November 7, 2018

Mr. Steve Morgan Florida Department of Environmental Protection – Southwest District Air & Solid Waste Permitting Manager Permitting and Waste Cleanup Program 13051 Northeast Telecom Parkway, Suite 101 Temple Terrace, FL 33637-0926

RE: Enterprise Recycling and Disposal Facility Cell 16 Construction Completion Report Response to 10/30/18 RAI Permit No.: 177982-023-SC/T3, Pasco County WACS No.: 87895

Dear Mr. Morgan:

As requested, we have addressed the items listed in your October 30, 2018 correspondence herein. The response follows each of the items listed in your letter request. We have elected to provide a complete revised report to the Department, which is enclosed.

CELL 6 CONSTRUCTION COMPLETION CERTIFICATION REPORT

Please provide the following additional information and revisions to this Construction Completion Certification Report. Please either provide a complete revised report or provide replacement pages with revisions noted (deletions may be struckthrough [struckthrough] and additions may be underlined [underlined] or a similar method may be used) and each page numbered with the document title and date of revision. The report will be reviewed in its entirety upon receipt of the request information and revisions.

RESPONSE: The additional information and revisions are provided herein. Any replacement pages have been edited to show the revisions (deletions identified with strikethrough and additions underlined).

October 15, 2018 Cover Letter

1. **Item g.**: Documentation of the installation of monitoring wells MW-5AR and MW-5BR provided to the Department by Locklear and Associates on May 10, 2018. Please revise the documentation as indicated:

a. The Installation of Monitoring Wells, Angelo's Recycled Materials – Enterprise Class III Landfill letter dated May 9, 2018 indicated that the subject wells (MW-5BR & MW-5BR), are scheduled to be surveyed "*at the next availability*." To date, the Department has not received the survey data for these wells. Per Subparagraph 62-701.510(3)(d)1., F.A.C. please provide survey data conducted by Florida Licensed Professional Surveyor and Mapper for the replacement monitoring wells. The survey should include the location of the well, horizontally

located in degrees, minutes and seconds of latitude and longitude, and the elevation of the top of the well casing and ground surface by the well casing to the nearest 0.01 foot, using an industry approved datum. b. Figures 2 & 3:

- 1) Revise figure title to remove "Proposed".
- 2) Update facility name to "Enterprise Recycling & Disposal Facility"
- 3) The figure indicates December 2016 was the date it was revised; please update to the current revision date.
- 4) The figure indicates that measurement "D" is from the top of the well cap (not casing) and extends to the top of the concrete pad (not existing grade/ground surface). Likewise, measurement "B" is shown as extending from the top of the concrete pad to the bottom of the PVC casing. Please verify that measurements "B" and "D" are from the <u>concrete pad</u> and <u>well cap</u> (measurement "D") and not from <u>existing grade/ground surface</u> and the <u>top of well casing</u> and revise figure as applicable.
- 5) Column "D" of the included table indicates only 3-<u>inches</u> of casing stickup. Please verify or revise as applicable.
- 6) The following columns of the included table need to be updated with actual elevations extrapolated from the survey data and boring field logs: a. Top of Screen Elevation
 - b. Bottom of Screen Elevation
 - c. Ground surface Elevation
 - d. Limestone Surface Elevation
- 7) The "Notes" provided on these figures appear to not be applicable to the installed monitoring wells MW-5AR and MW-5BR. Please revise the notes as applicable.

RESPONSE: The requested changes have been made to the MW-5AR and MW-5BR documentation with the exception of the items related to the survery information. Locklear & Associates has scheduled the wells to be surveyed and will provide copies of updated documentation to the Department under separate cover. Copies of the revised documentation are provided in Attachment D.

Attachment A – Certification of Construction Completion Form 62-701.900(2)

2. **Deviations from Plans and Application Approved by DEP**: It appear that there were deviations in construction of Cell 16 not described in this section of the form. Based on your response to comments below regarding the construction completion report, please revise the narrative provided for this section of the form, as appropriate.

RESPONSE: The construction completion form and narrative report have been revised to include the deviations in construction.

<u> Attachment B – Record Drawings</u>

3. Cell 16 Top of Clay:

a. An-built survey of the clay perimeter berm is not provided on this drawing and does not appear to have been provided in the certification report. Please verify and provide an as-built survey of the clay perimeter berm.

RESPONSE: Drawing AB-02 has been revised to include the as-built conditions of the clay perimeter berm.

b. The top of clay elevations provided on this drawing appear to be inconsistent with the survey elevations for Cell 16 presented on the Pickett October 2018 topographic survey. Please verify and explain this apparent discrepancy.

RESPONSE: We discussed the apparent elevation discrepancies with Mr. Jeff Young with Pickett Surveying and Associates, Inc. Mr. Young's explanation of the apparent differences between the data sets is provided in Attachment E. The apparent differences between the Pickett October 2018 survey and the spot elevations recorded during construction are within the acceptable tolerances.

Attachment C – Engineer of Record Narrative Report

4. **Clay Layer Construction**:

a. The clay CQA testing information provided in Attachment D indicate that only one inplace density test and one permeability test was conducted for the entire perimeter berm, rather than one test for each constructed lift of the berm, consistent with the cell floor construction and testing procedures. Please explain.

RESPONSE: Permeability testing was performed on the "3' clay layer" (cell floor) per Appendix 3.2.a of the Operations Permit. The 3' clay layer extends approximately half way beneath the clay perimeter berm and was included in the cell floor testing program. Consistent with the testing associated with the certification of Cells 6 and 7, an additional permeability test was performed on the clay perimeter berm. All of the clay used for the cell floor and perimeter berm came from the same source and was installed using the same means and methods and is expected to yield them same installed maximum hydraulic permeability requirement of 1x10-8cm/sec.

5. Leachate Pipe and Wet Well:

a. The record drawing provided in Attachment B did not include as-built drawings of the constructed leachate collection trench and piping system and the constructed wet well, pump, and float control system. Please verify and provide as-built drawings of these systems.

RESPONSE: Drawing C4.00 of the plan set has been included in Attachment B to show the as-built conditions of those systems.

b. Based on the narrative description in this section and a comparison of the leachate trench bottom elevations provided on the Top of Clay as-built drawing in Appendix B and the

leachate collection pipe elevations provided on the table titled "Cell 16 Pump Station and Leachate Collection Pipe As-Built Elevations", it appears that the leachate collection pipe was installed directly on the top of clay in the leachate collection trench rather than on top of an installed 3-inch aggregate layer as specified on the "Toe Drain" construction detail on Drawing C0.04 of the Permit Drawings for facility. A review of inspection photos taken during a February 2018 DEP facility operation compliance inspection seems to confirm that the leachate collection pipe was installed in this manner [see below]. Of particular concern with this installation deviation is that the clay will be in direct contact with and block the lower holes in perforated pipe. Please verify, explain, and provide supporting information demonstrating that the leachate collection system will function adequately under this construction condition and justification for this construction deviation.

RESPONSE: The leachate collection pipe was not installed directly on the clay and was constructed in accordance with the TOE DRAIN DETAIL shown on drawing C0.04. In the photo shown below, additional #4 gravel was placed over the constructed toe drain to protect it from UV degradation and subsequent waste placement.

Additional photos have been added to the construction completion report to document that the leachate pipe was installed within the geofabric. Additional commentary has also been added to the Narrative Report.

c. The "Toe Drain" construction detail on Drawing C0.04 of the Permit Drawing also specifies that the leachate pipe is surrounded by graded aggregate and then a non-woven filter fabric is installed over the aggregate. The inspection photo below and a photo of the leachate collection trench during construction provided at the end of the "Project Photographs" included in this report appear to depict a different installation configuration (i.e. installation of aggregate over no-woven filter fabric. Please verify this apparent construction deviation and based on the information provided on the as-built drawings for the leachate collection trench and piping system provided in response to Comment 5.a. above, provide support justification for the deviation if applicable.



RESPONSE: Please refer to the response directly above. The exposed gravel in this picture is the additional gravel that was used to cover the completed toe drain. The exposed pipe shown in the 02/09/18 photograph shows the approximate 30' long section of non-perforated pipe that leads from the end of the toe drain trench to the perimeter berm and then continues to the wetwell. This photograph was taken prior to that section of the trench being backfilled with clay. This section of pipe has since been backfilled with clay in substantial accordance with drawing C0.04

d. The easternmost top of clay elevation in the leachate trench reported on the "Top of Clay" as-built drawing in Appendix B is 74.90 ft. NGVD. The as-built invert elevation of the leachate collection pipe at the wet well [STM-1] is reported as 74.95 ft. NGVD. Based on the apparent installation of the leachate collection pipe directly on top of the installed clay layer in the leachate trench described in Comment #5.b. and shown on the photo above, it appears that gravity flow in that section of the solid leachate pipe will flow away from the wet well. Please verify this apparent construction deviation and based on the information provided on the asbuilt drawings for the leachate collection trench and piping system provided in response to Comment 5.a. above, provide supporting justification why the leachate collection system will function properly under these conditions.

RESPONSE: The difference between the two reported elevations is .05' or approximately ½" and is not expected to have any measurable influence on the leachate conveyance system. Using a vertical tolerance of +/- 1/16' (0.063') the elevation at the wetwell could be determined

to be as low as 74.95 - 0.063 = 74.89' and other elevation could be determined to be 74.90 + 0.063' = 74.96', showing a positive slope towards the wetwell.

e. Rule 62-701.500(8)(h), F.A.C. requires that "new leachate collection systems shall be water pressure cleaned or inspected by video recording after construction but prior to initial placement of waste". Please provide documentation that this has occurred included that results of jet cleaning and/or video inspection that verify the system was constructed and is functioning as designed to convey leachate to the wet well.

RESPONSE: The system was pressure cleaned by Jetclean and their report is included in the construction Narrative Report.

f. Please provide the supporting information utilized for CQA verification that the leachate collection system aggregate used on this project met the required material specifications.

RESPONSE: The gradation test for the #4 gravel used in the leachate collection system is included in the Narrative Report.

g. Please revise this section to describe the CQA activities conducted to verify that the constructed leachate level float control system in the wet well is functioning as designed.

RESPONSE: This section of the CQA has been revised to document the CQA activities conducted to verify that the constructed leachate level float control system in the wet well is functioning as designed.

6. Field Inspection, Review, Conformance Assessment, and Major Deviations:

a. This section indicates that the capacity of the leachate collection pipe was increased by increasing the proposed diameter of the pipe from 6" to 8". The design of the permitted leachate wet well was based on the calculated capacity of the designed and permitted 6" leachate collection pipe. Please provide supporting documentation that the increased capacity of the collection pipe can be accommodated by the constructed wet well and pump system.

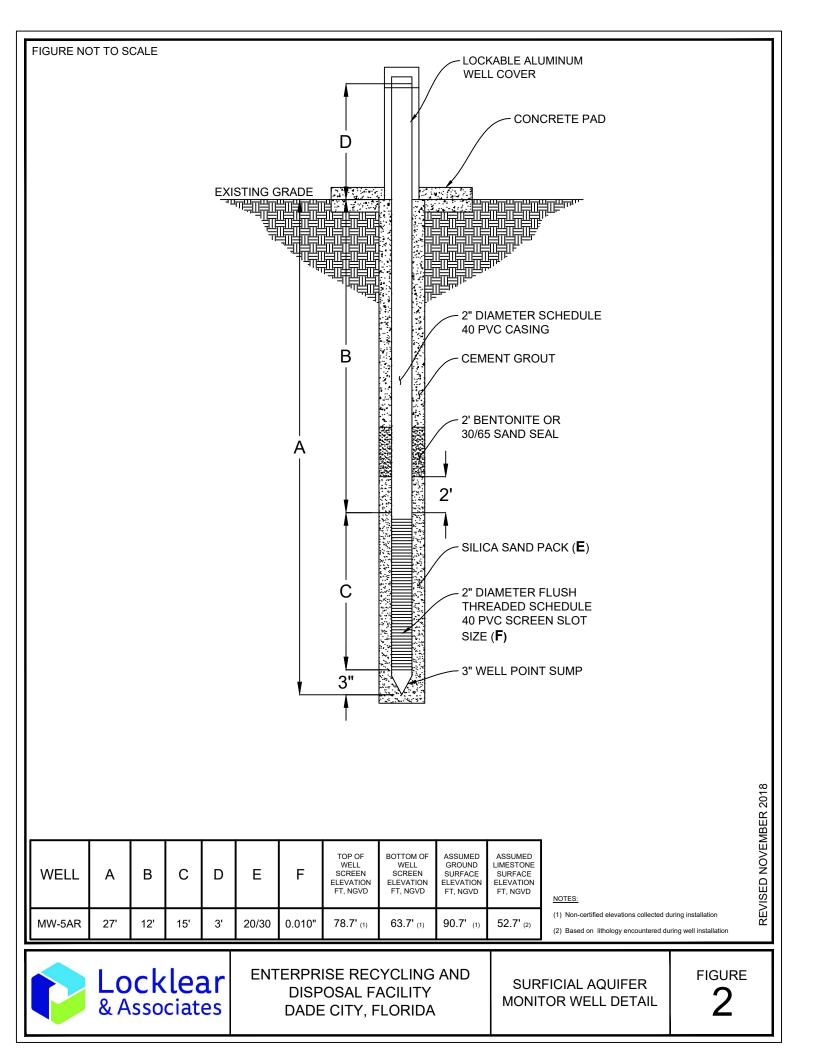
RESPONSE: The design flow rate for Cell 16 remains the same. The purpose of increasing the pipe diameter was to potentially provide excess capacity in the future, for cell 17. No additional capacity is expected for Cell 16 and we anticipate addressing those permitting issues with the Department separately during the Cell 17 permitting process.

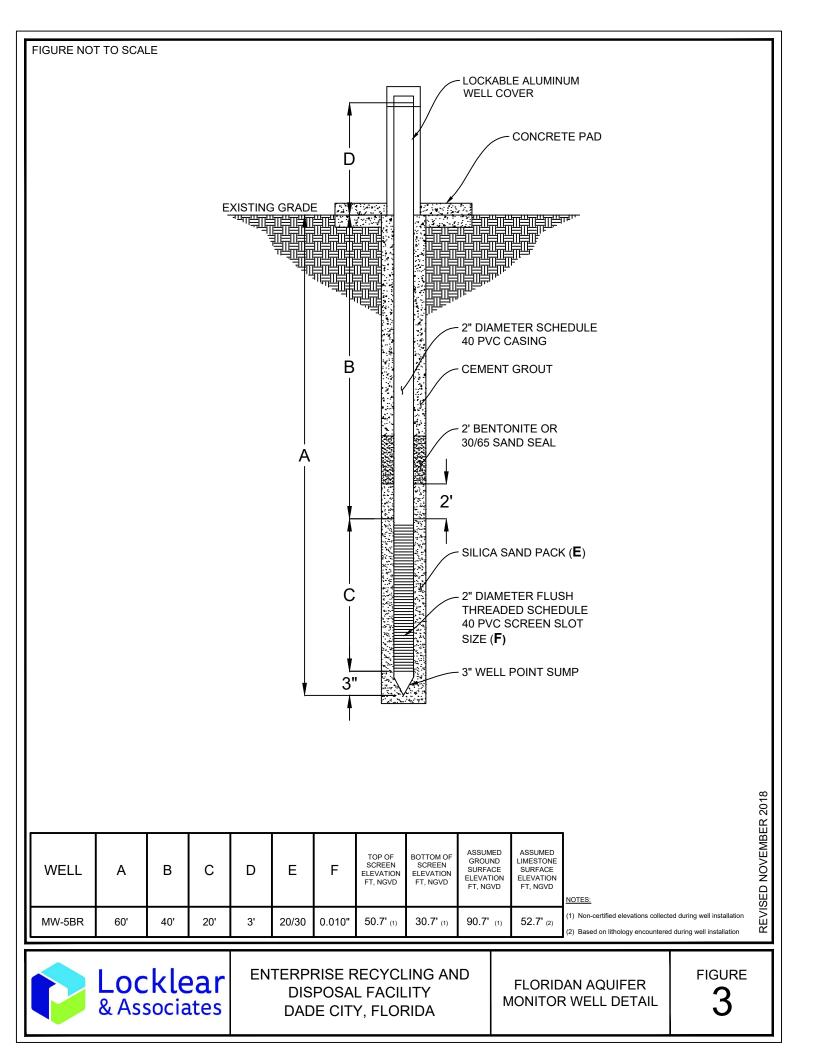
7. Please note that, in accordance with Specific Condition #2.B.2. of the above referenced permit, the construction deviations discussed in Comments #5.b., #5.c., and #6.a. above would appear to have required prior notification to the Department to determine whether a permit modification would be required for the proposed changes. The Department's Compliance Assurance Program by this letter has been informed of this matter for consideration of any potential non-compliance issues and follow up actions. This comment is provided for informational purposes only and does not necessarily require a response other than acknowledgement of the comment.

