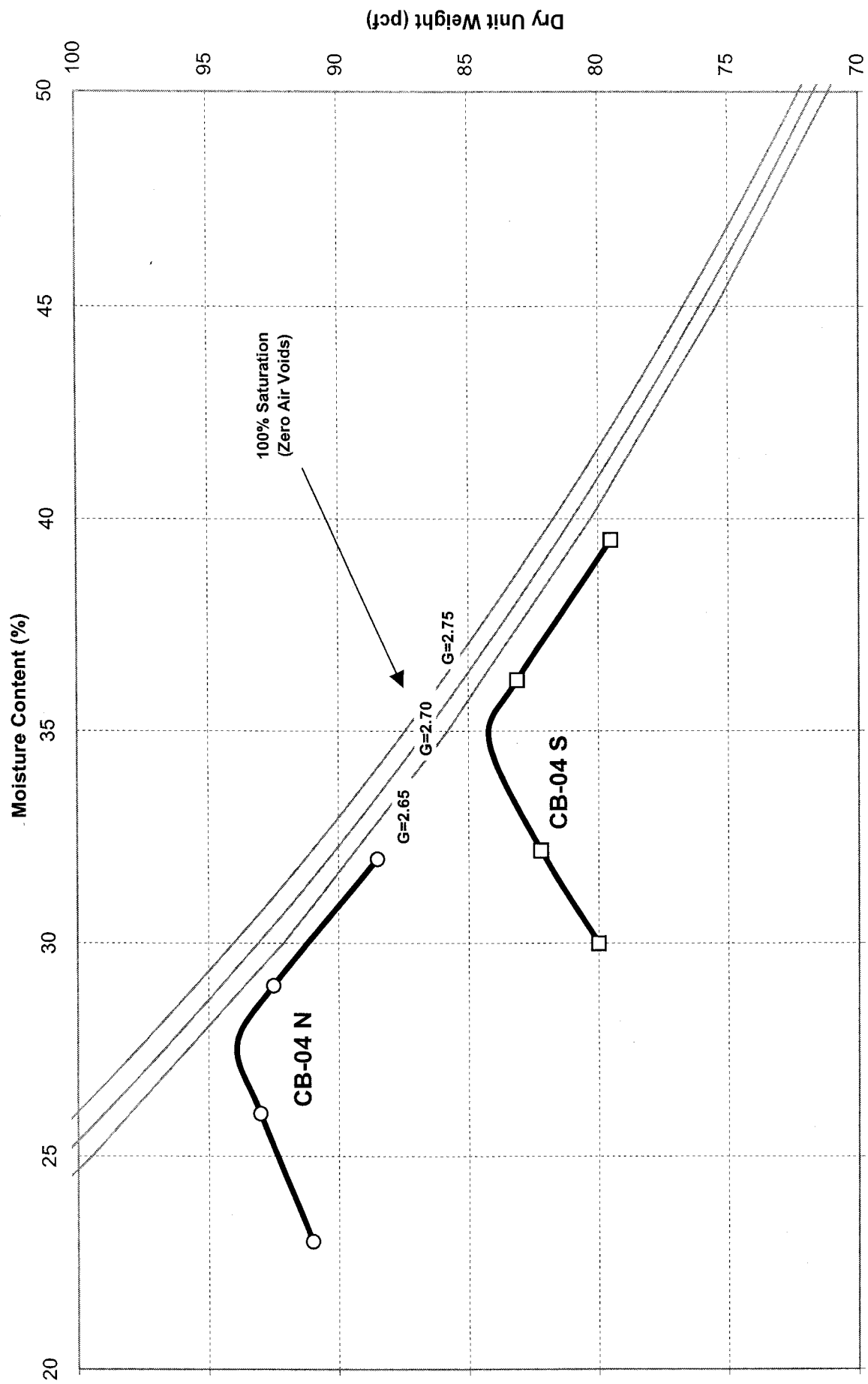


Figure 4-2: Standard Proctor Test Results for Low-Permeability Soil

5. CONSTRUCTION QUALITY ASSURANCE - GEOSYNTHETICS

5.1 General

GeoSyntec monitored the installation of the geosynthetic components of the double composite liner system in Cell 1A and the leachate storage area, as described in Section 2. At times, several liner system installation operations were conducted simultaneously during OHDF construction. When this occurred, the on-site CQA personnel monitored the operations that were considered most critical to the performance of the liner systems.

5.2 CQA of Geosynthetic Clay Liner

5.2.1 Conformance Testing and Documentation

A geosynthetic clay liner (GCL) was used in construction of the liner systems in Cell 1A and the leachate storage area. The Thermal Lock NWL-35 GCL used was manufactured by Bentofix Technologies, Inc. (Bentofix) in Barrie, Ontario, Canada. The GCL conformance samples were collected, from the rolls produced for the project, by Texas Research Institute (TRI) (an independent contractor authorized to collect samples for CQA testing) at Bentofix's manufacturing plant. TRI also performed the CQA conformance testing in accordance with the CQA Documents on the samples of GCL collected.

The MQC certificates and test results and the CQA conformance test results were reviewed by CQA personnel and were found to be in compliance with the CQA Documents. The results of the MQC and CQA conformance tests are summarized in Table 5-1. Table 5-1 also indicates the tests that were conducted, the required test frequencies, and the acceptance criteria in accordance with the CQA Documents.

It is noted that Table 5-1 is organized with respect to the GCL lot numbers and indicates the roll numbers in each lot of GCL that were sampled and tested as part of the MQC and CQA conformance testing. Each sheet of Table 5-1 also presents the total number of rolls (and square footage) of the GCL received from the respective lot number and also the cumulative number of rolls (and square footage) of the GCL received for the project (to evaluate the MQC and CQA test frequencies for each lot and for the project).

A total of 13 CQA conformance samples were tested for 1,860,000 square feet (ft²) of GCL delivered to the site for installation in Cell 1A, Cell 1B, and the leachate storage area. The CQA test frequency of 1 test per 143,000 ft² of GCL exceeded the minimum testing frequency of 1 test per 200,000 ft² required by the CQA Documents. As noted in Table 5-1, a minimum of one conformance sample was tested during CQA from each lot of GCL supplied for the project.

To evaluate the influence of leachate on the hydraulic conductivity of GCL, a sample from Roll # 86967 (Lot # 23092319) was tested using deionized water and leachate from St. Cloud Landfill (a similar solid waste landfill currently being used by City of St. Cloud in Osceola County) as permeant fluids during MQC testing. As noted in Table 5-1, the measured hydraulic conductivity of GCL with leachate as the permeant fluid was an order of magnitude lower than the measured hydraulic conductivity of GCL with deionized water as the permeant fluid. As a result, remaining MQC and CQA conformance tests were performed using water as the permeant fluid to avoid collection, handling, and transportation of leachate.

5.2.2 Field Monitoring Activities

5.2.2.1 Delivery and On-Site Storage

Upon delivery, GCL rolls were unloaded in an area located east of the Cell 1A construction area (i.e., in future Cell 2 footprint), stacked on a 1.5-ft high berm, and covered with a plastic sheets. The rolls were typically transported on site by a tractor with a special lifting attachment. CQA personnel periodically monitored the installer's delivery, unloading, and storage procedures and observed that the GCL was handled in an appropriate manner. The CQA personnel also compared the roll numbers of the GCL rolls delivered to the manufacturer's bill of lading. An inventory of the rolls delivered for the project was maintained by the CQA personnel and is available upon request. This inventory also includes the rolls that were approved for installation based on MQC and CQA test results and the rolls that were used during construction of Cell 1A and the leachate storage area. Only approved rolls were incorporated into the work.

5.2.2.2 Deployment

Prior to GCL deployment, the installer signed certificates of acceptance for the liner subbase, which are not included in the report but are available upon request. The GCL rolls were lifted using a spreader bar attached to a tractor. The panels were positioned using laborers assisted by a track-mounted, low-ground pressure, all-terrain vehicle (ATV).

CQA personnel monitored the deployment of the GCL rolls. During deployment, the CQA personnel checked for the following:

- manufacturing defects;
- damage that may have occurred during shipment, storage, and handling; and
- damage resulting from installation activities.

If any materials were observed to be damaged, the installer was notified and the damaged materials were either discarded or repaired. CQA personnel observed repair locations to verify conformance with the requirements of the CQA Documents.

CQA personnel also periodically monitored the deployment of the GCL as well as its condition after installation to ensure that the installer followed the following procedures:

- the GCL was unrolled and placed in a manner which kept the GCL in sufficient tension to avoid excessive wrinkling and was securely anchored in the anchor trench or ballasted with sand bags;
- the rolls were deployed with the non heat-treated, non-woven geotextile in contact with the geomembrane;
- adjacent GCL panels were overlapped a minimum of 6 inches along the length of the panels and 12 inches along the width of the panels; and
- granular bentonite was added between overlap areas;
- measures were taken to keep the GCL free of contamination and protected from premature hydration; and
- geomembrane installation immediately followed installation of the GCL.

Any observed holes or tears in the GCL were repaired by the installer by placing a patch of the same material over the hole or tear and at a distance of at least 1 ft beyond the edges of the hole or tear. In areas where premature hydration of the GCL was detected, the GCL was removed and replaced with new material.

5.3 CQA of Textured Geomembrane

5.3.1 Conformance Testing and Documentation

A 60-mil textured geomembrane was installed as primary and secondary liners in Cell 1A and the leachate storage area. The 60-mil textured geomembrane liner HDT 060A000 was supplied by GSE Lining Technologies, Inc. (GSE) in Houston, Texas. Conformance samples for textured geomembrane were collected (from the rolls produced for the project) by TRI at GSE's manufacturing plant in Houston Texas. TRI also performed the CQA conformance testing in accordance with the CQA Documents on the samples of textured geomembrane collected.

The MQC certificates and test results and the CQA conformance test results were reviewed by CQA personnel and were found to be in compliance with the CQA

Documents. The results of the MQC and CQA conformance tests are summarized in Table 5-2. Table 5-2 also indicates the tests that were conducted, the required test frequencies, and the acceptance criteria in accordance with the CQA Documents.

It is noted that Table 5-2 is organized with respect to the resin lot numbers and indicates the roll numbers from each resin lot that were sampled and tested as part of the MQC and CQA conformance testing. Each sheet of Table 5-2 also presents the total number of rolls (and square footage) of the textured geomembrane received from the respective resin lot number and the cumulative number of rolls (and square footage) of the textured geomembrane received for the project (to evaluate the MQC and CQA test frequencies for each lot and for the project).

A total of 21 CQA conformance samples were tested for approximately 2,021,000 ft² of textured geomembrane delivered to the site for installation in Cell 1A, Cell 1B, and the leachate storage area. The CQA test frequency of 1 test per 96,000 ft² for the textured geomembrane exceeded the minimum frequency of 1 test per 100,000 ft² required by the CQA Documents. As noted in Table 5-2, a minimum of one conformance sample was tested during CQA from each resin lot supplied for the project.

5.3.2 Field Monitoring Activities

5.3.2.1 Delivery and On-Site Storage

Upon delivery to the site, geomembrane rolls were stored in an area located east of the Cell 1A construction area (i.e., in future Cell 2 footprint) and stacked on HDPE perforated pipes. The rolls were typically transported by a tractor with a special lifting attachment. CQA personnel periodically monitored the installer's delivery, unloading, and storage procedures to ensure that the material was handled in an appropriate manner. The CQA personnel also compared the roll numbers of the geomembrane rolls delivered to the manufacturer's bill of lading. An inventory of the rolls delivered for the project was maintained by the CQA personnel and is available upon request. This inventory also includes the rolls that were approved for installation based on MQC and CQA test results and the rolls that were used during construction of Cell 1A and leachate storage area. Only approved rolls were incorporated into the work.

5.3.2.2 Deployment

The geomembrane rolls were lifted using a spreader bar attached to a tractor. The panels were positioned using laborers assisted by a track-mounted, low-ground pressure, ATV.

CQA personnel monitored the deployment of each geomembrane panel. During deployment, the CQA personnel checked for the following:

- manufacturing defects;
- damage that may have occurred during shipment, storage, and handling; and
- damage resulting from installation activities, including damage as a consequence of panel placement, seaming operations, or weather.

If any materials were observed to be damaged or deficient, the installer was notified and the damaged materials were either discarded or repaired. CQA personnel observed and documented the repair locations to verify compliance with the CQA Documents. Details of the geomembrane panel placement were recorded by CQA personnel on panel placement logs, which are not included in the report but are available upon request. Panel and repair locations are indicated on the Record Drawings included in Appendix A for the primary and secondary liners in Cell 1A and the leachate storage area.

5.3.2.3 Trial Seams

Prior to production seaming, the installer prepared geomembrane trial seams for each technician using each piece of seaming equipment. Additional trial seams were prepared every four to five hours. CQA personnel evaluated the trial seams as follows:

- trial seams were welded under similar conditions as production seaming;
- test strips were cut from the trial seams at random locations with a die press;
- three test strips were tested using a field tensiometer and compared to the passing criteria for the tests, which were as follows:

Fusion

- *Peel tests* - a minimum bonded seam strength of 78 lb/in.; and
- *Shear test* - a minimum bonded seam strength of 120 lb/in.

Extrusion

- *Peel test* - a minimum bonded seam strength of 70 lb/in.; and
- *Shear test* - a minimum bonded seam strength of 108 lb/in.

A total of 167 trial seams were observed by CQA personnel during OHDF construction; 112 trial seams were made using double-track fusion (i.e., hot wedge) welders and 55 were made using extrusion welders. If trial welds failed, the machine or welding process was adjusted and a new trial seam was made. The new sample was tested to ensure compliance with the above strength requirements. The procedure was repeated, as needed, until passing results were obtained.

Trial seam samples were not archived. Details of the trial seams, including the trial seam test results, are not included in the report but are available upon request.

5.3.2.4 Production Seams

Geomembrane production seaming operations were monitored by CQA personnel. The majority of the geomembrane production seams were fabricated using double-track fusion welders. Seam repairs were made using hand-held extrusion welders. Rub sheets were periodically used during production seaming to provide a clean surface to weld over. During or after fabrication, the geomembrane seams were visually examined for workmanship and continuity. Geomembrane seaming logs are not included in the report but are available upon request.

5.3.3 **Nondestructive Seam Testing**

5.3.3.1 Scope

Nondestructive testing of geomembrane seams was periodically monitored by CQA personnel. All geomembrane seams were nondestructively tested for continuity by the installer using the air pressure procedure for double-track fusion seams and the vacuum-box test procedure for extrusion welded seams. Failed air pressure seams, if applicable, were capped and then retested using vacuum-box test methods after determining the failed seam length. Leaks identified using the vacuum-box method were repaired and retested as described in Section 5.3.5.

5.3.3.2 Air Pressure Testing

Accessible double-track fusion seams were nondestructively tested using the air pressure test. The procedure used by the installer for air pressure testing was as follows:

- visually observe the integrity of the annulus of the section of seam being tested and isolating the section by sealing the ends using heat and pressure;
- insert the needle of a pressure test apparatus into the annulus at one end of the seam;

- inflate the annulus to a gauge pressure of a minimum 25 psi with an air pump and maintain the gauge pressure for at least 5 minutes;
- repair faulty area in accordance with Section 5.3.5 if the pressure loss exceeds 3 psi or if the pressure does not stabilize; and
- confirm airflow through the entire annulus by releasing the air from the seam at the opposite end from where the needle was inserted.

5.3.3.3 Vacuum-Box Testing

The vacuum-box was used by the installer to nondestructively test extrusion seams and repairs. The procedure used by the installer for vacuum testing was as follows:

- wet a strip of seam with a soapy solution;
- place the vacuum-box assembly over the wetted area, close the bleed valve and open the vacuum valve;
- force the box onto the sheet until a vacuum is observed;
- examine the seam through the viewing window for a period of approximately 20 seconds for the occurrence of air bubbles;
- remove the assembly and continue the process over the entire length of the seam; and
- record the location of any leaks.

Table 5-3 contains a summary of the nondestructive tests performed for the textured geomembrane seams. Table 5-3A presents nondestructive seam test results for primary liner in Cell 1A. Table 5-3B presents nondestructive seam test results for secondary liner in Cell 1A. Table 5-3C presents nondestructive seam test results for primary and secondary liners in the leachate storage area. If nondestructive testing indicated that repairs were necessary, repairs were made in accordance with procedures presented in Section 5.3.5. All repairs were tested using the vacuum-box test procedure.

5.3.4 Destructive Seam Sample Testing

5.3.4.1 Scope

In accordance with the CQA Documents, CQA personnel identified and collected geomembrane seam samples for destructive testing. The samples were tested by the CQA personnel in the on-site geosynthetics laboratory.

For a destructive seam sample to be considered as passing, the seam strength criteria described in Section 5.3.2.3 had to be met for at least four out of the five test specimens obtained from the sample. In addition, if one non-FTB failure was observed, the average of the five test specimens had to meet the specified strength criterion.

5.3.4.2 Sampling Procedures

Prior to the removal of the full seam sample, two geomembrane test strips were taken by the installer from either end of the proposed destructive sample. Each strip was peel-tested in the field. If the peel samples exhibited passing results, the adjacent destructive seam sample was removed and tested. At each destructive seam sample location, a test sample measuring approximately 12 in. across the seam and 42 in. along the seam, was obtained. The sample was divided into three pieces and distributed to: (i) the on-site geosynthetics laboratory for testing, (ii) the installer, and (iii) the owner as an archive sample.

5.3.4.3 Test Results

On-site laboratory testing of geomembrane seam samples was performed in accordance with the CQA Documents. At the on-site geosynthetics laboratory, five 1-in wide test specimens were removed from the destructive seam sample using a die press. On a calibrated tensiometer, five test specimens were peel-tested for adhesion strength. For fusion seams, peel tests were performed on both the bottom (inside track) and top (outside track) peels. Additionally, five specimens were tested for shear strength. The seam acceptance/rejection criteria described in Sections 5.3.2.3 and 5.3.4.1 were used to evaluate the destructive seam samples.

The destructive seam test results are summarized in Table 5-4. The destructive seam test results for primary and secondary liners installed in Cell 1A are presented in Tables 5-4A and 5-4B, respectively. For primary liner installed in Cell 1A, 61 destructive seam samples were tested for a total seam length of 25,000 ft (approx.). This corresponds to an approximate sample frequency of 1 per 400 lf of seam. For secondary liner installed in Cell 1A, 52 destructive seam samples were tested for a total seam length of 23,000 ft (approx.). This corresponds to an approximate sample frequency of 1 per 450 lf of seam.

The destructive seam test results for primary and secondary liners installed in the leachate storage area are presented in Table 5-4C. For primary liner installed in the leachate storage area, 11 destructive seam samples were tested for a total seam length of 5,500 ft (approx.). This corresponds to an approximate sample frequency of 1 per 500 lf of seam. For secondary liner installed in the leachate storage area, 12 destructive seam samples were tested for a total seam length of 4,700 ft (approx.). This corresponds to an approximate sample frequency of 1 per 400 lf of seam. As noted, the destructive seam test frequencies met or exceeded the minimum frequency of 1 per 500 lf of production seams required by the CQA Documents.

As part of destructive seam testing of geomembranes during CQA, a total of 136 destruct sample were tested. Out of all the seam samples tested destructively, 5 failed to meet the required criteria. For the failed samples, additional test strips were taken from the seam at locations approximately 10 ft from each side of the failing sample location. If the additional test strips had passing results, full destructive seam sample was collected. If the samples did not pass, test strips were obtained at another location approximately 10 ft further from the failure and the procedure was repeated until passing samples were obtained and the failing area was localized. These destructive seam samples were tested in accordance with procedures described previously in this section. Once the limits of the failing seam were evaluated, the entire seam length between the passing samples was repaired by the procedures described in Section 5.3.5.

5.3.5 Geomembrane Repairs

The repair procedures presented in this subsection were used by the installer to patch holes and tears, spot-extrude impact damage or other minor defects, and for grinding and extrusion welding small sections of failed fusion seams (if the exposed edge was accessible). In the cases where patches or caps were used to repair the damaged geomembrane (i.e., small holes, tears, or on seams which failed nondestructive or destructive testing), an approximately 12-in. wide capping strip was used.

During the repair or panel tie-in operations, the following procedures were implemented:

- technicians and seaming equipments used were required to pass trial welds;
- patches or caps extended at least 6 in. beyond the edge of the defect and all corners were rounded; and
- repairs were tested using vacuum box and visually observed for continuity.

Seam and panel repair logs prepared by GeoSyntec during CQA are not included in the report but are available upon request. Record drawings illustrating layout of panels and the location of seams and repairs are included in Appendix A.

5.4 CQA of Smooth Geomembrane

A 60-mil smooth geomembrane was used only to construct the four flexible storage containers in the leachate storage area. The 60-mil smooth geomembrane HDE 060A000 was supplied by GSE. Conformance samples for smooth geomembrane were collected (from the rolls produced for the project) by TRI at GSE's manufacturing plant in Houston Texas. TRI also performed the CQA conformance testing on the samples of the smooth geomembrane collected.

The MQC certificates and test results and the CQA conformance test results were reviewed by CQA personnel and were found to be in compliance with the CQA Documents. The results of the MQC and CQA conformance tests are summarized in Table 5-5. Table 5-5 includes the test that were conducted, the required test frequencies, and the acceptance criteria in accordance with the CQA Documents. Table 5-5 also presents the total number of rolls (and square footage) of the smooth geomembrane received for the project to evaluate the MQC and CQA test frequencies.

A total of 3 CQA conformance samples were tested for approximately 202,000 ft² of smooth geomembrane delivered to the site for use in the construction of the four flexible storage containers in the leachate storage area. The CQA test frequency of 1 test per 67,000 ft² (approx.) of smooth geomembrane exceeded the minimum frequency of 1 test per 100,000 ft² required by the CQA Documents.

All smooth geomembrane seams were nondestructively tested using the air pressure or the vacuum-box test procedure as discussed in Section 5.3.3. Table 5-6 summarizes the nondestructive tests performed for the smooth geomembrane seams. If nondestructive testing indicated that repairs were necessary, repairs were made in accordance with procedures presented in Section 5.3.5. All repairs were tested using the vacuum-box test procedure.

The destructive seam tests for the smooth geomembrane were performed in accordance with Section 5.3.4 and are summarized in Table 5-7. As noted, 10 destructive seam samples were tested for a total seam length of 4,000 ft (approx.). This corresponds to an approximate sample frequency of 1 per 400 lf of seam, which exceeds the minimum sample frequency of 1 per 500 lf of production seams required by the CQA Documents.

5.5 CQA of Primary Geocomposite

5.5.1 Conformance Testing and Documentation

A tri-planar geocomposite was used as the primary drainage geocomposite in the double composite liner system in Cell 1A. The primary geocomposite used was Tendrain 7100-2 manufactured by the Tenax Corporation (Tenax) in Baltimore, Maryland. The primary geocomposite conformance samples were collected (from the rolls produced for the project) by TRI at the Tenax's manufacturing plant in Evergreen, Alabama. TRI also performed the CQA conformance testing on the samples of primary geocomposite collected.

The MQC certificates and test results and the CQA conformance test results were reviewed by CQA personnel and were found to be in compliance with the CQA Documents. The results of the MQC and CQA conformance tests are summarized in Tables 5-8A, 5-8B, and 5-8C. Table 5-8A presents the CQA and MQC test results for the primary geocomposite rolls produced for the project. Table 5-8B presents the MQC test results for the geotextile rolls used to manufacture the primary geocomposite rolls produced for the project. Table 5-8C presents the MQC test results for the geonet rolls used to manufacture the primary geocomposite rolls for the project.

Table 5-8A presents the CQA and MQC test results for the primary geocomposite rolls and CQA test results for the geotextile component of the primary geocomposite. Table 5-8A also indicates the tests that were conducted, the required test frequencies, and the acceptance criteria in accordance with the CQA Documents. A total of 5 CQA conformance samples were tested for approximately 895,000 ft² of primary geocomposite approved for installation in Cells 1A and 1B. The CQA test frequency of 1 test per 179,000 ft² (approx.) of the primary geocomposite exceeded the minimum frequency of 1 test per 200,000 ft² required by the CQA Documents. As noted in Table 5-8A, a minimum of one conformance sample was tested during CQA from each geocomposite lot.

It is noted that during CQA and MQC testing, the transmissivity of the primary geocomposite was measured under compressive stresses of 500 psf and 13,500 psf for 100 hours. The tests were performed with the primary geocomposite sandwiched between 60-mil textured geomembrane and the soil actually used in the liner protective layer. The transmissivity of the primary geocomposite reported in Table 5-8A is the minimum transmissivity measured during the 100-hour test.

As noted in Table 5-8A, the transmissivity measured under compressive stress of 500 psf for the primary geocomposite Roll # 3506561 (PGC-3) marginally failed to meet the specified requirement. As a result, additional tests were performed to bracket the failing roll which included testing rolls on either side of the failing roll until results meeting the

specified requirements were obtained. Four rolls between the passing rolls (3506560 and 3506565) were rejected and not used for the project.

Table 5-8B presents the MQC test results for the geotextile component of the primary geocomposite rolls approved for the project. Several rolls of primary geocomposite were manufactured from the same roll of geotextile. Approximately 1,790,000 ft² of geotextile was used to manufacture the primary geocomposite rolls for the project. As part of the MQC testing, 27 geotextile rolls were tested for mass per unit area, grab strength, trapezoidal tear strength, and puncture strength. Apparent opening size and permittivity tests were also performed on 16 of these 27 geotextile samples. The approximate MQC test frequency of 1 test per 66,000 ft² (or 112,000 ft²) for the geotextile component of the primary geocomposite exceeded the minimum frequency of 1 test per 100,000 ft² (or 250,000 ft²) required by the CQA Documents for the respective tests.

Table 5-8C presents the MQC test results for the geonet component of the primary geocomposite rolls approved for the project. Several rolls of primary geocomposite were manufactured from the same roll of geonet. Thirty two geonet rolls were tested for approximately 895,000 ft² of geonet used to manufacture the primary geocomposite rolls for the project. The MQC test frequency of 1 test per 28,000 ft² (approx.) for the geonet component of the primary geocomposite exceeded the minimum frequency of 1 test per 100,000 ft² required by the CQA Documents.

5.5.2 Field Monitoring Activities

5.5.2.1 Delivery and On-Site Storage

Upon delivery to the site, primary geocomposite rolls were stored in an area located east of the Cell 1A construction area (i.e., in future Cell 2 footprint) and stacked on HDPE perforated pipes. The rolls were typically transported by a tractor with a special lifting attachment. CQA personnel periodically monitored the installer's delivery, unloading, and storage procedures to ensure that the material was handled in an appropriate manner. The CQA personnel also compared the roll numbers of the primary geocomposite rolls delivered to the manufacturer's bill of lading. An inventory of the rolls delivered for the project was maintained by the CQA personnel and is available upon request. This inventory also includes the rolls that were approved for installation based on MQC and CQA test results and the rolls that were used during construction of Cell 1A. Only approved rolls were incorporated into the work.

5.5.2.2 Deployment

CQA personnel monitored the deployment of the primary geocomposite for manufacturing defects, damage that may have occurred during shipment, storage, or

handling, and damage resulting from installation activities. If the materials were observed to be damaged, the installer was notified and the damaged materials were either discarded or repaired. CQA personnel observed repair locations to verify conformance with the CQA Documents. CQA personnel periodically monitored the deployment of the primary geocomposite, as well as its condition after installation, to confirm that the installer took measures to:

- securely anchor the geonet composite in the anchor trench or ballast it with sand bags;
- unroll the geonet composite down the slope (i.e., rolls were aligned perpendicular to the slope contours) in a manner that kept the panel in sufficient tension to avoid excessive wrinkling;
- avoid entrapment of dust, stones, or other objects that would damage or clog the geonet composite;
- avoid damaging the underlying geomembrane during deployment;
- secure the geonet composite panels with nylon fasteners, installed on a maximum 5-ft spacing laterally and at 1-ft spacing on end seams; and
- overlap and continuously sew the upper geotextile edges.

Any observed holes in the geotextile component of the primary geocomposite were repaired by placing a patch of non-woven geotextile over the hole that extended at least one foot beyond the edge of the hole. These patches were continuously thermally bonded to the undamaged portion of the geocomposite. This method was also used along the tie-in at the toe of the slope and along trimmed panels. Any observed holes or tears in the geonet component of the composite were repaired by the installer by placing a patch of the same material over or under the hole or tear, at least 2-ft beyond the edges of the hole or tear. These patches were secured using nylon fasteners, followed by thermal bonding of the uppermost geotextile of the patch to the undamaged portion of the geocomposite.

5.6 CQA of Secondary Geocomposite

5.6.1 Conformance Testing and Documentation

A bi-planar geocomposite was used as secondary drainage geocomposite in the double composite liner system in Cell 1A. The secondary geocomposite used was Fabrinet HS geocomposite manufactured by GSE in Kingstree, South Carolina. The secondary geocomposite conformance samples were collected (from the rolls produced

for the project) by TRI at GSE's manufacturing plant in Kingstree, South Carolina. TRI also performed the CQA conformance testing on the samples of the secondary geocomposite collected.

The MQC certificates and test results and the CQA conformance test results were reviewed by CQA personnel and were found to be in compliance with the CQA Documents. The results of the MQC and CQA conformance tests are summarized in Tables 5-9A, 5-9B, and 5-9C. Table 5-9A presents the CQA and MQC test results for the secondary geocomposite rolls produced for the project. Table 5-9B presents the MQC test results for the geotextile rolls used to manufacture the secondary geocomposite rolls for the project. Table 5-9C presents the MQC test results for the geonet rolls used to manufacture the secondary geocomposite rolls for the project.

Table 5-9A presents the CQA and MQC test results for the secondary geocomposite rolls and CQA test results for the geotextile component of the secondary geocomposite. It includes the test that were conducted, the required test frequencies, and the acceptance criteria in accordance with the CQA Documents. A total of 5 CQA conformance samples were tested for approximately 887,000 ft² of secondary geocomposite delivered to the site for installation in Cells 1A and 1B. The CQA test frequency of 1 test per 177,000 ft² (approx.) for the secondary geocomposite exceeded the minimum frequency of 1 test per 200,000 ft² required by the CQA Documents. As noted in Table 5-9A, a minimum of one conformance sample was tested during CQA from each geonet lot.

It is noted that during CQA and MQC testing, the transmissivity of the secondary geocomposite was measured under compressive stresses of 500 psf and 13,500 psf for 100 hours. The tests were performed with the secondary geocomposite sandwiched between a GCL and a 60-mil textured geomembrane. The transmissivity of the secondary geocomposite reported in Table 5-9A is the minimum transmissivity measured during the 100-hour test.

Table 5-9B presents the MQC test results for the geotextile component of the secondary geocomposite rolls delivered for the project. Several rolls of secondary geocomposite were manufactured from the same roll of geotextile. 42 geotextile rolls were tested for approximately 1,775,000 ft² of geotextile used to manufacture the secondary geocomposite rolls for the project. The MQC test frequency of 1 test per 42,000 ft² (approx.) for the geotextile component of the secondary geocomposite exceeded the minimum frequency of 1 test per 100,000 ft² required by the CQA Documents.

Table 5-9C presents the MQC test results for the geonet component of the secondary geocomposite rolls delivered for the project. Several rolls of secondary geocomposite were manufactured from the same roll of geonet. 40 geonet rolls were tested for approximately 887,000 ft² of geonet used to manufacture the secondary geocomposite rolls delivered for the project. The MQC test frequency of 1 test per 22,000 ft² (approx.)

for the geonet component of the secondary geocomposite exceeded the minimum frequency of 1 test per 100,000 ft² required by the CQA Documents.

5.6.2 Field Monitoring Activities

5.6.2.1 Delivery and On-Site Storage

Upon delivery to the site, secondary geocomposite rolls were stored in an area located east of the Cell 1A construction area (i.e., in future Cell 2 footprint) and stacked on HDPE perforated pipes. The rolls were typically transported by a tractor with a special lifting attachment. CQA personnel periodically monitored the installer's delivery, unloading, and storage procedures to ensure that the material was handled in an appropriate manner. The CQA personnel also compared the roll numbers of the secondary geocomposite rolls delivered to the manufacturer's bill of lading. An inventory of the rolls delivered for the project was maintained by the CQA personnel and is available upon request. This inventory also includes the rolls that were approved for installation based on MQC and CQA test results and the rolls that were used during construction of Cell 1A. Only approved rolls were incorporated into the work.

5.6.2.2 Deployment

CQA personnel monitored the deployment of the secondary geocomposite for manufacturing defects, damage that may have occurred during shipment, storage, or handling, and damage resulting from installation activities. If the materials were observed to be damaged, the installer was notified and the damaged materials were either discarded or repaired. CQA personnel observed repair locations to verify conformance with the CQA Documents. CQA personnel periodically monitored the deployment of the secondary geocomposite, as well as its condition after installation, to confirm that the installer took measures to:

- securely anchor the geonet composite in the anchor trench or ballast it with sand bags;
- unroll the geonet composite down the slope (i.e., rolls were aligned perpendicular to the slope contours) in a manner that kept the panel in sufficient tension to avoid excessive wrinkling;
- avoid entrapment of dust, stones, or other objects that would damage or clog the geonet composite;
- avoid damaging the underlying geomembrane during deployment;

- secure the geonet composite panels with nylon fasteners, installed on a maximum 5-ft spacing laterally and at 1-ft spacing on end seams; and
- overlap and continuously sew the upper geotextile edges.

Any observed holes in the geotextile component of the secondary geocomposite were repaired by placing a patch of non-woven geotextile over the hole that extended at least one foot beyond the edge of the hole. These patches were continuously thermally bonded to the undamaged portion of the geocomposite. This method was also used along the tie-in at the toe of the slope and along trimmed panels. Any observed holes or tears in the geonet component of the composite were repaired by the installer by placing a patch of the same material over or under the hole or tear, at least 2-ft beyond the edges of the hole or tear. These patches were secured using nylon fasteners, followed by thermal bonding of the uppermost geotextile of the patch to the undamaged portion of the geocomposite.

5.7 CQA of Non-Woven Geotextile

5.7.1 Conformance Testing and Documentation

A non-woven geotextile was used as filter/seperator fabric to surround the aggregate in the leachate collection system in Cell 1A, cover the gravel bed in Cell 1 sump, and to wrap the gravel in the leachate storage area sumps in the leak detection layer (between the primary and secondary liners). The 8-oz/yd², needle-punched, non-woven geotextile (GE 00808002) was manufactured by GSE in Kingstree, South Carolina. The CQA conformance samples for the non-woven geotextile were collected by GeoSyntec on-site and shipped to TRI. TRI performed the CQA conformance testing on the samples of the non-woven geotextile collected.

The MQC certificates and test results and the CQA conformance test results were reviewed by CQA personnel and were found to be in compliance with the CQA Documents. The results of the MQC and CQA conformance tests are summarized in Table 5-10. Table 5-10 also indicates the tests that were conducted, the required test frequencies, and the acceptance criteria in accordance with the CQA Documents.

A CQA conformance samples was tested for approximately 36,000 ft² of the non-woven geotextile delivered to the site for installation in Cell 1A, Cell 1B, and the leachate storage area. The CQA test frequency of 1 test per 36,000 ft² of non-woven geotextile exceeded the minimum testing frequency of 1 test per 200,000 ft² required by the CQA Documents.

5.7.2 Field Monitoring Activities

5.7.2.1 Delivery and On-Site Storage

Upon delivery to the site, secondary geocomposite rolls were stored in an area located east of the Cell 1A construction area (i.e., in future Cell 2 footprint) and stacked on HDPE perforated pipes. The rolls were typically transported by a tractor with a special lifting attachment. CQA personnel periodically monitored the installer's delivery, unloading, and storage procedures to ensure that the material was handled in an appropriate manner.

5.7.2.2 Deployment

CQA personnel monitored the deployment of the non-woven geotextile rolls for manufacturing defects; damage that may have occurred during shipment, storage, and handling; and damage resulting from installation activities. If any materials were observed to be damaged, the installer was notified and the damaged materials were either discarded or repaired. CQA personnel observed repair locations to verify conformance with the requirements of the CQA Documents.

After deployment of the geotextile, CQA personnel observed that the installer overlapped geotextile panels end-to-end a minimum of 24-in. and laterally a minimum of 6-in.

5.8 Interface Friction Testing

As discussed in Section 2, the liner system in Cell 1A consists (from top to bottom) of the liner protective layer, primary geocomposite, primary liner, primary GCL, secondary geocomposite, secondary liner, secondary GCL and prepared subbase. Tests were performed in accordance with the CQA Documents to evaluate the interface shear strength for the various components of the liner system and the internal strength of the GCL. All tests for interface shear strength and the internal strength of the GCL were performed by SGI Testing Services, LLC, (SGI) in Norcross, Georgia.

Tests were performed using samples of geosynthetics collected from rolls that were actually installed in Cell 1A. The soils for liner protective layer and liner subbase were obtained from the on-site borrow source, Borrow Area A, and were similar to the sandy soils used in construction of these two soil components of the liner system. The following rolls of geosynthetics were used in the interface friction tests as indicated in the tables presented above (i.e., Tables 5-1, 5-2, 5-8A, and 5-9A):

- GCL – Roll #87000 (Lot #23092319);
- Textured geomembrane – Roll #108101974 (Lot #8230906);
- Tri-Planar geocomposite – Roll #3506717 (Lot #35041); and
- Bi-Planar geocomposite – Roll #131131239 (Lot #CB13080403).

The 6 different interfaces between the various components of the liner system and the internal strength of the GCL were tested at normal stresses of 5,000, 10,000, and 15,000 psf. Peak (at small displacement) and residual (at large displacements) shear strengths were measured at each normal stress. The interface shear tests were conducted under wetted conditions. GCL was soaked and consolidated prior to testing. The following liner system interfaces were tested (from top to bottom):

- (1) Liner protective layer soil / Tri-planar geocomposite;
- (2) Tri-planar geocomposite / Textured geomembrane;
- (3) Textured geomembrane / GCL (non-heat treated side);
- (4) GCL (heat treated side) / Bi-planar geocomposite;
- (5) Bi-planar geocomposite / Textured geomembrane;
- (6) GCL (heat treated side) / Subbase soil; and
- (7) Internal strength of the GCL.

The minimum shear strengths required by the CQA Documents are presented in Table 5-11A. The measured peak and residual shear strengths are summarized in Table 5-11B and are presented in Figures 5-1 and 5-2, respectively. As noted, the measured peak and residual shear strengths exceeded the minimum specification requirements.

Table 5-1

CQA AND MQC TEST RESULTS FOR GEOSYNTHETIC CLAY LINER (BENTOFIX)

LOT NO. 23092319

PROPERTY	CONSTRUCTION QUALITY ASSURANCE (CQA)	MANUFACTURING QUALITY CONTROL (MQC)				
		Bentonite Content (lb/ft ²)	Bentonite Moisture Content ¹ (%)	Bentonite Free Swell ² (ml)	Peel Strength (lb)	Hydraulic Conductivity (cm/s)
TEST STANDARD	ASTM D 5887	ASTM D 5993	ASTM D 4643	ASTM D 5890	ASTM D 4632	ASTM D 5887
PROJECT SPECS.	$\leq 5.0 \times 10^{-9}$	≥ 0.75	≤ 25	≥ 24	≥ 15	$\leq 5.0 \times 10^{-9}$
TESTING FREQUENCY	1 per 200,000 ft ² (3)	1 per 40,000 ft ² (3)			1 per 100,000 ft ² (3)	

ROLL NUMBER	CQA SAMPLE ID	TEST RESULTS					PASS/FAIL (P/F)	
							CQA	MQC
86967	GCL-1	9.3×10^{-10}	0.92	8	33	48	P	P
86983			0.93	8	33	46		P
86999			0.96	8	33	50		P
87000 ⁴	GCL-2	2.6×10^{-9}					P	
87015			1.09	8	33	42		P
87031			0.99	8	33	38		P

Notes:

- 1 Four tests were performed per railcar (approximately 200,000 lbs). Reported value represents maximum bentonite moisture content for the 4 tests.
 2 Four tests were performed per railcar (approximately 200,000 lbs). Reported value represents minimum bentonite free swell for the 4 tests.
 3 A minimum of 1 test per lot was required.
 4 This roll was used to perform interface friction tests.

Average Roll Area (150 ft x 15.5 ft):	2,325	ft ²	SHEET NO.	1	of	11
No. of Rolls in Lot:	77		CUMULATIVE NUMBER OF ROLLS:	77		
Area in Lot:	179,025	ft ²	CUMULATIVE AREA:	179,025	ft ²	

Table 5-1 (continued)

**CQA AND MQC TEST RESULTS FOR GEOSYNTHETIC CLAY LINER (BENTOFIX)
LOT NO. 23092409**

CONSTRUCTION QUALITY ASSURANCE (CQA)		MANUFACTURING QUALITY CONTROL (MQC)				
PROPERTY	Hydraulic Conductivity (cm/s)	Bentonite Content (lb/ft ²)	Bentonite Moisture Content ¹ (%)	Bentonite Free Swell ² (ml)	Peel Strength (lb)	Hydraulic Conductivity (cm/s)
TEST STANDARD	ASTM D 5887	ASTM D 5993	ASTM D 4643	ASTM D 5890	ASTM D 4632	ASTM D 5887
PROJECT SPECS.	≤ 5.0x10 ⁻⁹	≥ 0.75	≤ 25	≥ 24	≥ 15	≤ 5.0x10 ⁻⁹
TESTING FREQUENCY	1 per 200,000 ft ² (3)	1 per 40,000 ft ² (3)				1 per 100,000 ft ² (3)

ROLL NUMBER	CQA SAMPLE ID	TEST RESULTS						PASS/FAIL (P/F)	
								CQA	MQC
87046			0.94	6	26	46			P
87055	GCL-3	1.2×10^{-9}	1.05	6	26	50		P	P
87062			0.97	6	26	43			P
87129			0.95	6	26	42			P
87153			0.92	6	26	48			P
87161	GCL-4	1.1×10^{-9}						P	
87177			0.93	6	26	41			P

Notes:

¹ Four tests were performed per railcar (approximately 200,000 lbs). Reported value represents maximum bentonite moisture content for the 4 tests.

² Four tests were performed per railcar (approximately 200,000 lbs). Reported value represents minimum bentonite free swell for the 4 tests.

³ A minimum of 1 test per lot was required.

Average Roll Area (150 ft x 15.5 ft): 2,325 ft²

No. of Rolls in Lot: 78

Area in Lot: 181,350 ft²

SHEET NO. 2

of 11

CUMULATIVE NUMBER OF ROLLS: 155

360,375

CUMULATIVE AREA: ft²

Table 5-1 (continued)

CQA AND MQC TEST RESULTS FOR GEOSYNTHETIC CLAY LINER (BENTOFIX)
LOT NO. 13093009

CONSTRUCTION QUALITY ASSURANCE (CQA)		MANUFACTURING QUALITY CONTROL (MQC)				
PROPERTY	Hydraulic Conductivity (cm/s)	Bentonite Content (lb/ft ²)	Bentonite Moisture Content ¹ (%)	Bentonite Free Swell ² (ml)	Peel Strength (lb)	Hydraulic Conductivity (cm/s)
TEST STANDARD	ASTM D 5887	ASTM D 5993	ASTM D 4643	ASTM D 5890	ASTM D 4632	ASTM D 5887
PROJECT SPECS.	$\leq 5.0 \times 10^{-9}$	≥ 0.75	≤ 25	≥ 24	≥ 15	$\leq 5.0 \times 10^{-9}$
TESTING FREQUENCY	1 per 200,000 ft ² (3)	1 per 40,000 ft ² (3)			1 per 100,000 ft ² (3)	

ROLL NUMBER	CQA SAMPLE ID	TEST RESULTS						PASS/FAIL (P/F)	
								CQA	MQC
96501	GCL-5	6.8×10^{-10}	0.94	6	28	44		P	P
96502			0.94	6	28	44			P
96517			1.12	6	28	37	1.31×10^{-9}		P
96533			1.01	6	28	42			P
96544			1.13	6	28	50	1.87×10^{-9}		P
96560			0.96	6	28	45			P
96576			0.99	6	28	48			P
Notes: 1 Four tests were performed per railcar (approximately 200,000 lbs). Reported value represents maximum bentonite moisture content for the 4 tests. 2 Four tests were performed per railcar (approximately 200,000 lbs). Reported value represents minimum bentonite free swell for the 4 tests. 3 A minimum of 1 test per lot was required.									
Average Roll Area (150 ft x 15.5 ft):		2,325	ft ²	SHEET NO.		3	of	11	
No. of Rolls in Lot:		77		CUMULATIVE NUMBER OF ROLLS:		232			
Area in Lot:		179,025	ft ²	CUMULATIVE AREA:		539,400			ft ²

Table 5-1 (continued)

**CQA AND MQC TEST RESULTS FOR GEOSYNTHETIC CLAY LINER (BENTOFIX)
LOT NO. 13100209**

MANUFACTURING QUALITY CONTROL (MQC)							CONSTRUCTION QUALITY ASSURANCE (CQA)
PROPERTY	Hydraulic Conductivity (cm/s)	Bentonite Content (lb/ft ²)	Bentonite Moisture Content ¹ (%)	Bentonite Free Swell ² (ml)	Peel Strength (lb)	Hydraulic Conductivity (cm/s)	
TEST STANDARD	ASTM D 5887	ASTM D 5993	ASTM D 4643	ASTM D 5890	ASTM D 4632	ASTM D 5887	
PROJECT SPECS.	≤ 5.0x10 ⁻⁹	≥ 0.75	≤ 25	≥ 24	≥ 15	≤ 5.0x10 ⁻⁹	
TESTING FREQUENCY	1 per 200,000 ft ² (3)			1 per 40,000 ft ² (3)		1 per 100,000 ft ² (3)	

ROLL NUMBER	CQA SAMPLE ID	TEST RESULTS	TEST RESULTS					PASS/FAIL (P/F)	
								CQA	MQC
96578	GCL-6	2.2×10^{-9}	1.02	10	38	47		P	P
96594			0.91	10	38	55	1.23×10^{-9}		P
96606			0.91	10	38	55			P
96617			0.95	10	38	40			P
96626			0.94	10	38	51	1.56×10^{-9}		P
96642			0.93	10	38	51			P

Notes:

- ¹ Four tests were performed per railcar (approximately 200,000 lbs). Reported value represents maximum bentonite moisture content for the 4 tests.
² Four tests were performed per railcar (approximately 200,000 lbs). Reported value represents minimum bentonite free swell for the 4 tests.
³ A minimum of 1 test per lot was required.

Average Roll Area (150 ft x 15.5 ft):

2,325 ft²

SHEET NO.

4

of

11

No. of Rolls in Lot:

78

CUMULATIVE NUMBER OF ROLLS:

310

ft²

Area in Lot:

181,350 ft²

CUMULATIVE AREA:

720,750

Table 5-1 (continued)

CQA AND MQC TEST RESULTS FOR GEOSYNTHETIC CLAY LINER (BENTOFIX)
LOT NO. 13100609

PROPERTY	CONSTRUCTION QUALITY ASSURANCE (CQA)	MANUFACTURING QUALITY CONTROL (MQC)				
		Bentonite Content (lb/ft ²)	Bentonite Moisture Content ¹ (%)	Bentonite Free Swell ² (ml)	Peel Strength (lb)	Hydraulic Conductivity (cm/s)
TEST STANDARD	ASTM D 5887	ASTM D 5993	ASTM D 4643	ASTM D 5890	ASTM D 4632	ASTM D 5887
PROJECT SPECS.	$\leq 5.0 \times 10^{-9}$	≥ 0.75	≤ 25	≥ 24	≥ 15	$\leq 5.0 \times 10^{-9}$
TESTING FREQUENCY	1 per 200,000 ft ² (3)	1 per 40,000 ft ² (3)				
		1 per 100,000 ft ² (3)				

ROLL NUMBER	CQA SAMPLE ID	TEST RESULTS	TEST RESULTS				PASS/FAIL (P/F)	
							CQA	MQC
96707			0.87	6	30	66		P
96719			0.89	6	30	66		P
96723	GCL-7	1.8×10^{-9}	0.94	6	30	41	P	P
96739			0.96	6	30	45		P
96755			1.01	6	30	48		P
96771			0.99	6	30	51		P
Notes: 1 Four tests were performed per railcar (approximately 200,000 lbs). Reported value represents maximum bentonite moisture content for the 4 tests. 2 Four tests were performed per railcar (approximately 200,000 lbs). Reported value represents minimum bentonite free swell for the 4 tests. 3 A minimum of 1 test per lot was required.								
Average Roll Area (150 ft x 15.5 ft):			2,325	ft ²	SHEET NO.		5	of 11
No. of Rolls in Lot:			77		CUMULATIVE NUMBER OF ROLLS:		387	
Area in Lot:			179,025	ft ²	CUMULATIVE AREA:		899,775	ft ²

Table 5-1 (continued)

CQA AND MQC TEST RESULTS FOR GEOSYNTHETIC CLAY LINER (BENTOFIX)
LOT NO. 23100809

PROPERTY	CONSTRUCTION QUALITY ASSURANCE (CQA)	MANUFACTURING QUALITY CONTROL (MQC)				
		Bentonite Content (lb/ft ²)	Bentonite Moisture Content (%)	Bentonite Free Swell ² (ml)	Peel Strength (lb)	Hydraulic Conductivity (cm/s)
TEST STANDARD	Hydraulic Conductivity (cm/s) ASTM D 5887	ASTM D 5993	ASTM D 4643	ASTM D 5890	ASTM D 4632	ASTM D 5887
PROJECT SPECS.	$\leq 5.0 \times 10^{-9}$	≥ 0.75	≤ 25	≥ 24	≥ 15	$\leq 5.0 \times 10^{-9}$
TESTING FREQUENCY	1 per 200,000 ft ² (3)	1 per 40,000 ft ² (3)				
		1 per 100,000 ft ² (3)				

ROLL NUMBER	CQA SAMPLE ID	TEST RESULTS	TEST RESULTS					PASS/FAIL (P/F)	
								CQA	MQC
87541	GCL-8	1.7×10^{-9}	0.90	6	27	46		P	P
87557			0.95	6	27	43	1.81×10^{-9}		P
87573			0.94	6	27	50			P
87589			0.94	6	27	41	1.60×10^{-9}		P
87597			0.92	6	27	51			P
87613			0.93	6	27	46			P
Notes: 1 Four tests were performed per railcar (approximately 200,000 lbs). Reported value represents maximum bentonite moisture content for the 4 tests. 2 Four tests were performed per railcar (approximately 200,000 lbs). Reported value represents minimum bentonite free swell for the 4 tests. 3 A minimum of 1 test per lot was required.									
Average Roll Area (150 ft x 15.5 ft):			2,325	ft ²	SHEET NO. 6		of	11	
No. of Rolls in Lot:			78		CUMULATIVE NUMBER OF ROLLS: 465				
Area in Lot:			181,350	ft ²	CUMULATIVE AREA: 1,081,125		ft ²		

Table 5-1 (continued)

CQA AND MQC TEST RESULTS FOR GEOSYNTHETIC CLAY LINER (BENTOFIX)
LOT NO. 23100909

PROPERTY	CONSTRUCTION QUALITY ASSURANCE (CQA)	MANUFACTURING QUALITY CONTROL (MQC)				
		Bentonite Content (lb/ft ²)	Bentonite Moisture Content ¹ (%)	Bentonite Free Swell ² (ml)	Peel Strength (lb)	Hydraulic Conductivity (cm/s)
TEST STANDARD	ASTM D 5887	ASTM D 5993	ASTM D 4643	ASTM D 5890	ASTM D 4632	ASTM D 5887
PROJECT SPECS.	$\leq 5.0 \times 10^{-9}$	≥ 0.75	≤ 25	≥ 24	≥ 15	$\leq 5.0 \times 10^{-9}$
TESTING FREQUENCY	1 per 200,000 ft ² (3)	1 per 40,000 ft ² (3)				
		1 per 100,000 ft ² (3)				

ROLL NUMBER	CQA SAMPLE ID	TEST RESULTS	TEST RESULTS				PASS/FAIL (P/F)	
							CQA	MQC
87619	GCL-9	2.0×10^{-9}	0.92	9	37	44	P	P
87635			0.91	9	37	42		P
87651			1.06	9	37	43		P
87667			0.92	9	37	47		P
87683			0.96	9	37	39		P
87690			0.96	9	37	39		P
Notes: 1 Four tests were performed per railcar (approximately 200,000 lbs). Reported value represents maximum bentonite moisture content for the 4 tests. 2 Four tests were performed per railcar (approximately 200,000 lbs). Reported value represents minimum bentonite free swell for the 4 tests. 3 A minimum of 1 test per lot was required.								
Average Roll Area (150 ft x 15.5 ft):			2,325	ft ²	SHEET NO. 7		of 11	
No. of Rolls in Lot:			77		CUMULATIVE NUMBER OF ROLLS: 542			
Area in Lot:			179,025	ft ²	CUMULATIVE AREA: 1,260,150		ft ²	

Table 5-1 (continued)

**CQA AND MQC TEST RESULTS FOR GEOSYNTHETIC CLAY LINER (BENTOFIX)
LOT NO. 23101009**

PROPERTY	CONSTRUCTION QUALITY ASSURANCE (CQA)	MANUFACTURING QUALITY CONTROL (MQC)				
		Bentonite Content (lb/ft ²)	Bentonite Moisture Content ¹ (%)	Bentonite Free Swell ² (ml)	Peel Strength (lb)	Hydraulic Conductivity (cm/s)
TEST STANDARD	Hydraulic Conductivity (cm/s)	ASTM D 5887	ASTM D 4643	ASTM D 5890	ASTM D 4632	ASTM D 5887
PROJECT SPECS.	$\leq 5.0 \times 10^{-9}$	≥ 0.75	≤ 25	≥ 24	≥ 15	$\leq 5.0 \times 10^{-9}$
TESTING FREQUENCY	1 per 200,000 ft ² (3)	1 per 40,000 ft ² (3)				
		1 per 100,000 ft ² (3)				

ROLL NUMBER	CQA SAMPLE ID	TEST RESULTS	TEST RESULTS				PASS/FAIL (P/F)	
							CQA	MQC
87696			0.90	8	26	47		P
87712			0.96	8	26	46		P
87713			0.96	8	26	46		P
87719	GCL-10	1.9×10^{-9}	0.92	8	26	48	P	P
87720			0.93	8	26	48		P
87732			0.92	8	26	48		P
87736			0.98	8	26	38		P
87752			0.92	8	26	50		P
87768			1.01	8	26	46		P
Notes: 1 Four tests were performed per railcar (approximately 200,000 lbs). Reported value represents maximum bentonite moisture content for the 4 tests. 2 Four tests were performed per railcar (approximately 200,000 lbs). Reported value represents minimum bentonite free swell for the 4 tests. 3 A minimum of 1 test per lot was required.								
Average Roll Area (150 ft x 15.5 ft):			2,325	ft ²	SHEET NO. 8		of 11	
No. of Rolls in Lot:			77		CUMULATIVE NUMBER OF ROLLS: 619			
Area in Lot:			179,025	ft ²	CUMULATIVE AREA: 1,439,175		ft ²	

Table 5-1 (continued)

CQA AND MQC TEST RESULTS FOR GEOSYNTHETIC CLAY LINER (BENTOFIX)
LOT NO. 23101409

PROPERTY	CONSTRUCTION QUALITY ASSURANCE (CQA)	MANUFACTURING QUALITY CONTROL (MQC)				
		Hydraulic Conductivity (cm/s)	Bentonite Content (lb/ft ²)	Bentonite Moisture Content ¹ (%)	Bentonite Free Swell ² (ml)	Peel Strength (lb)
TEST STANDARD	ASTM D 5887		ASTM D 5993	ASTM D 4643	ASTM D 5890	ASTM D 4632
PROJECT SPECS.	$\leq 5.0 \times 10^{-9}$		≥ 0.75	≤ 25	≥ 24	≥ 15
TESTING FREQUENCY	1 per 200,000 ft ² (3)			1 per 40,000 ft ² (3)		1 per 100,000 ft ² (3)

ROLL NUMBER	CQA SAMPLE ID	TEST RESULTS	TEST RESULTS				PASS/FAIL (P/F)	
							CQA	MQC
87773	GCL-11	1.7×10^{-9}	0.91	10	35	45	P	P
87789			0.94	10	35	50		P
87800			0.94	10	35	50		P
87805			0.93	10	35	45		P
87821			0.98	10	35	48		P
87835			0.97	10	35	48		P
87837			0.93	10	35	42		P
Notes: ¹ Four tests were performed per railcar (approximately 200,000 lbs). Reported value represents maximum bentonite moisture content for the 4 tests. ² Four tests were performed per railcar (approximately 200,000 lbs). Reported value represents minimum bentonite free swell for the 4 tests. ³ A minimum of 1 test per lot was required.								
Average Roll Area (150 ft x 15.5 ft):			2,325	ft ²	SHEET NO. 9		of 11	
No. of Rolls in Lot:			77		CUMULATIVE NUMBER OF ROLLS: 773			
Area in Lot:			179,025	ft ²	CUMULATIVE AREA: 1,797,225		ft ²	

Table 5-1 (continued)

**CQA AND MQC TEST RESULTS FOR GEOSYNTHETIC CLAY LINER (BENTOFIX)
LOT NO. 23101509**

PROPERTY	CONSTRUCTION QUALITY ASSURANCE (CQA)	MANUFACTURING QUALITY CONTROL (MQC)					
		Hydraulic Conductivity (cm/s)	Bentonite Content (lb/ft ²)	Bentonite Moisture Content ¹ (%)	Bentonite Free Swell ² (ml)	Peel Strength (lb)	Hydraulic Conductivity (cm/s)
TEST STANDARD	ASTM D 5887	ASTM D 5887	ASTM D 5993	ASTM D 4643	ASTM D 5890	ASTM D 4632	ASTM D 5887
PROJECT SPECS.	$\leq 5.0 \times 10^{-9}$	≥ 0.75	≤ 25	≥ 24	≥ 15	$\leq 5.0 \times 10^{-9}$	
TESTING FREQUENCY	1 per 200,000 ft ² (3)	1 per 40,000 ft ² (3)					1 per 100,000 ft ² (3)

ROLL NUMBER	CQA SAMPLE ID	TEST RESULTS	TEST RESULTS				PASS/FAIL (P/F)	
							CQA	MQC
87850			0.92	9	33	57		P
87858	GCL-12	2.3×10^{-9}	0.87	9	33	51	P	P
87876			0.96	9	33	41		P
87890			0.94	9	33	41		P
87906			0.96	9	33	44		P
87922			0.99	9	33	46		P

Notes:

¹ Four tests were performed per railcar (approximately 200,000 lbs). Reported value represents maximum bentonite moisture content for the 4 tests.

² Four tests were performed per railcar (approximately 200,000 lbs). Reported value represents minimum bentonite free swell for the 4 tests.

³ A minimum of 1 test per lot was required.