RESPONSE: We acknowledge this comment and hope that the additional information provides the Department to necessary information to approve the cell for use. As you are aware, we are anxious about operations at this location coming to a halt until we receive the necessary approvals and greatly appreciate your help. We would like to get your input as quickly as possible so that we can provide you with any additional information you need.

Sincerely. ohn Arnold, P.E.

 cc: Dominic Iafrate, Angelo's Aggregate Materials, <u>diafrate@iafrate.com</u> Lisa Baker, Locklear & Associates, <u>lisa@locklearconsulting.com</u> John Locklear, Locklear & Associates, <u>john@locklearconsulting.com</u> Walker Wrenn, Locklear & Associates, <u>walker@locklearconsulting.com</u> Justin Chamberlain, P.G., FDEP Tampa <u>Justin.Chamberlain@floridadep.gov</u> Steve Tafuni, FDEP Tampa Solid Waste CAP Manager <u>Steve.Tafuni@floridadep.gov</u>





November 7, 2018

Mr. Steve Morgan Solid Waste Section Florida Department of Environmental Protection - Southwest District 13051 North Telecom Parkway Temple Terrace, Florida 33637-0926

RE: Enterprise Recycling and Disposal Facility Cell 16 Construction Completion Report Revised Report per 10/30/18 FDEP Comments Angelo's Aggregate Materials, Ltd. FDEP Permit Nos. 177982-023-SC/T3 WACS No.: 87895 Pasco County, Florida

Dear Mr. Morgan,

This report contains the Certification of Construction Completion (Certification) and Construction Quality Assurance (CQA) data for Cell 16 of the Enterprise Class III landfill and is being submitted to the Florida Department of Environmental Protection (Department) for review and approval. This report has been revised to respond to the items contained in your October 30, 2018 correspondence.

The CQA program and certification reporting are based on the specific condition requirements contained in FDEP Permit No. 177982-023-SC/T3, which include the following:

- a. The owner or operator shall submit a Certification of Construction Completion, Form 62-701.900(2), signed and sealed by the professional engineer in charge of construction and quality assurance to the Department for approval (Specific Condition 177982-023-SC/T3, Part B, 2.a.1). The Certification of Construction Completion is provided in Attachment A.
- b. The permittee shall submit Record Drawings/Documents showing all changes (i.e. additions, deletions, revisions to the plans previously approved by the Department including site grades and elevations). The Record Documents shall include, but not be limited to, as-built elevations of the disposal areas (surveys), details and elevations of limerock encountered, and other details as appropriate (Specific Condition 177982-023-SC/T3, Part B, 2.a.2). Record Drawings are provided in Attachment B.
- c. The owner or operator shall submit a narrative indicating all changes in plans, the cause of the deviations, and certification of the Record Drawings/Documents by the Engineer to the Department (Specific Condition 177982-023-SC/T3, Part B, 2.a.3). The narrative report prepared by the professional engineer responsible for the construction quality assurance (CQA Engineer of Record) program is provided in

Attachment C.

- d. The CQA Engineer of Record shall submit to the Department a final report to verify conformance with the project specifications, including all test results for the development of each cell (Specific Condition 177982-023-SC/T3, Part B, 2.a.4). These documents including the Construction Quality Assurance Testing performed by Universal Engineering Sciences, Inc. are provided in Attachment D.
- e. Prepare and submit financial assurance for the facility in accordance with F.A.C. 62-701.630 and Specific Condition 177982-023-SC/T3, Part D.4). The approved financial assurance estimate and existing letter of credit on file with the Department include Cell 16.
- f. Limerock Details and Observations. There was no limerock observed or encountered as part of Cell 16 construction.
- g. Groundwater Monitoring Wells and Sampling. Installation, initial sampling, and reporting of the groundwater monitoring wells associated with Cell 16 construction is being coordinated by our sub-consultant, Mr. Locklear P.G. All of the requested materials have been provided to the Department by them.

We trust this submittal, along with the financial assurance update, will satisfy the Department's certification requirements. Please call me at (352) 339-1408 if you have any questions or require any additional information.

Sincer CENSE John Arnold, P. ETE OFDate State of Florida P.E. No 1530 McDuff Jacksonville, F Tel.: (352) 339-1408

attachments

cc: Dominic lafrate, Angelo's Recycled Materials Lisa Baker, Locklear and Associates, Inc. John Locklear, Locklear and Associates, Inc.

Revised to respond to 10/30/18 comments from the Department.

Attachment A

Certification of Construction Completion FDEP Form 62-701.900(2)

Revised to respond to 10/30/18 comments from the Department.

R	leset	t Foi	m



Department of Environmental Protection

Bob Martinez Center 2600 Blair Stone Road Tallahassee, Florida 32399-2400 DEP Form # 62-701.900(2) Form Title Certification of Construction Completion of a Solid Waste Management Facility Effective Date May 19, 1994

Certification of Construction Completion of a Solid Waste Management Facility

DEP Construction Permit No: 177982-023-SC/T3 County: PASCO	_
Name of Project: ENTERPRISE RECYCLING & DISPOSAL FACILITY	_
Name of Owner: ANGELO'S AGGREGATE MATERIALS, LTD	-
Name of Engineer: JOHN P. ARNOLD, P.E.	_
Type of Project: CELL 16 OF THE CLASS III LANDFILL; CERTIFICATION OF AS-BUILT DRAWINGS	_
AND CERTIFICATION OF CLAY LINER CONSTRUCITON AND CONFORMANCE TESTING	
Cost: Estimate \$250,000 estActual \$ 250,000 est	_
Site Design Quantity: <u>1,500</u> ton/day Site Acreage: <u>5.5 apprx. Cell 16</u> Acres	5
Deviations from Plans and Application Approved by DEP (attach additional pages as needed)	*
The top of clay within the cell is higher at some locations, but meets the minimum 3' installed thickness. The	
perimeter berm is apprx. 2' higher. A larger (8" DIA) leachate collection pipe was installed. A fiberglass 4'	
DIA wetwell was installed with same pumping capacity. East side Pond 3 shifted to accommodate landscape	e
berm. Perimeter road widened in areas to for 2-way vehicle traffic more safety.	

Address and Telephone No. of Site: 41111 ENTERPRISE RD., DADE CITY, FL 33525

Name(s) of Site Supervisor: Mr. Phil Curtin

Date Site inspection is requested: November 14, 2018

This is to certify that, with the exception of any deviation noted above, the construction of the project has been completed in substantial accordance with the plans authorized by Construction

Permit No.: 177982-023-SC/T3

Dated: July 9, 2013 (Issued)

Date: 11/8/18

Signature of Professional Engineer

* Please refer to CQA Engineer of Record Narrative Report.

Northwest District 160 Governmental Center Pensacola, FL 32501-5794 850-595-8360 Northeast District 7825 Baymeadows Way, Ste. B200 Jacksonville, FL 32256-7590 904-448-4300 Central District 3319 Maguire Blvd., Ste. 232 Orlando, FL 32803-3767 407-894-7555 Southwest District 3804 Coconut Palm Dr. Tampa, FL 33619 813-744-6100 South District 2295 Victoria Ave., Ste. 364 Fort Myers, FL 33901-3881 941-332-6975 Southeast District 400 North Congress Ave. West Palm Beach, FL 33401 561-681-6600

Print Form

Attachment B

Record Drawings & Documents

A topographic survey depicting as-built conditions of the site was prepared by Pickett and Associates, Inc. based on the aerial reconnaissance performed on September 17, 2018. Surveying ground control for the site was established by Simmons and Beall, Inc. As-built elevations documenting the 3' clay over-excavation, top of Cell 16 clay, wetwell (pump station) and leachate collection pipe were collected under the direction of John Arnold, P.E. as the Professional Engineer responsible for the Construction Quality Assurance (CQA) plan in accordance with Chapter 471, Florida Statues.

Supporting Record Drawings and Documents include the following:

- Pickett and Associates, Inc. Topographic Survey
- Drawing AB-1 Cell 16 Over-Excavation
- Drawing AB-2 Cell 16 Top Of Clay
- Drawing AB-3 Leachate Collection Wetwell Section and Details
- Non-Woven Geofabric Cut Sheet
- No. 4 Aggregate Gradation Test
- Wetwell Start-Up Pump Test
- Florida Jet Clean Report
- Wetwell Pump, Floats, and Panel Cut Sheets
- IW Pond As-Built Volume Calculation

SURVEYOR'S REPORT

ENTERPRISE ROAD LANDFILL

Prepared for:



Prepared by:



PICKETT AND ASSOCIATES PROJECT NO.: 14094-9 TITLE/TYPE OF SURVEY: Topographic Survey DATE OF SURVEY: This Map is based on LiDAR data & aerial imagery flown 09/17/18

NOTE: THIS REPORT AND ACCOMPANYING MAP TITLED ENTERPRISE ROAD LANDFILL, ARE NOT FULL AND COMPLETE WITHOUT THE OTHER AND ARE NOT VALID WITHOUT THE SIGNATURE AND ORIGINAL RAISED SEAL OF A FLORIDA LICENSED SURVEYOR AND MAPPER.

Pickett and Associates, Inc. • 475 South First Avenue • Bartow, FL 33830 • (863) 533-9095

DATUM:

HORIZONTAL:

Coordinates are referenced to the West Zone of the Florida State Plane Coordinate System, NAD 83, and were provided by Simmons and Beall Surveying.

VERTICAL:

Elevations are to National Geodetic Vertical Datum of 1929 and were provided by Simmons and Beall Surveying

Control Points Used:

Pt#	Easting	Northing	Elevation
4000	612277.73	1454997.54	105.81
4001	612338.97	1452175.37	139.98
4002	614249.29	1452235.24	113.56
4003	614271.09	1454880.23	85.32

ACCURACY STATEMENT: The following stated plus or minus tolerances encompass a minimum of 90% of the difference between photogrammetrically measured values and any ground truth of all well-identified features. Mapped features will meet or exceed the Florida Standards of Practice.

VERTICAL:

Contours have an estimated vertical positional accuracy of 0.5'. Spot elevations, on paved surfaces, have an estimated vertical positional accuracy of 0.25'.

HORIZONTAL:

Well-identified features have an estimated horizontal positional accuracy of 1.66'. All measurements are in U.S. Survey Feet.

Measurement Methods:

The planimetrics shown are limited to those features visible on aerial imagery. Color digital imagery was acquired at an average altitude of 2100' using a metric precision digital camera whose focal length is 51.58mm. Mapping was performed using LiDAR and softcopy photogrammetric techniques. The LiDAR data has an estimated point sample distance of 0.4 foot and a density of 6.4 points per square foot (\pm 68.889 points per square meter). For a vertical accuracy check, the LiDAR data was compared to the four (4) points set as targets for aerial imagery. The Root Mean Square Error of the Elevations (RMSEZ) is 0.074 foot, being the equivalent of 0.145' FGDC/NSSDA Vertical Accuracy. All measurements are in U.S. Survey Feet.

Limitations:

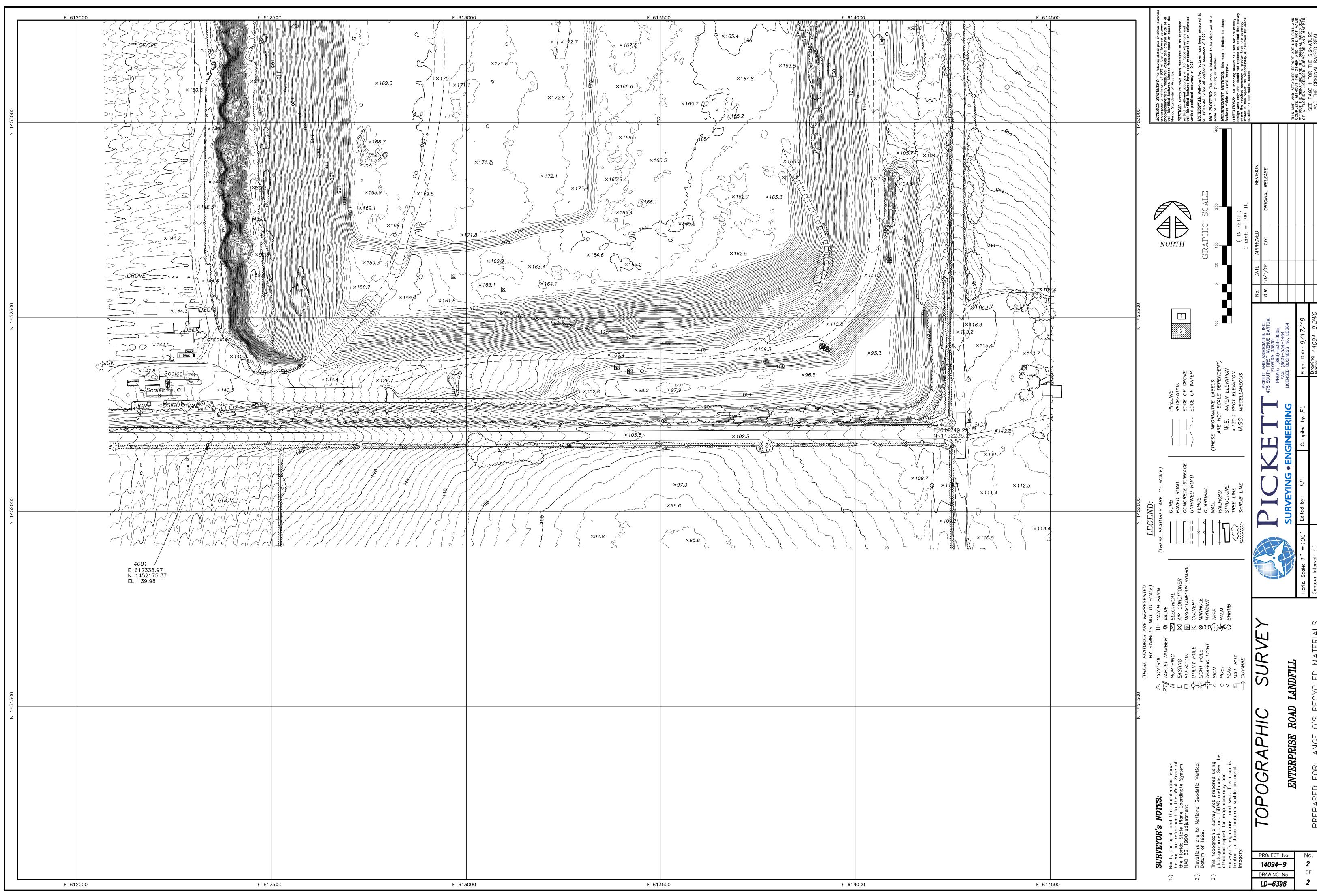
This mapping should be used for preliminary design work only and should not replace an actual field survey where the required accuracy is greater than the accuracy stated in this report. No responsibility is assumed for areas outside the contracted scope or for the control provided by Simmons and Beall Surveying, Dade City, Florida.