Average Roll Area (150 ft x 15.5 ft):	2,325	ft ²	SHEET NO.	10	of	11
No. of Rolls in Lot:	78		CUMULATIVE NUMBER OF ROLLS:	774		
Area in Lot:	181,350	ft ²	CUMULATIVE AREA:	1,799,550	ft ²	

Table 5-1 (continued)

CQA AND MQC TEST RESULTS FOR GEOSYNTHETIC CLAY LINER (BENTOFIX)
LOT NO. 23101609

PROPERTY	CONSTRUCTION QUALITY ASSURANCE (CQA)	MANUFACTURING QUALITY CONTROL (MQC)				
		Bentonite Content (lb/ft ²)	Bentonite Moisture Content ¹ (%)	Bentonite Free Swell ² (ml)	Peel Strength (lb)	Hydraulic Conductivity (cm/s)
TEST STANDARD	ASTM D 5887	ASTM D 5993	ASTM D 4643	ASTM D 5890	ASTM D 4632	ASTM D 5887
PROJECT SPECS.	$\leq 5.0 \times 10^{-9}$	≥ 0.75	≤ 25	≥ 24	≥ 15	$\leq 5.0 \times 10^{-9}$
TESTING FREQUENCY	1 per 200,000 ft ² (3)	1 per 40,000 ft ² (3)				
		1 per 100,000 ft ² (3)				

ROLL NUMBER	CQA SAMPLE ID	TEST RESULTS	TEST RESULTS				PASS/FAIL (P/F)	
							CQA	MQC
87928	GCL-13	8.8×10^{-10}	0.98	9	34	41	P	P
87944			0.92	9	34	53		P
Notes: 1 Four tests were performed per railcar (approximately 200,000 lbs). Reported value represents maximum bentonite moisture content for the 4 tests. 2 Four tests were performed per railcar (approximately 200,000 lbs). Reported value represents minimum bentonite free swell for the 4 tests. 3 A minimum of 1 test per lot was required.								
Average Roll Area (150 ft x 15.5 ft):			2,325	ft ²	11	of	11	
No. of Rolls in Lot:			26		800			
Area in Lot:			60,450	ft ²	1,860,000			
			CUMULATIVE AREA:		1,860,000	ft ²		

Table 5-2

**CQA AND MQC TEST RESULTS FOR 60-mil TEXTURED GEOMEMBRANE (GSE)
RESIN LOT NO. 8230906**

CONSTRUCTION QUALITY ASSURANCE (CQA)										MANUFACTURING QUALITY CONTROL (MQC)									
PROPERTY	Thickness (mil)	Density (g/cm ³)	Carbon Black Content (%)	Carbon Black Dispersion	Yield Strength ² (lb/in)	Break Strength ² (lb/in)	Yield Elongation ² (%)	Break Elongation ² (%)	Thickness ¹ (mil)	Density (g/cm ³)	Carbon Black Content (%)	Carbon Black Dispersion	Yield Strength ² (lb/in)	Break Strength ² (lb/in)	Yield Elongation ² (%)	Break Elongation ² (%)	Tear Resistance ² (lb)	Puncture Resistance (lb)	
TEST STANDARD	ASTM D 5994	ASTM D 1505	ASTM D 1603	ASTM D 5596	ASTM D 6693	ASTM D 6693	ASTM D 6693	ASTM D 6693	ASTM D 5994	ASTM D 1505	ASTM D 1603	ASTM D 5596	ASTM D 6693	ASTM D 6693	ASTM D 6693	ASTM D 6693	ASTM D 1004	ASTM D 4833	
PROJECT SPECS.	≥ 60	≥ 0.94	2 to 3	See Note 3	≥ 130	≥ 72	≥ 12	≥ 100	≥ 60	≥ 0.94	2 to 3	See Note 3	≥ 130	≥ 72	≥ 12	≥ 100	≥ 40	≥ 80	
TESTING FREQUENCY	1 per 100,000 ft ² (4)										1 per 50,000 ft ² (4)								

ROLL NUMBER	CQA SAMPLE ID	TEST RESULTS										TEST RESULTS										PASS/FAIL (PIF)	
		62	0.946	2.58	10	181	205	17	534	61	0.946	2.6	10	167	177	16	399	62	156	P	P	COA	MQC
108101974 ⁵	GMT-1																						
108101976																							
108101980																							
108101984	GMT-2	68	0.945	2.46	10	180	191	17	430	61	0.946	2.7	10	160	187	15	414	61	153	P	P		
108101988																							
108101992																							
108101993																							
108101994	GMT-3	69	0.944	2.54	10	180	170	17	375	61	0.945	2.6	10	153	143	17	389	57	149	P	P		
108101997																							

Notes:

- Thickness was measured for every roll.
- Smaller of machine direction (MD) and transverse direction (TD).
- Project specifications for carbon black dispersion are: 8 of 10 in Category 1 or 2 and all in Category 1, 2, or 3. Results are for Category 1 or 2.
- A minimum of 1 test per lot was required.
- This roll was used to perform interface friction tests.

Average Roll Area (460 ft x 23 ft):	10,580	ft ²	SHEET NO.	1	OF	7
No. of Rolls in Lot:	25		CUMULATIVE NUMBER OF ROLLS:	25		
Area in Lot:	264,500	ft ²	CUMULATIVE AREA:	264,500	ft ²	

Table 5-2 (continued)

CQA AND MQC TEST RESULTS FOR 60-mil TEXTURED GEOMEMBRANE (GSE)
RESIN LOT NO. 8230879

	CONSTRUCTION QUALITY ASSURANCE (CQA)								MANUFACTURING QUALITY CONTROL (MQC)									
PROPERTY	Thickness (mil)	Density (g/cm ³)	Carbon Black Content (%)	Carbon Black Dispersion	Yield Strength ² (lb/in)	Break Strength ² (lb/in)	Yield Elongation ² (%)	Break Elongation ² (%)	Thickness ¹ (mil)	Density (g/cm ³)	Carbon Black Content (%)	Carbon Black Dispersion	Yield Strength ² (lb/in)	Break Strength ² (lb/in)	Yield Elongation ² (%)	Break Elongation ² (%)	Tear Resistance ² (lb)	Puncture Resistance (lb)
TEST STANDARD	ASTM D 5994	ASTM D 1505	ASTM D 1603	ASTM D 5596	ASTM D 6693	ASTM D 6693	ASTM D 6693	ASTM D 6693	ASTM D 5994	ASTM D 1505	ASTM D 1603	ASTM D 5596	ASTM D 6693	ASTM D 6693	ASTM D 6693	ASTM D 6693	ASTM D 1004	ASTM D 4833
PROJECT SPECS.	≥ 60	≥ 0.94	2 to 3	See Note 3	≥ 130	≥ 72	≥ 12	≥ 100	≥ 60	≥ 0.94	2 to 3	See Note 3	≥ 130	≥ 72	≥ 12	≥ 100	≥ 40	≥ 80
TESTING FREQUENCY	1 per 100,000 ft ² (4)								1 per 50,000 ft ² (4)									

ROLL NUMBER	CQA SAMPLE ID	TEST RESULTS										TEST RESULTS										PASS/FAIL (P/F)	
																						CQA	MQC
108102147	GMT-4	68	0.945	2.6	10	173	181	15	429	61	0.946	2.5	10	166	201	16	419	59	155	P	P		
108102151										61	0.947	2.5	10	167	160	16	379	58	156		P		
108102155										61	0.945	2.7	10	171	176	15	449	57	151		P		
108102156	GMT-5	64	0.947	2.66	10	172	207	15	556	60										P	P		

Notes:

1 Thickness was measured for every roll.

2 Smaller of machine direction (MD) and transverse direction (TD).

3 Project specifications for carbon black dispersion are: 8 of 10 in Category 1 or 2 and all in Category 1,2, or 3. Results are for Category 1 or 2.

4 A minimum of 1 test per lot was required.

Average Roll Area (460 ft x 23 ft):	10,580	ft ²	SHEET NO.	2	OF	7
No. of Rolls in Lot:	10		CUMULATIVE NUMBER OF ROLLS:	35		
Area in Lot:	105,800	ft ²	CUMULATIVE AREA:	370,300	ft ²	

Notes:
1 Thickness was measured for every roll.
2 Smaller of machine direction (MD) and transverse direction (TD).
3 Project specifications for carbon black dispersion are: 8 of 10 in Category 1 or 2 and all in Category 1, 2, or 3. Results are for Category 1 or 2.
4 A minimum of 1 test per lot was required.

Average Roll Area (460 ft x 23 ft):	10,580	ft ²	SHEET NO.	2	OF	7	
No. of Rolls in Lot:	10		CUMULATIVE NUMBER OF ROLLS:				35
Area in Lot:	105,800	ft ²	CUMULATIVE AREA:				370,300
							ft ²

Table 5-2 (continued)

CQA AND MQC TEST RESULTS FOR 60-mil TEXTURED GEOMEMBRANE (GSE)
RESIN LOT NO. 8230884

PROPERTY	CONSTRUCTION QUALITY ASSURANCE (CQA)							MANUFACTURING QUALITY CONTROL (MQC)						
	Thickness (mil)	Density (g/cm ³)	Carbon Black Content (%)	Carbon Black Dispersion	Yield Strength ² (lb/in)	Break Strength ² (lb/in)	Yield Elongation ² (%)	Break Elongation ² (%)	Yield Elongation ² (%)	Break Strength ² (lb/in)	Yield Strength ² (lb/in)	Carbon Black Dispersion	Carbon Black Content (%)	Carbon Black Dispersion
TEST STANDARD	ASTM D 5994	ASTM D 1505	ASTM D 1603	ASTM D 5596	ASTM D 6693	ASTM D 6693	ASTM D 6693	ASTM D 6693	ASTM D 6693	ASTM D 6693	ASTM D 6693	ASTM D 5596	ASTM D 1603	ASTM D 5596
PROJECT SPECS.	≥ 60	≥ 0.94	2 to 3	See Note 3	≥ 130	≥ 72	≥ 12	≥ 100	≥ 12	≥ 72	≥ 130	See Note 3	2 to 3	See Note 3
TESTING FREQUENCY	1 per 100,000 ft ² (4)							1 per 50,000 ft ² (4)						

ROLL NUMBER	COA SAMPLE ID	TEST RESULTS										TEST RESULTS										PASS/FAIL (P/F)			
																						CQA	MQC		
108102159															60	0.944	2.6	10	163	182	14	380	62	155	P
108102164															61	0.945	2.6	10	168	192	16	458	60	150	P
108102168	GMT-6	64	0.947	2.58	10	174	191	22	489						61	0.944	2.5	10	166	189	16	410	62	157	P
108102172															62	0.946	2.6	10	174	199	15	376	62	162	P
108102177															61	0.948	2.6	10	167	185	15	458	62	135	P
108102181															61	0.947	2.4	10	169	191	16	448	60	130	P
108102185															61	0.946	2.5	10	167	216	16	462	62	161	P
108102186	GMT-7	64	0.946	2.66	10	179	201	17	484						61										P
108102189															61	0.944	2.5	10	172	217	16	462	60	160	P
108102195															61	0.945	2.5	10	162	184	16	464	55	157	P
108102197	GMT-8	65	0.946	2.71	10	178	192	15	502						61	0.946	2.6	10	159	196	17	479	55	157	P
108102198															61	0.946	2.6	10	159	196	17	479	54	152	P

Notes:

1 Thickness was measured for every roll.

2 Smaller of machine direction (MD) and transverse direction (TD).

3 Project specifications for carbon black dispersion are: 8 of 10 in Category 1 or 2 and all in Category 1, 2, or 3. Results are for Category 1 or 2.

4 A minimum of 1 test per lot was required.

Table 5-2 (continued)

**CQA AND MQC TEST RESULTS FOR 60-mil TEXTURED GEOMEMBRANE (GSE)
RESIN LOT NO. 8230884**

CONSTRUCTION QUALITY ASSURANCE (CQA)										MANUFACTURING QUALITY CONTROL (MQC)									
PROPERTY	Thickness (mil)	Density (g/cm ³)	Carbon Black Content (%)	Carbon Black Dispersion	Yield Strength ² (lb/in)	Break Strength ² (lb/in)	Yield Elongation ² (%)	Break Elongation ² (%)	Thickness ¹ (mil)	Density (g/cm ³)	Carbon Black Content (%)	Carbon Black Dispersion	Yield Strength ² (lb/in)	Break Strength ² (lb/in)	Yield Elongation ² (%)	Break Elongation ² (%)	Tear Resistance ² (lb)	Puncture Resistance (lb)	
TEST STANDARD	ASTM D 5994	ASTM D 1505	ASTM D 1603	ASTM D 5596	ASTM D 6693	ASTM D 6693	ASTM D 6693	ASTM D 6693	ASTM D 5994	ASTM D 1505	ASTM D 1603	ASTM D 5596	ASTM D 6693	ASTM D 6693	ASTM D 6693	ASTM D 6693	ASTM D 1004	ASTM D 4833	
PROJECT SPECS.	≥ 60	≥ 0.94	2 to 3	See Note 3	≥ 130	≥ 72	≥ 12	≥ 100	≥ 60	≥ 0.94	2 to 3	See Note 3	≥ 130	≥ 72	≥ 12	≥ 100	≥ 40	≥ 80	
TESTING FREQUENCY	1 per 100,000 ft ² (4)										1 per 50,000 ft ² (4)								

ROLL NUMBER	CQA SAMPLE ID	TEST RESULTS										TEST RESULTS						PASS/FAIL (P/F)	
		Thickness ¹ (mil)	Density (g/cm ³)	Carbon Black Content (%)	Carbon Black Dispersion	Yield Strength ² (lb/in)	Break Strength ² (lb/in)	Yield Elongation ² (%)	Break Elongation ² (%)	Thickness ¹ (mil)	Density (g/cm ³)	Carbon Black Content (%)	Carbon Black Dispersion	Yield Strength ² (lb/in)	Break Strength ² (lb/in)	Yield Elongation ² (%)	Break Elongation ² (%)	Tear Resistance ² (lb)	Puncture Resistance (lb)
108102201										62	0.947	2.4	10	168	175	17	406	57	157
108102203										61	0.947	2.4	10	165	184	16	417	61	159
108102206	GMT-9	68	0.947	2.51	10	173	202	16	500	61	0.947	2.4	10	168	175	17	406	57	157
108102210										61	0.946	2.6	10	162	155	17	480	55	154
108102211										61	0.946	2.6	10	164	204	18	480	55	154
108102213										61	0.946	2.6	10	162	164	18	476	55	154
108102214										61	0.947	2.5	10	166	191	17	441	61	153
108102215	GMT-10	67	0.945	2.68	10	180	214	13	559	61									P
108102218										61	0.948	2.5	10	164	185	16	430	57	156

Notes:

1 Thickness was measured for every roll.

2 Smaller of machine direction (MD) and transverse direction (TD).

3 Project specifications for carbon black dispersion are: 8 of 10 in Category 1 or 2 and all in Category 1, 2, or 3. Results are for Category 1 or 2.

4 A minimum of 1 test per lot was required.

Average Roll Area (460 ft x 23 ft): 10,580 ft²

No. of Rolls in Lot: 58

Area in Lot: 613,640 ft²

SHEET NO. 3 OF 7

CUMULATIVE NUMBER OF ROLLS: 93

CUMULATIVE AREA: 983,940 ft²

LOT SHEET 2 OF 2

Table 5-2 (continued)

CQA AND MQC TEST RESULTS FOR 60-mil TEXTURED GEOMEMBRANE (GSE)
RESIN LOT NO. 8230913

CONSTRUCTION QUALITY ASSURANCE (CQA)										MANUFACTURING QUALITY CONTROL (MQC)									
PROPERTY	Thickness (mil)	Density (g/cm ³)	Carbon Black Content (%)	Carbon Black Dispersion	Yield Strength ² (lb/in)	Break Strength ² (lb/in)	Yield Elongation ² (%)	Break Elongation ² (%)	Thickness ¹ (mil)	Density (g/cm ³)	Carbon Black Content (%)	Carbon Black Dispersion	Yield Strength ² (lb/in)	Break Strength ² (lb/in)	Yield Elongation ² (%)	Break Elongation ² (%)	Tear Resistance ² (lb)	Puncture Resistance (lb)	
TEST STANDARD	ASTM D 5994	ASTM D 1505	ASTM D 1603	ASTM D 5596	ASTM D 6693	ASTM D 6693	ASTM D 6693	ASTM D 6693	ASTM D 5994	ASTM D 1505	ASTM D 1603	ASTM D 5596	ASTM D 6693	ASTM D 6693	ASTM D 6693	ASTM D 6693	ASTM D 1004	ASTM D 4833	
PROJECT SPECS.	≥ 60	≥ 0.94	2 to 3	See Note 3	≥ 130	≥ 72	≥ 12	≥ 100	≥ 60	≥ 0.94	2 to 3	See Note 3	≥ 130	≥ 72	≥ 12	≥ 100	≥ 40	≥ 80	
TESTING FREQUENCY	1 per 100,000 ft ² (4)										1 per 50,000 ft ² (4)								

ROLL NUMBER	CQA SAMPLE ID	TEST RESULTS										TEST RESULTS										PASS/FAIL (P/F)	
		Thickness ¹ (mil)	Density (g/cm ³)	Carbon Black Content (%)	Carbon Black Dispersion	Yield Strength ² (lb/in)	Break Strength ² (lb/in)	Yield Elongation ² (%)	Break Elongation ² (%)	Thickness ¹ (mil)	Density (g/cm ³)	Carbon Black Content (%)	Carbon Black Dispersion	Yield Strength ² (lb/in)	Break Strength ² (lb/in)	Yield Elongation ² (%)	Break Elongation ² (%)	Tear Resistance ² (lb)	Puncture Resistance (lb)	CQA	MQC	P	P
108102222										61	0.949	2.5	10	168	196	16	445	57	152				
108102229	GMT-11	67	0.945	2.63	10	184	179	15	424	61	0.946	2.6	9	171	174	15	250	59	160			P	P
108102230										61	0.946	2.6	9	162	199	17	473	59	153			P	P
108102233										61	0.945	2.7	10	170	206	17	411	62	156			P	P
108102237										60	0.944	2.6	10	170	211	16	454	62	162			P	P
108102238	GMT-12	66	0.946	2.47	10	181	211	18	459	60												P	P

- Notes:
- Thickness was measured for every roll.
 - Smaller of machine direction (MD) and transverse direction (TD).
 - Project specifications for carbon black dispersion are: 8 of 10 in Category 1 or 2 and all in Category 1, 2, or 3. Results are for Category 1 or 2.
 - A minimum of 1 test per lot was required.

Average Roll Area (460 ft x 23 ft):	10,580	ft ²	SHEET NO.	4	OF	7	
No. of Rolls in Lot:	13		CUMULATIVE NUMBER OF ROLLS:				106
Area in Lot:	137,540	ft ²	CUMULATIVE AREA:				1,121,480
							ft ²

Table 5-2 (continued)

CQA AND MQC TEST RESULTS FOR 60-mil TEXTURED GEOMEMBRANE (GSE)
RESIN LOT NO. 8230912

CONSTRUCTION QUALITY ASSURANCE (CQA)										MANUFACTURING QUALITY CONTROL (MQC)							
PROPERTY	Thickness (mil)	Density (g/cm ³)	Carbon Black Content (%)	Yield Strength ² (lb/in)	Break Strength ² (lb/in)	Yield Elongation ² (%)	Break Elongation ² (%)	Thickness ¹ (mil)	Density (g/cm ³)	Carbon Black Content (%)	Carbon Black Dispersion	Yield Strength ² (lb/in)	Break Strength ² (lb/in)	Yield Elongation ² (%)	Break Elongation ² (%)	Tear Resistance ² (lb)	Puncture Resistance (lb)
TEST STANDARD	ASTM D 5994	ASTM D 1505	ASTM D 1603	ASTM D 6693	ASTM D 6693	ASTM D 6693	ASTM D 6693	ASTM D 5994	ASTM D 1505	ASTM D 1603	ASTM D 5596	ASTM D 6693	ASTM D 6693	ASTM D 6693	ASTM D 6693	ASTM D 1004	ASTM D 4833
PROJECT SPECS.	≥ 60	≥ 0.94	2 to 3	See Note 3	≥ 72	≥ 12	≥ 100	≥ 60	≥ 0.94	2 to 3	See Note 3	≥ 130	≥ 72	≥ 12	≥ 100	≥ 40	≥ 80
TESTING FREQUENCY	1 per 100,000 ft ² (4)										1 per 50,000 ft ² (4)						

ROLL NUMBER	CQA SAMPLE ID	TEST RESULTS										TEST RESULTS										PASS/FAIL (P/F)	
												61	0.944	2.5	10	172	208	15	466	61	158	CQA	MQC
108102241											61	0.944	2.5	10	172	208	15	466	61	158		P	
108102245											61	0.946	2.7	10	154	204	17	486	58	145		P	
108102247	GMT-13	66	0.947	2.62	10	180	208	17	559		61										P		
108102249											61	0.945	2.5	10	153	191	17	514	57	145		P	
108102254											61	0.94	2.8	10	149	187	18	509	55	142		P	
108102257	GMT-14	66	0.947	2.62	10	175	213	15	503		61										P		
108102258											62	0.943	2.3	10	162	210	17	456	57	150		P	
108102262											61	0.946	2.7	9	161	200	17	447	62	149		P	
108102266	GMT-15	65	0.946	2.82	10	184	206	17	540		61	0.946	2.5	10	165	216	16	471	60	157	P	P	
108102270											61	0.944	2.7	10	161	175	16	438	61	163		P	
108102274											61	0.948	2.4	10	170	205	16	454	64	164		P	
108102275	GMT-16	68	0.947	2.61	10	180	214	17	568		63										P		

Notes:

- Thickness was measured for every roll.
- Smaller of machine direction (MD) and transverse direction (TD).
- Project specifications for carbon black dispersion are: 8 of 10 in Category 1 or 2 and all in Category 1, 2, or 3. Results are for Category 1 or 2.
- A minimum of 1 test per lot was required.

Average Roll Area (460 ft x 23 ft):	10,580	ft ²	SHEET NO.	5	OF	7
No. of Rolls in Lot:	34		CUMULATIVE NUMBER OF ROLLS:	140		
Area in Lot:	359,720	ft ²	CUMULATIVE AREA:	1,481,200	ft ²	

Table 5-2 (continued)

**CQA AND MQC TEST RESULTS FOR 60-mil TEXTURED GEOMEMBRANE (GSE)
RESIN LOT NO. 8230914**

PROPERTY	CONSTRUCTION QUALITY ASSURANCE (CQA)							MANUFACTURING QUALITY CONTROL (MQC)						
	Thickness (mil)	Density (g/cm ³)	Carbon Black Content (%)	Carbon Black Dispersion	Yield Strength ² (lb/in)	Break Strength ² (lb/in)	Yield Elongation ² (%)	Break Elongation ² (%)	Yield Elongation ² (%)	Break Strength ² (lb/in)	Carbon Black Dispersion	Yield Strength ² (lb/in)	Break Strength ² (lb/in)	Puncture Resistance (lb)
TEST STANDARD	ASTM D 5994	ASTM D 1505	ASTM D 1603	ASTM D 5596	ASTM D 6693	ASTM D 6693	ASTM D 6693	ASTM D 6693	ASTM D 6693	ASTM D 6693	ASTM D 5596	ASTM D 6693	ASTM D 6693	ASTM D 4893
PROJECT SPECS.	≥ 60	≥ 0.94	2 to 3	See Note 3	≥ 130	≥ 72	≥ 12	≥ 100	≥ 60	≥ 0.94	2 to 3	≥ 130	≥ 72	≥ 40
TESTING FREQUENCY	1 per 100,000 ft ² (4)							1 per 50,000 ft ² (4)						

ROLL NUMBER	CQA SAMPLE ID	TEST RESULTS													PASS/FAIL (P/F)	
		Thickness (mil)	Density (g/cm ³)	Carbon Black Content (%)	Carbon Black Dispersion	Yield Strength ² (lb/in)	Break Strength ² (lb/in)	Yield Elongation ² (%)	Break Elongation ² (%)	Yield Strength ² (lb/in)	Carbon Black Dispersion	Yield Strength ² (lb/in)	Break Strength ² (lb/in)	Yield Elongation ² (%)	Break Elongation ² (%)	Puncture Resistance (lb)
108102280																158
108102283																157
108102286	GM T-17	67	0.946	2.49	10	182	222	15	581							
108102287																
108102291																
108102295	GMT-18	67	0.945	2.58	10	181	219	17	576							
108102298																
108102302																
108102304	GMT-19	67	0.946	2.6	10	184	210	15	544							
108102306																
108102310																
108102313	GMT-20	66	0.946	2.51	10	176	217	16	557							
108102314																

Notes:

1 Thickness was measured for every roll.

2 Smaller of machine direction (MD) and transverse direction (TD).

3 Project specifications for carbon black dispersion are: 8 of 10 in Category 1 or 2 and all in Category 1, 2, or 3. Results are for Category 1 or 2.

4 A minimum of 1 test per lot was required.

Average Roll Area (480 ft x 23 ft): 10,580 ft²
 No. of Rolls in Lot: 40
 Area in Lot: 423,200 ft²

SHEET NO. 6 OF 7
 CUMULATIVE NUMBER OF ROLLS: 180
 CUMULATIVE AREA: 1,904,400 ft²

Table 5-2 (continued)

CQA AND MQC TEST RESULTS FOR 60-mil TEXTURED GEOMEMBRANE (GSE)
RESIN LOT NO. 8230916

	CONSTRUCTION QUALITY ASSURANCE (CQA)										MANUFACTURING QUALITY CONTROL (MQC)							
PROPERTY	Thickness (mil)	Density (g/cm ³)	Carbon Black Content (%)	Carbon Black Dispersion	Yield Strength ² (lb/in)	Break Strength ² (lb/in)	Yield Elongation ² (%)	Break Elongation ² (%)	Thickness ¹ (mil)	Density (g/cm ³)	Carbon Black Content (%)	Carbon Black Dispersion	Yield Strength ² (lb/in)	Break Strength ² (lb/in)	Yield Elongation ² (%)	Break Elongation ² (%)	Tear Resistance ² (lb)	Puncture Resistance (lb)
TEST STANDARD	ASTM D 5994	ASTM D 1505	ASTM D 1603	ASTM D 5596	ASTM D 6693	ASTM D 6693	ASTM D 6693	ASTM D 6693	ASTM D 5994	ASTM D 1505	ASTM D 1603	ASTM D 5596	ASTM D 6693	ASTM D 6693	ASTM D 6693	ASTM D 6693	ASTM D 1004	ASTM D 4833
PROJECT SPECS.	≥ 60	≥ 0.94	2 to 3	See Note 3	≥ 130	≥ 72	≥ 12	≥ 100	≥ 60	≥ 0.94	2 to 3	See Note 3	≥ 130	≥ 72	≥ 12	≥ 100	≥ 40	≥ 80
TESTING FREQUENCY	1 per 100,000 ft ² (4)										1 per 50,000 ft ² (4)							

ROLL NUMBER	CQA SAMPLE ID	TEST RESULTS										TEST RESULTS										PASS/FAIL (P/F)	
																			CQA	MQC			
108102318										60	0.947	2.7	10	166	182	16	446	61	160	P			
108102322	GMT-21	70	0.946	2.56	10	182	201	19	539	61	0.948	2.6	10	167	166	15	430	60	163	P			
108102326										60	0.947	2.5	10	165	185	16	451	63	163	P			
108102333										62	0.946	2.3	10	169	203	17	462	62	155	P			
Notes:																							
1 Thickness was measured for every roll.																							
2 Smaller of machine direction (MD) and transverse direction (TD).																							
3 Project specifications for carbon black dispersion are: 8 of 10 in Category 1 or 2 and all in Category 1, 2, or 3. Results are for Category 1 or 2.																							
4 A minimum of 1 test per lot was required.																							

Table 5-3A

**NONDESTRUCTIVE SEAM TEST RESULTS FOR PRIMARY LINER
INSTALLED IN CELL 1A**

Panel No.	Seam Length (ft)	Vacuum Box ¹ Pass/Fail (P/F)	Air Pressure		
			Applied Pressure (psi)	Loss of Pressure ² (psi)	Pass/Fail (P/F)
P1-P2	453	-	34	0	P
P1-P27	23	-	35	0	P
P2-P3	455	-	34	0	P
P2-P27	22	-	35	0	P
P3-P4	458	-	33	0	P
P3-P27	22	-	35	0	P
P4-P5	456	-	34	0	P
P4-P27	23	-	35	0	P
P5-P6	455	-	34	0	P
P5-P26	17	-	34	0	P
P5-P27	6	-	33	0	P
P6-P7	457	-	34	0	P
P6-P26	22	-	35	0	P
P7-P8	457	-	34	0	P
P7-P26	22	-	35	0	P
P8-P9	460	-	34	0	P
P8-P26	23	-	35	0	P
P9-P10	458	-	34	0	P
P9-P26	22	-	35	0	P
P10-P11	379	-	30	0	P
P10-P11A	76	-	30	0	P
P10-P26	22	-	35	0	P
P11-P11A	23	-	30	0	P
P11-P12	379	-	30	0	P
P11A-P12	76	-	30	0	P
P11A-P26	22	-	33	0	P
P12-P13	458	-	30	0	P
P12-P26	24	-	34	0	P
P13-P14	458	-	30	0	P
P13-P26	22	-	35	0	P
P14-P15	458	-	30	0	P
P14-P26	22	-	35	0	P
P15-P16	460	-	30	0	P
P15-P26	23	-	34	0	P
P16-P17	458	-	30	0	P
P16-P26	22	-	35	0	P
P17-P18	455	-	30	1	P
P17-P26	22	-	34	1	P
P18-P19	455	-	30	0	P
P18-P26	23	-	34	1	P
P19-P20	455	-	30	1	P
P19-P26	22	-	34	1	P
P20-P21	455	-	30	0	P
P20-P26	22	-	34	0	P

Notes:

¹ Using 5 psi gauge suction in the box.² Acceptance criterion: Loss of pressure does not exceed 3 psi for at least 5 minutes.