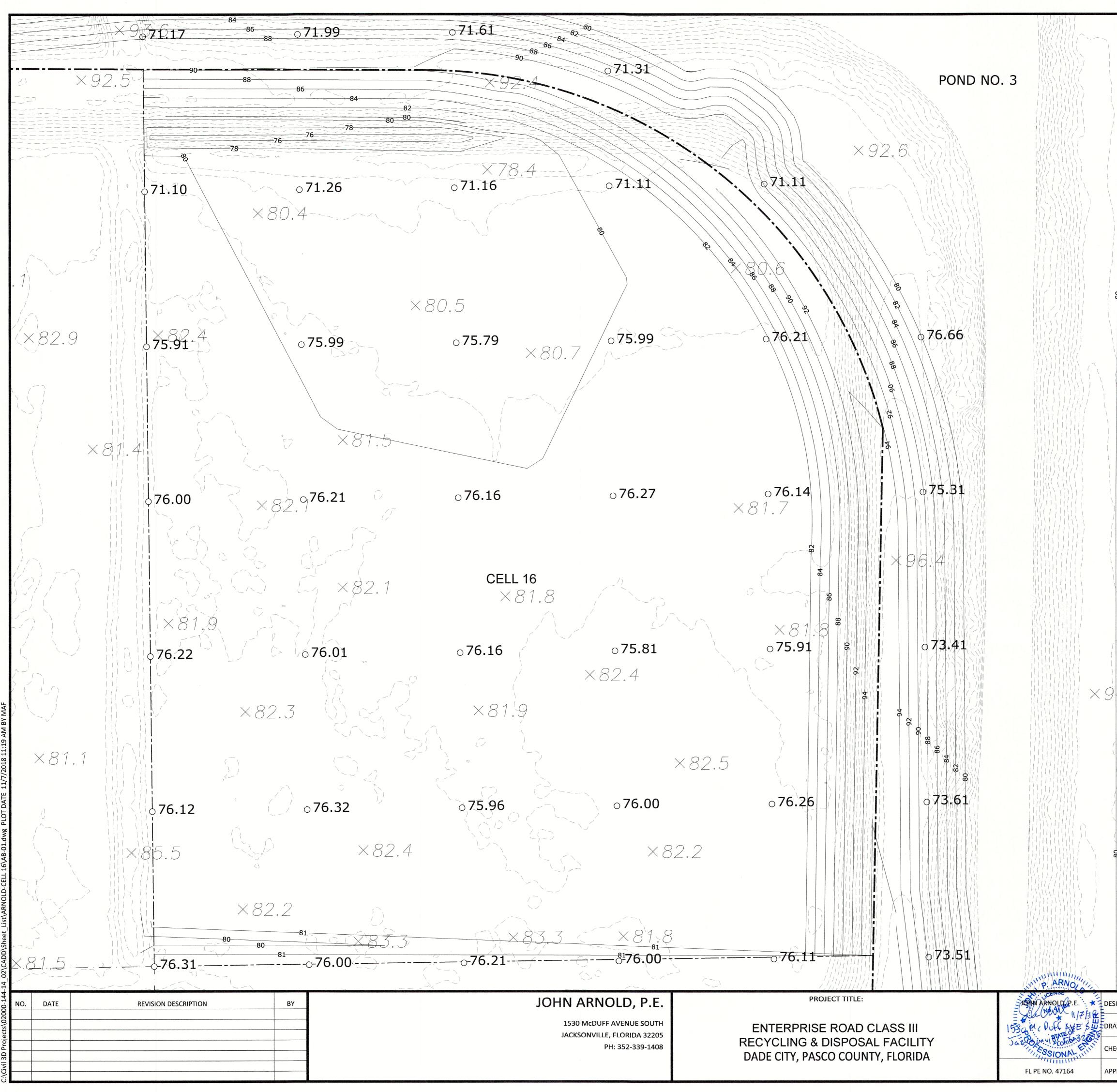
MAP PLOTTING:

This map may be displayed at a scale of 1'' = 50' (1:600) or smaller.

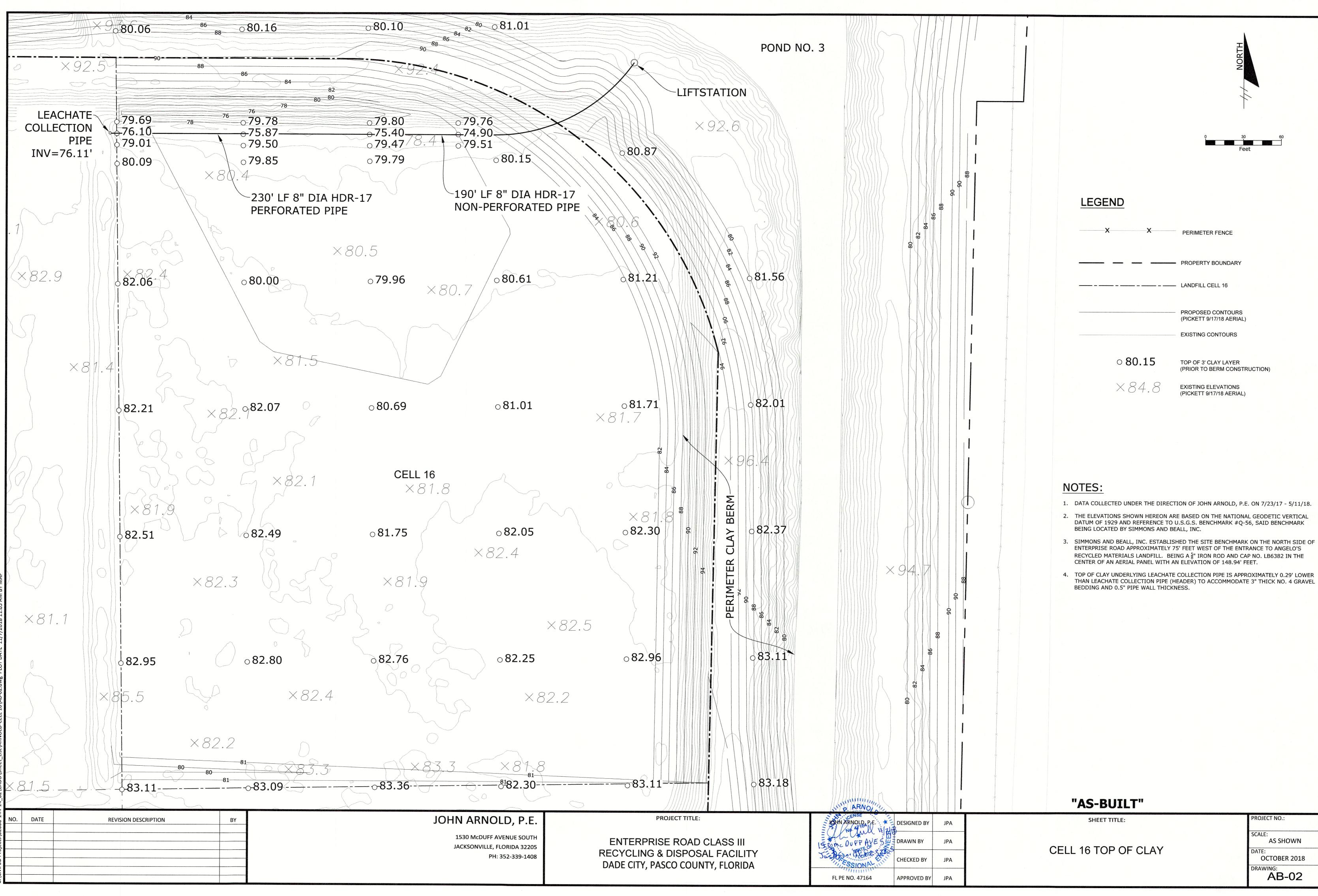
T. JEFFREY YOUNG, PSM, CP FLORIDA REGISTRATION NO. 5440 PICKETT AND ASSOCIATES, INC. FLORIDA REGISTRATION NO. 364 SURVEY DATE



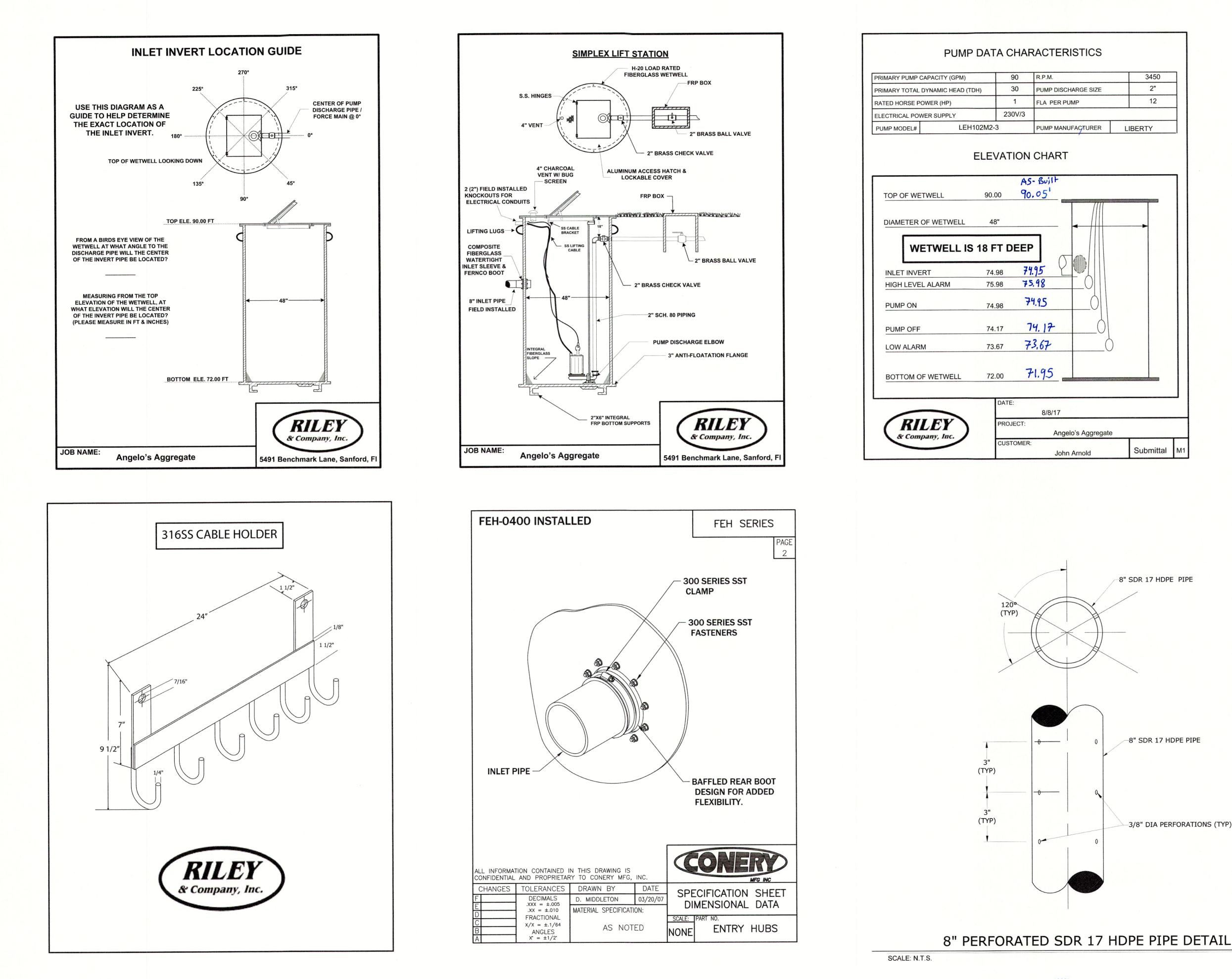




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		PROPOSED CONTOURS	
		EXISTING CONTOURS	
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		$\times 84.8$ existing elevations	
		NOTES:	
		1. DATA COLLECTED UNDER THE DIRECTION OF JOHN ARNOLD, P.E. ON 4/24 GROUND CONTROL BY SIMMONS AND BEALL, INC.	
		2. THE ELEVATIONS SHOWN HEREON ARE BASED ON THE NATIONAL GEODE OF 1929 AND REFERENCE TO U.S.G.S. BENCHMARK #Q-56, SAID BENCHMA BY SIMMONS AND BEALL, INC.	
		3. SIMMONS AND BEALL, INC. ESTABLISHED THE SITE BENCHMARK ON THE ENTERPRISE ROAD APPROXIMATELY 75' FEET WEST OF THE ENTRANCE T RECYCLED MATERIALS LANDFILL. BEING A 5" IRON ROD AND CAP NO. LBG	TO ANGELO'S
4.1/		OF AN AERIAL PANEL WITH AN ELEVATION OF 148.94' FEET. 4. SPOT ELEVATIONS ARE BASED ON FLORIDA STATE PLANE COORDINATE	SYSTEM, WEST ZONE.
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	JPA		OCTOBER 2018 DRAWING: AB-01
ROVED B	Y JPA		



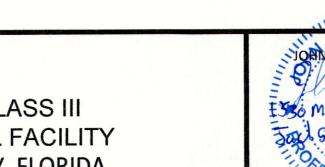
ivil 3D Projects\02000-144-14_02\CADD\Sheet_List\ARNOLD-CELL 16\AB-02.dwg PLOT DATE 11/7/2018 11:

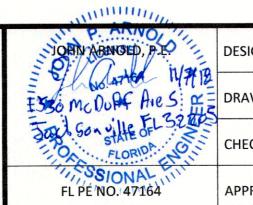




NO.	DATE	REVISION DESCRIPTION	BY

DATE:	
	8/8/17
PROJECT:	
	Angelo's Aggregate
CUSTOMER:	
	John Arnold



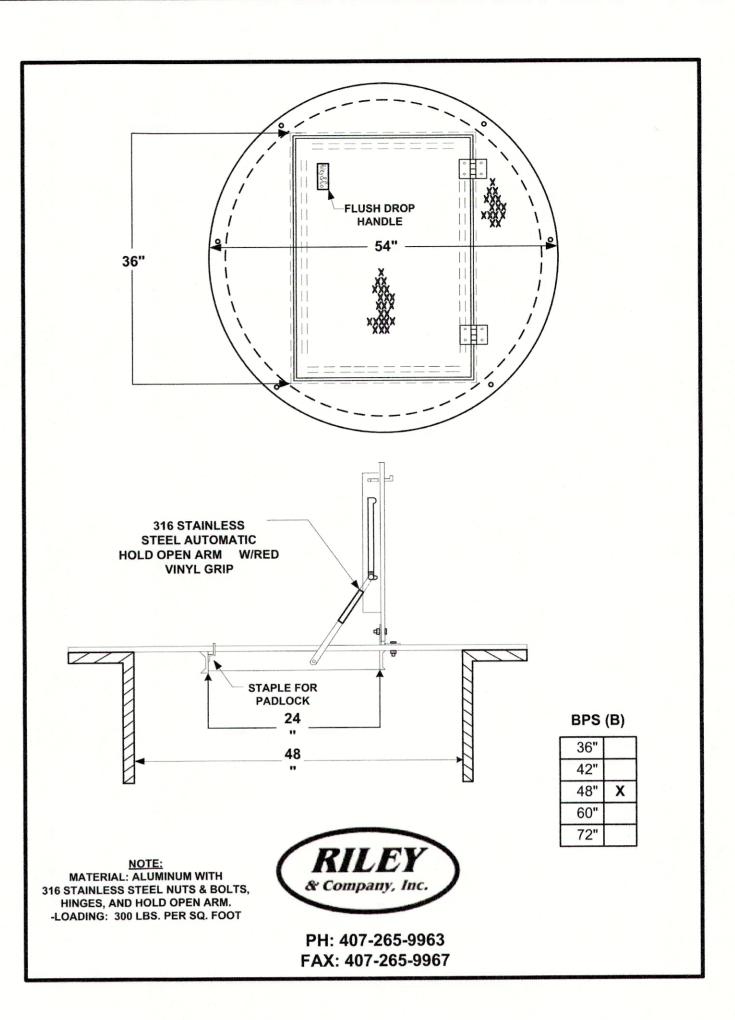


JOHN ARNOLD, P.E.

1530 McDUFF AVENUE SOUTH JACKSONVILLE, FLORIDA 32205 PH: 352-339-1408

ENTERPRISE ROAD CLASS III **RECYCLING & DISPOSAL FACILITY** DADE CITY, PASCO COUNTY, FLORIDA

PROJECT TITLE:



-3/8" DIA PERFORATIONS (TYP)

"AS-BUILT"

IGNED BY	JPA	SHEET TITLE:	PROJECT NO.:
WN BY	JPA		SCALE: AS SHOWN
CKED BY	JPA		DATE: OCTOBER 2018
PROVED BY	JPA		DRAWING: AB-03



P. O. Box 16549 | Tampa, Florida 33687 Telephone: (800) 330-2333

QU	DTE
QUOTE DATE	12/22/17
QUOTE #	3134
QUOTED BY	Mike Nester

PROJECT	Landfill	
BID DATE		
COMPANY	Angelos Recycled Materials 855 28th Street South St. Petersburg, FL 33712	
ATTENTION	Nero	

SHIP TO Angelos Recycled Materials Dade City, FL

FREIGHT TERMS	
Plus Freight	
EST. DELIVERY TIME	
Same Business Day ARO	Reading

ITEM	DESCRIPTION	QTY	U/M	UNIT PRICE	LINE TOTAL
2010-020	Mirafi 160N - nonwoven geotextile Roll size: 15' X 300' (500 SQYD) (Meets FDOT Type D-3)	2	rl	460.00	920.00
Freight	 Estimated freight to the Dade City, Florida jobsite based on a one-time shipment of the quantity listed via Ed Nunez Trucking. 			125.00	125.00
	MATERIAL PRICING DOES NOT INCLUDE SALES TAX	 	SUBTO TAX (7.0	and the second	\$1,045.00
All prices are valid for 30 days from the date of the quotation Changes in quantities will require creation of a new quote Terms: Net 30 Days (with an open account) Above pricing is for materials only Materials in stock may be picked up from our Tampa Warehouse location			\$1,109.40		

Please call (800) 330-2333 or send an email to Sales@rhmooreassociates.com if you need more information, technical support or to place your order

ACCEPTED BY

SIGNATURE



ENGINEERING, TESTING & INSPECTION P.O. BOX 15732 • TAMPA, FLORIDA 33684 • 813/872-7821 CA No. 1450

PROJECT: Product Check 1170.15.1 **PROJECT NO:** Largo, FL DATE: 1/22/2018 LAB NO: B-11077 Angelo's Recycled Materials **CLIENT:**

Composite sample from stockpile **SAMPLE LOCATION:**

No. 4 Crushed Concrete **SAMPLE DESCRIPTION:** (Coarse Aggregate)

Sieve Number	Percent Passing	FDOT Specifications Section 901-1.4
2"	100	100
1 ¹ / ₂ "	92	90-100
1"	35	20-55
³ / ₄ "	6	0-15
¹ / ₂ "	3	
³ / ₈ "	2	0-5

FINENESS MODULUS:	8.00
	0.00

DATE TESTED: 1/16/18

TEST LAB, INC.

SA.LabB-11077 (No 4)

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Design Point

<u>70</u> GPM @ <u>30</u> FT. TDH



5491 Benchmark Lane Sanford, FL 32773 Ph: (407) 265-9963 Fx: (407) 265-9967

START-UP REPORT

I. PROJECT INFOR	MATION - Complete	d by Installing (Contractor			
JOB NAME: Ange	Angelo's Aggregate INSTALLING CONTRACTOR: John Arnold					
ENGINEER:	START UP DATE & TIME : 5/1/18 @ 11am					
LOCATION: 4111	1 Enterprise Rd. Dad	de City, FL 335	25 Jo	ohn: 813-477-1719		
II. EQUIPMENT INI	FORMATION - Com	pleted by Install	ing Contractor			
Pump Manufacturer:	LIBERTY	Model Number	-: FL63M-3	Wetwell Elevations	Тор:	92.00
Serial No. Pump 1:	B78108ZMC	Voltage & Phase	230/3	Invert:	Off:	
		FLA: 20	HP: 0.0	6 Bottom: <u>74.00</u>	Lead:	
				HLA:		
Control Panel (If used, 1	Model, Mfr. Serial #, Ty	/pe)	<u>SIM- 092617-1</u>	Basin Dim (LXW)		
Float Switches (If pump			40' Roto Float	Installation Type:	Prepackaged R	ail System
					DEADDIGG	
III. INSTALLER CH by installing contract		wing should be	completed	V. ELECTRICAL	READINGS	
			Nganio 2014 interne	SINGLE PHASE:	Pump #1	
X Pit Clean				pply (Pump Off) L1-L2		
X Pump Rotatio				upply (Pump On L1-L2		
X Impeller Turr			An	np Draw (Pump On) L1		
X Panel securel				L2	1	
X Short Circuit				THREE PHASE:		
	good condition		Voltage Su	pply (Pump Off) L1-L2	211.0	
	discharge pipe, and ve			L2-L3	212.0	
	installed by electrician	(not required 460	0v)	L1-L3	212.0	
IV. START-UP VER	FICATION LIST		Voltage S	upply (Pump On L1-L2	210.0	
	ers operational			L2-L3	210.0	
X Pump submer	ged at least 2/3			L1-L3	211.0	
	nt pipe installed		An	np Draw (Pump On) L1	6.0	
	scharge pipe installed			L2	6.0	
	wer supply cable length			L3	6.0	
	nstalled in correct local	tion and direction	1			
VI. PERFORMANCE				COMMENTS AND S	UGGESTIONS	2
P1 86	GPM @			an a		
		F	T. TDH			
Static Pressure:	N/A					
	PUMP 1			ture:		
Pump Off:	199			ame:		
Start/Pump On:	188		Contractor Signa			
Pump Down / Inch:	11			ame: John Arnold		
Total GPM:	86			hers:		
Pump On Pressure:	N/A	ntify this start		ame:	$\overline{\Omega}$	0
		••	-up report to be a	(//)	/lh	$\sqrt{0} = 86 \text{Gpn}$
	Tech:	•	Nik Marku		A.M	(q=86gpn
				•)c	hy Mind	

FLORIDA JETCLEAN

HIGH PRESSURE WATER JETTING EXPLOSION PROOF VIDEO INSPECTION VACUUM TRUCK SERVICES WWW.FLORIDAJETCLEAN.COM 7538 DUNBRIDGE DRIVE ODESSA, FL 33556 T: 800-226-8013 / F: 813-926-4616 FLORIDAJETCLEAN@YAHOO.COM

Angelo's Recycled Materials Dade City, FL New Cell 16 Toe Drain Leachate Collection System Jetcleaning

Work Performed November 2018

> Conducted By: Florida Jetclean 800-226-8013

FLORIDA JETCLEAN

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REPORT

DATE	: 11/5/2018
ТО	: John Arnold - Angelo's Recycled Materials
FROM	: Ralph Calistri (floridajetclean@yahoo.com)
SUBJECT	: 2018 - New Cell 16 Toe Drain - LCS Pipe Jetcleaning Project

Florida Jetclean was mobilized to Angelo's Recycled Materials in Dade City, FL on 11/2/2018 to provide leachate collection piping high-pressure water-jetting services (4,000 PSI) on the new Cell 16 Toe Drain piping.

The below jetting log summarizes the work performed during this mobilization.

LOCATION	JETTED LENGTH	DESCRIPTION
Cell 16 Toe Drain P.S. to Cleanout	450'	Entire Pipe Length Jetcleaned
Cell 16 Toe Drain Cleanout to P.S.	450'	Entire Pipe Length Jetcleaned

The above pipes were clean and blockage free at the completion of our site services.

Please call us with questions or concerns.

Regards,

Calisti'

Ralph Calistri - Florida Jetclean - 800-226-8013

LIFT STATION SUBMITTALS

PROJECT:

Angelo's Aggregate

Date: 8/10/17 REVISED

Contractor: Angelo's Aggregate

Attn: John Arnold



5491 Benchmark Lane Sanford, FL 32773 P. 407-265-9963 F. 407-265-9967

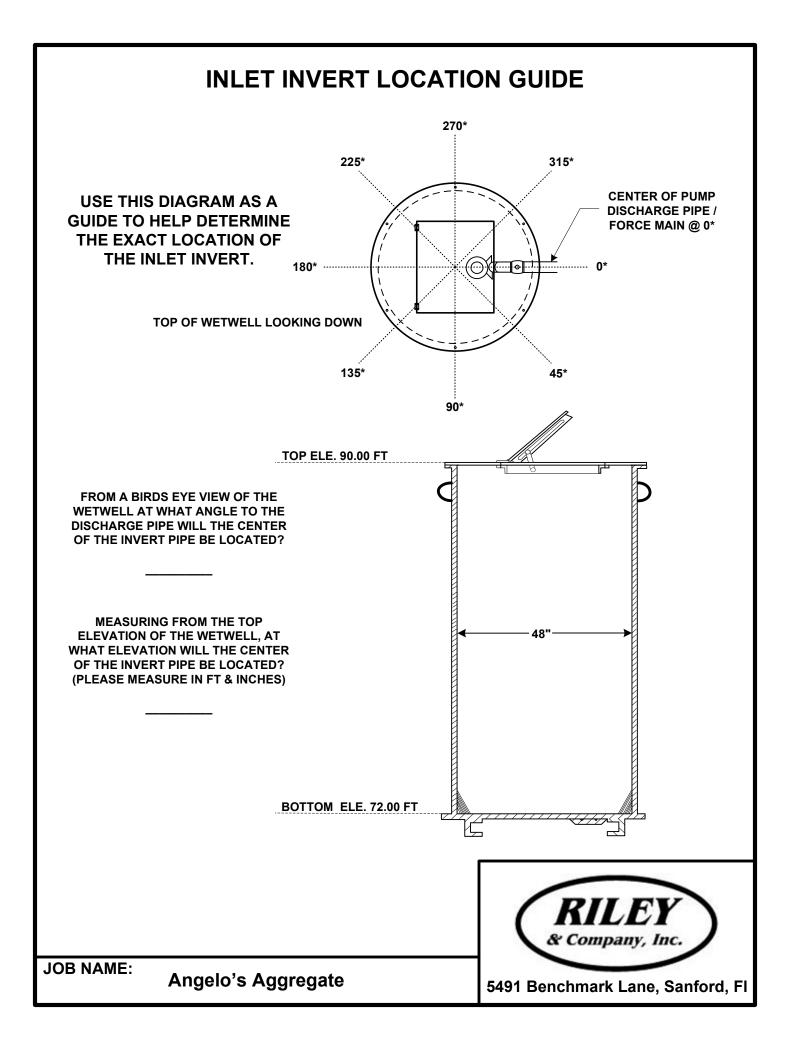


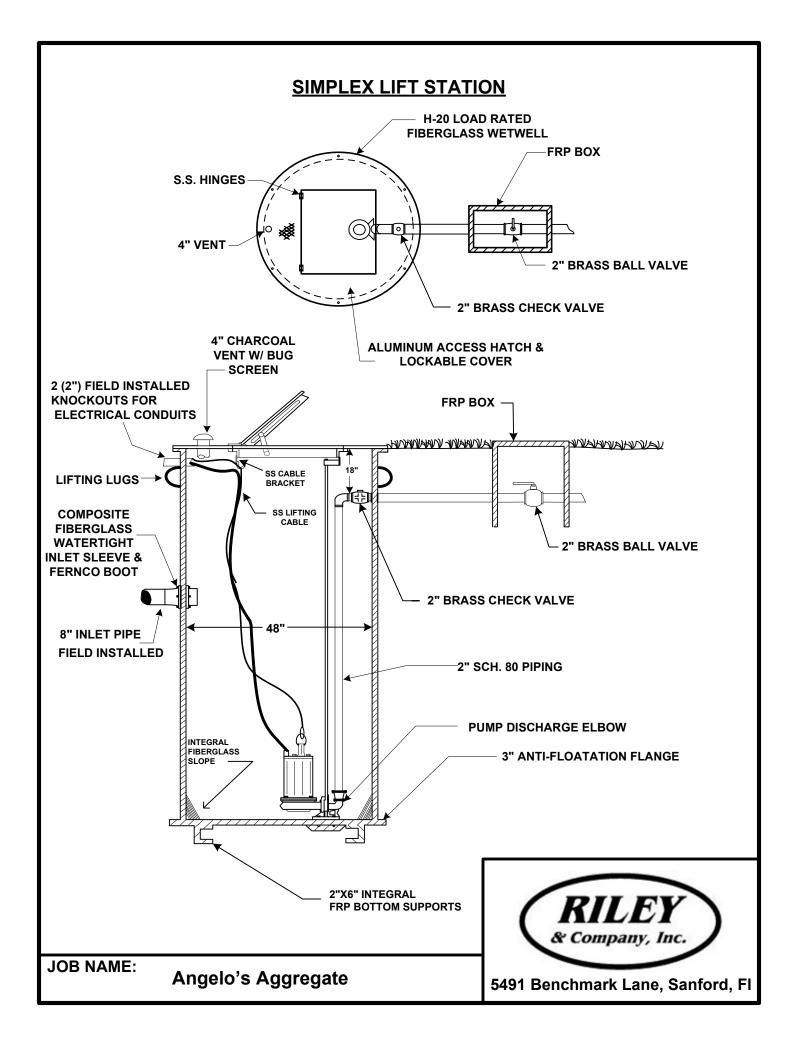
5491 Benchmark Lane Sanford, FL 32773

PH. 407-265-9963 FX. 407-265-9967

SCOPE OF SUPPLY

ITEM	QTY.	DESCRIPTION
1 2 3 4 5 6 7 8 9 10 11 12 13 14	1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1	LIBERTY LEH102M2 Pumps, 230V/3P, 35' Cables Cast-Iron Pump Discharge Bases Pre-Plumbed (H-20) Fiberglass Wetwell (48" x 216" Deep) w/ Lockable Alum. Cover FRP Valve Cover for Ball Valve Fiberglass Simplex Control Panel Stainless Steel Lifting Chains Stainless Steel Cable Holder Brass Steel Cable Holder Brass Check Valves (2") Brass Ball Valves (2") 8" Composite Inlet Sleeve 8" Rubber FERNCO Boot Float Switches Charcoal Filter Vent (4")





RIMARY PUMP CAPAC	ITY (GPM)	90	R.P.M.	3450
PRIMARY TOTAL DYNAM	/IC HEAD (TDH)	30	PUMP DISCHARGE SIZE	2"
RATED HORSE POWER	(HP)	1	FLA PER PUMP	12
ELECTRICAL POWER SU	JPPLY	230V/3		
PUMP MODEL#	LEH102M2	-3	PUMP MANUFACTURER	LIBERTY
TOP OF WETWEL	.L 90	.00		
DIAMETER OF WI	ETWELL 4	8"		>
	/ELL IS 18 I	FT DEEF		
HIGH LEVEL ALA	RM 75	5.98	0	
PUMP ON	74	.98		
PUMP OFF		.17		
LOW ALARM	73	9.67	C)
BOTTOM OF WE	TWELL 72	2.00		
		DATE:		
	_	D/ (1 E.		
	V		8/8/17	
RILE	Y	PROJECT:		
RILE & Company,	Inc.		Angelo's Aggregate	Submitt

Liberty Pumps[®]

LEH100-Series

High Head Sewage Pumps

1 hp 2" Solids-Handling 2" or 3" Flanged Discharge 53' Maximum TDH

Features:

 Rugged 2 vane, semi-open cast iron impellers

 Cast iron housings and volute with all stainless steel and brass fasteners

• 17-4 PH stainless steel shaft

Oil-filled, hermetically sealed motors

Built-in thermal protection on single
 phase models

• 2" or 3" flanged discharge

 Permanently lubricated upper and lower ball bearings

Unitized shaft seals

 Single float mechanical level control with series plug for manual bypass operation– standard on single-phase automatic models

Adjustable pumping range

 Quick-disconnect 10' standard power cord allows replacement of cord in seconds without breaking seals to motor (25' and 35' lengths optional)





Models:

SINGLE PHASE

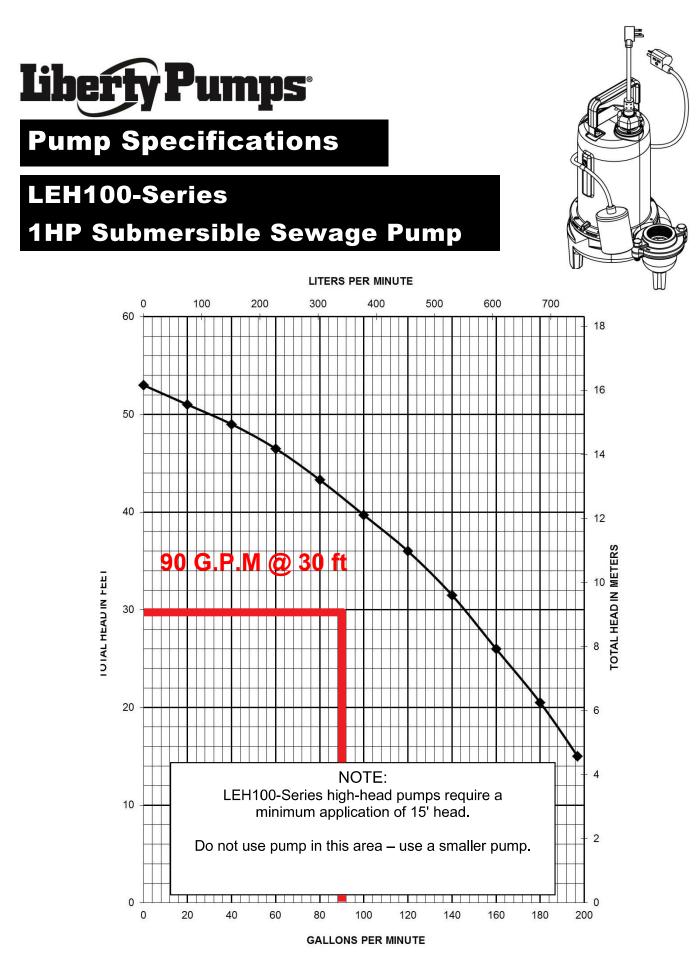
LEH102M	230V, 12a,	manual
LEH102A	230V, 12a,	automatic

3-PHASE

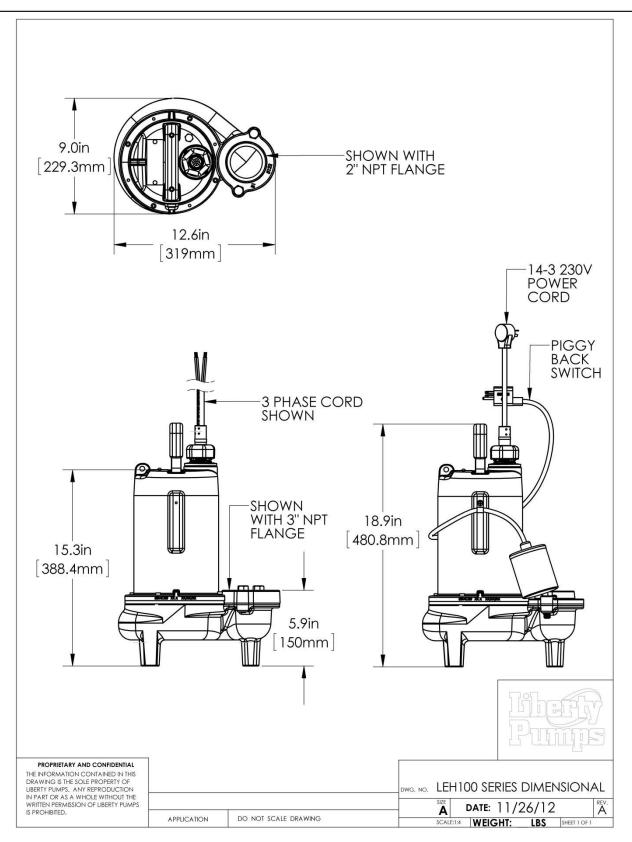
LEH103M	208-230V, 9a, manual*
LEH104M	440-480V, 4.5a, manual*
LEH105M	575V, 3.3a, manual*

*Note: 3-phase models require control panel for automatic operation. See sewage accessories literature for complete information on all simplex and duplex controls.

innovate./evolve.