Table 5-3A (continued)

**NONDESTRUCTIVE SEAM TEST RESULTS FOR PRIMARY LINER
INSTALLED IN CELL 1A**

Panel No.	Seam Length (ft)	Vacuum Box ¹ Pass/Fail (P/F)	Air Pressure		
			Applied Pressure (psi)	Loss of Pressure ² (psi)	Pass/Fail (P/F)
P21-P22	455	-	30	0	P
P21-P26	22	-	35	1	P
P22-P23	455	-	30	0	P
P22-P26	22	-	35	0	P
P23-P24	455	-	33	0	P
P23-P26	23	-	35	0	P
P24-P25	455	-	35	0	P
P24-P26	23	-	35	0	P
P25-P93	4	-	30	1	P
P26-P27	23	-	35	1	P
P26-P28	453	-	34	0	P
P26-P93	20	-	30	0	P
P27-P32	97	-	34	0	P
P28-P29	459	-	35	0	P
P28-P32	22	-	35	0	P
P28-P94	21	-	30	0	P
P29-P30	459	-	35	0	P
P29-P33	22	-	34	0	P
P29-P97	22	-	30	0	P
P30-P31	459	-	34	0	P
P30-P34	22	-	33	0	P
P30-P96	23	-	30	0	P
P31-P35	22	-	34	0	P
P31-P36	458	-	35	0	P
P31-P98	18	-	30	0	P
P32-P33	96	-	35	0	P
P33-P34	96	-	34	0	P
P34-P35	96	-	34	0	P
P35-P39	97	-	35	0	P
P36-P37	458	-	35	0	P
P36-P39	22	-	35	0	P
P37-P38	458	-	35	0	P
P37-P40	22	-	35	0	P
P38-P42 A	37	-	35	0	P
P38-P42 B	424	-	35	0	P
P39-P40	97	-	35	0	P
Cap8-P39	242	P	-	-	-
P41-P38	22	-	35	0	P
P41-P40	97	-	35	0	P
P41-P43	97	-	35	0	P
P42-P43	22	-	35	0	P
P44-P43	22	-	33	0	P
P44-P45 A	62	-	34	1	P
P44-P45 B	58	-	35	0	P

Notes:

¹ Using 5 psi gauge suction in the box.² Acceptance criterion: Loss of pressure does not exceed 3 psi for at least 5 minutes.

Table 5-3A (continued)

**NONDESTRUCTIVE SEAM TEST RESULTS FOR PRIMARY LINER
INSTALLED IN CELL 1A**

Panel No.	Seam Length (ft)	Vacuum Box ¹ Pass/Fail (P/F)	Air Pressure		
			Applied Pressure (psi)	Loss of Pressure ² (psi)	Pass/Fail (P/F)
P45-P43	22	-	33	0	P
P45-P46	58	-	34	0	P
P46-P43	22	-	33	0	P
P46-P47	58	-	36	0	P
P47-P43	22	-	33	0	P
P47-P48	60	-	36	1	P
P48-P42	12	-	35	0	P
P48-P43	10	-	33	0	P
P48-P49	60	-	35	0	P
P49-P42	22	-	35	0	P
P49-P50	60	-	35	0	P
P50-P42	23	-	35	0	P
P50-P51	60	-	40	2	P
P51-P42	24	-	35	0	P
P51-P52	60	-	35	0	P
P52-P42	21	-	35	0	P
P52-P53	60	-	35	0	P
P53-P42	22	-	35	0	P
P53-P54	60	-	34	0	P
P54-P42	21	-	35	0	P
P54-P55	60	-	34	2	P
P55-P42	24	-	35	0	P
P55-P56	60	-	35	0	P
Cap4-P55	16	P	-	-	-
Cap8-P55	16	P	-	-	-
P56-P42	23	-	35	0	P
P56-P57	60	-	33	0	P
Cap5-P56	16	P	-	-	-
Cap8-P56	21	P	-	-	-
P57-P42	22	-	35	0	P
P57-P58	60	-	34	2	P
Cap6-P57	16	P	-	-	-
Cap8-Cap6	2	P	-	-	-
Cap8-P57	23	P	-	-	-
P58-P42	23	-	35	0	P
P58-P59	60	-	35	1	P
Cap6-P58	16	P	-	-	-
Cap8-P58	21	P	-	-	-
P59-P42	22	-	35	0	P
P59-P60	60	-	35	0	P
Cap8-P59	22	P	-	-	-
P60-P42	23	-	35	0	P
P60-P61	60	-	36	2	P
Cap8-P60	23	P	-	-	-

Notes:

¹ Using 5 psi gauge suction in the box.² Acceptance criterion: Loss of pressure does not exceed 3 psi for at least 5 minutes.

Table 5-3A (continued)

**NONDESTRUCTIVE SEAM TEST RESULTS FOR PRIMARY LINER
INSTALLED IN CELL 1A**

Panel No.	Seam Length (ft)	Vacuum Box ¹ Pass/Fail (P/F)	Air Pressure		
			Applied Pressure (psi)	Loss of Pressure ² (psi)	Pass/Fail (P/F)
P61-P42	22	-	35	0	P
P61-P62	60	-	36	0	P
P61-Cap3	51	P	-	-	-
Cap8-P61 A	13	P	-	-	-
Cap8-P61 B	5	P	-	-	-
Cap8-Cap7	2	P	-	-	-
P62-P42	22	-	35	0	P
P62-P63	60	-	34	0	P
P62-Cap2	41	P	-	-	-
P62-Cap3	51	P	-	-	-
Cap8-P62 A	27	P	-	-	-
Cap8-P62 B	20	P	-	-	-
P63-P42	23	-	35	0	P
P63-P64	60	-	35	0	P
P63-Cap1	22	P	-	-	-
P63-Cap2	41	P	-	-	-
P64-P42	23	-	35	0	P
P64-P65	60	-	35	0	P
P64-Cap1	22	P	-	-	-
Cap8-P64	23	P	-	-	-
P65-P42	22	-	35	0	P
P65-P66	4	P	-	-	-
Cap8-P65	22	P	-	-	-
P68-P99	63	-	34	0	P
P69-P25	455	-	35	0	P
P69-P70	452	-	33	0	P
P69-P93	22	-	30	0	P
P69-RS134	25	P	-	-	P
P70-71	450	-	32	0	P
P70-P93	23	-	30	0	P
P70-RS134	13	P	-	-	-
P71-P72	310	-	36	0	P
P71-P93	22	-	30	0	P
P72-P89	22	-	30	0	P
P72-P90	22	-	30	0	P
P72-P91	22	-	30	0	P
P72-P92	22	-	30	0	P
P72-P93	22	-	30	0	P
P73-P71	22	-	32	0	P
P73-RS134	14	P	-	-	-
P74-P71	22	-	32	0	P
P74-P73	45	-	32	0	P
P75-P71	22	-	33	1	P
P75-P74	56	-	33	0	P

Notes:

¹ Using 5 psi gauge suction in the box.² Acceptance criterion: Loss of pressure does not exceed 3 psi for at least 5 minutes.

Table 5-3A (continued)

**NONDESTRUCTIVE SEAM TEST RESULTS FOR PRIMARY LINER
INSTALLED IN CELL 1A**

Panel No.	Seam Length (ft)	Vacuum Box ¹ Pass/Fail (P/F)	Air Pressure		
			Applied Pressure (psi)	Loss of Pressure ² (psi)	Pass/Fail (P/F)
P76-P71	22	-	33	0	P
P76-P75	66	-	30	0	P
P77-P71	22	-	33	0	P
P77-P72	20	-	33	0	P
P77-P76	75	-	32	0	P
P77-RS134	14	P	-	-	-
P78-P72	22	-	32	0	P
P78-P77	58	-	33	0	P
P79-P72	23	-	32	0	P
P79-P78	67	-	30	0	P
P80-P72	22	-	32	0	P
P80-P79	67	-	32	0	P
P81-P72	22	-	32	0	P
P81-P80	67	-	32	0	P
P82-P72	23	-	32	0	P
P82-P81	68	-	33	0	P
P83-P72	23	-	34	0	P
P83-P82	67	-	32	0	P
P84-P72	23	-	34	0	P
P84-P83	67	-	35	0	P
P85-P72	23	-	34	0	P
P85-P84	67	-	31	0	P
P86-P72	22	-	34	0	P
P86-P85	67	-	34	0	P
P87-P72	23	-	34	0	P
P87-P86	67	-	34	0	P
P88-P72	22	-	34	0	P
P88-P87	67	-	33	0	P
P88-P89	63	-	30	0	P
P89-P90	63	-	40	0	P
P90-P91	63	-	35	0	P
P91-P92	63	-	30	0	P
P92-P93	63	-	30	0	P
P93-P94	160	-	30	0	P
P94-P37	22	-	31	0	P
P94-P95	115	-	30	0	P
P94-P97	45	-	30	0	P
P95-P96	114	-	30	0	P
P95-P97	22	-	30	0	P
P96-P98	160	-	30	0	P
P97-P96	46	-	30	0	P
P98-P99	22	-	30	0	P
P98-P100	22	-	34	0	P
P98-P101	116	-	34	0	P

Notes:

¹ Using 5 psi gauge suction in the box.² Acceptance criterion: Loss of pressure does not exceed 3 psi for at least 5 minutes.

Table 5-3A (continued)
NONDESTRUCTIVE SEAM TEST RESULTS FOR PRIMARY LINER
INSTALLED IN CELL 1A

Panel No.	Seam Length (ft)	Vacuum Box ¹ Pass/Fail (P/F)	Air Pressure		
			Applied Pressure (psi)	Loss of Pressure ² (psi)	Pass/Fail (P/F)
P99-P31	-	P	-	-	-
P99-P36	23	-	32	0	P
P99-P37	22	-	31	0	P
P99-P38	23	-	30	0	P
P99-P42	22	-	30	0	P
P99-P100 A	155	-	34	0	P
P99-P100 B	185	-	34	0	P
P100-P101	22	-	32	0	P
P100-P117	44	-	34	0	P
P101-P100	22	-	32	0	P
P101-P102	25	-	32	0	P
P101-P104	39	-	34	0	P
P101-P105	6	-	32	0	P
P101-P106	23	-	32	0	P
P102-P103	25	-	34	0	P
P103-P104	64	-	36	0	P
P104-P106	23	-	32	0	P
P104-P107	23	-	31	0	P
P104-P108	24	-	35	0	P
P104-P109	18	-	32	0	P
P104-P113	19	-	34	0	P
P105-P100	7	-	31	0	P
P105-P106	10	-	33	0	P
P106-P100	29	-	30	0	P
P107-P100	30	-	32	0	P
P107-P106	34	-	32	0	P
P108-P100	2	-	33	0	P
P108-P107	79	-	30	0	P
P108-P109	74	-	34	0	P
P108-P116	18	-	30	0	P
P108-P117	36	-	33	0	P
P109-P110	28	-	30	0	P
P109-P113	19	-	34	0	P
P109-P116 A	55	-	36	0	P
P109-P116 B	37	-	34	0	P
P109-P117	16	-	30	0	P
P110-P111	11	-	-	-	-
P110-P112	19	-	34	0	P
P110-P113	13	-	-	-	-
P111-P112	15	-	-	-	-
P112-P113	25	-	31	0	P
P113-P114	23	-	30	0	P
P115-P113	19	-	-	-	-
P115-P114	11	-	-	-	-
P115-P116	12	-	-	-	-
P116-P114	17	-	30	0	P
P118-P119	61	P	-	-	-
P118-P120	6	P	-	-	-
P119-P120	22.5	P	-	-	-

Notes:

¹ Using 5 psi gauge suction in the box.

² Acceptance criterion: Loss of pressure does not exceed 3 psi for at least 5 minutes.

Table 5-3B

**NONDESTRUCTIVE SEAM TEST RESULTS FOR SECONDARY LINER
INSTALLED IN CELL 1A**

Panel No.	Seam Length (ft)	Vacuum Box ¹ Pass/Fail (P/F)	Air Pressure		
			Applied Pressure (psi)	Loss of Pressure ² (psi)	Pass/Fail (P/F)
S2-S1	457	-	35	0	P
S3-S2 A	13	-	35	0	P
S3-S2 B	444	-	35	0	P
S4-S3	457	-	35	1	P
S5-S4	456	-	35	1	P
S5-S26	13	P	-	-	-
S6-S5	457	-	35	0	P
S6-S26	23	P	-	-	-
S7-S6	457	-	35	0	P
S7-S26	23	-	31	0	P
S8-S7	455	-	35	0	P
S8-S9	455	-	32	0	P
S8-S26	23	-	31	0	P
S9-S10	455	-	30	0	P
S9-S26	23	-	32	0	P
S10-S11	455	-	30	0	P
S10-S26	23	-	31	0	P
S11-S12	456	-	30	0	P
S11-S26	23	-	31	0	P
S12-S13	457	-	30	0	P
S12-S26	23	-	32	0	P
S13-S14	456	-	30	0	P
S13-S26	23	-	32	0	P
S14-S15	457	-	35	0	P
S14-S26	23	-	31	0	P
S15-S16	458	-	34	0	P
S15-S26	23	-	32	0	P
S16-S17	460	-	34	0	P
S16-S26	23	-	31	0	P
S17-S18	456	-	32	0	P
S17-S26	23	-	32	0	P
S18-S19	460	-	35	0	P
S18-S26	23	-	32	0	P
S19-S20	461	-	35	0	P
S19-S26	23	-	31	0	P
S20-S21	460	-	35	0	P
S20-S26	23	-	32	0	P
S21-S22	458	-	35	0	P
S21-S26	23	-	32	0	P
S22-S23	457	-	35	0	P
S22-S26	23	-	32	0	P
S23-S24	455	-	35	0	P
S23-S26	23	-	32	0	P
S24-S25	457	-	35	0	P
S24-S26	23	-	32	0	P

Notes:

¹ Using 5 psi gauge suction in the box.² Acceptance criterion: Loss of pressure does not exceed 3 psi for at least 5 minutes.

Table 5-3B (continued)

**NONDESTRUCTIVE SEAM TEST RESULTS FOR SECONDARY LINER
INSTALLED IN CELL 1A**

Panel No.	Seam Length (ft)	Vacuum Box ¹ Pass/Fail (P/F)	Air Pressure		
			Applied Pressure (psi)	Loss of Pressure ² (psi)	Pass/Fail (P/F)
S25-S26	23	-	32	0	P
S25-S79 A	385	-	35	0	P
S25-S79 B	27	-	34	0	P
S26-S27	463	-	35	0	P
S26-S31	23	-	35	0	P
S26-S79	22	-	31	0	P
S27-S28	463	-	35	0	P
S27-S32	23	-	33	0	P
S27-S79	24	-	32	0	P
S28-S29	463	-	35	0	P
S28-S33	23	-	35	0	P
S28-S84	22	-	33	0	P
S29-S30	463	-	35	0	P
S29-S34	23	-	34	0	P
S29-S84	22	-	33	0	P
S30-S35	23	-	34	0	P
S30-S36	463	-	35	0	P
S30-S84	22	-	33	0	P
S31-S32 A	100	-	35	0	P
S31-S32 B	23	P	-	-	-
S31-S1	23	P	-	-	-
S31-S2	23	P	-	-	-
S31-S4	23	P	-	-	-
S31-S5	10	P	-	-	-
S31-S6	23	P	-	-	-
S32-S33	100	-	35	0	P
S33-S34	100	-	35	0	P
S34-S35	100	-	35	0	P
S35-S40	97	-	35	0	P
S36-S37	463	-	35	0	P
S36-S84	22	-	33	0	P
S36-S130	18	P	-	-	-
S37-S38	462	-	35	0	P
S37-S84	22	-	33	0	P
S37-S130	22	P	-	-	-
S38-S39	462	-	35	0	P
S38-S84	22	-	33	0	P
S38-S130	22	P	-	-	-
S39-S43	22	-	35	0	P
S39-S82	22	-	30	0	P
S39-S83	22	-	34	0	P
S39-S84	22	-	33	0	P
Cap3-S39	123	P	-	-	-
S39-S130	21	P	-	-	-
S40-S41	98	-	35	0	P

Notes:

¹ Using 5 psi gauge suction in the box.² Acceptance criterion: Loss of pressure does not exceed 3 psi for at least 5 minutes.

Table 5-3B (continued)

**NONDESTRUCTIVE SEAM TEST RESULTS FOR SECONDARY LINER
INSTALLED IN CELL 1A**

Panel No.	Seam Length (ft)	Vacuum Box ¹ Pass/Fail (P/F)	Air Pressure		
			Applied Pressure (psi)	Loss of Pressure ² (psi)	Pass/Fail (P/F)
S40-S36	22	-	34	1	P
S41-S42	100	-	35	0	P
S41-S37	22	-	35	0	P
S42-S43	100	-	35	0	P
S44-S43	22	-	35	0	P
S44-S45	57	-	35	0	P
S45-S43	22	-	35	0	P
S45-S46	59	-	35	0	P
S46-S43	22	-	35	0	P
S46-S47	57	-	34	0	P
S47-S43	22	-	35	0	P
S47-S48	59	-	35	0	P
S48-S39	10	-	35	0	P
S48-S43	12	-	35	0	P
S48-S49	59	-	35	0	P
S49-S39	23	-	35	0	P
S49-S50	59	-	35	0	P
S50-S39	22	-	35	0	P
S50-S51	59	-	34	1	P
S51-S39	23	-	35	0	P
S51-S52	59	-	34	0	P
S52-S39	22	-	35	0	P
S52-S53	59	-	35	0	P
S53-S39	22	-	35	0	P
S53-S54	59	-	34	0	P
S54-S39	23	-	35	0	P
S54-S55	60	-	35	0	P
S55-S39	22	-	35	0	P
S55-S56	60	-	35	1	P
S56-S39	23	-	35	0	P
S56-S57	60	-	35	0	P
S57-S39	22	-	35	0	P
S57-S58	60	-	35	0	P
S58-S39	22	-	35	0	P
S58-S59	60	-	34	1	P
S59-S39	22	-	35	0	P
S59-S60	60	-	36	1	P
S60-S39	22	-	35	0	P
S60-S61	60	-	35	0	P
S61-S39	22	-	35	0	P
S61-S62	60	-	35	0	P
S62-S39	22	-	35	0	P
S62-S63	61	-	35	0	P
Cap3-Cap2-P	4	P	-	-	-
Cap3-S63	12	P	-	-	-

Notes:

¹ Using 5 psi gauge suction in the box.² Acceptance criterion: Loss of pressure does not exceed 3 psi for at least 5 minutes.

Table 5-3B (continued)

**NONDESTRUCTIVE SEAM TEST RESULTS FOR SECONDARY LINER
INSTALLED IN CELL 1A**

Panel No.	Seam Length (ft)	Vacuum Box ¹ Pass/Fail (P/F)	Air Pressure		
			Applied Pressure (psi)	Loss of Pressure ² (psi)	Pass/Fail (P/F)
S63-S39	22	-	35	0	P
S63-S64	57	-	35	0	P
S63-S65	4	-	35	0	P
S64-S65	22	-	35	0	P
S64-S66	57	-	35	0	P
S65-S39	22	-	35	0	P
S65-S66	4	P	-	-	-
Cap3-S65	20	P	-	-	-
S66-S39	23	-	35	0	P
S66-S67	62	-	35	0	P
Cap3-S66 A	17	P	-	-	-
Cap3-S66 B	22	P	-	-	-
S67-S39	22	-	35	0	P
S67-S82	62	-	30	0	P
Cap3-S67	4	P	-	-	-
S68-S69 A	26	-	30	0	P
S68-S69 B	44	-	31	1	P
S68-S78 A	52	-	30	0	P
S68-S78 B	0	P	-	-	-
S68-S78 C	0	-	30	0	P
S68-S127	60	P	-	-	-
S69-S70 A	43	-	30	0	P
S69-S70 B	41	-	30	0	P
S70-S71	15	-	32	0	P
S70-S73	32	-	30	0	P
S71-S72	20	-	31	0	P
S72-S73	15	-	30	0	P
S73-S74	41	-	31	0	P
S74-S75	15	-	30	0	P
S74-S77	32	-	30	0	P
S76-S77 A	20	-	31	0	P
S76-S77 B	10	P	-	-	-
S79-S80	460	-	34	0	P
S79-S88	5	P	-	-	-
S80-S81	385	-	36	0	P
S80-S88	18	-	34	0	P
S82-S83	61	-	30	0	P
Cap3-S82	22	P	-	-	-
S83-S84	57	-	36	0	P
Cap3-S83	19	P	-	-	-
S84-S79	18	-	35	0	P
S84-S85	18	-	34	0	P
S84-S86	50	-	34	0	P
S84-S88	128	-	36	0	P
S84-S131	22	P	-	-	-

Notes:

¹ Using 5 psi gauge suction in the box.² Acceptance criterion: Loss of pressure does not exceed 3 psi for at least 5 minutes.

Table 5-3B (continued)

**NONDESTRUCTIVE SEAM TEST RESULTS FOR SECONDARY LINER
INSTALLED IN CELL 1A**

Panel No.	Seam Length (ft)	Vacuum Box ¹ Pass/Fail (P/F)	Air Pressure		
			Applied Pressure (psi)	Loss of Pressure ² (psi)	Pass/Fail (P/F)
S84-S130	21	P	-	-	-
S84A-S130	20	-	30	0	P
S85-S86	17	-	32	0	P
S86-S87	65	-	34	0	P
S86-S131	22	P	-	-	-
S87-S68	24	-	32	0	P
S87-S69	24	-	32	0	P
S87-S88	22	-	36	0	P
S87-S78	14	-	32	0	P
S87-S131	19	P	-	-	-
S114-S128	60	-	29	0	P
S117-S128	6	P	-	-	-
S119-S128	23	P	-	-	-
S127-S87	22	P	-	-	-
S127-S131	5	P	-	-	-
S127-S132 A	23	-	30	0	P
S127-S132 B	23	-	30	0	P
S127-S133 A	22	-	30	0	P
S127-S133 B	22	-	30	0	P
S128-S68	20	P	-	-	-
S128-S69	20	P	-	-	-
S128-S127	24	P	-	-	-
S128-S129	19	-	30	0	P
S128-S131 A	8	-	30	0	P
S128-S131 B	6	-	30	0	P
S128-S132 A	39	-	30	0	P
S128-S132 B	38	-	30	0	P
S129-S68	2	P	-	-	-
S130-S131 A	71	-	30	0	P
S130-S131 B	77	-	30	0	P
S131-S116	22	-	30	0	P
S131-S132 A	25	-	30	0	P
S131-S132 B	23	-	30	0	P
S131-S133 A	24	-	30	0	P
S131-S133 B	22	-	30	0	P
S132-S133 A	18	-	30	0	P
S132-S133 B	18	-	30	0	P

Notes:

¹ Using 5 psi gauge suction in the box.² Acceptance criterion: Loss of pressure does not exceed 3 psi for at least 5 minutes.

Table 5-3C

**NONDESTRUCTIVE SEAM TEST RESULTS FOR PRIMARY AND SECONDARY
LINERS INSTALLED IN THE LEACHATE STORAGE AREA**

Panel No.	Seam Length (ft)	Vacuum Box ¹ Pass/Fail (P/F)	Air Pressure		
			Applied Pressure (psi)	Loss of Pressure ² (psi)	Pass/Fail (P/F)
P1-P2 A	25	-	30	0	P
P1-P2 B	291	-	30	0	P
P2-P3	316	-	30	0	P
P3-P15	23	-	30	< 3	P
P3-P17	22	-	30	< 3	P
P3-P20	76	-	30	< 3	P
P3-P23	23	-	30	< 3	P
P3-P24	22	-	30	< 3	P
P3-P25	22	-	30	< 3	P
P3-P26	22	-	30	< 3	P
P4-P1	12	-	30	0	P
P4-P5	40	-	30	0	P
P4-P9	40	-	30	0	P
P4-P53	23	P	-	-	-
P5-P1	22	-	30	0	P
P5-P54	22	P	-	-	-
P5-P6	40	-	30	0	P
P6-P7	40	-	30	0	P
P6-P55	23	P	-	-	-
P7-P1	22	-	30	0	P
P7-P8	40	-	30	0	P
P7-P56	23	P	-	-	-
P8-P1	22	-	30	0	P
P8-P12	29	-	30	0	P
P8-P57	19	P	-	-	-
P9-P1	2	P	-	-	-
P9-P10	32	-	30	0	P
P9-P11	14	-	30	0	P
P9-P52	23	P	-	-	-
P10-P1	28	-	33	0	P
P11-P10	11	-	30	0	P
P12-P1	24	-	30	0	P
P13-P3	46	-	30	0	P
P14-P3	22	-	30	0	P
P14-P15	53	-	30	0	P
P14-P16	31	-	30	0	P
P15-P17	53	-	30	0	P
P16-P44	21	-	30	0	P
P17-P18	31	-	30	0	P
P17-P20	22	-	30	< 3	P
P18-P19	21	-	30	0	P
P18-P20	8	-	30	< 3	P
P18-P31	12	-	30	0	P
P19-P30	15	-	30	0	P

Notes:

¹ Using 5 psi gauge suction in the box.² Acceptance criterion: Loss of pressure does not exceed 3 psi for at least 5 minutes.

Table 5-3C (continued)

**NONDESTRUCTIVE SEAM TEST RESULTS FOR PRIMARY AND SECONDARY
LINERS INSTALLED IN THE LEACHATE STORAGE AREA**

Panel No.	Seam Length (ft)	Vacuum Box ¹ Pass/Fail (P/F)	Air Pressure		
			Applied Pressure (psi)	Loss of Pressure ² (psi)	Pass/Fail (P/F)
P20-P21	65	-	30	0	P
P21-P22	30	-	30	0	P
P22-P23	31	-	30	0	P
P22-P30	12	P	-	-	-
P23-P20	21	-	30	0	P
P23-P24	52	-	30	0	P
P24-P25	52	-	30	0	P
P25-P26	52	-	30	0	P
P26-P27	22	-	30	< 3	P
P26-P28	30	-	30	0	P
P27-P3	33	-	30	< 3	P
P28-P29	29	-	30	< 3	P
P29-P27	33	-	30	< 3	P
P30-P20	12	-	30	< 3	P
P30-P21	20	-	30	< 3	P
P30-P22	12	-	30	< 3	P
P30-P31	24	-	30	< 3	P
P31-P30	9	-	30	< 3	P
P32-P1	38	-	30	0	P
P32-P33	15	-	30	0	P
P32-P34	2	P	-	-	-
P32-P35	32	-	30	0	P
P33-P34	17	-	30	0	P
P34-P35	14	-	30	0	P
P34-P67	7	P	-	-	-
P35-P1	2	P	-	-	-
P35-P36	40	-	30	0	P
P35-P66	6	P	-	-	-
P35-P67	16	P	-	-	-
P36-P1	22	-	30	0	P
P36-P37	40	-	30	0	P
P36-P65	6	P	-	-	-
P36-P66	17	P	-	-	-
P37-P1	23	-	30	1	P
P37-P38	40	-	30	0	P
P37-P64	7	P	-	-	-
P37-P65	16	P	-	-	-
P38-P1	22	-	30	0	P
P38-P39	40	-	30	0	P
P38-P63	7	P	-	-	-
P38-P64	15	P	-	-	-
P39-P40	44	-	30	0	P
P39-P62	6	P	-	-	-
P39-P63	17	P	-	-	-

Notes:

¹ Using 5 psi gauge suction in the box.² Acceptance criterion: Loss of pressure does not exceed 3 psi for at least 5 minutes.

Table 5-3C (continued)

**NONDESTRUCTIVE SEAM TEST RESULTS FOR PRIMARY AND SECONDARY
LINERS INSTALLED IN THE LEACHATE STORAGE AREA**

Panel No.	Seam Length (ft)	Vacuum Box ¹ Pass/Fail (P/F)	Air Pressure		
			Applied Pressure (psi)	Loss of Pressure ² (psi)	Pass/Fail (P/F)
P40-P42	21	-	30	< 3	P
P40-P61	3	P	-	-	-
P40-P62	16	P	-	-	-
P42-P12	7	-	30	< 3	P
P42-P41	13	-	30	< 3	P
P42-P43	16	-	30	< 3	P
P42-P58	4	P	-	-	-
P42-P61	10	P	-	-	-
P43-P8	16	-	30	< 3	P
P43-P12	9	-	30	< 3	P
P43-P57	3	P	-	-	-
P43-P58	7	P	-	-	-
P44-P13	40	-	30	0	P
P44-P45	9	-	30	< 3	P
P45-P16	11	-	30	< 3	P
P46-P47	312	-	30	0	P
P46-P71	33	-	30	0	P
P46-P74	22	-	30	0	P
P46-P75	22	-	30	0	P
P46-P76	23	-	30	0	P
P46-P77	23	-	30	0	P
P46-P80	68	-	30	0	P
P46-P82	21	-	30	0	P
P46-P83	23	-	30	0	P
P46-P84	22	-	30	0	P
P46-P85	17	-	30	0	P
P46-P88	38	-	30	0	P
P47-P48	313	-	30	1	P
P48-P49	31	-	30	0	P
P48-P53	22	-	30	0	P
P48-P54	22	-	30	0	P
P48-P55	22	-	30	0	P
P48-P56	22	-	30	0	P
P48-P60	70	-	30	0	P
P48-P63	22	-	30	0	P
P48-P64	23	-	30	0	P
P48-P65	22	-	30	0	P
P48-P66	22	-	30	0	P
P48-P70	34	-	30	0	P
P49-P50	20	-	30	0	P
P49-P52	12	-	30	0	P
P49-P53	10	-	30	0	P
P50-P51	7	-	30	0	P
P50-P52	19	-	30	0	P

Notes:

¹ Using 5 psi gauge suction in the box.² Acceptance criterion: Loss of pressure does not exceed 3 psi for at least 5 minutes.

Table 5-3C (continued)

**NONDESTRUCTIVE SEAM TEST RESULTS FOR PRIMARY AND SECONDARY
LINERS INSTALLED IN THE LEACHATE STORAGE AREA**

Panel No.	Seam Length (ft)	Vacuum Box ¹ Pass/Fail (P/F)	Air Pressure		
			Applied Pressure (psi)	Loss of Pressure ² (psi)	Pass/Fail (P/F)
P51-P52	7	-	30	0	P
P52-P53	39	-	30	1	P
P53-P54	49	-	30	0	P
P54-P55	48	-	30	1	P
P55-P56	48	-	30	0	P
P56-P57	48	-	30	0	P
P56-P60	8	-	30	0	P
P57-P58	13	-	30	0	P
P57-P59	12	-	30	0	P
P57-P60	15	-	30	0	P
P58-P59	14	-	30	0	P
P59-P60	42	-	30	0	P
P60-P62	16	-	30	0	P
P60-P63	7	-	30	0	P
P61-P59	14	-	30	0	P
P62-P59	13	-	30	0	P
P62-P63	39	-	30	1	P
P63-P64	48	-	30	0	P
P64-P65	48	-	30	0	P
P65-P66	48	-	30	1	P
P66-P67	39	-	30	0	P
P66-P70	9	-	30	0	P
P67-P68	13	-	30	0	P
P67-P69	12	-	30	0	P
P67-P70	15	-	30	0	P
P68-P69	14	-	30	1	P
P72-P71	15	-	30	0	P
P73-P71	15	-	30	0	P
P73-P74	35	-	30	0	P
P74-P71	7	-	30	0	P
P74-P75	44	-	30	1	P
P75-P76	45	-	30	1	P
P76-P77	44	-	30	0	P
P77-P78	37	-	30	1	P
P77-P80	7	-	30	0	P
P78-P79	22	-	30	0	P
P78-P80	17	-	30	1	P
P79-P80	38	-	30	0	P
P79-P81	22	-	30	0	P
P80-P81	20	-	30	0	P
P81-P82	38	-	30	1	P
P82-P83	43	-	30	0	P
P83-P84	43	-	30	0	P
P84-P85	43	-	30	0	P

Notes:

¹ Using 5 psi gauge suction in the box.² Acceptance criterion: Loss of pressure does not exceed 3 psi for at least 5 minutes.