LEH100-Series Electrical Data

MODEL	HP	VOLTAGE	PHASE	SF	FULL LOAD AMPS	LOCKED ROTOR AMPS	THERMAL OVERLOAD TEMP	STATOR WINDING CLASS	CORD LENGTH FT	DISCHARGE	AUTOMATIC
LEH102M2-3	1	230	1	1.00	12	28.6	120°C 248°F	В	35	2" Flanged	NO

LEH100-Series Technical Data

IMPELLER	2 VANE CLASS 25 CAST IRON
SOLIDS HANDLING SIZE	2"
PAINT	POWDER COAT
MAX LIQUID TEMP	60°C 140°F
MAX STATOR TEMP	130°C 266°F
THERMAL OVERLOAD	120°C 248°F (single-phase only)
	SJOOW (1-PH) or
POWER CORD TYPE	SEOOW (3-PH)
MOTOR HOUSING	CLASS 25 CAST IRON
VOLUTE	CLASS 25 CAST IRON
SHAFT	STAINLESS
HARDWARE	STAINLESS
ORINGS	BUNA N
MECHANICAL SEAL	UNITIZED CARBON CERAMIC - SINGLE PHASE UNITIZED SILICON CARBIDE - THREE PHASE
MIN BEARING LIFE	50,000 HRS

LEH100-P3 R8/5/2014

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LEH100-SERIES TECHNICAL SPECIFICATIONS

PUMP

The pump(s) shall be model **LEH102M2-3** as manufactured by Liberty Pumps, Bergen, N.Y. or equal. The pump(s) shall have a capacity of **90** GPM at a total dynamic head of **30** feet. Motor size shall be 1 horsepower, ____ 3 phase, 60 hz. and 230 volt operation.

MOTOR

The pump motor shall be of the submersible type, oil filled, and hermetically sealed. Single phase motors shall have thermal overload protection embedded in the windings, and shall automatically reset when motor cools. Three-phase motors shall have overloads incorporated into the control panel, properly sized for the horsepower and amperage of the pump(s).

The rotor shaft shall be made of 17-4 PH stainless steel and shall be supported by upper and lower ball bearings.

The power cord shall be of the quick-disconnect design.

ALL MODELS: 1 HP, 3450 RPM

IMPELLER

The pump impeller shall be cast iron, 2 vane, semi-open, and shall be capable of passing a 2" spherical solid.

SEAL

The shaft seal on single phase units shall be of the carbon/ceramic unitized design, with BUNA N elastomers and stainless housings. 3 phase units are silicon carbide.

EXTERNAL CONSTRUCTION

The pump volute, legs and motor housing shall be heavy gray iron castings, class 25 or better. All castings shall be powder coated before assembly.

All fasteners shall be of 300-series stainless steel.

LEVEL CONTROL

The pump shall be controlled by an adjustable mechanical switch sealed in a PVC float, and shall have a series plug for manual bypass operation.

	MODELS	VOLTS	PHASE	AMPS	DISCHARGE	AUTOMATIC
SINGLE PHASE	LEH102M2	230	1	12	2" FLANGED	NO
10' cord standard o					x to model number. Ex ple: LEH102A2-3	ample: LEH102A2-2
					panel for automatic op I simplex and duplex c	

DIMENSIONAL DATA:

Weight: LEH102M: 64 LBS.

Height: 18.9"

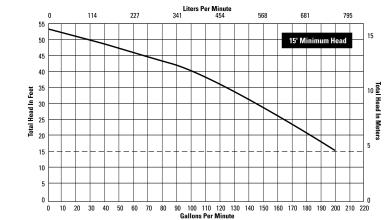
Major Width: 12.5"

Maximum fluid temperature: 140 degrees F.

NOTE: LEH-Series high-head pumps require a minimum application of 15' head.



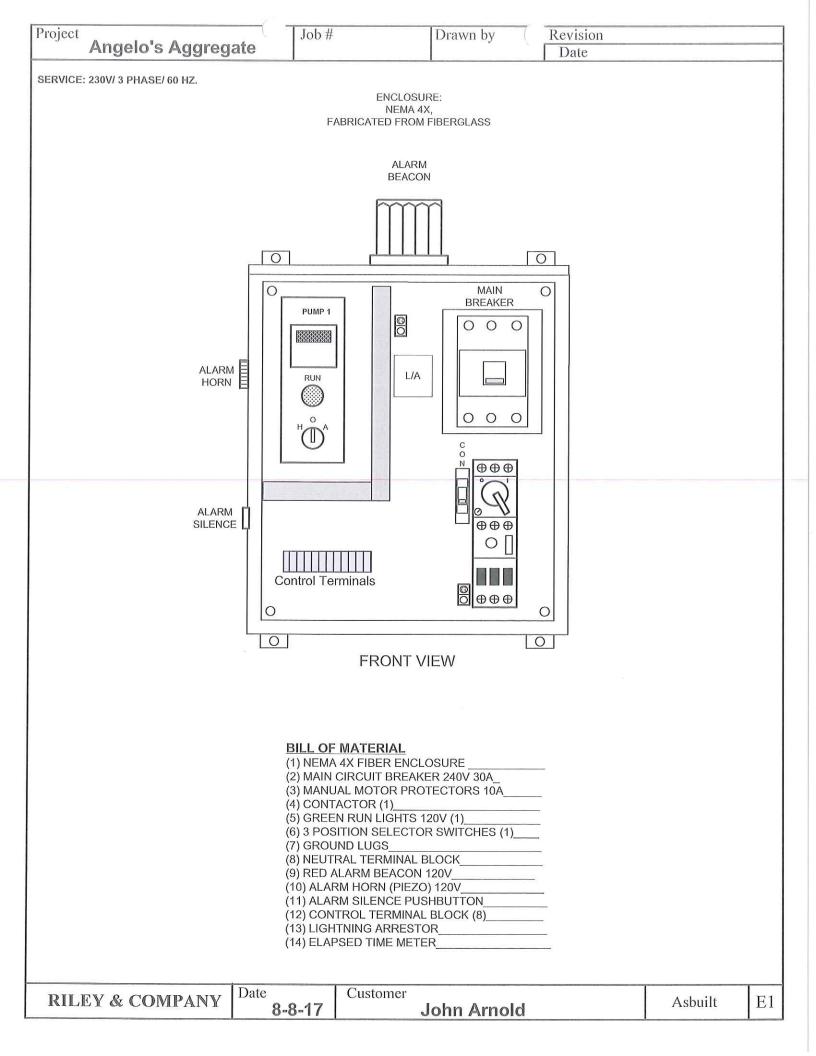
PERFORMANCE CURVE 60 HZ



Liberty Pumps • 7000 Apple Tree Avenue • Bergen, New York 14416 • Phone 800-543-2550 Fax (585) 494-1839

www.libertypumps.com

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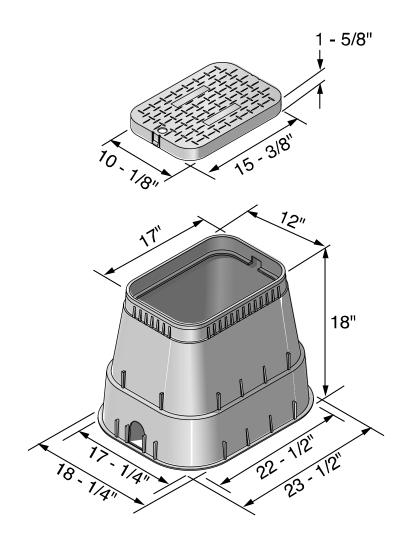


NDS METER BOXES – STANDARD COMMERCIAL GRADE

NDS D1800 Meter Boxes

Specifications: NDS 14" x 19" x 18" meter boxes and covers are injection molded of structural foam recycled polyolefin material with a melt index between 10-12. Coloring and UV stabilizers are added, along with processing lubricants when needed. The 14" x 19" body is tapered and has a minimum wall thickness of 0.250". The body has a double wall at the top cover seat area with a minimum thickness of 0.22". The cover seat area has 26 structural support ribs on the underside of the seat, each with a minimum thickness of 0.12". The bottom of the body has a 0.50" flange. The 14" x 19" cover has an average thickness of 0.20".

Part No.	Description - Marking	Color (Box/Cover)	Pallet Qty.	Wt. Ea. Ibs.	Product Class
Drop-in Box & Cover					
D1800-DISB	14" x 19" x 18" Box, Drop-in Solid Plastic Cover	Black/Black	48	12.20	20ME



3" x 4" Pipe Slot

521 Brass Check Valve • Spec Sheet



FEATURES & BENEFITS

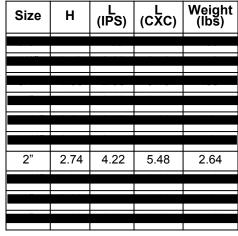
- 200 WOG
- 125 SWP
- Cast Brass Body
- Swing Type
- Threaded Ends Comply with ANSI B2.1 3/8" 4"
- Solder Ends Comply with ANSI B16.18 1/2" 4"
- Valves are Tested in Accordance w/MSS-SP-82

Application: Commercial, Light Industrial for Water, Oil, Gas or Steam.

MATERIAL SPECIFICATIONS

No.	Part	Material
1	Name Plate	Aluminum A1100
2	Сар	Brass B584 C85710
3	Packing	Fibre "H"
4	Disc	Brass B584 C85710
5	Body	Brass B584 C85710
6	Bolt	Brass B16 C36000
7	Pin	Brass B124 C37700

DIMENSIONS



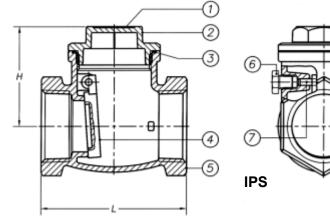


 CALIFORNIA
 5593 Fresca Dr., La Palma CA 90623
 •Toll Free: 866-532-8306
 •Fax: 866-532-8307

 TEXAS
 1150 Silber Rd., Houston TX 77055
 •Toll Free: 800-935-5456
 •Fax: 713-680-2999

 NEW YORK
 PO Box 27, Rt. 22, Brewster NY 10509
 •Toll Free: 800-431-2082
 •Fax: 845-278-9056

 WEB: www.matco-norca.com
 EMAIL: mail@matco-norca.com



759 Brass Ball Valve With Locking Handles • Spec Sheet



• Full Port

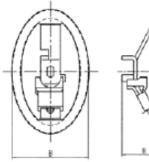
- Forged Brass
- Two Piece Body
- PTFE Seats
- Blow-Out Proof Stem
- Chrome Plated Solid
- Brass Ball

FEATURES & BENEFITS

- Thread Ends Comply Per ANSI B2.1
- Conforms to MSS-SP-110
- 1/4" 2" Rated: 600 PSI Non- Shock CWP - 150 PSI SWP
- 2-1/4" 4" Rated: 400 PSI Non- Shock CWP - 100 PSI SWP
- CSA Gas Approved to 2" (IPS Only)

APPLICATIONS

· Commercial & Light Industrial/ Steam, Water, Oil, Gas, Air

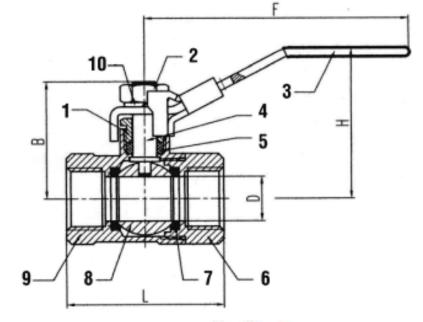


	HAN
ĩ_1	 Size
	1/4" - 1
114	3/4" –
×U.	1-1/4"
Q	

Size	L	В	Н	
1/4" - 1/2"	3.62	2.29	1.34	
3/4" – 1"	3.78	2.36	1.69	
1-1/4" - 2"	4.65	3.51	1.93	

DIMENSIONS

Size	L	D	Н	F	В
2"	4.41	1.97	3.45	5.91	2.96



MATERIAL SPECIFICATION

No.	Part	Material
1	Gland Nut	Brass
2	Hex Nut	Steel
3	Handle	Steel
4	Stem	Brass
5	Stem Packing	PTFE
6	End Cap	Brass
7	Seat	PTFE
8	Ball	C.P. Brass
9	Body	Brass
10*	Lock Washer	Steel

*Not present on 2-1/2" valves

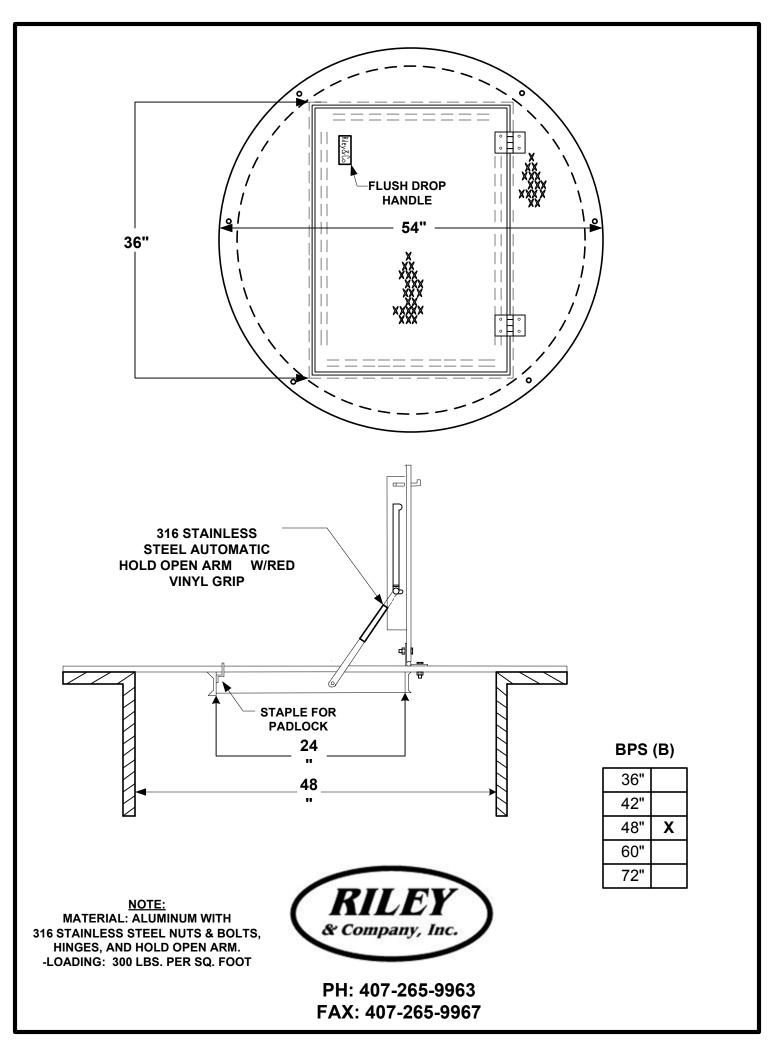


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 WEB: www.matco-norca.com
 EMAIL: mail@matco-norca.com





5491 BENCHMARK LANE SANFORD, FL 32773

PH. 407-265-9963 FX. 407265-9967

FLOAT SWITCHES

roto-float TYPE S-SUSPENDED & TYPEP- PIPE MOUNTED

TYPES-SUSPENDED & TYPE P-PIPE MOUNTED

The **ROTO-FLOAT** is a direct acting float switch. Each **ROTO- FLOAT** contains a single pole mercury switch which activates when the longitudinal axis of the float is horizontal, and deactuates when the liquid level falls 1" below the actuation elevation.

The float is a chemical resistant polypropylene casing with a firmly bonded electrical cable protruding. One end of the cable is permanently connected to the enclosed mercury switch and the entire assembly is encapsulated to form a completely watertight and impact resistant unit. Type S- Suspended has built in weight.