Table 5-3C (continued)

**NONDESTRUCTIVE SEAM TEST RESULTS FOR PRIMARY AND SECONDARY
LINERS INSTALLED IN THE LEACHATE STORAGE AREA**

Panel No.	Seam Length (ft)	Vacuum Box ¹ Pass/Fail (P/F)	Air Pressure		
			Applied Pressure (psi)	Loss of Pressure ² (psi)	Pass/Fail (P/F)
P85-P86	34	-	30	0	P
P85-P88	6	-	30	0	P
P86-P87	7	-	30	0	P
P86-P88	19	-	30	1	P
P87-P88	9	-	30	0	P
S1-S2	314	-	34	0	P
S2-S3	314	-	35	0	P
S3-S33	76	-	34	0	P
S4-S5	33	-	34	0	P
S5-S6	32	-	35	0	P
S6-S7	35	-	35	0	P
S7-S8	33	-	35	0	P
S8-S9	33	-	35	0	P
S9-S10	33	-	34	0	P
S10-S11	32	-	35	0	P
S10-S13	3	P	-	-	-
S11-S12	17	-	33	0	P
S11-S13	25	-	34	0	P
S12-S13	12	-	35	0	P
S12-S14	32	-	35	0	P
S12-S15	8	-	34	0	P
S13-S14	22	-	33	0	P
S14-S15	34	-	34	0	P
S14-S16	4	P	-	-	-
S15-S16	42	-	34	0	P
S16-S17	39	-	34	0	P
S17-S18	39	-	34	0	P
S18-S19	40	-	35	0	P
S19-S20	40	-	35	0	P
S20-S21	38	-	34	0	P
S21-S22	18	-	33	0	P
S23-S3	32	-	34	0	P
S23-S24	32	-	35	0	P
S27-S3	22	-	35	0	P
S28-S3	23	-	33	1	P
S29-S3	23	-	34	0	P
S30-S3	20	-	33	0	P
S30-S31	28	-	33	0	P
S30-S33	18	-	34	1	P
S31-S32	4	-	35	0	P
S32-S33	67	-	35	0	P
S34-S31	13	-	33	0	P
S34-S32	26	-	33	0	P
S35-S36 A	51	-	33	0	P

Notes:

¹ Using 5 psi gauge suction in the box.² Acceptance criterion: Loss of pressure does not exceed 3 psi for at least 5 minutes.

Table 5-3C (continued)

**NONDESTRUCTIVE SEAM TEST RESULTS FOR PRIMARY AND SECONDARY
LINERS INSTALLED IN THE LEACHATE STORAGE AREA**

Panel No.	Seam Length (ft)	Vacuum Box ¹ Pass/Fail (P/F)	Air Pressure		
			Applied Pressure (psi)	Loss of Pressure ² (psi)	Pass/Fail (P/F)
S35-S36 B	16	-	34	0	P
S36-S37 A	51	-	34	0	P
S36-S37 B	22	-	33	0	P
S37-S38 A	49	-	32	0	P
S37-S38 B	22	-	35	0	P
S38-S39 A	33	P	-	-	-
S38-S39 B	22	-	34	0	P
S38-S43	15	-	33	0	P
S39-S41	32	-	34	0	P
S39-S42	3	P	-	-	-
S39-S43	9	-	34	0	P
S41-S42	39	-	35	0	P
S43-S3	35	-	33	0	P
S44-S45	15	-	35	0	P
S44-S66	15	-	33	0	P
S45-S65	8	-	33	0	P
S45-S66	25	-	35	0	P
S46-S47	40	-	35	0	P
S46-S62	19	-	35	0	P
S46-S65	15	-	35	0	P
S46-S85	4	P	-	-	-
S47-S48	40	-	35	1	P
S47-S85 A	23	-	35	0	P
S47-S85 B	23	-	35	0	P
S48-S62	23	-	34	0	P
S48-S85	41	P	-	-	-
S49-S48	45	-	35	0	P
S49-S50	32	-	35	0	P
S49-S52	11	-	33	0	P
S49-S62	21	-	34	0	P
S50-S51	23	-	33	1	P
S51-S52	46	-	33	1	P
S52-S50	23	-	35	0	P
S52-S62 A	73	-	35	0	P
S52-S62 B	74	-	34	0	P
S53-S51	25	-	35	0	P
S53-S52	18	-	33	0	P
S54-S52	10	-	32	0	P
S54-S53	39	-	34	1	P
S54-S55	45	-	35	0	P
S54-S62	23	-	35	0	P
S55-S56	44	-	32	0	P
S55-S62	22	-	35	0	P
S56-S62	23	-	33	0	P

Notes:

¹ Using 5 psi gauge suction in the box.² Acceptance criterion: Loss of pressure does not exceed 3 psi for at least 5 minutes.

Table 5-3C (continued)

**NONDESTRUCTIVE SEAM TEST RESULTS FOR PRIMARY AND SECONDARY
LINERS INSTALLED IN THE LEACHATE STORAGE AREA**

Panel No.	Seam Length (ft)	Vacuum Box ¹ Pass/Fail (P/F)	Air Pressure		
			Applied Pressure (psi)	Loss of Pressure ² (psi)	Pass/Fail (P/F)
S57-S58	28	-	35	0	P
S57-S62	22	-	35	0	P
S60-S58	28	-	34	0	P
S61-S57	14	-	34	1	P
S61-S58	7	-	32	0	P
S61-S60	28	-	35	0	P
S62-S63	314	-	35	0	P
S62-S65	28	-	35	0	P
S62-S85 A	11	-	35	0	P
S62-S85 B	12	-	35	0	P
S63-S64	314	-	35	0	P
S64-S67	31	-	35	0	P
S64-S84	32	-	35	0	P
S65-S66	30	-	35	0	P
S67-S68	31	-	35	0	P
S67-S70 A	7	-	33	0	P
S67-S70 B	2	P	-	-	-
S69-S70	33	-	33	0	P
S70-S71	45	-	35	0	P
S71-S64	23	-	35	0	P
S71-S72	44	-	34	0	P
S72-S64	22	-	34	0	P
S72-S73	45	-	33	0	P
S73-S64	23	-	32	0	P
S73-S74	44	-	35	0	P
S74-S75	23	-	35	0	P
S74-S76	26	-	34	0	P
S75-S76	37	-	35	0	P
S75-S77	25	-	35	0	P
S76-S64	68	-	33	0	P
S76-S77	18	-	35	0	P
S76-S78	11	-	33	0	P
S77-S78	35	-	35	0	P
S78-S64	22	-	34	0	P
S78-S79	42	-	35	0	P
S79-S64	23	-	34	0	P
S80-S64	22	-	34	0	P
S80-S79	44	-	35	0	P
S80-S81	42	-	35	0	P
S81-S64	21	-	34	0	P
S81-S82	42	-	35	0	P
S82-S64	5	P	-	-	-
S82-S83	32	-	34	0	P
S82-S84	2	P	-	-	-
S83-S84	33	-	32	0	P

Notes:

¹ Using 5 psi gauge suction in the box.² Acceptance criterion: Loss of pressure does not exceed 3 psi for at least 5 minutes.

Table 5-4A

**DESTRUCTIVE SEAM TEST RESULTS FOR PRIMARY LINER
INSTALLED IN CELL 1A**

Sample No.	Panel No.	Weld Type ¹	Peel Strength ² (lb/in)														Shear Strength ³ (lb/in)						Failure Type ⁴	Pass/Fail (P/F)	Retest No.
			Bottom Peel (inside)							Top Peel (outside)															
			114	113	122	125	130	133	134	130	119	119	180	188	182	183	182								
DSP-1	P1-P2	F	128	134	145	124	141	138	135	127	128	123	123	123	123	192	181	180	185	190	FTB	P			
DSP-2	P2-P3	F	131	120	128	124	129	126	124	126	126	134	213	180	186	179	185				FTB	P			
DSP-3	P3-P4	F	130	78	127	126	117	110	137	113	129	150	182	176	178	176	177				FTB	P			
DSP-4	P4-P5	F	120	122	118	121	134	130	120	133	132	121	196	174	169	170	183				FTB	P			
DSP-5	P5-P6	F	123	129	139	119	127	135	136	123	134	119	182	174	171	170	173				FTB	P			
DSP-6	P6-P7	F	123	131	148	140	133	126	150	104	128	155	189	170	188	192	191				FTB	P			
DSP-7	P7-P8	F	130	129	133	144	81	125	152	132	152	150	185	183	181	182	180				FTB	P			
DSP-8	P8-P9	F	151	132	131	130	130	128	135	140	123	128	183	184	193	185	184				FTB	P			
DSP-9	P9-P10	F	130	118	113	114	111	119	112	125	126	122	184	183	177	181	181				FTB	P			
DSP-10	P10-P11	F	123	124	145	148	119	121	123	122	127	136	177	182	177	176	173				FTB	P			
DSP-11	P11-P12	F	144	116	111	116	125	117	114	130	134	119	184	193	165	173	168				FTB	P			
DSP-12	P12-P13	F	118	124	127	115	130	125	107	116	121	117	209	179	145	183	174				FTB	P			
DSP-13	P13-P14	F	131	115	135	141	119	115	118	105	113	121	183	179	202	183	219				FTB	P			
DSP-14	P14-P15	F	148	128	139	120	146	122	113	117	122	126	178	178	175	177	175				FTB	P			
DSP-15	P15-P16	F	156	125	132	133	141	129	127	128	138	140	166	179	183	176	179				FTB	P			
DSP-16	P16-P17	F	131	126	130	142	119	126	140	130	139	136	195	192	190	184	196				FTB	P			
DSP-17	P17-P18	F	124	130	148	130	118	135	131	137	127	141	183	185	184	186	186				FTB	P			
DSP-18	P18-P19	F	132	133	131	131	162	128	139	153	130	117	200	187	194	183	182				FTB	P			
DSP-19	P19-P20	F	136	127	144	140	137	126	137	133	139	128	188	189	192	198	213				FTB	P			
DSP-20	P20-P21	F	134	149	123	136	80	146	137	138	143	139	197	201	208	197	191				FTB	P			
DSP-21	P21-P22	F	129	128	135	133	138	133	123	127	125	125	200	202	201	196	196				FTB	P			
DSP-22	P22-P23	F	118	131	121	122	132	117	131	122	116	122	189	192	191	199	205				FTB	P			
DSP-23	P23-P24	F	154	141	164	156	136	120	114	128	125	122	197	208	202	204	204				FTB	P			
DSP-24	P24-P25	F	142	149	142	144	146	138	142	142	159	137	191	196	196	192	193				FTB	P			
DSP-25	P6-P26	F																							

Notes:

- 1 "F" is fusion and "E" is extrusion weld.
- 2 Specified peel strength: 78 lb/in for fusion and 70 lb/in for extrusion
- 3 Specified shear strength: 120 lb/in for fusion and 108 lb/in for extrusion
- 4 "FTB" is Film Tear Bond (with maximum 10 percent seam separation) and "AD" is adhesion failure (Non-FTB).

Table 5-4A (continued)
DESTRUCTIVE SEAM TEST RESULTS FOR PRIMARY LINER
INSTALLED IN CELL 1A

Sample No.	Panel No.	Weld Type ¹	Peel Strength ² (lb/in)													Shear Strength ³ (lb/in)				Failure Type ⁴	Pass/Fail (P/F)	Retest No.
			Bottom Peel (inside)						Top Peel (outside)													
			123	131	131	136	133	129	148	155	141	145	181	180	174	175	183					
DSP-26	P26-P27	F	123	131	131	136	133	129	148	155	141	145	181	180	174	175	183	FTB	P			
DSP-27	P32-P33	F	141	136	128	127	132	126	123	126	124	127	202	199	186	198	195	FTB	P			
DSP-28	P26-P28	F	133	140	126	125	125	115	118	117	112	109	191	197	182	183	175	FTB	P			
DSP-29	P28-P29	F	120	129	122	126	125	122	120	127	118	123	193	186	190	183	188	FTB	P			
DSP-30	P29-P30	F	127	127	130	124	129	127	130	132	123	134	183	194	178	190	175	FTB	P			
DSP-31	P30-P31	F	131	147	127	154	138	130	134	135	134	131	180	186	193	190	193	FTB	P			
DSP-32	P31-P36	F	131	130	166	132	168	135	149	140	146	130	196	202	194	205	198	FTB	P			
DSP-33	P36-P37	F	130	130	140	145	140	137	137	142	133	135	194	193	189	195	194	FTB	P			
DSP-34	P40-P41	F	145	140	149	150	140	148	143	149	156	141	169	189	181	197	193	FTB	P			
DSP-35	P37-P38	F	152	155	144	156	136	132	132	130	140	135	198	184	204	192	180	FTB	P			
DSP-36	P41-P43	F	129	140	142	138	136	139	144	153	147	155	194	187	186	201	187	FTB	P			
DSP-37	P48-P49	F	148	134	144	134	147	136	145	132	147	136	201	185	190	187	186	FTB	P			
DSP-38	P55-P56	F	143	127	135	152	137	136	139	144	135	140	192	192	201	197	177	FTB	P			
DSP-39	P62-P63	F	135	-	136	154	148	-	140	133	128	133	-	-	-	-	-	AD	F	39A, 39B		
DSP-39A	P63-P64	F	131	140	144	137	144	123	128	135	144	142	186	182	180	187	181	FTB	P			
DSP-39B	P61-P62	F	136	138	126	141	126	139	131	-	-	-	183	177	173	179	181	AD	F	39B1		
DSP-39B1	P61-P62	F	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	AD	F ⁵	39B2		
DSP-39B2	P61-P62	F	130	134	153	136	130	136	135	136	135	133	203	188	188	183	184	FTB	P			
DSP-40	P51-P42	F	154	135	137	150	142	144	142	140	130	130	187	195	185	183	191	FTB	P			
DSP-41	Cap2-P62	E	129	144	134	146	126	-	-	-	-	-	172	175	169	154	170	FTB	P			
DSP-42	Cap8-P62	E	133	136	165	162	150	-	-	-	-	-	171	175	170	174	176	AD	F	42A, 42B		
DSP-42A	P60-Cap8	E	133	131	129	119	123	-	-	-	-	-	180	186	191	193	192	FTB	P			
DSP-42B	P62-Cap8	E	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	AD	F ⁵	42B1		
DSP-42B1	P62-Cap8	E	114	152	124	123	119	-	-	-	-	-	184	178	183	180	183	FTB	P			
DSP-43	P42-P68	E	146	145	148	139	155	-	-	-	-	-	177	175	173	172	167	FTB	P			

Notes:

- ¹ "F" is fusion and "E" is extrusion weld.
- ² Specified peel strength: 78 lb/in for fusion and 70 lb/in for extrusion
- ³ Specified shear strength: 120 lb/in for fusion and 108 lb/in for extrusion
- ⁴ "FTB" is Film Tear Bond (with maximum 10 percent seam separation) and "AD" is adhesion failure (Non-FTB).
- ⁵ No lab test was performed on this sample because field test failed.

Table 5-4A (continued)

**DESTRUCTIVE SEAM TEST RESULTS FOR PRIMARY LINER
INSTALLED IN CELL 1A**

Sample No.	Panel No.	Weld Type ¹	Peel Strength ²												Shear Strength ³ (lb/in)					Failure Type ⁴	Pass/Fail (P/F)	Retest No.
			Bottom Peel (inside)						Top Peel (outside)													
			139	124	136	127	137	128	137	144	141	137	153	161	172	160	171					
DSP-44	P25-P69	F	139	124	136	127	137	128	137	144	141	137	153	161	172	160	171	FTB	P			
DSP-45	P69-P70	F	141	123	137	117	147	133	133	124	121	126	174	182	177	170	180	FTB	P			
DSP-46	P70-P71	F	144	143	143	150	129	121	126	133	129	130	177	177	151	167	176	FTB	P			
DSP-47	P71-P72	F	128	122	130	127	131	144	126	120	124	124	175	175	173	171	175	FTB	P			
DSP-48	P74-P73	F	117	127	145	123	125	133	126	136	131	120	176	176	175	176	180	FTB	P			
DSP-49	P75-P74	F	120	130	130	122	122	123	115	119	115	121	164	176	173	182	179	FTB	P			
DSP-50	P79-P78	F	113	126	124	121	142	131	116	135	127	134	168	168	166	168	166	FTB	P			
DSP-51	P88-P87	F	132	133	130	129	121	115	123	136	146	124	174	171	175	170	170	FTB	P			
DSP-52	P86-P85	F	132	128	120	136	137	132	138	142	129	133	175	176	181	174	173	FTB	P			
DSP-53	P87-P72	F	124	113	147	125	135	112	139	123	109	114	167	163	162	170	166	FTB	P			
DSP-54	P96-P97	F	123	152	131	128	125	145	132	150	129	135	166	179	185	164	169	FTB	P			
DSP-55	P30-P96	F	136	137	135	138	141	116	143	129	135	142	159	155	167	167	162	FTB	P			
DSP-56	RP149-P96	E	112	122	124	147	118	-	-	-	-	-	153	163	163	167	164	FTB	P			
DSP-57	P71-P70	E	153	139	141	121	125	-	-	-	-	-	163	156	165	165	165	FTB	P			
DSP-58	P99-P100	F	131	130	123	126	117	126	122	126	140	122	192	190	196	183	185	FTB	P			
DSP-59	P98-P101	F	138	122	131	136	119	133	131	141	130	128	199	190	192	195	190	FTB	P			
DSP-60	P108-P109	F	128	141	136	129	135	143	140	139	137	141	196	188	193	200	192	FTB	P			
DSP-61	P38-P99	F	143	133	126	131	130	131	130	125	123	147	186	199	197	191	190	FTB	P			

Notes:

- 1 "F" is fusion and "E" is extrusion weld.
2 Specified peel strength: 78 lb/in for fusion and 70 lb/in for extrusion
3 Specified shear strength: 120 lb/in for fusion and 108 lb/in for extrusion
4 "FTB" is Film Tear Bond (with maximum 10 percent seam separation) and "AD" is adhesion failure (Non-FTB).

Table 5-4B
DESTRUCTIVE SEAM TEST RESULTS FOR SECONDARY LINER
INSTALLED IN CELL 1A

Sample No.	Panel No.	Weld Type ¹	Peel Strength ²															Shear Strength ³ (lb/in)					Failure Type ⁴	Pass/Fail (P/F)
			Bottom Peel (Inside)					Top Peel (outside)																
			151	143	139	139	137	123	133	123	134	126	188	183	183	178	187							
DSS-1	S1-S2	F	-	136	136	140	133	-	129	124	120	132	180	182	179	183	177	FTB	P					
DSS-2	S2-S3	F	146	141	142	147	142	120	126	135	147	129	179	172	181	183	187	FTB	P					
DSS-3	S3-S4	F	135	138	141	138	140	127	128	138	125	149	190	191	188	186	184	FTB	P					
DSS-4	S4-S5	F	133	133	139	137	130	123	136	136	138	126	182	181	180	183	183	FTB	P					
DSS-5	S5-S6	F	138	132	147	131	143	132	132	129	133	130	188	185	189	187	187	FTB	P					
DSS-6	S6-S7	F	134	145	139	139	136	141	131	137	130	134	185	180	182	177	180	FTB	P					
DSS-7	S7-S8	F	131	146	150	142	147	119	130	100	133	136	190	183	179	183	185	FTB	P					
DSS-8	S8-S9	F	131	137	128	130	134	132	142	131	125	138	184	174	169	182	183	FTB	P					
DSS-9	S9-S10	F	148	150	140	140	141	146	146	115	119	115	189	187	181	181	180	FTB	P					
DSS-10	S10-S11	F	145	136	133	141	121	121	122	126	115	137	179	179	185	185	186	FTB	P					
DSS-11	S11-S12	F	123	122	132	129	143	133	134	133	121	143	167	179	177	170	179	FTB	P					
DSS-12	S12-S13	F	137	125	130	140	131	130	121	123	128	121	181	179	179	179	179	FTB	P					
DSS-13	S13-S14	F	133	146	135	149	136	137	131	128	131	123	176	175	171	184	170	FTB	P					
DSS-14	S14-S15	F	130	122	139	133	118	112	117	121	115	117	170	170	174	194	192	FTB	P					
DSS-15	S15-S16	F	126	128	134	133	136	124	110	116	118	117	168	172	177	174	176	FTB	P					
DSS-16	S16-S17	F	117	128	133	117	129	125	114	121	123	117	170	171	172	172	168	FTB	P					
DSS-17	S17-S18	F	125	122	137	134	120	126	126	115	125	115	172	171	173	176	174	FTB	P					
DSS-18	S18-S19	F	128	113	146	119	116	118	124	118	121	123	171	195	171	171	173	FTB	P					
DSS-19	S19-S20	F	123	136	119	119	125	121	120	118	126	137	175	173	173	174	170	FTB	P					
DSS-20	S20-S21	F	126	123	109	127	116	139	127	124	131	118	174	160	167	165	164	FTB	P					
DSS-21	S21-S22	F	125	129	114	115	132	123	115	120	123	138	188	199	170	168	166	FTB	P					
DSS-22	S22-S23	F	115	119	127	126	143	130	132	144	126	136	183	177	174	173	182	FTB	P					
DSS-23	S23-S24	F	113	119	115	119	157	114	119	120	114	127	186	176	175	171	166	FTB	P					
DSS-24	S24-S25	F	128	124	124	120	140	133	132	116	124	131	208	183	193	177	175	FTB	P					
DSS-25	S26-S27	F																						

Notes:

¹ "F" is fusion and "E" is extrusion weld.

² Specified peel strength: 78 lb/in for fusion and 70 lb/in for extrusion

³ Specified shear strength: 120 lb/in for fusion and 108 lb/in for extrusion

⁴ "FTB" is Film Tear Bond (with maximum 10 percent seam separation) and "AD" is adhesion failure (Non-FTB).

Table 5-4B (continued)
DESTRUCTIVE SEAM TEST RESULTS FOR SECONDARY LINER
INSTALLED IN CELL 1A

Sample No.	Panel No.	Weld Type ¹	Peel Strength ² (lb/in)										Shear Strength ³ (lb/in)					Failure Type ⁴	Pass/Fail (P/F)	
			Bottom Peel (inside)					Top Peel (outside)												
			147	126	147	127	135	129	136	136	141	126								
DSS-26	S27-S28	F	131	122	129	125	119	117	136	125	121	123	182	175	181	187	183	185	FTB	P
DSS-27	S28-S29	F	127	131	136	128	133	126	120	124	127	129	188	181	176	177	177	177	FTB	P
DSS-28	S29-S30	F	119	132	146	148	128	147	151	126	148	155	176	172	168	165	172	172	FTB	P
DSS-29	S28-S33	F	139	132	133	139	143	143	146	136	132	144	202	182	195	181	182	182	FTB	P
DSS-30	S32-S33	F	85	109	120	110	105	-	-	-	-	-	193	172	166	175	169	172	FTB	P
DSS-31	S4-S31	E	124	114	137	142	124	146	134	126	122	138	171	177	177	176	170	177	FTB	P
DSS-32	S8-S26	F	124	122	124	126	130	118	127	120	131	128	180	182	177	191	185	182	FTP	P
DSS-33	S30-S36	F	133	127	121	130	128	121	117	133	119	127	183	192	184	187	180	192	FTP	P
DSS-34	S36-S37	F	134	126	129	131	127	131	128	125	127	124	194	188	185	185	196	194	FTP	P
DSS-35	S37-S38	F	126	122	119	130	131	120	118	126	122	124	181	183	186	180	182	183	FTP	P
DSS-36	S38-S39	F	136	128	131	137	130	130	126	125	129	120	190	190	192	188	187	190	FTP	P
DSS-37	S46-S43	F	126	124	128	120	130	124	124	122	129	133	179	185	183	180	184	179	FTP	P
DSS-38	S47-S48	F	139	129	132	131	136	133	134	130	126	130	187	181	184	183	182	187	FTP	P
DSS-39	S55-S56	F	144	158	161	153	156	136	163	148	137	140	188	199	184	185	183	188	FTP	P
DSS-40	S64-S65	F	141	137	140	141	138	130	131	136	129	131	185	194	188	179	186	194	FTP	P
DSS-41	S39-S61	F	147	134	145	146	140	140	132	138	125	143	180	202	178	187	180	202	FTP	P
DSS-42	S25-S79	F	137	127	143	130	144	154	132	127	129	158	171	171	187	155	185	171	FTP	P
DSS-43	S79-S80	F	124	119	134	137	138	120	148	134	128	126	187	181	188	180	179	187	FTP	P
DSS-44	S67-S82	F	124	131	127	108	120	-	-	-	-	-	122	166	169	167	170	122	FTP	P
DSS-45	Cap2-S39	F	130	119	126	125	128	119	123	124	126	127	190	183	185	184	190	190	FTP	P
DSS-46	S81-S89	F	133	128	125	119	120	128	123	126	117	123	191	188	185	183	180	188	FTP	P
DSS-47	S117-S119	F	131	127	120	119	114	127	125	120	114	117	190	183	180	193	180	190	FTP	P
DSS-48	S89-S98	F	135	122	134	136	129	139	141	132	136	139	182	179	180	187	185	182	FTP	P
DSS-49	S107-S106	F	128	142	134	143	143	145	141	125	130	135	181	184	177	187	185	181	FTP	P
DSS-50	S100-S101	F																		P

Notes:

¹ "F" is fusion and "E" is extrusion weld.

² Specified peel strength: 78 lb/in for fusion and 70 lb/in for extrusion

³ Specified shear strength: 120 lb/in for fusion and 108 lb/in for extrusion

⁴ "FTB" is Film Tear Bond (with maximum 10 percent seam separation) and "AD" is adhesion failure (Non-FTB).

Table 5-4B (continued)

DESTRUCTIVE SEAM TEST RESULTS FOR SECONDARY LINER
INSTALLED IN CELL 1A

Sample No.	Panel No.	Weld Type ¹	Peel Strength ² (lb/in)										Shear Strength ³ (lb/in)					Failure Type ⁴	Pass/Fail (P/F)																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																		
			Bottom Peel (inside)					Top Peel (outside)																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																													
			145	130	139	137	130	133	134	138	135	127	173	163	171	171	171																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																				
DSS-51	S93-S94	F																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																			

Table 5-4C

**DESTRUCTIVE SEAM TEST RESULTS FOR PRIMARY AND SECONDARY
LINERS INSTALLED IN THE LEACHATE STORAGE AREA**

Sample No.	Panel No.	Weld Type ¹	Peel Strength ²																Shear Strength ³ (lb/in)						Failure Type ⁴	Pass/Fail (P/F)	Retest No.		
			Bottom Peel (inside)								Top Peel (outside)																		
			131	126	130	124	122	122	129	118	117	120	196	189	187	190	184	127	118	123	131	126	119	115				130	124
DSSLP-1	S7-S8	F	127	118	123	131	126	119	126	122	122	129	118	117	120	196	189	187	190	184	FTB	P							
DSSLP-2	S17-S1	F	130	120	122	119	126	122	126	122	122	117	117	123	119	188	187	187	190	189	FTB	P							
DSSLP-3	S37-S38	F	129	132	123	126	126	120	126	120	126	126	120	120	119	193	190	180	185	187	FTB	P							
DSSLP-4	S23-S27	F	124	124	131	123	131	126	130	126	126	130	122	118	128	186	186	185	187	189	FTB	P							
DSSLP-5	S2-S3	F	124	126	119	122	120	126	121	126	126	121	128	120	117	189	183	187	185	180	FTB	P							
DSSLP-6	S57-S62	F	126	122	117	120	131	121	120	121	121	120	126	130	124	179	187	180	182	179	FTB	P							
DSSLP-7	S63-S64	F	114	122	130	119	121	120	123	120	120	123	127	127	119	186	177	186	190	188	FTB	P							
DSSLP-8	S73-S64	F	129	119	125	126	117	116	124	116	116	124	125	130	128	177	188	192	190	183	FTB	P							
DSSLP-9	S46-S65	F	126	123	116	127	130	126	118	126	126	118	122	133	119	177	181	179	186	185	FTB	P							
DSSLP-10	S62-S63	F	113	115	108	107	110	-	-	-	-	-	-	-	-	169	157	170	163	149	AD	F	11A, 11B						
DSSLP-11	S40-S50	E	120	105	83	91	106	-	-	-	-	-	-	-	-	139	150	163	174	168	AD	F	11C						
DSSLP-11A	S40-S50	E	131	120	133	121	114	-	-	-	-	-	-	-	-	161	166	172	168	165	FTB	P							
DSSLP-11B	S35-S49	E	113	109	103	111	106	-	-	-	-	-	-	-	-	156	164	161	160	153	AD	F	11D						
DSSLP-11C	S34-S51	E	101	97	132	143	106	-	-	-	-	-	-	-	-	167	173	170	166	172	FTB	P							
DSSLP-11D	Cap1-S53	E	116	145	140	136	154	-	-	-	-	-	-	-	-	168	165	169	163	164	FTB	P							
DSSLP-12	S55-S29	E	131	133	124	143	149	136	130	137	136	130	137	136	126	175	174	182	181	172	FTB	P							
DSPLP-1	P1-P2	F	123	150	138	153	149	145	128	145	126	128	145	126	126	178	180	177	178	185	FTB	P							
DSPLP-2	P1-P2	F	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	F ⁵	3A, 3B						
DSPLP-3	P9-P10	F	117	125	118	119	120	114	120	118	107	120	118	107	111	147	151	153	157	144	FTB	P							
DSPLP-3A	P1-P12	F	119	107	123	118	129	105	124	120	120	124	120	120	121	159	157	142	153	156	FTB	P							
DSPLP-3B	P6-P7	F	122	130	121	119	125	119	121	115	126	121	115	126	128	188	180	183	185	180	FTB	P							
DSPLP-4	P1-P37	F	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	F ⁵	5A						
DSPLP-5	P60-P51	E	115	119	123	128	120	-	-	-	-	-	-	-	-	189	191	188	183	185	FTB	P							
DSPLP-5A	Cap1-P22	E																											

Notes:

- ¹ "F" is fusion and "E" is extrusion weld.
- ² Specified peel strength: 78 lb/in for fusion and 70 lb/in for extrusion
- ³ Specified shear strength: 120 lb/in for fusion and 108 lb/in for extrusion
- ⁴ "FTB" is Film Tear Bond (with maximum 10 percent seam separation) and "AD" is adhesion failure (Non-FTB).
- ⁵ Sample failed in field testing.