ROTO-FLOAT can be mounted on a support pip (typeP) or suspended from above (type S). Advantages of the **ROTO-FLOAT** are low cost, simplicity, and reliability

NOTE: Mercury switches are not to be used in potable wate

MATERIALS OF CONSTRUCTION

Float housing......Polypropylene Cable clamp......Polypropylen Cable jacket......PVC

CONTACTS: Normally Open

Applications *Pilot Duty *Industrial Control Equipment

CABLE

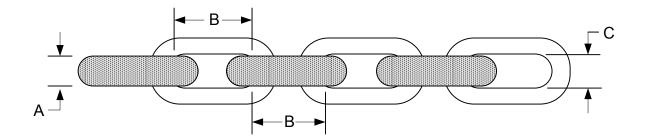
PVC type STO#18 conductors (41 strand) rated 600 volts *Various lengths available *See table of models *Non-standard lengths also available on special order

anchor scientific inc.

WWW.RILEYANDCO.COM

roto-float



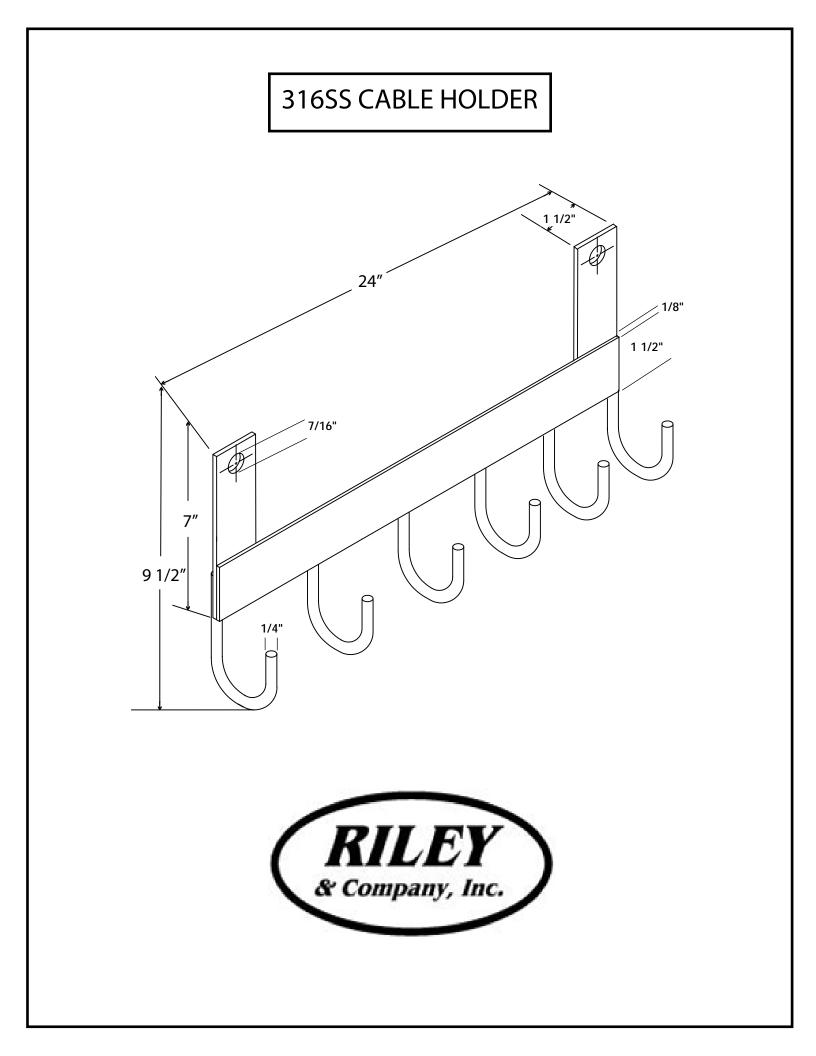


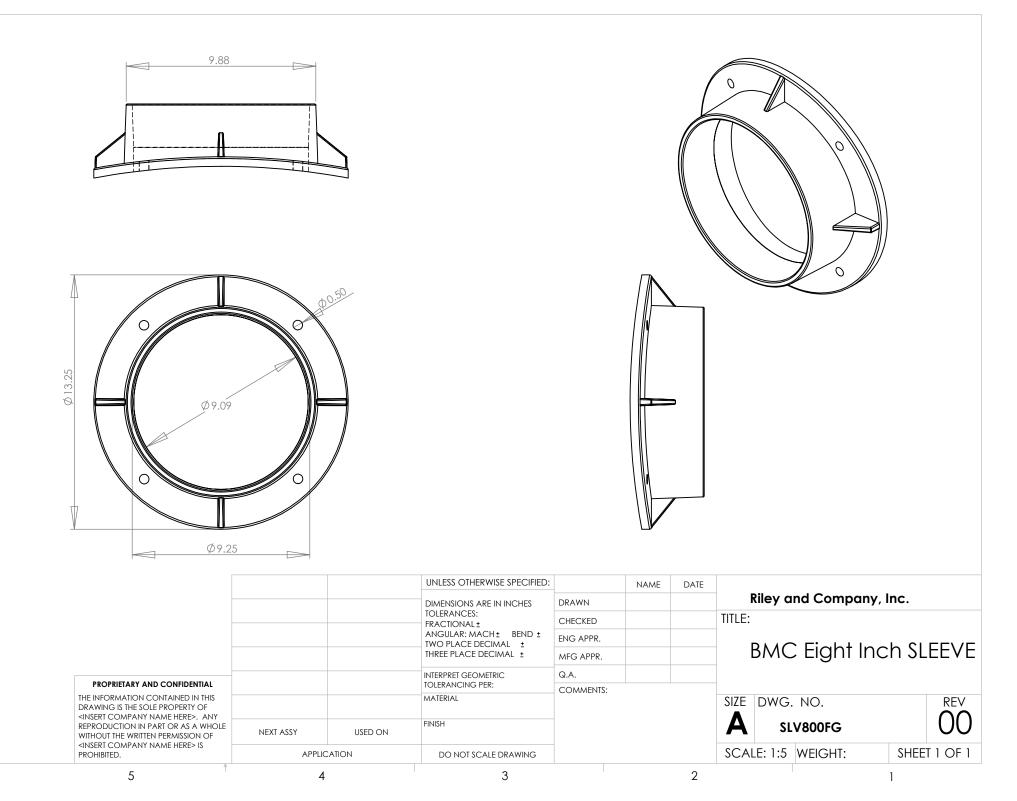
DIMENSIONS AND SPECS

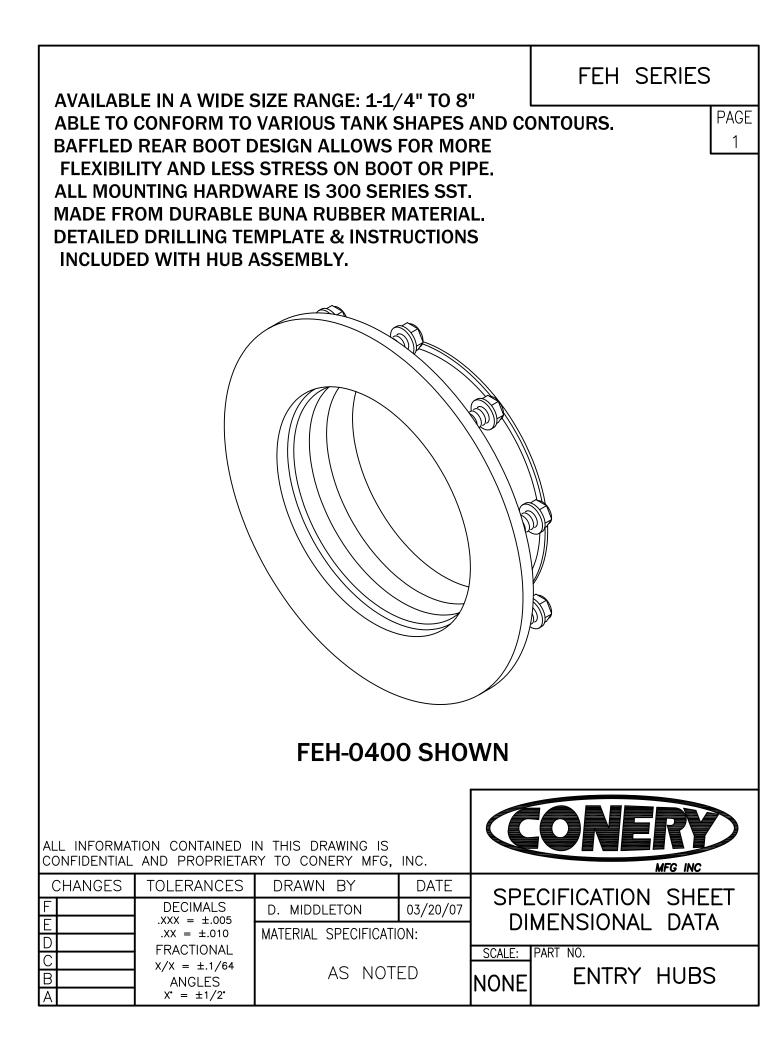
SIZE	А	В		С		WT/FT	WORKING LOAD LIMIT
3/16"	0.2	0.94		0.38		0.3	800
576/11	k k	<u>II INNI</u>	\overline{D}	1 <i>44</i> 61	\overline{D}	18981	

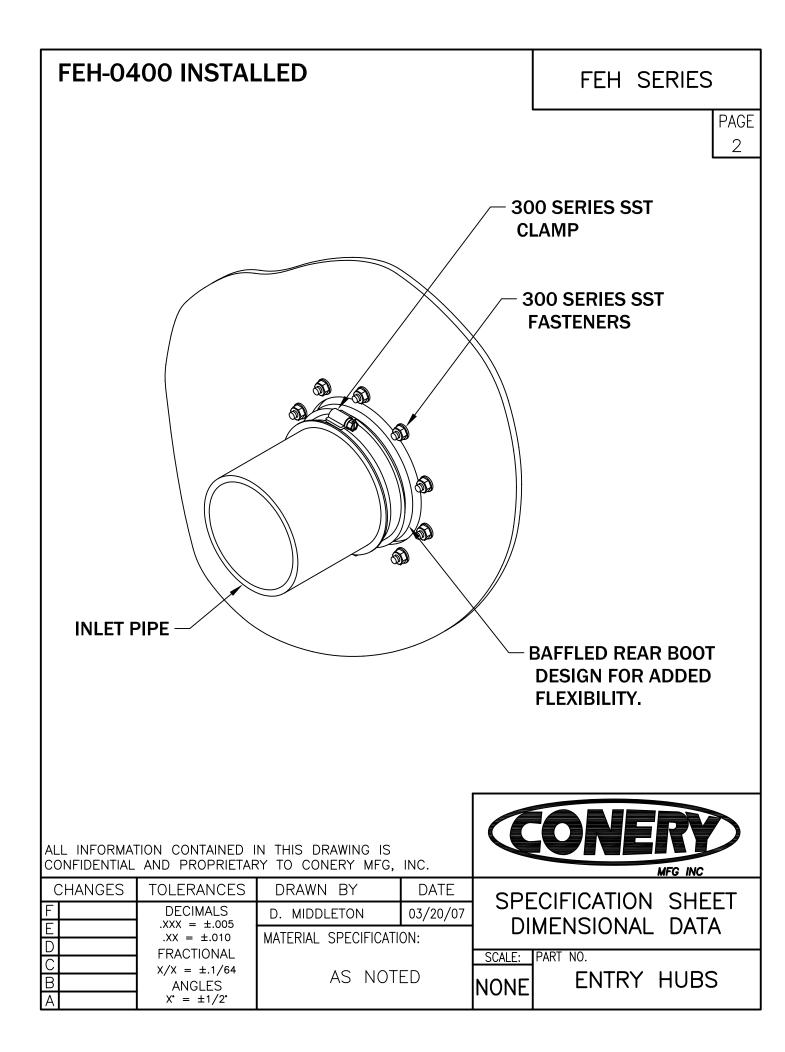


	TYPE OF MATERIAL
	304 STAINLESS STEEL
\square	316 STAINLESS STEEL
	SIZE
M	











				Cumulative	Cumulative
Depth	Area	Area	Volume	Volume	Volume
(ft - NGVD)	(sf)	(ac)	(ac-ft)	(ac-ft)	(cf)
80	103278	2.37	0.00	0.00	0
81	115389	2.65	2.51	2.51	109,334
82	131019	3.01	2.83	5.34	232,539
83	140406	3.22	3.12	8.45	368,252
84	150506	3.46	3.34	11.79	513,708
85	170629	3.92	3.69	15.48	674,277
86	185097	4.25	4.08	19.56	852,140
87	198951	4.57	4.41	23.97	1,044,165
88	212506	4.88	4.72	28.69	1,249,894
89	225955	5.19	5.03	33.73	1,469,125
90	239555	5.50	5.34	39.07	1,701,881
91	254064	5.83	5.67	44.74	1,948,692
92	269363	6.18	6.01	50.74	2,210,406

ASBUILT POND 3 VOLUME CALCULATION

Provided Volume w/ 1' of Freeboard

> 35.42 Ac-ft for 25-yr 24 hr

Provided Volume w/ 0.5' of Freeboard 47.74 Ac-ft

44.74 Ac-ft

> 47.17 Ac-ft for 100-yr 24 hr

Total Provided Volume Top of Pond 50.74 Ac-ft

Project | Enterprise Wetwell Design Project | 02000-144-14

Purpose: Determine wetwell working volume, associated working depth, and operational level.

Given:			
Pump Information			
1. Pump Manufacturer/Model =			
2. Design Pumping Capacity (Min Head Case) =	90	gpm	Qmh
3. Max # of Starts per Hour Avg Flow =	2		# of starts (Avg)
4. Max # of Starts per Hour Peak Flow =	12		# of starts (Peak; limited duration of operation <5 min per cycle)
Number of Pumps Used =	1		1 Pump Non-Alternating Operation.
Required Depth of Submergence of Pump =	0.17	ft	
Marken Hander and Aller			
Wetwell Information 1. Wetwell Inside Diameter =	4	ft	D
Wetwell Inside Diameter = Inflow Inv. Elevation =	4 74.95	ft	As-Built Info
3. Bottom of Wetwell Elevation =	74.95	ft	Bottom Well 71.95
4. Total Depth (Inv Elev to Bottom of Wetwell)	3.00	ft	Height Pum 1.58
			Top of Pumi 73.53
Required Depth of Submergence of Pump =	0.17	ft	Height of Water Above pump
6. Total Height of Pump =	1.58	ft	
Total Depth Required =	1.75	ft	Yes Total Depth is OK; water below invert
Working Depth volume from Pipe IE to Submergence)	1.25	ft	Difference from Pipe IE to Pump Submergence (Pump off)
la ferra la ferra a l'an	14.96	inches	
Inflow Information 1. Design Average Inflow (Qdai)=	5	gpm =	0.011 cfs
2. Design Peak Inflow (Qda)=	90	gpm =	0.200 cfs
		gpin -	0.200 013
Solution:			
Determine Actual Working Depth and Volume			
 Allowable Cycle Time (avg) = 	30	min	tc = 60/# of starts (Avg) No less than 5 minutes or
2. Allowable Cycle Time (peak) =	5	min	tc = 60/# of starts (Peak) the average cycle not exceed 30 minutes
3. Wetwell Volume per Foot of Depth =	94	gal/ft	$V/ft = \pi/4^*D^2$
 Selected Working Depth = Actual Working Volume = 	1.25	ft	da Va = (π/4*D^2)*7.48*da
5. Actual Working Volume -	117	gal	va = (n4 D Z) 7.40 ua
Determine Fill, Empty, and Cyle Times for Design Average In	flow		
 Time to Fill (Design Average Inflow) = 	23.44	min	tf = Va/Qdai 0.39 hours
2. Time to Empty (Design Average Inflow) =	1.38	min	te = V/(Qmh-Qdai) 0.02 hours
Cycle Time (Design Average Inflow) =	24.82	min	tc = tf + te 0.41 hours
			2.42 starts per hour
Determine Fill, Empty, and Cycle Times for Design Peak Inflo			
 Time to Fill (Peak Inflow) = Time to Empty (Peak Inflow) = 	1.30 1171.82	min min	tf = Va/Qpi te = V/(Qmh-Qpi)
 Cycle Time (Based on Peak Inflow) = 	1173.13	min	tc = tf + te
······································			
Proposed Operational Levels:			
HWL Alarm:	75.9		Ground EL = 90.5 ft
Lag Pump On: Lead Pump On:	74.9 74.9		Bottom EL = 71.95 ft Wetwell depth = 18.55 ft (min Static Head)
Lead Pump On. LWL/ All Pumps Off (Includes Redundancy):	74.9		Inflow Pipe Inv EL = 74.95 ft
Calculated Wetwell Bottom with water over pump:	73.7		Depth of Working Volume = 1.25 ft
Selected Wetwell Bottom:	71.9		
	Inflow Pipe		
0.5000		HWL ALAR	ξM
0.5000		LAG PUMP	
t EL74.95		LEAD PUMP	
WORKING WORKING VOLUME			
DEPTH, a			
EL73.53		ALL PUMPS	OFF (INCLUDES REDUNDANCY)
MINIMUM REQUIRED SUBMERGENCE			
EL 71.95			

Attachment C

CQA Engineer of Record Narrative Report

Construction Quality Assurance Engineerof Record Narrative Report

Enterprise Recycling and Disposal Facility Cell 16 Construction FDEP Permits No.: 177982-023-SC/T3 WACS No.: 87895

Prepared For:

Angelo's Aggregate Materials, Ltd. 855 28th Street South St. Petersburg, FL 33712

Prepared By:

epared By: CQA Engineer of Record: John P. Arnold, P.E Date: 1 State of Florida P.E. 0.: 36 McDuf Jacksonville, Tel.: (352) 339-1408

Background

This report documents the activities and methods of construction for Cell 16 (approximately 5.5 acres in size) in accordance with FDEP Permit No. 177982-023-SC/T3 and in response to the Department's 10/30/18 request for additional information.