Table 5-4C (continued)

**DESTRUCTIVE SEAM TEST RESULTS FOR PRIMARY AND SECONDARY
LINERS INSTALLED IN THE LEACHATE STORAGE AREA**

Sample No.	Panel No.	Weld Type ¹	Peel Strength ² (lb/in)												Shear Strength ³ (lb/in)					Failure Type ⁴	Pass/Fail (P/F)	Retest No.
			Bottom Peel (inside)						Top Peel (outside)													
			119	115	130	121	123	124	124	125	127	126	156	164	161	154	160					
DSPLP-6	P46-P47	F	122	112	108	111	111	112	119	119	118	121	168	177	180	163	157	FTB	P			
DSPLP-7	P47-P48	F	172	139	139	139	150	-	-	-	-	-	162	158	168	168	163	FTB	P			
DSPLP-8	P4-P53	E	147	131	129	130	119	134	128	133	131	132	175	165	195	177	172	FTB	P			
DSPLP-9	P74-P75	F	115	119	120	130	120	120	134	130	119	119	162	164	165	164	155	FTB	P			
DSPLP-10	P46-P75	F	130	126	122	100	109	-	-	-	-	-	179	191	183	190	185	FTB	P			
DSPLP-11	Cap2-P36	E																FTB				

Notes:

¹ "F" is fusion and "E" is extrusion weld.

² Specified peel strength: 78 lb/in for fusion and 70 lb/in for extrusion

³ Specified shear strength: 120 lb/in for fusion and 108 lb/in for extrusion

⁴ "FTB" is Film Tear Bond (with maximum 10 percent seam separation) and "AD" is adhesion failure (Non-FTB).

Notes:

1 "F" is fusion and "E" is extrusion weld.

2 Specified peel strength: 78 lb/in for fusion and 70 lb/in for extrusion

3 Specified shear strength: 120 lb/in for fusion and 108 lb/in for extrusion

4 "FTB" is Film Tear Bond (with maximum 10 percent seam separation) and "AD" is adhesion failure (Non-FTB).

Table 5-5

**CQA AND MQC TEST RESULTS FOR 60-mil SMOOTH GEOMEMBRANE (GSE)
RESIN LOT NO. D30520959**

	CONSTRUCTION QUALITY ASSURANCE (CQA)								MANUFACTURING QUALITY CONTROL (MQC)									
PROPERTY	Thickness (mil)	Density (g/cm ³)	Carbon Black Content (%)	Carbon Black Dispersion	Yield Strength ² (lb/in)	Break Strength ² (lb/in)	Yield Elongation ² (%)	Break Elongation ² (%)	Thickness ¹ (mil)	Density (g/cm ³)	Carbon Black Content (%)	Carbon Black Dispersion	Yield Strength ² (lb/in)	Break Strength ² (lb/in)	Yield Elongation ² (%)	Break Elongation ² (%)	Tear Resistance ² (lb)	Puncture Resistance (lb)
TEST STANDARD	ASTM D 5994	ASTM D 1505	ASTM D 1603	ASTM D 5596	ASTM D 6693	ASTM D 6693	ASTM D 6693	ASTM D 6693	ASTM D 5994	ASTM D 1505	ASTM D 1603	ASTM D 5596	ASTM D 6693	ASTM D 6693	ASTM D 6693	ASTM D 6693	ASTM D 1004	ASTM D 4833
PROJECT SPECS.	≥ 60	≥ 0.94	2 to 3	See Note 3	≥ 130	≥ 72	≥ 12	≥ 100	≥ 60	≥ 0.94	2 to 3	See Note 3	≥ 130	≥ 72	≥ 12	≥ 100	≥ 40	≥ 80
TESTING FREQUENCY	1 per 100,000 ft ² (4)								1 per 50,000 ft ² (4)									

ROLL NUMBER	CQA SAMPLE ID	TEST RESULTS										TEST RESULTS						PASS/FAIL (P/F)	
		Thickness ¹ (mil)	Density (g/cm ³)	Carbon Black Content (%)	Carbon Black Dispersion	Yield Strength ² (lb/in)	Break Strength ² (lb/in)	Yield Elongation ² (%)	Break Elongation ² (%)	Thickness ¹ (mil)	Density (g/cm ³)	Carbon Black Content (%)	Carbon Black Dispersion	Yield Strength ² (lb/in)	Break Strength ² (lb/in)	Yield Elongation ² (%)	Break Elongation ² (%)	Tear Resistance ² (lb)	Puncture Resistance (lb)
115143502	GMS-1	61	0.944	2.51	10	158	305	18	800	60	0.943	2.5	10	150	325	18	834	50	152
115143503										60	0.943	2.5	10	150	343	18	889	50	152
115143505										60	0.943	2.5	10	150	434	18	889	50	152
115143506										60	0.943	2.4	10	150	336	17	859	50	152
115143509	GMS-2	61	0.943	2.34	10	157	307	19	787	61	0.943	2.5	10	151	331	17	842	51	152
115143512										61	0.943	2.1	10	152	339	18	858	50	152
115143515										61	0.943	2.5	10	150	342	18	888	50	150
115143516	GMS-3	62	0.943	2.42	10	158	298	17	764	61									P

Notes:

¹ Thickness was measured for every roll.² Smaller of machine direction (MD) and transverse direction (TD).³ Project specifications for carbon black dispersion are: 8 of 10 in Category 1 or 2 and all in Category 1, 2, or 3. Results are for Category 1 or 2.⁴ A minimum of 1 test per lot was required.Average Roll Area (560 ft x 22.5 ft): 12,600 ft²

No. of Rolls in Lot: 16

Area in Lot: 201,600 ft²

SHEET NO. 1 OF 1

CUMULATIVE NUMBER OF ROLLS: 16

CUMULATIVE AREA: 201,600 ft²

Table 5-6

**NONDESTRUCTIVE SEAM TEST RESULTS FOR SMOOTH GEOMEMBRANE
IN FLEXIBLE STORAGE CONTAINERS**

Panel No. ¹	Seam Length (ft)	Vacuum Box ² Pass/Fail (P/F)	Air Pressure		
			Applied Pressure (psi)	Loss of Pressure ³ (psi)	Pass/Fail (P/F)
BB1-BB2	26	-	30	0	P
BB1-BB14	33	-	30	0	P
BB3-BB1	10	-	30	0	P
BB3-BB14	22	-	30	0	P
BB3-BB2	30	-	30	0	P
BB4-BB14	22	-	30	0	P
BB4-BB3	40	-	30	0	P
BB5-BB14	21	-	30	0	P
BB5-BB4	40	-	30	0	P
BB6-BB14	23	-	30	0	P
BB6-BB5	40	-	30	0	P
BB7-BB14	23	-	30	0	P
BB7-BB17	7	-	30	0	P
BB7-BB18	36	-	30	0	P
BB8-BB14	21	-	30	0	P
BB8-BB7	44	-	30	0	P
BB9-BB8	44	-	30	0	P
BB10-BB9	44	-	30	0	P
BB11-1113	25	-	30	0	P
BB11-BB10	33	-	30	1	P
BB13-BB10	8	-	30	0	P
BB14-BB19	22	-	30	< 3	P
BB17-BB14	67	-	30	0	P
BB18-BB17	21	-	30	0	P
BB19-BB9	23	-	30	1	P
BB19-BB10	23	-	30	1	P
BB22-BB23	22	-	30	0	P
BB22-BB21	22	-	30	0	P
BB24-BB25	32	-	30	0	P
BB25-BB24	144	-	33	0	P
BB25-BB26 A	27	-	31	0	P
BB25-BB26 B	144	-	33	0	P
BB25-BB28	8	-	30	0	P
BB26-BB27	37	-	32	0	P
BB27-BB28	36	-	31	0	P
BB28-BB29	20	-	31	0	P
BB28-BB30	11	-	31	0	P
BB29-BB23	21	-	30	0	P
BB29-BB30 A	86	-	30	0	P
BB29-BB30 B	20	-	31	0	P
BB29-BB31	25	-	31	0	P
BB29-BB48	23	-	30	0	P
BB29-BB22	15	-	30	0	P
BB30-BB22	18	-	30	0	P

Notes:

¹ "BB" and "BT" represent bottom and top of bladders, respectively.

² Using 5 psi gauge suction in the box.

³ Acceptance criterion: Loss of pressure does not exceed 3 psi for at least 5 minutes.

Table 5-6 (continued)

**NONDESTRUCTIVE SEAM TEST RESULTS FOR SMOOTH GEOMEMBRANE
IN FLEXIBLE STORAGE CONTAINERS**

Panel No. ¹	Seam Length (ft)	Vacuum Box ² Pass/Fail (P/F)	Air Pressure		
			Applied Pressure (psi)	Loss of Pressure ³ (psi)	Pass/Fail (P/F)
BB30-BB23	5	-	30	0	P
BB30-BB31 A	86	-	30	0	P
BB30-BB31 B	22	-	32	0	P
BB30-BB48	22	-	30	0	P
BB31-BB21	7	-	30	0	P
BB31-BB32 A	86	-	30	0	P
BB31-BB32 B	48	-	31	0	P
BB31-BB48	22	-	30	0	P
BB32-BB21	23	-	30	0	P
BB32-BB26	21	-	31	0	P
BB32-BB33 A	74	-	30	0	P
BB32-BB33 B	49	-	31	0	P
BB32-BB34	12	-	30	0	P
BB32-BB48	22	-	30	0	P
BB33-BB26	23	-	32	0	P
BB33-BB36	18	-	30	0	P
BB33-BB37	24	-	30	0	P
BB33-BB48	5	P	-	-	-
BB34-BB21	70	-	30	0	P
BB34-BB33	23	-	30	0	P
BB34-BB35	45	-	30	0	P
BB34-BB38	22	-	31	0	P
BB35-BB33	16	-	30	0	P
BB35-BB38	24	-	31	0	P
BB36-BB37	25	-	31	0	P
BB36-BB38	18	-	34	0	P
BB38-BB37	30	-	31	0	P
BB38-BB39	86	-	31	0	P
BB38-BB48	1	P	-	-	-
BB39-BB21	23	-	30	0	P
BB39-BB40	86	-	31	0	P
BB39-BB48	22	-	32	0	P
BB40-BB21	22	-	30	0	P
BB40-BB41	86	-	31	0	P
BB40-BB48	23	-	34	0	P
BB41-BB21	22	-	30	0	P
BB41-BB42	86	-	31	0	P
BB42-BB21	23	-	30	0	P
BB42-BB43	25	-	31	0	P
BB43-BB44	19	-	31	0	P
BB44-BB21	34	-	31	0	P
BB44-BB42	10	-	31	0	P
BB44-BB79	12	-	31	0	P
BB45-BB43	22	-	31	0	P

Notes:

¹ "BB" and "BT" represent bottom and top of bladders, respectively.² Using 5 psi gauge suction in the box.³ Acceptance criterion: Loss of pressure does not exceed 3 psi for at least 5 minutes.

Table 5-6 (continued)

**NONDESTRUCTIVE SEAM TEST RESULTS FOR SMOOTH GEOMEMBRANE
IN FLEXIBLE STORAGE CONTAINERS**

Panel No. ¹	Seam Length (ft)	Vacuum Box ² Pass/Fail (P/F)	Air Pressure		
			Applied Pressure (psi)	Loss of Pressure ³ (psi)	Pass/Fail (P/F)
BB45-BB77	13	-	31	0	P
BB46-BB45	31	-	31	0	P
BB46-BB47	30	-	32	0	P
BB47-BB41	4	P	-	-	-
BB47-BB42	23	-	31	0	P
BB47-BB48	22	-	30	0	P
BB47-BB50	34	-	31	0	P
BB48-BB37	66	-	30	0	P
BB48-BB41	18	-	31	0	P
BB48-BB49 A	132	-	30	0	P
BB48-BB49 B	120	-	30	0	P
BB47-BB49 C	260	-	30	0	P
BB50-BB51	34	-	31	0	P
BB50-BB49	22	-	30	0	P
BB51-BB52	35	-	32	0	P
BB52-BB49	21	-	31	0	P
BB52-BB53	22	-	31	0	P
BB53-BB49	29	-	31	0	P
BB53-BB54	22	-	31	0	P
BB54-BB49	37	-	31	0	P
BB54-BB55	22	-	30	0	P
BB55-BB49	192	-	31	0	P
BB55-BB60	22	-	31	0	P
BB55-BB56	43	-	31	0	P
BB56-BB57	42	-	31	0	P
BB56-BB60	22	-	31	0	P
BB57-BB58	23	-	31	0	P
BB57-BB59	13	-	31	0	P
BB58-BB59	21	-	31	0	P
BB59-BB60	31	-	31	0	P
BB60-BB61 A	35	-	31	0	P
BB60-BB61 B	20	-	31	0	P
BB61-BB55	22	-	31	0	P
BB61-BB62	55	-	31	0	P
BB62-BB63	38	-	31	0	P
BB62-BB55	23	-	31	0	P
BB63-BB67	17	-	31	0	P
BB63-BB64	33	-	31	0	P
BB65-BB64	8	-	31	0	P
BB65-BB66	18	-	31	0	P
BB65-BB68	7	-	31	0	P
BB66-BB64	31	-	31	0	P
BB66-BB67	62	-	29	0	P
BB66-BB68	29	-	31	0	P

Notes:

¹ "BB" and "BT" represent bottom and top of bladders, respectively.

² Using 5 psi gauge suction in the box.

³ Acceptance criterion: Loss of pressure does not exceed 3 psi for at least 5 minutes.

Table 5-6 (continued)

**NONDESTRUCTIVE SEAM TEST RESULTS FOR SMOOTH GEOMEMBRANE
IN FLEXIBLE STORAGE CONTAINERS**

Panel No. ¹	Seam Length (ft)	Vacuum Box ² Pass/Fail (P/F)	Air Pressure		
			Applied Pressure (psi)	Loss of Pressure ³ (psi)	Pass/Fail (P/F)
BB67-BB55 A	10	-	31	0	P
BB67-BB55 B	46	-	31	0	P
BB67-BB55 C	23	-	30	0	P
BB67-BB62	17	-	31	0	P
BB67-BB68	22	-	33	0	P
BB67-BB69	24	-	33	0	P
BB68-BB69	30	-	31	0	P
BB69-BB54	22	-	30	0	P
BB69-BB70	54	-	31	0	P
BB70-BB53	9	-	30	0	P
BB70-BB54	12	-	31	0	P
BB70-BB71	54	-	30	0	P
BB71-BB53	23	-	30	0	P
BB71-BB72	56	-	31	0	P
BB72-BB52	21	-	32	0	P
BB73-BB72	31	-	32	0	P
BB74-BB73	30	-	29	0	P
BB75-BB74	29	-	30	0	P
BB75-BB76	35	-	31	0	P
BB76-BB51	35	-	31	0	P
BB76-BB72 A	21	-	31	0	P
BB76-BB72 B	25	-	32	0	P
BB77-BB46	12	-	31	0	P
BB77-BB78	11	-	31	0	P
BB78-BB43	13	-	31	0	P
BB79-BB43	13	-	31	0	P
BB79-BB78	12	-	31	0	P
BB81-BB48	33	-	31	0	P
BT1-BT2	149	-	33	0	P
BT2-BT3	26	-	32	0	P
BT2-BT5	22	-	30	0	P
BT3-BT4	24	-	30	0	P
BT3-BT5	7	-	30	0	P
BT4-BT5	27	-	30	0	P
BT5-BT6	34	-	30	0	P
BT6-BT2	22	-	30	0	P
BT6-BT7	34	-	30	0	P
BT7-BT2	23	-	30	0	P
BT7-BT8	34	-	30	0	P
BT8-BT2	22	-	30	0	P
BT8-BT9	21	-	31	0	P
BT8-BT11	11	-	31	0	P
BT9-BT10	15	-	31	0	P
BT9-BT11	20	-	31	0	P

Notes:

¹ "BB" and "BT" represent bottom and top of bladders, respectively.

² Using 5 psi gauge suction in the box.

³ Acceptance criterion: Loss of pressure does not exceed 3 psi for at least 5 minutes.

Table 5-6 (continued)

**NONDESTRUCTIVE SEAM TEST RESULTS FOR SMOOTH GEOMEMBRANE
IN FLEXIBLE STORAGE CONTAINERS**

Panel No. ¹	Seam Length (ft)	Vacuum Box ² Pass/Fail (P/F)	Air Pressure		
			Applied Pressure (psi)	Loss of Pressure ³ (psi)	Pass/Fail (P/F)
BT10-BT11	11	-	31	0	P
BT11-BT2	27	-	30	0	P
BT11-BT12	149	-	33	0	P
BT13-BT12	26	-	30	0	P
BT13-BT14	18	-	30	0	P
BT13-BT15	14	-	31	0	P
BT13-BT16	14	-	33	0	P
BT14-BT15	21	-	31	0	P
BT15-BT16	25	-	30	0	P
BT16-BT12	22	-	32	0	P
BT16-BT17	39	-	31	0	P
BT17-BT12	23	-	32	0	P
BT17-BT18	39	-	30	0	P
BT18-BT12	23	-	30	0	P
BT19-BT12	21	-	33	0	P
BT19-BT18	39	-	32	0	P
BT23-BT24	24	-	32	0	P
BT23-BT44	30	-	31	0	P
BT31-BT26	24	-	30	0	P
BT33-BT34	22	-	31	0	P
BT34-BT35	18	-	30	0	P
BT34-BT36	15	-	30	0	P
BT34-BT37	12	-	31	0	P
BT35-BT36	19	-	31	0	P
BT37-BT26	44	-	31	0	P
BT37-BT33	44	-	31	0	P
BT37-BT36	28	-	31	0	P
BT40-BT24	21	-	30	0	P
BT40-BT44	27	-	30	0	P
BT41-BT24	23	-	32	0	P
BT41-BT40	27	-	31	0	P
BT42-BT24	21	-	32	0	P
BT42-BT41	27	-	30	0	P
BT43-BT24	24	-	30	0	P
BT43-BT42	27	-	30	0	P

Notes:

¹ "BB" and "BT" represent bottom and top of bladders, respectively.

² Using 5 psi gauge suction in the box.

³ Acceptance criterion: Loss of pressure does not exceed 3 psi for at least 5 minutes.

Table 5-7

**DESTRUCTIVE SEAM TEST RESULTS FOR SMOOTH GEOMEMBRANE
IN FLEXIBLE STORAGE CONTAINERS**

Sample No.	Panel No.	Weld Type ¹	Peel Strength ² (lb/in)														Shear Strength ³ (lb/in)						Failure Type ⁴	Pass/Fail (P/F)
			Bottom Peel (inside)							Top Peel (outside)														
			121	123	124	126	124	120	116	122	123	124	180	177	173	172	175							
DSB-1	BB7-BB18	F	120	118	124	122	129	118	116	122	122	121	181	173	170	168	173	FTB	P					
DSB-2	BB6-BB14	F	131	127	125	124	127	127	126	123	123	124	169	173	172	170	170	FTB	P					
DSB-3	BB20-BB21	F	130	128	126	125	127	128	128	125	125	126	180	182	182	185	183	FTB	P					
DSB-4	BB32-BB21	F	120	126	129	133	131	118	117	122	119	118	177	173	168	170	171	FTB	P					
DSB-5	BB32-BB31	F	133	126	129	123	126	120	120	121	122	125	168	173	158	163	160	FTB	P					
DSB-6	BB36-BB38	F	140	135	129	138	130	136	127	125	131	131	164	170	165	163	166	FTB	P					
DSB-7	BB48-BB49	F	133	131	148	136	128	130	126	131	132	133	166	164	160	161	169	FTB	P					
DSB-8	BB60-BB61	F	136	124	129	121	136	130	124	120	128	127	173	169	170	170	172	FTB	P					
DSB-9	BB55-BB54	F	126	129	129	130	129	118	126	134	130	117	168	168	173	170	170	FTB	P					
DSB-10	BB70-BB71	F																						

Notes:

1 "F" is fusion and "E" is extrusion weld.

2 Specified peel strength: 78 lb/in for fusion and 70 lb/in for extrusion

3 Specified shear strength: 120 lb/in for fusion and 108 lb/in for extrusion

4 "FTB" is Film Tear Bond (with maximum 10 percent seam separation) and "AD" is adhesion failure (Non-FTB).

Table 5-8A

CQA AND MQC TEST RESULTS FOR PRIMARY GEOCOMPOSITE (TENAX)

	CONSTRUCTION QUALITY ASSURANCE (CQA)							MANUFACTURING QUALITY CONTROL (MQC)			
PROPERTY	GEOTEXTILE					GEOCOMPOSITE		GEOCOMPOSITE			
	Mass per Unit Area (oz/yd ²)	Grab Strength ¹ (lb)	Trapezoidal Tear Strength ¹ (lb)	Apparent Opening Size (mm)	Permittivity (sec ⁻¹)	Transmissivity (m ² /sec)	Peel Strength ² (lb/in)	Transmissivity (m ² /sec)	Peel Strength ² (lb/in)		
TEST STANDARD	ASTM D 5261	ASTM D 4632	ASTM D 4533	ASTM D 4751	ASTM D 4491	ASTM D 4716	ASTM F 904	ASTM D 4716	GRI GC7		
PROJECT SPECS.	≥ 8	≥ 180	≥ 75	≤ 0.21	≥ 0.5	≥ 7.8x10 ⁻³ at 500 psf	≥ 2.6x10 ⁻³ at 13,500 psf	≥ 1	≥ 7.8x10 ⁻³ at 500 psf	≥ 2.6x10 ⁻³ at 13,500 psf	≥ 1
TESTING FREQUENCY	1 per 200,000 ft ² (3)			1 per 500,000 ft ² (3)		1 per 200,000 ft ² (3)		1 per 100,000 ft ² (3)			

GEOCOMPOSITE ROLL NUMBER	LOT NO.	CQA SAMPLE ID	CQA TEST RESULTS								MQC TEST RESULTS			PASS/FAIL (P/F)		
														CQA	MQC	
3506332	35041										7.8x10 ⁻³	4.77x10 ⁻³	3.2		P	
3506338													5.4		P	
3506356													3.5		P	
3506376		PGC-1	9.6	324	97.0	0.11	1.0	8.1x10 ⁻³	5.5x10 ⁻³	1.7				2.9	P	P
3506416										2.1	1.04x10 ⁻²	4.5x10 ⁻³		P	P	
3506417														2.8		P
3506436														3.2		P
3506490		PGC-2	8.8	335	107.0	0.10	1.3	1.0x10 ⁻²	5.6x10 ⁻³	2.6	1.1x10 ⁻²	3.66x10 ⁻³		P	P	P
3506528										2.1	8.55x10 ⁻³	5.82x10 ⁻³		P	P	P
3506531													3.2		P	P
3506549													4.4		P	P
3506560		PGC-3						7.9x10 ⁻³							P	
3506561			8.7	284	107.0	0.14	1.5	6.8x10 ⁻³	6.2x10 ⁻³	1.5					F ⁴	
3506562															F ⁴	
3506563									7.7x10 ⁻³						F ⁴	
3506564															F ⁴	
3506565									9.9x10 ⁻³							P
3506568											9.28x10 ⁻³	5.22x10 ⁻³	3.2		P	
3506587													4.3		P	
3506606													4.6		P	
3506611											8.65x10 ⁻³	3.29x10 ⁻³			P	
3506624													5.0		P	
3506642		PGC-4	9.1	300	101.0			1.2x10 ⁻²	6.4x10 ⁻³	3.9	9.33x10 ⁻³	2.85x10 ⁻³		P	P	
3506643													4.2		P	
3506662											7.93x10 ⁻³	4.48x10 ⁻³	3.5		P	
3506692											8.32x10 ⁻³	6.76x10 ⁻³			P	
3506699													2.6		P	
3506717 ⁵															P	
3506719													3.3		P	
3507016	35043	PGC-5	8.2	287	112.0	0.13	1.5	7.3x10 ⁻³	6.8x10 ⁻³	1.9	8.69x10 ⁻³	3.77x10 ⁻³	2.0	P ⁶	P	
3507035													1.9		P	
3507036													2.6		P	

Notes:

- ¹ Smaller value in machine and cross-machine directions.
- ² Smaller of top and bottom peel strength.
- ³ A minimum of 1 test per lot was required.
- ⁴ Additional tests performed to bracket failing roll. Rolls rejected between passing bracket rolls are shown.
- ⁵ This roll was used to perform interface friction tests.
- ⁶ The hydraulic transmissivity under compressive stress of 500 psf was accepted because the average of CQA and MQC tests was 8.0x10⁻³ m²/sec.