Record Drawings of the as-built conditions, including the top of the 3' thick clay barrier layer were performed by Pickett and Associates, Inc. and John Arnold, P.E., with ground control provided by Simmons and Beall, Inc. Elevations of the excavation/undercut (prior to installation of the 3' thick clay barrier layer), top-of-clay (after installation of the 3' thick clay barrier layer), pump station (wetwell) and leachate collection pipe were performed by the Engineer of Record (Engineer) using the ground control data provided by Simmons and Beall, Inc. Topographic survey and elevation data were evaluated by the Engineer for conformance with the Department requirements. All Record Drawings are provided in Attachment B and include the clay perimeter berm and leachate collection system. The elevations on the surveys show that the subgrade was over-excavated by a minimum of 3-feet and then backfilled with clay to construct a 3' thick clay layer. The 3' clay layer (cell floor) was placed in three (3) approximately 12-inch thick lifts, with each lift being compacted. Geotechnical soils tests were performed on each completed clay lift of the 3' clay barrier layer to ensure the installed clay layer met the Department requirements in accordance with Appendix 3.2.a of the Operations Permit

Universal Engineering Sciences, Inc. (UES) performed all field and laboratory testing in accordance with the Construction Quality Assurance (CQA) requirements. Mr. John Arnold, P.E. served as the CQA professional engineer of record and he, or his designee was on-site at all times during construction to monitor construction activities.

Clay Layer Construction

Cell 16 was over-excavated by a minimum of 3 feet so that the finished 3-ft thick clay layer could be installed. The over-excavation was performed using tracked excavating equipment. The Engineer verified grades to ensure that the excavation was sufficient to meet the 3-foot over-excavation criteria. Clay was placed and compacted in the over-excavated using approximatly12-inch lifts to construct the clay layer. Clay was also placed and compacted in approximate 12-inch lifts to construct the perimeter berm (road). Signed and Sealed drawings documenting the As-Built conditions are provided in Attachment B.

Clay from on-site was used to construct the 3' clay layer and the clay berms that extend along the east and north sides of Cell 16. The clay was installed in approximately 12-inch lifts and compacted to within at least 95% of the maximum dry density in accordance with ASTM D698. The clay for each lift was spread with a bull dozer and compacted with multiple passes of loaded off-road (articulating) dump trucks. The in-place density and moisture content for the clay lifts of the 3' clay layer were evaluated by the Universal representative using nuclear-density testing and Speedy Moisture Content devices, respectively. Cell 16 was subdivided by row (1, 2, and 3) and columns (A and B) into sections for testing. Each section was less than 1 acre in size, which was the approved testing frequency used for in-place materials, per lift. Lifts were Revised to respond to 10/30/18 comments from the Department.

designated as Lift 1, 2, or 3 (from bottom to top). A figure depicting the Cell 16 Test Plan is attached.

The perimeter clay berm was constructed in approximate 12" lifts up to the finished grades, which are approximately 2' higher than in the permit plans. The perimeter berm was widened to accommodate truck 2-way traffic safely. The line and grade of the earthwork of the perimeter clay berm and Pond 3 are as represented on the Pickett topographic survey. The top of bank along the east side of the IW Pond was moved westward to preserve the existing 8' high landscaping berm that is required by the Pasco County conditional use. Pond 3 (IW Pond), as built, provides 50.74 Ac-ft of storage volume at elevation 92 and exceeds the required containment volume (runoff from 100-yr 24-hr storm) of 40.23 Ac-ft. The as-built IW Pond calculations are provided in Attachment B.

The UES field technician collected undisturbed Shelby tube samples for each test section of the 3' clay layer, per completed lift, to verify that the installed permeability met or exceeded the Department approved criteria. An additional sample was collected from the clay perimeter berm at the location of the wetwell at an approximate elevation of 85' for permeability testing. Permeability testing was performed on the undisturbed Shelby tube samples in the laboratory using a triaxial-permeameter device. The collected samples were also used to evaluate Atterberg Limits.

Results of the density, permeability, and moisture content tests, including the testing plan key map, are provided as Attachment D and show that the installed, compacted clay for the 3' clay layer and perimeter berm satisfied the maximum installed hydraulic permeability of 1x10-8 cm/sec.

Leachate Pipe and Wetwell

The leachate pipe along the north end of Cell 16 was installed by Comanco Environmental Corporation. The leachate pipe was 8" DIA SDR 17 HDPE and was fusion welded by Comanco. The perforated portion of the pipe included 3/8" DIA holes at 3" linear spacing per the approved drawings. The pipe was backfilled with No. 4 aggregate and encapsulated with non-woven filter fabric. The wet well was installed by Riley and Company, Inc. A copy of the start up test is provided and documents a flow rate of 86 (90 rounded up) gpm. A gradation test of the aggregate used to bed and backfill the leachate collection pipe is provided in Attachment B.

The wetwell installed was a 48" diameter fiberglass H20 rated system provided by Riley Pump, Inc. The pump start-up test (Attachment B) measured the installed capacity of the system to pump at 90 gpm. As-built elevations and hydraulic calculations for the system are provided in Attachment B.

The toe drain was constructed in accordance with the drawings. Non-woven geofabric was placed in the bottom of the trench and the pipe was placed on a 3-inch thick layer of gravel. The pipe was then backfilled with gravel and encased in the non-woven geofabric. The completed toe drain was covered with excess (surplus) No. 4 gravel that was on-site to provide additional protection to the installed system.

Limerock

Limerock was not observed or encountered within the area of Cell 16.

Field Inspection, Review, Conformance Assessment, and Major Deviations

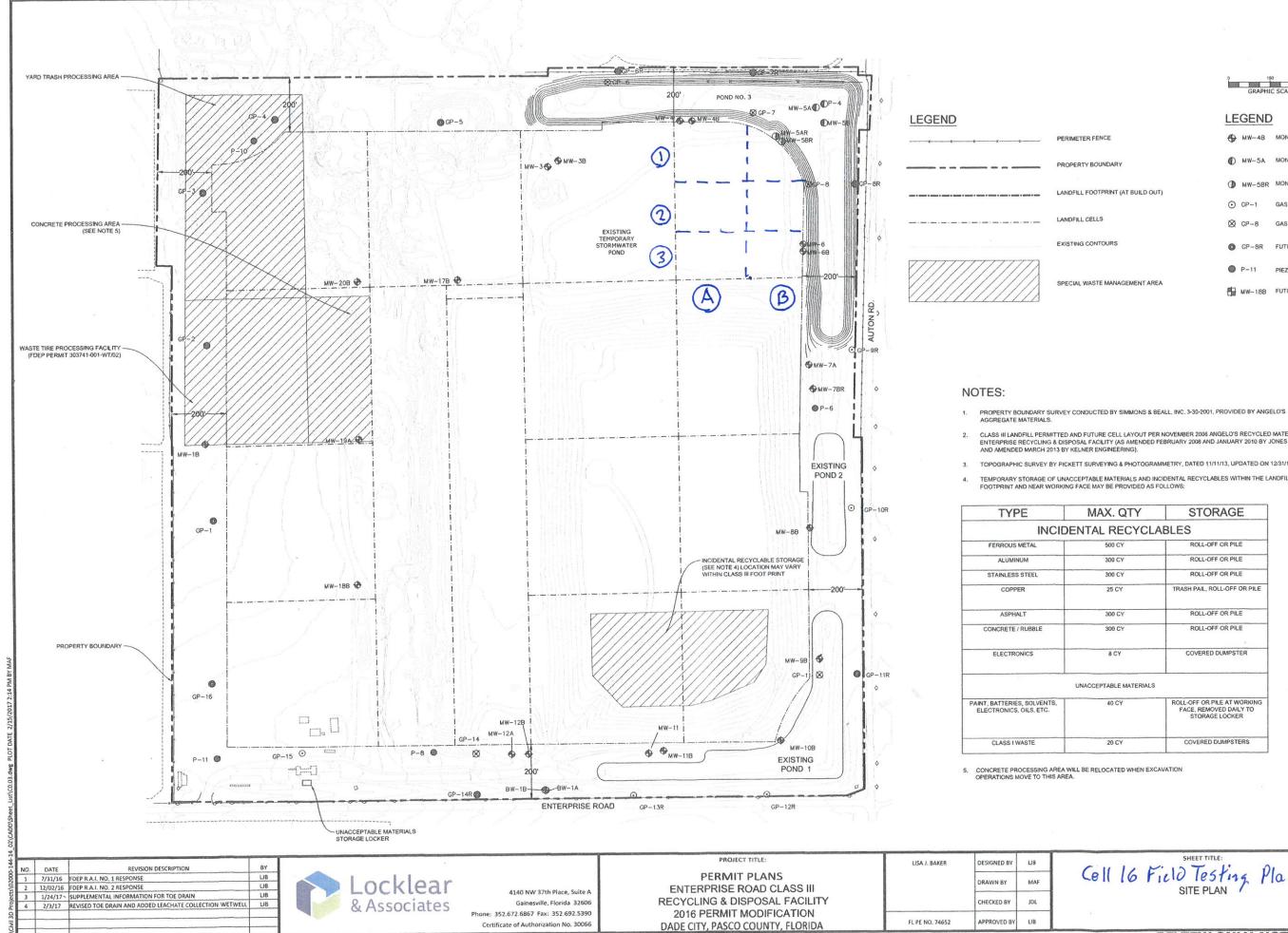
John Arnold, P.E., serving as the CQA Engineer of Record reviewed the UES Testing Report, As-Built (Record) drawings including Pickett topographic survey, performed daily field inspections/observations, and prepared and submitted this report and Certification of Construction Completion to the Department for review and approval. In accordance with requirements of Specific Condition 177982-023-SC/T3, Part B, 6.b.:

- 1. There were no occurrences of sinkholes, soft zones, ravel areas, or unstable conditions associated with the construction of Cell 16.
- 2. Deviations associated with the construction of Cell 16 maintain the approved functions and requirements. The capacity of the leachate collection pipe was increased by enlarging the diameter of the pipe from 6" to 8" HDPE (SDR 17) and will continue to convey the design leachate flow to the wet well. The wet well pumping rate remains unchanged and will convey leachate to Pond 3 as intended. The perimeter berm and resulting Pond 3 provide the required treatment volume.
- 3. Weekly progress meeting were informal and minutes were not taken.
- 4. Daily observation reports and photographs of construction activity are attached to this CQA Engineer of Record Narrative Report.

Summary

Review of the UES Testing Report, Record Drawings, and field observations during construction indicate that Cell 16 has been constructed in substantial accordance with the Department approved permit requirements. Specifically, the 3' clay layer and perimeter clay berm meet the maximum installed permeability requirement, the lift station pumps at the design rate of 90 gpm, and the leachate collection pipe in the toe drain has a greater conveyance capacity than the design pipe and will adequately convey the anticipated leachate generated in Cell 16. Changes to the line and grade of the perimeter berm maintain the minimum 200' setback from the property line and provide the required treatment volume for Pond 3.

Cell 16 Test Plan





LEGEND

NORTH	
11	1 ili

RIMETER FENCE
OPERTY BOUNDARY
NDFILL FOOTPRINT (AT BUILD OUT)
NDFILL CELLS
STING CONTOURS

SPECIAL WASTE MANAGEMENT AREA

•	MW-48	MONITORING WELL LOCATION
Ø	MW-5A	MONITORING WELL TO BE ABANDONE
0	₩₩58R	MONITORING WELL TO BE INSTALLED
\odot	GP-1	GAS PROBE LOCATION
\otimes	GP-8	GAS PROBE TO BE ABANDONED
0	GP-8R	FUTURE GAS PROBE LOCATION
•	P-11	PIEZOMETER WELL LOCATION
	MW-188	FUTURE MONITOR WELL LOCATION*

2. CLASS III LANDFILL PERMITTED AND FUTURE CELL LAYOUT PER NOVEMBER 2006 ANGELO'S RECYCLED MATERIALS ENTERPRISE RECYCLING & DISPOSAL FACILITY (AS AMENDED FEBRUARY 2008 AND JANUARY 2010 BY JONES EDMUNDS AND AMENDED MARCH 2013 BY KELNER ENGINEERING).

3. TOPOGRAPHIC SURVEY BY PICKETT SURVEYING & PHOTOGRAMMETRY, DATED (1/11/13, UPDATED ON 12/31/13.

TEMPORARY STORAGE OF UNACCEPTABLE MATERIALS AND INCIDENTAL RECYCLABLES WITHIN THE LANDFILL FOOTPRINT AND NEAR WORKING FACE MAY BE PROVIDED AS FOLLOWS:

MAX. QTY	STORAGE
ENTAL RECYCI	ABLES
500 CY	ROLL-OFF OR PILE
300 CY	ROLL-OFF OR PILE
300 CY	ROLL-OFF OR PILE
25 CY	TRASH PAIL, ROLL-OFF OR PILE
300 CY	ROLL-OFF OR PILE
300 CY	ROLL-OFF OR PILE
8 CY	COVERED DUMPSTER
UNACCEPTABLE MATERIAL	s
40 CY	ROLL-OFF OR PILE AT WORKING FACE, REMOVED DAILY TO STORAGE LOCKER
20 CY	COVERED DUMPSTERS

			EET TITLE:	
Cell	16	Field	Testing	Plan

ROJECT NO .:
02000-144-14
CALE:
AS SHOWN
DATE:
MARCH 2016
RAWING:
C0.03

REVIEW ONLY-NOT FOR CONSTRUCTION

Project Photographs

















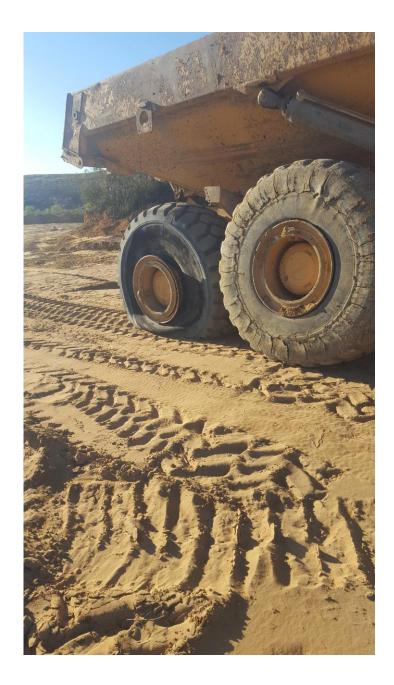






























3-inch deep gravel below pipe



Gravel over pipe prior to wrapping geotextile



Gravel over pipe prior to wrapping geotextile



Daily Observation Reports

	Resident	Temp.		
Date	Observer	(F)	Rainfall	Observations and Comments
4/24/17	JPA	72	0.00	Clearing vegetation from construction area
4/25/17	JPA	70	0.00	
4/26/17	JPA	74	0.00	
4/27/17	JPA	68	0.04	
4/28/17		67	0.00	
4/29/17				
4/30/17				
5/1/17	JPA	77	0.00	Clearing vegetation from construction area
5/2/17	JPA	79	0.31	
5/3/17	JPA	68	0.10	
5/4/17	JPA	68	0.00	
5/5/17	JPA	68	0.10	
5/6/17				
5/7/17				
5/8/17	JPA	76	0.00	
5/9/17	JPA	72	0.00	
5/10/17	JPA	74	0.00	Undercut of cell and berm areas
5/11/17	JPA	75	0.00	
5/12/17	JPA	78	0.00	
5/13/17				
5/14/17				
5/15/17		67	0.00	Wet conditions from weekend.
5/16/17	JPA	74	0.00	
5/17/17	JPA	73	0.00	
5/18/17		76	0.00	
5/19/17	JPA	78	0.00	
5/20/17				
5/21/17				
5/22/17		80		Wet conditions from weekend.
5/23/17		81	0.00	
5/24/17		79	0.10	
5/25/17		72	0.00	
5/26/17	JPA	72	0.00	
5/27/17				
5/28/17				
5/29/17		81		Earthwork cut
5/30/17	JPA	82	0.00	