Average Roll Area (200 ft x 12.5 ft): 2,500 ft²
 Total No. of Rolls: 358
 Total Area of Rolls: 895,000 ft²

Table 5-8B

**MQC TEST RESULTS FOR GEOTEXTILE USED TO MANUFACTURE
PRIMARY GEOCOMPOSITE (TENAX)**

PROPERTY	Mass per Unit Area (oz/yd ²)	Grab Strength ¹ (lb)	Trapezoidal Tear Strength ¹ (lb)	Puncture Strength (lb)	Apparent Opening Size (mm)	Permittivity (sec ⁻¹)
TEST STANDARD	ASTM D 5261	ASTM D 4632	ASTM D 4533	ASTM D 4833	ASTM D 4751	ASTM D 4491
PROJECT SPECS.	≥ 8	≥ 180	≥ 75	≥ 75	≤ 0.21	≥ 0.5
TESTING FREQUENCY	1 per 100,000 ft ²				1 per 250,000 ft ²	

GEOTEXTILE ROLL NUMBER	LOT NO.	TEST RESULTS						PASS/FAIL (P/F)
3012141	35041	9.4	285	116	159	0.09	1.3	P
3012149		8.7	366	113	150	0.10	1.0	P
3012153		8.4	349	119	135			P
3012157		8.1	268	113	144	0.11	1.3	P
3012161		8.2	274	137	148			P
3012163		8.4	277	120	159	0.09	1.1	P
3012167		8.6	284	120	146			P
3012171		8.2	291	111	150	0.13	1.2	P
3012180		8.6	271	102	141	0.12		P
3012182		8.2	309	117	145	0.11	1.0	P
3012186		8.4	267	91	149			P
3012190		8.8	263	87	156	0.09	1.0	P
3012195		8.7	293	104	143	0.09	0.8	P
3012199		9.0	291	149	147	0.08	1.1	P
3012203		8.7	293	101	148		1.4	P
3012207		8.6	280	112	153	0.08	1.2	P
3012215		8.4	281	114	156	0.10	1.1	P
3012219		8.2	286	115	159		1.1	P
3012223		8.5	293	117	156	0.10	1.3	P
3012227		8.4	280	110	152			P
3012231		8.0	239	101	147	0.09	1.4	P
3012238		8.2	287	117	142			P
3012241		8.2	255	102	141	0.09	1.1	P
3012245		8.4	298	110	152			P
3012246		8.1	291	127	148	0.09	1.2	P
3012250		8.4	303	116	144			P
3013888	35043	8.2	307	124	148	0.08	1.2	P

Note:

1 Smaller value in machine and cross-machine directions.

Total Area of Geotextile Rolls: 1,790,000 ft²

No. of Geotextile Rolls Tested: 27 / 16

Test Frequency: 1 per 66,296 / 111,875 ft²

Table 5-8C

**MQC TEST RESULTS FOR GEONET USED TO MANUFACTURE
PRIMARY GEOCOMPOSITE (TENAX)**

PROPERTY	Polymer Density (g/cm ³)	Carbon Black Content (%)	Thickness (mil)
TEST STANDARD	ASTM D 1505	ASTM D 1603	ASTM D 5199
PROJECT SPECS.	≥ 0.93	2 to 3	≥ 200
TESTING FREQUENCY	1 per 100,000 ft ²		

GEONET ROLL NUMBER	LOT NO.	TEST RESULTS			PASS/FAIL (P/F)
3302796	35041	0.947	2.2	337	P
3302815		0.951	2.5	369	P
3302795		0.948	2.1	347	P
3302825		0.949	2.3	345	P
3302900		0.948	2.5	355	P
3302903		0.949	2.5	360	P
3302901		0.951	2.5	353	P
3302889		0.955	2.7	380	P
3302902		0.956	2.9	362	P
3302915		0.952	2.5	336	P
3302909		0.953	2.8	335	P
3302925		0.948	2.4	330	P
3302921		0.952	2.5	359	P
3302931		0.947	2.4	360	P
3302937		0.949	2.3	385	P
3302944		0.945	2.7	333	P
3302950		0.953	2.6	336	P
3302955		0.948	2.7	345	P
3302960		0.953	2.7	354	P
3302961		0.948	2.7	364	P
3302962		0.953	2.7	341	P
3302968		0.945	2.7	332	P
3302975		0.952	2.4	333	P
3302976		0.947	2.6	331	P
3302978		0.954	2.6	325	P
3302984		0.953	2.5	332	P
3302990		0.949	2.5	340	P
3302996		0.947	2.5	352	P
3303002		0.947	2.5	355	P
3303100	35043	0.950	2.5	346	P
3303106		0.950	2.4	358	P
3303109		0.949	2.9	351	P

Area of Geonet Rolls: 895,000 ft²

No. of Geonet Rolls Tested: 32

Test Frequency: 1 per 27,969 ft²

Table 5-9A

CQA AND MQC TEST RESULTS FOR SECONDARY GEOCOMPOSITE (GSE)

PROPERTY	CONSTRUCTION QUALITY ASSURANCE (CQA)						MANUFACTURING QUALITY CONTROL (MQC)			
	GEOTEXTILE					GEOCOMPOSITE		GEOCOMPOSITE		
	Mass per Unit Area (oz/yd ²)	Grab Strength ¹ (lb)	Trapezoidal Tear Strength ¹ (lb)	Apparent Opening Size (mm)	Permittivity (sec ⁻¹)	Transmissivity (m ² /sec)	Peel Strength ² (lb/in)	Transmissivity (m ² /sec)	Peel Strength ² (lb/in)	
TEST STANDARD	ASTM D 5261	ASTM D 4632	ASTM D 4533	ASTM D 4751	ASTM D 4491	ASTM D 4716	ASTM F 904	ASTM D 4716	GRI GC7	
PROJECT SPECS.	≥ 8	≥ 180	≥ 75	≤ 0.21	≥ 0.5	≥ 5.0x10 ⁻⁴ at 500 psf	≥ 1.5x10 ⁻⁴ at 13,500 psf	≥ 1	≥ 5.0x10 ⁻⁴ at 500 psf	≥ 1.5x10 ⁻⁴ at 13,500 psf
TESTING FREQUENCY	1 per 200,000 ft ² (3)			1 per 500,000 ft ² (3)		1 per 200,000 ft ² (3)		1 per 100,000 ft ² (3)		

GEOCOMPOSITE ROLL NUMBER	LOT NO.	CQA SAMPLE ID	TEST RESULTS									TEST RESULTS			PASS/FAIL (P/F)	
															CQA	MQC
131131239 ⁴	CB13080403	SGC-1	8.4	281	95.0	0.15	1.3	8.4x10 ⁻⁴	1.5x10 ⁻⁴	3.2				P		
131131245												1.12x10 ⁻³	2.06x10 ⁻⁴	1.0		P
131132511	RPG120092											1.70x10 ⁻³	6.37x10 ⁻⁴	1.1		P
131132551												1.87x10 ⁻³	5.83x10 ⁻⁴	2.2		P
131132591												1.31x10 ⁻³	4.24x10 ⁻⁴	2.0		P
131132594		SGC-2	14.1	284	112.0	0.12	1.5	2.3x10 ⁻³	8.1x10 ⁻⁴	2.2					P	
131132631	RPG120091											1.63x10 ⁻³	1.77x10 ⁻⁴	1.7		P
131132670		SGC-3	14.4	267	106.0	0.13	1.4	2.3x10 ⁻³	6.7x10 ⁻⁴	2.5					P	
131132671												1.69x10 ⁻³	3.51x10 ⁻⁴	2.2		P
131132711												1.16x10 ⁻³	3.3x10 ⁻⁴	3.2		P
131132750		SGC-4	9.0	267	100.0	-	-	1.4x10 ⁻³	5.8x10 ⁻⁴	2.9	1.26x10 ⁻³	2.64x10 ⁻⁴	2.3	P	P	
131132790												1.27x10 ⁻³	6.15x10 ⁻⁴	2.4		P
131132830		SGC-5	8.2	267	98.0	-	-	1.8x10 ⁻³	5.3x10 ⁻⁴	2.2	1.79x10 ⁻³	3.04x10 ⁻⁴	2.5	P	P	
<div>Notes:</div> <div><div>¹ Smaller value in machine and cross-machine directions.</div><div>² Smaller of top and bottom peel strength.</div><div>³ A minimum of 1 test per lot was required.</div><div>⁴ This roll was used to perform interface friction tests.</div></div>																
Average Roll Area (170 ft x 14.5 ft):			2,465 ft ²													
Total No. of Rolls:			360													
Total Area of Rolls:			887,400 ft ²													

Table 5-9B

**MQC TEST RESULTS FOR GEOTEXTILE USED TO MANUFACTURE
SECONDARY GEOCOMPOSITE (GSE)**

PROPERTY	Mass per Unit Area (oz/yd ²)	Grab Strength ¹ (lb)	Trapezoidal Tear Strength ¹ (lb)	Puncture Strength (lb)	Burst Strength (lb/in ²)	Apparent Opening Size (mm)	Permittivity (sec ⁻¹)
TEST STANDARD	ASTM D 5261	ASTM D 4632	ASTM D 4533	ASTM D 4833	ASTM D 3786	ASTM D 4751	ASTM D 4491
PROJECT SPECS.	≥ 8	≥ 180	≥ 75	≥ 75	≥ 350	≤ 0.21	≥ 0.5
TESTING FREQUENCY	1 per 100,000 ft ²					1 per 250,000 ft ²	

GEOTEXTILE ROLL NUMBER	TEST RESULTS								PASS/FAIL (P/F)
130154602	8.2	258	128	148	454	0.18	2.0		P
130154606	8.2	275	142	172	480	0.18	2.0		P
130154609	8.8	281	141	179	494	0.18	2.0		P
130154613	8.5	277	133	163	482	0.18	1.8		P
130154617	9.1	288	189	184	522	0.18	1.8		P
130154622	8.4	261	156	151	486	0.18	1.8		P
130154629	8.6	275	158	178	473	0.18	1.6		P
130154645	8.5	276	124	157	476	0.18	1.8		P
130154823	8.7	265	120	147	456	0.18	2.1		P
130154828	8.3	258	122	155	426	0.18	2.1		P
130154832	8.1	250	127	162	460	0.18	2.1		P
130154837	8.6	263	144	155	470	0.18	2.1		P
130154842	8.8	261	139	176	486	0.18	2.1		P
130154843	8.9	286	144	171	476	0.18	2.1		P
130154854	8.2	243	165	167	412	0.18	1.9		P
130155191	9.0	304	144	150	506	0.18	2.1		P
130155195	8.7	295	149	174	480	0.18	2.1		P
130155198	8.0	264	130	151	416	0.18	2.1		P
130155202	8.6	266	126	163	428	0.18	1.9		P
130155206	8.6	291	115	162	444	0.18	1.9		P
130155210	8.2	258	117	140	470	0.18	1.9		P
130155214	8.7	278	128	159	498	0.18	2.0		P
130155220	8.3	263	118	149	440	0.18	2.0		P
Note: ¹ Smaller value in machine and cross-machine directions.									

Table 5-9B (continued)

**MQC TEST RESULTS FOR GEOTEXTILE USED TO MANUFACTURE
SECONDARY GEOCOMPOSITE (GSE)**

PROPERTY	Mass per Unit Area (oz/yd ²)	Grab Strength ¹ (lb)	Trapezoidal Tear Strength ¹ (lb)	Puncture Strength (lb)	Burst Strength (lb/in ²)	Apparent Opening Size (mm)	Permittivity (sec ⁻¹)
TEST STANDARD	ASTM D 5261	ASTM D 4632	ASTM D 4533	ASTM D 4833	ASTM D 3786	ASTM D 4751	ASTM D 4491
PROJECT SPECS.	≥ 8	≥ 180	≥ 75	≥ 75	≥ 350	≤ 0.21	≥ 0.5
TESTING FREQUENCY	1 per 100,000 ft ²					1 per 250,000 ft ²	

GEOTEXTILE ROLL NUMBER	TEST RESULTS								PASS/FAIL (P/F)
130155224	8.6	267	130	147	454	0.18	2.0		P
130155228	8.2	264	126	147	436	0.18	2.0		P
130155230	8.3	277	118	145	466	0.18	2.0		P
130155234	8.3	284	136	147	464	0.18	1.8		P
130155238	8.1	269	109	139	460	0.18	2.0		P
130155242	8.5	272	120	148	518	0.18	2.0		P
130155246	8.4	270	117	159	478	0.18	2.0		P
130155250	8.4	271	111	149	434	0.18	2.0		P
130155254	8.5	262	122	168	466	0.18	2.0		P
130155258	8.3	268	112	160	479	0.18	2.0		P
130155262	8.0	268	119	145	442	0.18	2.3		P
130155267	8.4	258	138	164	480	0.18	2.1		P
130155270	8.0	274	148	170	426	0.18	2.1		P
130155275	8.2	268	149	163	470	0.18	2.1		P
130155279	8.3	254	144	150	454	0.18	2.0		P
130155282	8.3	247	100	161	452	0.18	2.0		P
130155286	8.3	267	101	158	450	0.18	2.0		P
130155339	8.4	264	112	167	460	0.18	2.0		P
130155344	8.7	264	128	183	429	0.18	2.0		P

Note:

¹ Smaller value in machine and cross-machine directions.Total Area of Geotextile Rolls: 1,774,800 ft²

No. of Geotextile Rolls Tested: 42

Test Frequency: 1 per 42,257 ft²

Table 5-9C

**MQC TEST RESULTS FOR GEONET USED TO MANUFACTURE
SECONDARY GEOCOMPOSITE (GSE)**

PROPERTY	Polymer Density (g/cm ³)	Carbon Black Content (%)	Thickness (mil)
TEST STANDARD	ASTM D 1505	ASTM D 1603	ASTM D 5199
PROJECT SPECS.	≥ 0.93	2 to 3	≥ 200
TESTING FREQUENCY	1 per 100,000 ft ²		

GEONET ROLL NUMBER	LOT NO.	TEST RESULTS			PASS/FAIL (P/F)	
131131239	CB13080403	0.961	2.2	290	P	
131131246		0.961	2.4	291	P	
131131256		0.961	2.4	286	P	
131131266		0.961	2.4	287	P	
131132511	RPG120092	0.961	2.2	310	P	
131132512		0.961	2.3	303	P	
131132522		0.961	2.4	311	P	
131132532		0.959	2.3	300	P	
131132542		0.961	2.6	301	P	
131132552		0.961	2.6	297	P	
131132562		0.961	2.5	308	P	
131132572		0.961	2.4	302	P	
131132582		0.961	2.2	302	P	
131132592		0.961	2.3	305	P	
131132602		0.960	2.4	312	P	
131132612		0.961	2.4	306	P	
131132622	RPG120091	0.961	2.4	314	P	
131132632		0.961	2.5	301	P	
131132642		0.961	2.4	306	P	
131132652		0.960	2.3	302	P	
131132662		0.961	2.3	309	P	
131132672		0.960	2.7	304	P	
131132682		0.961	2.3	300	P	
131132692		0.961	2.6	301	P	
131132702		0.960	2.5	305	P	
131132712		0.960	2.3	305	P	
131132722		0.961	2.5	311	P	
131132731		0.961	2.5	307	P	
131132741		0.960	2.3	307	P	
131132751		0.960	2.9	303	P	
131132761		0.961	2.3	305	P	
131132771		0.961	2.3	308	P	
131132781		0.961	2.8	311	P	
131132791		0.961	2.4	303	P	
131132801		0.961	2.2	294	P	
131132811		0.961	2.9	302	P	
131132821		0.961	2.5	304	P	
131132831		0.961	2.6	304	P	
131132841		0.961	2.5	304	P	
131132857		0.960	2.4	307	P	
Area of Geonet Rolls: 887,400 ft ²						
No. of Geonet Rolls Tested: 40						
Test Frequency: 1 per 22,185 ft ²						

Table 5-10

CQA AND MQC TEST RESULTS FOR NON-WOVEN GEOTEXTILE (GSE)

LOT NO: 20059427

CONSTRUCTION QUALITY ASSURANCE (CQA)									MANUFACTURING QUALITY CONTROL (MQC)						
PROPERTY	Mass per Unit Area (oz/yd ²)	Grab Strength ¹ (lb)	Trapezoidal Tear Strength ¹ (lb)	Puncture Strength (lb)	Burst Strength (lb/in ²)	Apparent Opening Size (mm)	Permittivity (sec ⁻¹)	Mass per Unit Area (oz/yd ²)	Grab Strength ¹ (lb)	Trapezoidal Tear Strength ¹ (lb)	Puncture Strength (lb)	Burst Strength (lb/in ²)	Apparent Opening Size (mm)	Permittivity (sec ⁻¹)	
TEST STANDARD	ASTM D 5261	ASTM D 4632	ASTM D 4533	ASTM D 4833	ASTM D 3786	ASTM D 4751	ASTM D 4491	ASTM D 5261	ASTM D 4632	ASTM D 4533	ASTM D 4833	ASTM D 3786	ASTM D 4751	ASTM D 4491	
PROJECT SPECS.	≥ 8	≥ 180	≥ 75	≥ 75	≥ 350	≤ 0.21	≥ 0.5	≥ 8	≥ 180	≥ 75	≥ 75	≥ 350	≤ 0.21	≥ 0.5	
TESTING FREQUENCY	1 per 200,000 ft ²						1 per 500,000 ft ²	1 per 100,000 ft ²						1 per 250,000 ft ²	

ROLL NUMBER	CQA SAMPLE ID	CQA TEST RESULTS								MQC TEST RESULTS					
130146805	GF-1	8.5	284	101	145	460	0.12	1.5	8.2	236	106	144	431	0.18	2.1
130146811									8.5	257	105	156	430	0.18	2.1
130146869									8.4	248	116	149	432	0.18	1.4
130147130									8.0	221	101	129	386	0.18	1.7
130154657									8.4	248	124	168	460	0.18	2.0

Note: 1 Smaller value in machine and cross-machine directions.

Total No. of Rolls:	5
Total Area of Rolls:	36,090 ft ²

Table 5-11A

MINIMUM INTERFACE SHEAR STRENGTHS REQUIRED BY THE CQA DOCUMENTS

Normal Stress (psf)	Shear Strength (psf)
2,005	429
5,013	787
10,025	1,143
15,038	1,594

Table 5-11B

INTERFACE SHEAR STRENGTHS FROM CQA TESTING

Normal Stress (psf)	(1) Protective Layer Soil / Tri-Planar Geocomposite		(2) Tri-Planar / Textured Geomembrane		(3) Textured Geomembrane / GCL		(4) GCL / Bi-Planar Geocomposite		(5) Bi-Planar / Textured Geomembrane		(6) GCL / Subbase Soil		(7) GCL Internal Strength	
	Peak Shear Strength (psf)	Residual Shear Strength (psf)	Peak Shear Strength (psf)	Residual Shear Strength (psf)	Peak Shear Strength (psf)	Residual Shear Strength (psf)	Peak Shear Strength (psf)	Residual Shear Strength (psf)	Peak Shear Strength (psf)	Residual Shear Strength (psf)	Peak Shear Strength (psf)	Residual Shear Strength (psf)	Peak Shear Strength (psf)	Residual Shear Strength (psf)
5,000	3,714	3,458	2,905	1,902	2,810	1,381	3,057	1,824	2,750	1,738	2,807	2,293	3,300	1,190
10,000	7,056	6,700	5,025	3,035	4,522	1,839	4,637	1,658	5,177	2,870	4,320	2,166	4,863	1,763
15,000	10,177	9,776	7,352	4,410	5,768	2,112	5,874	1,872	7,265	4,334	5,865	2,022	6,197	2,383

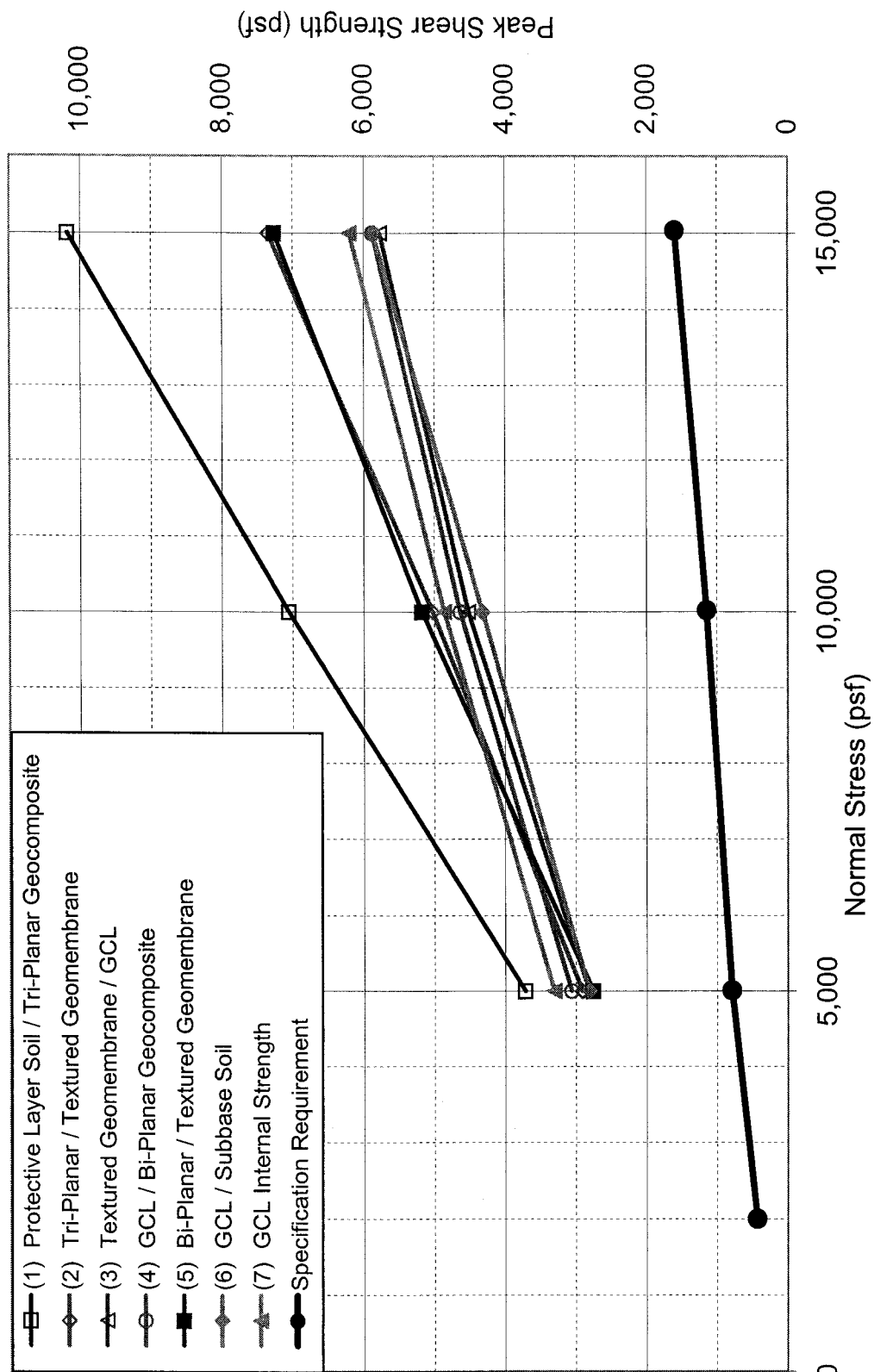


Figure 5-1: Peak Shear Strengths from Interface Friction Tests

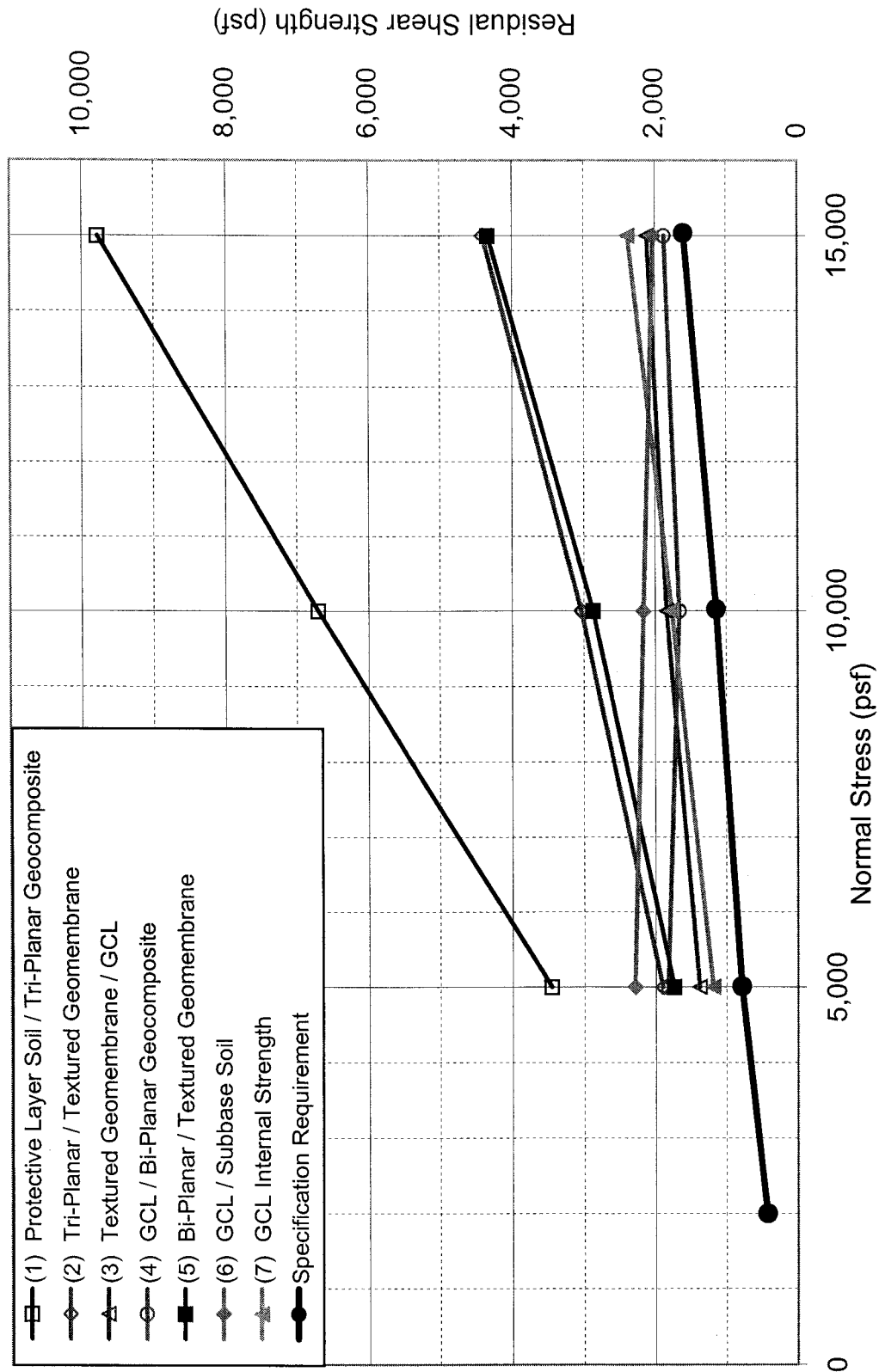


Figure 5-2: Residual Shear Strengths from Interface Friction Tests

6. CONSTRUCTION QUALITY ASSURANCE -- LEACHATE MANAGEMENT SYSTEM

6.1 General

The leachate management system is comprised of the leachate collection, transmission, and storage systems. The construction of the initial leachate management system for Phase 1 development of the OHDF landfill included construction of leachate collection system in Cell 1A, Cell 1 sump, leachate transmission system for Phase 1, and the initial leachate storage area.

The leachate collection system in Cell 1A consists of a primary and a secondary leachate collection system. The primary leachate collection system included 6-in diameter SDR 11 HDPE perforated leachate collection pipe surrounded by gravel aggregate and geotextile filter fabric at the toe of the perimeter berm on the north and the west sides of Cell 1A. The leachate collection pipe was provided with two rows of 1/2-in perforations in the bottom 1/3 of the pipe section. Granular drainage materials meeting the requirements of #57 and #4 stone (per ASTM D 448) were used as the gravel aggregate. An 8-oz/yd² needle-punched, non-woven geotextile was used as the filter fabric. Four cleanouts were installed along the inside slope of the perimeter dike in Cell 1A to maintain the leachate collection system piping. The cleanouts were constructed using 6-in diameter SDR 11 HDPE pipe and were finished with a blind flange. The secondary leachate collection system included an additional 6-ft wide secondary geocomposite layer at the toe of the perimeter berm on the north and west sides of Cell 1A.

The Cell 1 sump included gravel beds covered with geotextile separator fabric and three sump risers. The gravel beds for the primary and secondary sumps were separated by the primary liner system. Granular drainage materials meeting the requirements for #4 stone (per ASTM D 448) were used in the 1.5-ft to 3-ft thick drainage beds. The drainage beds were separated from the overlying liner protective layer by an 8-oz/yd² needle-punched, non-woven geotextile separator fabric. Two primary and one secondary sump risers were installed in the Cell 1 sump. The sump risers were constructed using 48-in diameter SDR 32.5 HDPE pipe and included a 2-in thick base plate and a bolted top lid. Each sump riser was provided with perforations in the bottom 1-ft and a 20-ft long, 8-in diameter, SDR 11 HDPE perforated collection pipe. The primary sump risers were connected to the secondary sump riser using a 3-in diameter SDR 11 HDPE overflow pipe with a 3-in gate valve in accordance with the CQA Documents. The Cell 1 sump area included a 2-ft thick compacted low permeability soil layer as discussed in Section 4. The geosynthetics in the sump area were installed as discussed in Section 5.

The Phase 1 leachate transmission system included an 8-inch diameter SDR 11 HDPE leachate transmission pipe from the Cell 5 sump area (south of Cell 4) to the leachate storage area (south of Cell 3). Seven manholes were installed along the leachate transmission line adjacent to the sump area of Cells 1 through 6 and Cell 9. The leachate transmission manholes were constructed using 48-in diameter SDR 32.5 HDPE pipe and included a 2-in thick base plate and a bolted top lid. The leachate transmission manholes included pipe stub-outs (for future connection to the cell sump risers) and valves (to allow isolation of the cell sump areas or leachate transmission line segments, as needed) in accordance with the CQA Documents.

The initial leachate storage area includes four flexible storage containers constructed using 60-mil thick smooth HDPE geomembrane. Two of these four flexible storage containers (#1 and #4) are complete at this time. The construction of the remaining two flexible storage containers (#2 and #3) will be completed in conjunction with the completion of Cell 1B. The flexible leachate storage containers were/are being constructed within a double-lined containment area as discussed in Section 2. The geosynthetics in the sump area were installed as discussed in Section 5. The construction of the leachate storage area also included a leachate truck load out pad with associated leachate transfer pumps, instrumentation, and piping system. Leak detection and toe drains were also installed in each of the four compartments of the leachate storage area in accordance with the CQA Documents.

Leachate from Cell 1A will be collected in the leachate collection system at the toe of the perimeter dike and will gravity flow to the Cell 1 sump. Leachate will be pumped from the sump risers through the leachate transmission line to the leachate storage area. Leachate from the leachate storage area will be loaded and trucked off-site to a waste water treatment plant for proper treatment and disposal.

To control the pumping and transfer of leachate, two control panels were installed as part of the leachate management system for the operation of Cell 1A. These control panels included sump control panel at the Cell 1 sump area and the leachate transfer control panel at the truck load-out pad. A third control panel will be installed during the completion of Cell 1B. This third panel will provide additional sensors and alarms for leachate levels in the flexible storage containers and controls for the automated valves used for routing leachate flow based on these levels.

GeoSyntec's CQA personnel monitored the construction of the leachate management system. The field monitoring and testing activities performed by the CQA personnel during construction of the leachate management system are discussed below.

6.2 HDPE Pipe

All pipes used in the construction of the leachate management system were SDR 11 HDPE pipes except for the sump risers and leachate transmission manholes, which were constructed using SDR 32.5 HDPE pipes. The MQC certificates for the HDPE pipes were reviewed by the CQA personnel and were found to be in compliance with the requirements of the CQA Documents.

HDPE pipe sections were joined using butt-fusion welding techniques. CQA personnel periodically monitored the butt-fusion welding techniques to ensure that industry-accepted procedures were used during construction. CQA personnel also verified the diameter of and perforation details (size, number of rows, orientation) for the various pipes used in the leachate management system and periodically monitored the installation of the various pipes to ensure compliance with the requirements of the CQA Documents. Record Drawing presenting the as-built location of the leachate transmission line is included in Appendix A

6.3 Granular Drainage Materials

Granular drainage materials meeting the requirements of #57 and/or #4 stone (per ASTM D 448) were used in Cell 1A leachate collection system, Cell 1 sump, and the leak detection drains in the leachate storage area. The #57 and #4 stones were supplied by Martin Marietta in Shorters, Alabama.

The QC certificates and test results and the CQA conformance test results were reviewed by CQA personnel and were found to be in compliance with the CQA Documents. The results of the QC and CQA conformance tests are summarized in Table 6-1. Table 6-1 presents the tests conducted, test frequencies, and the acceptance criteria in accordance with the CQA Documents. In addition to the tests presented in Table 6-1, a carbonate content test (per ASTM D 3042) was also performed on the granular drainage materials during the QC testing. The #57 and #4 stones used in construction of the leachate management system were found to be insoluble to 6N hydrochloric acid used in the tests.

CQA personnel periodically monitored the placement of the granular drainage material to ensure (i) the underlying geosynthetics were not damaged; (ii) the perforated pipes were properly surrounded by the drainage materials; and (iii) the drainage materials were placed in accordance with the requirements of the CQA Documents.

6.4 Pressure Testing

The leachate transmission line was pressure tested to detect any leaks or defective pipe joints. The hydrostatic pressure testing was performed in 2 segments. The first

segment included the leachate transmission line from MH-5 (manhole adjacent to Cell 5 sump area) to MH-1 (manhole adjacent to Cell 1 sump area). The second segment included the leachate transmission line from MH-1 to the leachate storage area.

The hydrostatic pressure testing was performed in two phases. In the first phase, the leachate transmission line segment was filled with water and the internal pressure was increased to 130 psi. This hydrostatic pressure was maintained for at least 3 hours to allow expansion of the pipe line. The second/test phase was performed for at least 1 hour. No drop in the hydrostatic pressure was allowed or observed during the second test phase.

During final grading of the outer slope of the perimeter dike on the east side of Cell 2, the leachate transmission line was accidentally damaged. The damaged length of the leachate transmission pipe was replaced using flange connections on both sides. The location of the repair is indicated on the Record Drawing for the leachate transmission line included in Appendix A. After completion of the repair, the leachate transmission line segment between manholes MH-2 and MH-3 was retested using the procedure discussed above.

6.5 Sump Pumps and Control Panel

Leachate collected in the leachate sumps will be extracted and pumped to the leachate storage area by two 5-hp electric submersible pumps located in the primary sump risers and one 3-hp electric submersible pump located in the secondary sump riser. The pumps are controlled by a control panel located at the Cell 1 sump near the top of the sump risers. The sump pumps and the associated control panel were provided by the EPG Companies (EPG) in Maple Grove, MN.

The following tests will be performed after installation of the pumps and control system at the Cell 1 sump area to confirm proper operation of the pumps and pump control panel. The testing is expected to be performed on 22 January 2004.

- Sump pumps tested in place by flooding the Cell 1 sump. Each pump will be connected to a piping assembly containing a pressure gauge, meter valve, and a flow meter. The pumps will be turned on and the pressure and flow rate of each pump will be recorded. The pressure and flow rate data for each pump will be compared to the pump curves provided by EPG.
- Float switches in each of the sump risers will be activated by hand to confirm that the switches are activating the correct pumps and alarms.

- The system will be tested to confirm that a manual signal from the leachate storage area is being received to shut down the Cell 1 sump pumps. A control panel to automatically indicate that all four storage containers are full will be installed during construction of Cell 1B.

6.6 Leachate Storage Area Control Panel

Control for the automatic valves located in the leachate storage area is provided by a control panel located at the truck load out station and a series of level switches located in each storage container. The leachate storage area control panel is currently being assembled. It is expected to be delivered to the site by early February 2004 and installed by end of February 2004.

It is noted that the Cell 1A can be safely operated without the leachate storage control panel because a manual cut-off switch is being installed (in addition to the controls indicated in the CQA Documents) in the leachate storage area to turn-off all pumps in the Cell 1 sump from the leachate storage area.

The control panel for the leachate storage area is being assembled by EPG. Level switches, and leak detection switches will also be supplied by EPG. The control panel; associated power and control conduit; and pipe valves and actuators will be supplied and installed by Comanco. Conductors for power and control will be supplied and installed by Curren Electric Co. in Okeechobee, Florida.

The level switches and leak detection switches in each storage area will be tested by immersing the switches in a tank of water. Each set point (LSL, LSH, LSHH) on the level switches will be marked on the cabling prior to immersion to confirm that set points are set accurately. As each level switch is immersed in water, GeoSyntec will confirm the response at the appropriate valve actuator and alarms.

Once the individual responses are verified, all four high level switches will be activated simultaneously to confirm that the pump disable signal is being sent to and received by the sump pump control panel and alarms.

The low level condition on all four pumps will also be activated to confirm the pump disable signal to the truck load out pumps.

6.7 Truck Load-Out Control Panel

Leachate stored in the leachate storage area will be loaded onto trucks using two 5-hp self-priming pumps. Pump operation is controlled by a separate control panel that will be tied into the leachate storage area control panel. The pumps and the control panel for the truck load out area were supplied by Barney's Pumps in Lakeland, Florida.

Operation of the leachate pump-out system will be confirmed by starting each pump to confirm that its direction of rotation is correct and there are no leaks in the piping system. The automated shut-off system will be tested to confirm that the signal from the leachate storage area indicating that all four low level switches are activated, will shut down the truck loading pumps when the leachate storage area control panel is installed. The operational testing of the leachate pump-out system is expected to be performed on 22 January 2004.

Table 6-1

CQA AND MQC TEST RESULTS FOR GRANULAR DRAINAGE MATERIAL

		PARTICLE SIZE ANALYSIS								SOIL CLASSIFICATION	HYDRAULIC CONDUCTIVITY	
TEST STANDARD		ASTM D 422								ASTM D 2847	ASTM D 2434	
TESTING FREQUENCY		1 test per 2,000 yd ³								1 test per 2,000 yd ³	1 test per 3,000 yd ³	
TEST RESULTS												
	Sample No.	Stone #	Percent Passing by Weight Through U.S. Standard Sieve							USCS Classification	Hydraulic Conductivity ¹ (cm/s)	Pass/Fail (P/F)
			3-in	2-in	1.5-in	1.0-in	3/4-in	3/8-in	#4			
CQA ²	SS-2	57	100.0	100.0	100.0	99.0	69.0	3.0	0.0	GP	31	P
	I-45	4	100.0	100.0	81.0	10.0	2.0	1.0	0.0	GP	74	P
MQC		57	-	-	100.0	82.8	-	0.7	0.4	GP	31	P
		4	-	100.0	79.7	11.7	0.9	0.2	-	GP	52	P
Notes:												
1 Hydraulic conductivity greater than or equal to 1 cm/sec and 10 cm/sec was required for # 57 and # 4 stones, respectively.												
2 CQA conformance tests were performed by Excel Geotechnical Testing.												

7. CONSTRUCTION QUALITY ASSURANCE - OTHER CONSTRUCTION ACTIVITIES

7.1 Ground Water Monitoring Wells

Forty five groundwater monitoring wells were installed in 15 clusters (MW-1 through MW-15) around the Phase 1 development of the OHDF landfill in accordance with the CQA Documents. Three monitoring wells (Type "A", Type "B", and Type "C") were installed at each cluster location.

Groundwater monitoring well clusters MW-1 through MW-6 and MW-8 through MW-14 were installed by Environmental Drilling Services, Orlando, Florida. Groundwater monitoring well clusters MW-7 and MW-15 were installed by Prosonic Corporation, Fort Lauderdale, Florida. GeoSyntec monitored the construction and performed the development and sampling of the groundwater monitoring wells. It is noted that a separate report is being prepared for groundwater monitoring installation, development, and sampling. The report will be submitted to FDEP after the laboratory testing of the groundwater samples is completed. Therefore, only a brief description of the groundwater monitoring wells is included herein.

Groundwater monitoring wells MW-1 through MW-13 were installed along the outer edge of the perimeter berm from approximate Elevation 92 feet, NGVD. Groundwater monitoring wells MW-14 and MW-15 were installed in the perimeter berm on south side of the Phase 1 development, as indicated in the CQA Documents, from approximate Elevation 84 feet, NGVD. All wells had 10-ft screened sections with the depth of the top of screen as discussed below:

- *Type "A" Shallow Wells:* The depth of top of screen was approximately 10 ft below existing (post-construction) grade at monitoring well clusters MW-1 through MW-13 and approximately 2.5 ft below existing grade at monitoring well clusters MW-14 and MW-15.
- *Type "B" Intermediate Wells:* The depth of top of screen was approximately 35 ft below existing grade at monitoring well clusters MW-1 through MW-13 and approximately 25 ft below existing grade at monitoring well clusters MW-14 and MW-15.
- *Type "C" Deep Wells:* The depth of top of screen was approximately 55 to 60 ft below existing grade at monitoring well clusters MW-1 through MW-13 and approximately 50 ft below existing grade at monitoring well clusters MW-14 and MW-15.

7.2 Ground Water Monitoring Well Abandonment

As part of the original site investigation during the permit process, several piezometers were installed within the footprint of the OHDF, landfill. Two of these piezometers, DP-1 and DP-2, were within the footprint of Cell 1 (see Figure 2 in Appendix E of the Solid Waste Permit Application). These 2 piezometers were abandoned during the construction of Cell 1. The abandonment of the piezometers included removing the steel casing around the piezometers; grouting the piezometers from the bottom to the ground surface using a tremie pipe; and cutting the well casing 1-ft below the ground surface.

7.3 Culvert Crossings

A total of five culvert crossings were constructed along the site access road in accordance with the CQA Documents with two exceptions. The wetland crossings at Sta. 85+00 and Sta. 96+00, which were originally designed with a 24-in and a 12-in diameter corrugated HDPE pipe, respectively, were both constructed using a 24-in diameter reinforced concrete pipe (RCP). Thus, all five culvert crossings along the site access road were constructed using RCPs.

The RCP for the culvert crossings were supplied by Rinker Materials in Apopka, Florida. The MQC certificates were reviewed by the CQA personnel and were found to be in compliance with the CQA Documents.

7.4 Storm Water Structures Along Site Access Road

Twenty-four storm water structures were constructed along the site access road in accordance with the CQA Documents with two exceptions. First, the storm water structures included 2 ft by 2 ft pre-cast concrete structures (with a slotted cast iron grate on top) in lieu of the 18-in diameter corrugated HDPE pipe structure indicated on Sheet 50 of 50 of the Permit Drawings. The number and diameter of the orifices in and the height of the pre-cast concrete storm water structures used in construction, are presented in Table 7-1. Second, the invert elevations of some of the storm water structures were adjusted due to the conditions encountered in the field. The invert elevations of the storm water structures used during construction are also presented in Table 7-1.

The pre-cast concrete storm water structures were manufactured by Mack Concrete Industries in Astatula, Florida. The Advanced Drainage Systems, Inc. N-12 corrugated HDPE pipes were used for the 12-in diameter outfall pipes. The MQC certificates supplied by the manufacturers were reviewed by the CQA personnel and were found to be in compliance with the CQA Documents.

7.5 Storm Water Drainage Structures

Five storm water drainage structures were installed along the perimeter berm around Phase 1 development of the OHDF landfill in accordance with the CQA Documents. These structures will be used as part of the storm water management system after installation of the final cover system (see Sheet 36 of 50 of the Permit Drawings).

The pre-cast concrete storm water drainage structures were manufactured by Mack Concrete Industries in Astatula, Florida. The MQC certificates supplied by the manufacturer were reviewed by the CQA personnel and were found to be in compliance with the CQA Documents.

7.6 Utilities

Power and a phone lines were installed from U.S. Hwy 441 to the administrative area of the OHDF landfill. The power and phone lines to the administrative area are active at this time.

The overhead power lines were installed by Progress Energy. The power lines run along the north side of the site access road up to Bull Creek, cross the access road near Bull Creek, and run along the south side of the site access road from Bull Creek to the administrative area.

The installation of power lines around the landfill (i.e., to leachate storage area and sump areas of cells in Phase 1) are being installed by Irby Construction of Casselberry, Florida, a subcontractor to Progress Energy. The installation of power lines around the landfill commenced on 4 January 2004 and is expected to be completed by end of February 2004. Therefore, at the start of operation of the OHDF landfill, the pumps at the Cell 1 sump and at the leachate storage area will be operated using portable electric generators.

The below grade phone line was installed by Team Fischel. The phone line runs approximately 4 ft from the edge of the future pavement along the north side of the site access road. The phone is buried approximately 2.5 ft below the existing grade. At the locations of the culvert crossings along the site access road, the phone line was typically installed approximately 7 ft below the invert of the pipes using horizontal directional drilling. At the culvert crossing at Sta. 143+74 (i.e., the dry retention area to wet retention area crossing) the phone line was installed approximately 1 ft below the invert of the pipes.

7.7 Limerock Base Course for Access and Haul Roads

Limerock was placed as the base course on the site access road and the landfill haul roads (including the haul road to the leachate storage area and the cell access road). A 14-in and 12-in thick layer of limerock base course was placed and compacted on top of the site access and the landfill haul roads subbase, respectively. The access road will be paved at a later time.

The limerock for the project was supplied by Bedrock Resources, Inc. in Ocala, Florida from a FDOT certified mine. CQA personnel periodically monitored the placement and compaction of the limerock base course and ensured compliance with the CQA Documents. The limerock was placed on the access and haul roads in two lifts and compacted in accordance with the CQA Documents. CQA personnel measured the density and percent compaction of the limerock base course at a frequency of one test per 200 linear feet per lane per lift in accordance with the CQA Documents. The results of the density tests along the site access and landfill haul roads are not included in this certification report but will be made available upon request.

7.8 Farm Fence

FDOT Type A farm fence was installed on both sides of the site access road, along the property boundary on east and north sides of the landfill footprint, and around Borrow Area A. The farm fence along the property boundary and around Borrow Area A was tied into the existing farm fence on the site to provide a continuous fence around the site access road, landfill footprint, and Borrow Area A. At 3 locations along the site access road, gates were provided on both sides of the site access road to serve as cattle crossings.

The farm fence was installed by Carroll Fencing in Kissimmee, Florida. CQA personnel periodically monitored the installation of the farm fence and ensured compliance with the CQA Documents.

7.9 Fire and Mosquito Control Ponds

The fire pond adjacent to the administrative area and the mosquito control pond in the dry retention basin on north side of the OHDF landfill footprint were excavated by Lundquist in accordance with the CQA Documents. CQA personnel periodically monitored the excavation of the fire and mosquito control ponds and ensured compliance with the CQA Documents.

7.10 Emergency Spillway

The emergency spillway along the storm water management berm on east side of the OHDF landfill footprint was constructed by Lundquist in accordance with the CQA Documents. CQA personnel periodically monitored the construction of the emergency spillway and ensured compliance with the CQA Documents.

Table 7-1

STORM WATER STRUCTURES ALONG SITE ACCESS ROAD

Structure ID	Invert El. (ft NGVD)	Top of Riser El. (ft NGVD)	Height of Riser (ft)	Height of Back-splash (in)	Number of Structures	Number of 6-in Dia. Orifices		Number of 7-in Dia. Orifices	
						Each	Total	Each	Total
NN1	77.7	80.95	3.25	0	1	0	0	3	3
NS1	78.4	81.65	3.25	0	1	1	1	0	0
NN2	77.1	78.35	1.25	4	1	1	1	0	0
NS2	77.3	78.55	1.25	4	1	1	1	0	0
NN3	76.2	77.45	1.25	4	1	0	0	3	3
NS3	76.2	77.45	1.25	4	1	0	0	3	3
NN4	74.6	75.85	1.25	4	1	2	2	0	0
NS4	74.6	75.85	1.25	4	1	2	2	0	0
NN5	74.2	75.45	1.25	4	1	1	1	0	0
NS5	74.5	75.75	1.25	4	1	1	1	0	0
NN6	74.0	75.25	1.25	4	1	3	3	0	0
NS6	74.1	75.35	1.25	4	1	3	3	0	0
NN7	74.0	75.25	1.25	4	2	2	4	0	0
NS7	74.0	75.25	1.25	4	2	2	4	0	0
NN8	71.5	72.70	1.25	4	2	2	4	0	0
NS8	71.5	72.70	1.25	4	2	2	4	0	0
NN9	71.2	72.45	1.25	4	2	3	6	0	0
NS9	71.1	72.35	1.25	4	2	3	6	0	0

8. CONCLUSIONS

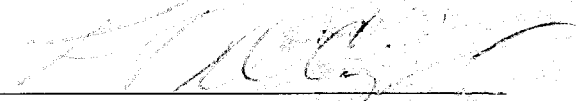
Observation of the construction of Cell 1A and associated facilities at the OHDF was performed by GeoSyntec during the period of 26 June 2003 to 19 January 2004. During this time, CQA personnel monitored the installation of the following components of Cell 1A and the associated facilities:

- earthwork (landfill perimeter berm, storm water management system, Cell 1A subgrade and intercell berm, Cell 1 sump area, low permeability soil layer in sumps areas, liner protective layer, leachate storage area, site access road and landfill haul roads, administration and future use areas, and other ancillary construction);
- geosynthetics in Cell 1A and the leachate storage area;
- leachate collection, transmission, and storage system (Cell 1A leachate collection system, Cell 1 sump, Phase 1 leachate transmission system, and leachate storage area with truck load-out pad).
- other construction including groundwater monitoring wells, culvert crossings and storm water structures along site access road, storm water drainage structures, site utilities, limerock base course on access and haul roads, facility fence, and other ancillary construction.

During construction of the above components, CQA personnel verified that performance and conformance testing was performed at the frequencies required by the CQA Documents and that the installation met or exceeded the requirements of the CQA Documents. CQA personnel also verified that conditions or materials identified as not conforming to the CQA Plan were replaced, repaired, and/or retested, as described in the report.

The results of the CQA activities undertaken by GeoSyntec as described in this report indicate that Cell 1A and associated facilities at the OHDF landfill were constructed in accordance with the CQA Documents and the solid waste permit.


Kirk Wills
Field Services Manager


Kenneth W. Cargill, P.E.
Project Manager, CQA Engineer-of-Record

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GEOSYNTEC CONSULTANTS
Photographic Record

Client: Omni Waste of Osceola County, LLC

Project Number:

FX 0521

Project Name: Oak Hammock Disposal Facility – Cell 1A and Leachate Storage Area

Project Location: Near Holopaw in Osceola County, Florida

Photograph 1

Date: June 27, 2003

Direction:

Comments:

Access road with silt
fence.



Photograph 2

Date: June 25, 2003

Direction: East

Comments:

Silt fence installed to
protect wetland area.
North side of access road.



GEOSYNTEC CONSULTANTS
Photographic Record

Client: Omni Waste of Osceola County, LLC

Project Number:

FX 0521

Project Name: Oak Hammock Disposal Facility – Cell 1A and Leachate Storage Area

Project Location: Near Holopaw in Osceola County, Florida .

Photograph 3

Date: July 1, 2003

Direction:

Comments:

Palmetto plow in Borrow
Area A.



Photograph 4

Date: June 25, 2003

Direction: South-West

Comments:

Stripping wetland area
within limits of access
road construction.



GEOSYNTEC CONSULTANTS
Photographic Record

Client: Omni Waste of Osceola County, LLC

Project Number: FX 0521

Project Name: Oak Hammock Disposal Facility – Cell 1A and Leachate Storage Area

Project Location: Near Holopaw in Osceola County, Florida

Photograph 5

Date: July 3, 2003

Direction: East

Comments:

Excavation of rim ditch
in Borrow Area A.



Photograph 6

Date: Aug 22, 2003

Direction: North-West

Comments:

Dozer shaping top and
outer north wall of
Borrow Area A.



GEOSYNTEC CONSULTANTS
Photographic Record

Client: Omni Waste of Osceola County, LLC

Project Number:

FX 0521

Project Name: Oak Hammock Disposal Facility – Cell 1A and Leachate Storage Area

Project Location: Near Holopaw in Osceola County, Florida

Photograph 7

Date: July 2, 2003

Direction: North-West

Comments:

Pan stripping at north-west berm area.



Photograph 8

Date: July 9, 2003

Direction: South

Comments:

Tractor with pans constructing storm water berm.



GEOSYNTEC CONSULTANTS
Photographic Record

Client: Omni Waste of Osceola County, LLC

Project Number:

FX 0521

Project Name: Oak Hammock Disposal Facility – Cell 1A and Leachate Storage Area

Project Location: Near Holopaw in Osceola County, Florida

Photograph 9

Date: July 14, 2003

Direction: North-East

Comments:

Subcontractor (BUL-HED) placing material for storm water management berm at Sta. 108+00.



Photograph 10

Date: July 2, 2003

Direction:

Comments:

Dozer tracking storm water management berm material.



GEOSYNTEC CONSULTANTS
Photographic Record

Client: Omni Waste of Osceola County, LLC

Project Number: FX 0521

Project Name: Oak Hammock Disposal Facility – Cell 1A and Leachate Storage Area

Project Location: Near Holopaw in Osceola County, Florida

Photograph 11

Date: July 17, 2003

Direction:

Comments:

Access road earthwork.



Photograph 12

Date: July 16, 2003

Direction:

Comments:

Access road earthwork.



GEOSYNTEC CONSULTANTS
Photographic Record

Client: Omni Waste of Osceola County, LLC

Project Number: FX 0521

Project Name: Oak Hammock Disposal Facility – Cell 1A and Leachate Storage Area

Project Location: Near Holopaw in Osceola County, Florida

Photograph 13

Date: July 22, 2003

Direction:

Comments:

Subgrade compaction for
access road and storm
water berms.



Photograph 14

Date: Aug 26, 2003

Direction: West

Comments:

Dump trucks placing
materials in haul road.



GEOSYNTEC CONSULTANTS
Photographic Record

Client: Omni Waste of Osceola County, LLC

Project Number:

FX 0521

Project Name: Oak Hammock Disposal Facility – Cell 1A and Leachate Storage Area

Project Location: Near Holopaw in Osceola County, Florida

Photograph 15

Date: Aug 27, 2003

Direction:

Comments:

Placement and
compaction of materials
in Cell 1A.



Photograph 16

Date: Aug 22, 2003

Direction: South-West

Comments:

Sod placement along
outer slope of storm
water berm of Borrow
Area A.



GEOSYNTEC CONSULTANTS
Photographic Record

Client: Omni Waste of Osceola County, LLC

Project Number:

FX 0521

Project Name: Oak Hammock Disposal Facility – Cell 1A and Leachate Storage Area

Project Location: Near Holopaw in Osceola County, Florida

Photograph 17

Date: 15 December 2003

Direction:

Comments: Installation
of primary liner over
GCL in Cell 1A.



Photograph 18

Date: 9 December 2003

Direction:

Comments:
Geosynthetics installation
in Cell 1A.



GEOSYNTEC CONSULTANTS
Photographic Record

Client: Omni Waste of Osceola County, LLC

Project Number:

FX 0521

Project Name: Oak Hammock Disposal Facility – Cell 1A and Leachate Storage Area

Project Location: Near Holopaw in Osceola County, Florida

Photograph 19

Date: 9 December 2003

Direction:

Comments: Compaction
of low-permeability layer
in Cell 1 sump.



Photograph 20

Date: 10 December 2003

Direction:

Comments: Liner
installation in Cell 1
sump.



GEOSYNTEC CONSULTANTS
Photographic Record

Client: Omni Waste of Osceola County, LLC

Project Number:

FX 0521

Project Name: Oak Hammock Disposal Facility – Cell 1A and Leachate Storage Area

Project Location: Near Holopaw in Osceola County, Florida

Photograph 21

Date: 13 December 2003

Direction:

Comments: Cell 1 sump
riser.



Photograph 22

Date: 13 December 2003

Direction:

Comments: Construction
of Cell 1 sump.



GEOSYNTEC CONSULTANTS
Photographic Record

Client: Omni Waste of Osceola County, LLC

Project Number:

FX 0521

Project Name: Oak Hammock Disposal Facility – Cell 1A and Leachate Storage Area

Project Location: Near Holopaw in Osceola County, Florida

Photograph 23

Date: 28 November 2003

Direction:

Comments: Secondary
liner installation in
leachate storage area.



Photograph 24

Date: 2 December 2003

Direction:

Comments: Leak
detection drain in
leachate storage area.



GEOSYNTEC CONSULTANTS
Photographic Record

Client: Omni Waste of Osceola County, LLC

Project Number:

FX 0521

Project Name: Oak Hammock Disposal Facility – Cell 1A and Leachate Storage Area

Project Location: Near Holopaw in Osceola County, Florida

Photograph 25

Date: 2 December 2003

Direction:

Comments: Placement of
liner protective layer in
leachate storage area.



Photograph 26

Date: 13 December 2003

Direction:

Comments: Placement of
primary liner in leachate
storage area.



GEOSYNTEC CONSULTANTS
Photographic Record

Client: Omni Waste of Osceola County, LLC

Project Number:

FX 0521

Project Name: Oak Hammock Disposal Facility – Cell 1A and Leachate Storage Area

Project Location: Near Holopaw in Osceola County, Florida

Photograph 27

Date: 28 November 2003

Direction:

Comments: Extrusion
welding.



Photograph 28

Date: 28 November 2003

Direction:

Comments: Vacuum box
test for extrusion welds.



GEOSYNTEC CONSULTANTS
Photographic Record

Client: Omni Waste of Osceola County, LLC

Project Number: FX 0521

Project Name: Oak Hammock Disposal Facility – Cell 1A and Leachate Storage Area

Project Location: Near Holopaw in Osceola County, Florida

Photograph 29

Date: 13 December 2003

Direction:

Comments: Double track
fusion welding.

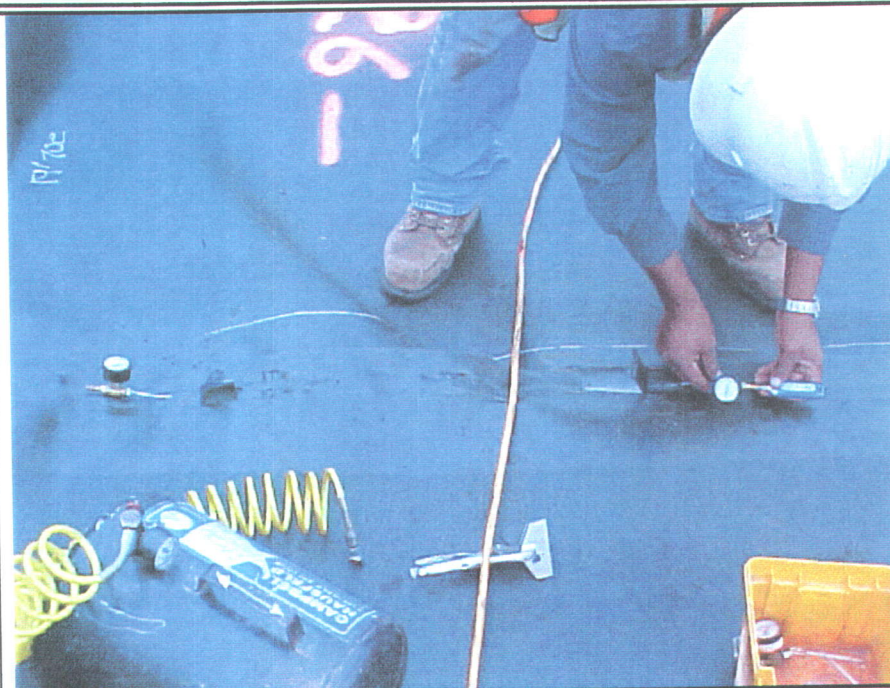


Photograph 30

Date: 10 December 2003

Direction:

Comments: Air pressure
test for fusion weld.



GEOSYNTEC CONSULTANTS
Photographic Record

Client: Omni Waste of Osceola County, LLC

Project Number:

FX 0521

Project Name: Oak Hammock Disposal Facility – Cell 1A and Leachate Storage Area

Project Location: Near Holopaw in Osceola County, Florida

Photograph 31

Date: 2 December 2003

Direction:

Comments: Liner
installation and repairs.



Photograph 32

Date: 6 December 2003

Direction:

Comments: Leachate
transmission manhole
and power pull/junction
box.



GEOSYNTEC CONSULTANTS
Photographic Record

Client: Omni Waste of Osceola County, LLC

Project Number:

FX 0521

Project Name: Oak Hammock Disposal Facility – Cell 1A and Leachate Storage Area

Project Location: Near Holopaw in Osceola County, Florida

Photograph 33

Date: 23 December 2003

Direction:

Comments:
Moisture/density
evaluation using nuclear
gauge.

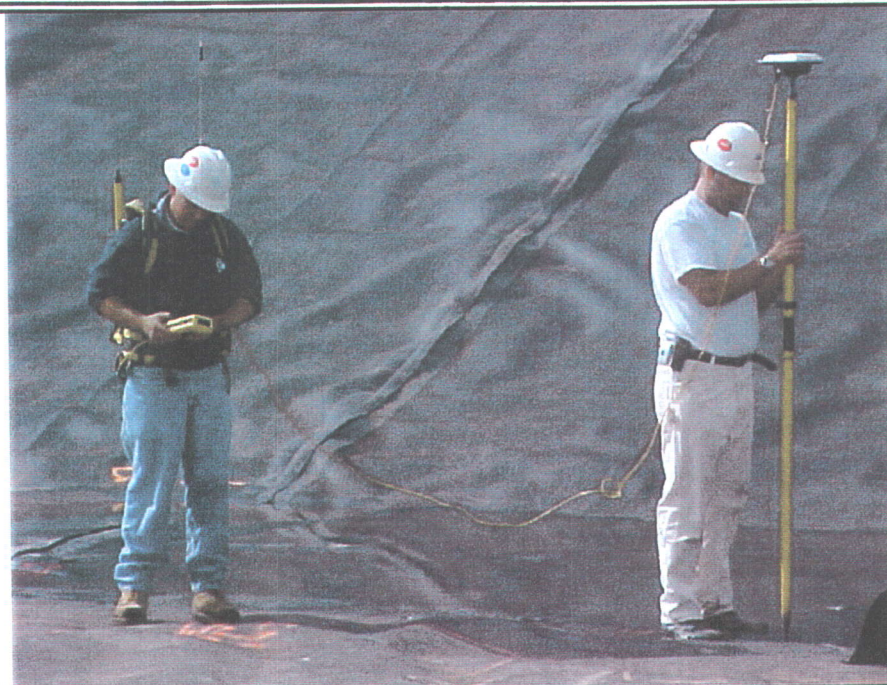


Photograph 34

Date: 23 December 2003

Direction:

Comments: As-built
survey of liner by CQA
personnel.



GEOSYNTEC CONSULTANTS
Photographic Record

Client: Omni Waste of Osceola County, LLC

Project Number:

FX 0521

Project Name: Oak Hammock Disposal Facility – Cell 1A and Leachate Storage Area

Project Location: Near Holopaw in Osceola County, Florida

Photograph 35

Date: 23 December 2003

Direction:

Comments: Development
of groundwater
monitoring wells.



Photograph 36

Date: 21 November 2003

Direction:

Comments: Construction
of truck scales.



GEOSYNTEC CONSULTANTS
Photographic Record

Client: Omni Waste of Osceola County, LLC

Project Number:

FX 0521

Project Name: Oak Hammock Disposal Facility – Cell 1A and Leachate Storage Area

Project Location: Near Holopaw in Osceola County, Florida

Photograph 37

Date: 11 December 2003

Direction:

Comments: Construction
of site utilities.



Photograph 38

Date: 11 December 2003

Direction:

Comments: Construction
of site utilities.