	Resident	Temp.		
Date	Observer	(F)	Rainfall	Observations and Comments
5/31/17	JPA	79	0.10	
6/1/17	JPA	85	0.00	
6/2/17	JPA	82	0.10	
6/3/17				
6/4/17				
6/5/17	JPA	8	0.00	Earthwork cut
6/6/17	JPA	78	0.00	
6/7/17	JPA	78	0.00	
6/8/17	JPA	80	0.00	
6/9/17	JPA	79	0.00	
6/10/17				
6/11/17				
6/12/17	JPA	80	0.00	Grading and cut cell and pond area
6/13/17	JPA	80	0.00	
6/14/17	JPA	80	0.00	
6/15/17	JPA	80	0.00	
6/16/17	JPA	79	0.00	
6/17/17				
6/18/17				
6/19/17	JPA	80	0.00	Cut and grading cell and pond
6/20/17	JPA	81	0.00	
6/21/17	JPA	84	0.00	
6/22/17	JPA	84	0.00	
6/23/17	JPA	48	0.00	
6/24/17				
6/25/17				
6/26/17	JPA	80	0.20	Earthwork activities
6/27/17	JPA	80	0.00	
6/28/17	JPA	80	0.00	
6/29/17	JPA	79	0.00	
6/30/17	JPA	80	0.00	
7/1/17				
7/2/17				
7/3/17	JPA	8,281	0.20	Wet from 1" weekend rain
7/4/17	JPA	82	0.00	
7/5/17	JPA	82	0.00	
7/6/17	JPA	82	0.00	

	Resident	Temp.		
Date	Observer	(F)	Rainfall	Observations and Comments
7/7/17	JPA	82	0.10	
7/8/17				
7/9/17				
7/10/17	JPA	80	1.50	Earthwork activities
7/11/17	JPA	81	0.10	
7/12/17	JPA	80	0.20	
7/13/17	JPA	80	0.10	
7/14/17		80	0.17	
7/15/17				
7/16/17				
7/17/17	JPA	8,182	0.10	Coordinate clay backfill
7/18/17		80	0.30	
7/19/17		80	0.00	Clay haul to cell and berms
7/20/17		80	0.00	
7/21/17	JPA	80	0.10	
7/22/17				
7/23/17				
7/24/17	JPA	83	0.25	Earthwork activities and clay backfill
7/25/17	JPA	83	0.25	
7/26/17		83	0.00	
7/27/17	JPA	84	0.00	
7/28/17	JPA	84	0.20	
7/29/17				
7/30/17				
7/31/17		78	0.50	Earthwork activities and clay backfill
8/1/17		76	2.00	
8/2/17		78	0.00	
8/3/17		78	0.50	
8/4/17	JPA	76	1.50	
8/5/17				
8/6/17				
8/7/17		84	0.30	Prepare hurricane Irma
8/8/17		83		same
8/9/17	JPA	84	0.00	same
8/10/17	JPA	84	0.00	same
8/11/17	JPA	84	0.00	same
8/12/17				

	Resident	Temp.		
Date	Observer	(F)	Rainfall	Observations and Comments
8/13/17				
8/14/17	JPA	84	0.25	Evaluate rain. Wet conditions.
8/15/17	JPA	83	0.10	
8/16/17		83	0.00	
8/17/17		83	0.00	
8/18/17	JPA	83	0.10	
8/19/17				
8/20/17				
8/21/17		82	0.00	Earthwork activities and clay backfill
8/22/17	JPA	82	0.00	
8/23/17		82	0.00	
8/24/17		82	0.25	
8/25/17	JPA	83	0.00	
8/26/17				
8/27/17				
8/28/17	JPA	80	1.50	Earthwork activities and clay backfill
8/29/17	JPA	80	0.50	
8/30/17	JPA	80	0.30	
8/31/17	JPA	82	0.00	
9/1/17	JPA	82	0.25	
9/2/17				
9/3/17				
9/4/17	JPA	81	1.00	Earthwork activities and clay backfill
9/5/17	JPA	81	0.00	
9/6/17		81	0.00	
9/7/17	JPA	8	0.00	
9/8/17	JPA	82	0.30	
9/9/17				
9/10/17				
9/11/17	JPA	83	2.00	Earthwork activities and clay backfill
9/12/17	JPA	81	0.00	
9/13/17	JPA	83	0.00	
9/14/17		82	0.25	
9/15/17	JPA	82	0.11	
9/16/17				
9/17/17				
9/18/17	JPA	80	0.00	Earthwork activities and clay backfill

	Resident	Temp.		
Date	Observer	(F)	Rainfall	Observations and Comments
9/19/17	JPA	80	0.00	
9/20/17	JPA	80	0.00	
9/21/17	JPA	80	0.00	
9/22/17	JPA	80	0.00	
9/23/17				
9/24/17				
9/25/17	JPA	80	0.00	Earthwork activities and clay backfill
9/26/17	JPA	81	0.00	
9/27/17	JPA	81	0.00	
9/28/17	JPA	81	0.00	
9/29/17	JPA	81	0.40	
9/30/17				
10/1/17				
10/2/17	JPA	76	0.00	Earthwork activities and clay backfill
10/3/17	JPA	76	0.30	
10/4/17	JPA	78	0.00	
10/5/17	JPA	78	0.10	
10/6/17	JPA	76	0.25	
10/7/17				
10/8/17				
10/9/17		78	0.35	Earthwork activities and clay backfill
10/10/17	JPA	78	0.10	
10/11/17	JPA	79	0.00	
10/12/17		80	0.00	
10/13/17	JPA	82	0.00	
10/14/17				
10/15/17				
10/16/17	JPA	74	0.00	Earthwork activities and clay backfill
10/17/17	JPA	74	0.15	
10/18/17	JPA	77	0.00	
10/19/17	JPA	78	0.00	
10/20/17	JPA	78	0.00	
10/21/17				
10/22/17				
10/23/17		71	1.00	Earthwork activities and clay backfill
10/24/17	JPA	71	0.10	
10/25/17	JPA	65	0.00	

	Resident	Temp.		
Date	Observer	(F)	Rainfall	Observations and Comments
10/26/17	JPA	59	0.00	
10/27/17	JPA	62	0.00	
10/28/17				
10/29/17				
10/30/17		57	0.00	Earthwork activities and clay backfill
10/31/17	JPA	64	0.00	
11/1/17		68	0.00	
11/2/17		70	0.00	
11/3/17	JPA	70	0.00	
11/4/17				
11/5/17				
11/6/17		68		Earthwork activities and clay backfill
11/7/17		72	0.00	Universal Site Visit
11/8/17		70	0.00	
11/9/17	JPA	65	0.00	
11/10/17	JPA	68	0.00	
11/11/17				
11/12/17				
11/13/17	JPA	74	0.00	Earthwork activities and clay backfill
11/14/17		70	0.00	
11/15/17		64	0.00	
11/16/17		64	0.00	Universal Testing Soil Sample Collection
11/17/17	JPA	62	0.00	
11/18/17				
11/19/17				
11/20/17		60		Earthwork activities and clay backfill
11/21/17		66	0.00	
11/22/17		70		Universal Testing Soil Sample Collection
11/23/17		72	0.20	
11/24/17		63	0.20	
11/25/17				
11/26/17				
11/27/17		62		Earthwork activities and clay backfill
11/28/17		69	0.00	
11/29/17		72	0.00	
11/30/17		68	0.00	
12/1/17	JPA	66	0.00	

	Resident	Temp.		
Date	Observer	(F)	Rainfall	Observations and Comments
12/2/17				
12/3/17				
12/4/17	JPA	66	0.00	Earthwork activities and clay backfill
12/5/17	JPA	66	0.00	
12/6/17	JPA	68	0.00	
12/7/17	JPA	70	0.00	
12/8/17	JPA	66	0.20	
12/9/17				
12/10/17				
12/11/17	JPA	46	0.00	Earthwork activities and clay backfill
12/12/17		54	0.00	
12/13/17		50	0.00	
12/14/17	JPA	55	0.00	
12/15/17	JPA	64	0.00	
12/16/17				
12/17/17				
12/18/17		70	0.00	Earthwork activities and clay backfill
12/19/17	JPA	68	0.00	
12/20/17	JPA	68	0.00	
12/21/17		70	0.00	
12/22/17		63	0.00	
12/23/17				
12/24/17				
12/25/17	JPA	66	0.10	Earthwork activities and clay backfill
12/26/17	JPA	63	0.00	
12/27/17		66	0.00	
12/28/17		65	0.00	
12/29/17	JPA	70	0.00	
12/30/17				
12/31/17				
1/1/18		48		Equipment - off road truck out
1/2/18		44	0.00	
1/3/18		46	0.00	
1/4/18		38	0.00	
1/5/18	JPA	41	0.00	
1/6/18				
1/7/18				

	Resident	Temp.		
Date	Observer	(F)	Rainfall	Observations and Comments
1/8/18	JPA	69	0.00	Earthwork activities and clay backfill
1/9/18	JPA	69	0.50	
1/10/18	JPA	67	0.00	
1/11/18		66	0.00	
1/12/18	JPA	69	0.21	
1/13/18				
1/14/18				
1/15/18		63		No work
1/16/18		64	0.00	
1/17/18		66	0.00	
1/18/18		39	0.00	
1/19/18	JPA	48	0.00	
1/20/18				
1/21/18				
1/22/18	JPA	66	0.10	No work
1/23/18	JPA	56	0.00	
1/24/18	JPA	66	0.00	
1/25/18	JPA	66	0.00	
1/26/18	JPA	67	2.00	
1/27/18				
1/28/18				
1/29/18		62	0.00	No work
1/30/18		64	0.00	
1/31/18		65	0.00	
2/1/18		64	0.00	
2/1/18		64	0.50	
2/2/18		04	0.50	
2/3/18				
2/4/18		00	0.00	No work
		66		
2/6/18		68	0.00	
2/7/18		70	0.00	
2/8/18		72	0.00	
2/9/18		76	0.00	
2/10/18				
2/11/18				

	Resident	Temp.		
Date	Observer	(F)	Rainfall	
2/12/18	JPA	76	0.50	No work
2/13/18	JPA	73	0.00	
2/14/18	JPA	72	0.00	
2/15/18	JPA	72	0.00	
2/16/18	JPA	72	0.00	
2/17/18				
2/18/18				
2/19/18	JPA	77	0.00	Earthwork activities and clay backfill
2/20/18	JPA	78	0.10	
2/21/18	JPA	72	0.00	
2/22/18	JPA	74	0.00	
2/23/18	JPA	74	0.00	
2/24/18				
2/25/18				
2/26/18	JPA	72	0.00	Earthwork activities and clay backfill
2/27/18	JPA	72	0.00	
2/28/18	JPA	72	0.00	
3/1/18	JPA	60	0.00	
3/2/18	JPA	58	0.00	
3/3/18				
3/4/18				
3/5/18	JPA	51	0.00	Earthwork activities and clay backfill
3/6/18	JPA	52	0.00	
3/7/18	JPA	69	0.00	
3/8/18	JPA	60	0.11	
3/9/18	JPA	52	0.00	
3/10/18				
3/11/18				
3/12/18		53	0.10	Earthwork activities and clay backfill
3/13/18		62	0.00	
3/14/18	JPA	70	0.00	
3/15/18	JPA	59	0.00	
3/16/18		61	0.00	
3/17/18				

	Resident	Temp.		
Date	Observer	(F)	Rainfall	Observations and Comments
3/18/18				
3/19/18	JPA	60	0.00	Earthwork activities and clay backfill
3/20/18	JPA	62	0.00	
3/21/18	JPA	63	0.00	
3/22/18	JPA	65	0.00	
3/23/18	JPA	68	0.00	
3/24/18				
3/25/18				
3/26/18		67	0.00	Earthwork activities and clay backfill
3/27/18	JPA	69	0.75	
3/28/18	JPA	74	0.25	
3/29/18	JPA	68	0.00	
3/30/18	JPA	74	0.00	
3/31/18				
4/1/18				
4/2/18	JPA	72	0.75	Earthwork activities and clay backfill
4/3/18	JPA	70	0.25	
4/4/18	JPA	72	0.00	
4/5/18	JPA	66	0.00	
4/6/18	JPA	68	0.00	
4/7/18				
4/8/18				
4/9/18	JPA	66	1.00	No work
4/10/18	JPA	70	3.50	
4/11/18	JPA	66	0.15	
4/12/18	JPA	65	0.00	
4/13/18	JPA	68	0.00	
4/14/18				
4/15/18				
4/16/18			0.00	Earthwork activities and clay backfill
4/17/18	JPA		0.00	
4/18/18	JPA		0.30	
4/19/18			0.20	
4/20/18	JPA		0.00	

	Resident	Temp.		
Date	Observer	(F)	Rainfall	Observations and Comments
4/21/18				
4/22/18				
4/23/18	JPA		0.00	Earthwork activities and clay backfill
4/24/18	JPA		0.00	
4/25/18	JPA		0.00	Universal Testing
4/26/18	JPA		0.00	
4/27/18	JPA		0.00	
4/28/18	JPA			
4/29/18	JPA			
4/30/18	JPA		0.00	Clay installation substantially complete
5/1/18	JPA		0.00	Pump Station Start Up
5/2/18	JPA		0.00	
5/3/18	JPA		0.00	
5/4/18	JPA		0.00	
5/5/18	JPA			
5/6/18	JPA			
5/7/18	JPA		0.25	
5/8/18	JPA		0.20	
5/9/18	JPA		0.00	
5/10/18	JPA		0.00	
5/11/18	JPA		0.00	Field Elevations
5/12/18	JPA			
5/13/18	JPA			

Attachment D

Construction Quality Assurance Test Results Universal Engineering Science, Inc.



LOCATIONS: Atlanta

- Daytona Beach
- Fort Myers
- Fort Pierce
- Gainesville
- Jacksonville
- Miami
- Ocala
 Orlando (Heador
- Orlando (Headquarters)
 Palm Coast
- Panama City
- Pensacola
- Rockledge
- Sarasota
- St. Petersburg
 Tampa
- Tampa
 Tifton

October 11, 2018

West Palm Beach

Angelo's Recycled Materials 41111 Enterprise Road Dade City, Florida 33525

Attention: John Arnold

Reference: John Arnold Enterprise Class III Landfill Cell 16 Dade City, Florida UES Project No. 0810.1500214.0000

Mr. Arnold:

Pursuant to your request, please find attached all related testing reports for the Landfill Cell 16. This letter certifies reports for:

- Proctor Report: Page 1-11
- Permeability Reports: Pages 12-13
- Site Density Reports: Pages 14-16

We trust that these testing reports bound herein, are acceptable to your current needs. However, if you should require additional information please contact us.

We appreciate the opportunity to work with you on this project and look forward to a continued association with Angelo's Recycled Materials. Please do not hesitate to contact us if you should have any questions or if we may further assist you as your plans proceed.

Respectfully submitted, UNIVERSAL ENGINEERING SCIE	INCES, INC.
Certificate of Authorization No.000	00549
IN A LICENSE	
Mattin 57000	111111
NO. 57233	1111
Mark Hardy, P.E.	*=
Tampa Regional Manager OF	E
Florida PE Registration Number 57	233
	93
	Nº.
Date: SSIONAL ENGIN	S.
(III)	
and the more	

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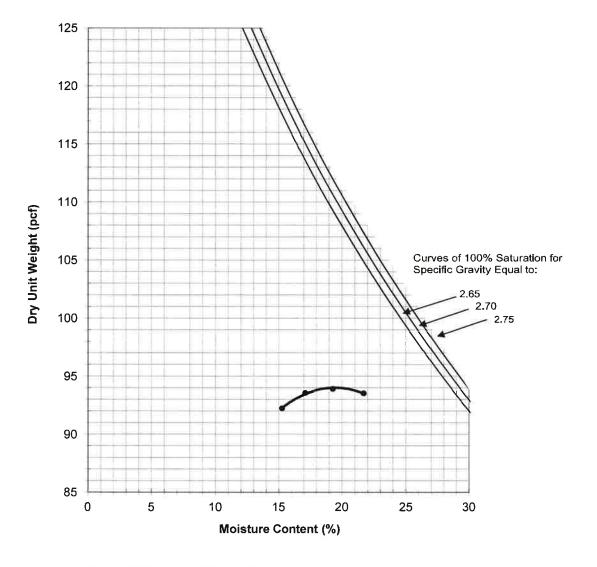


Project Number: 810.1500214.00 Lab Sample No.: 18-P1364 Work Order No.: 81060

Client:	Angelo's Materials		
Project Name:	Cell 16		
Sample Location:	Cell 16 Section 1 Lift 2	Area Covered:	N/A
Intended Use:	Other		
Material Description:	Clay	Date Plotted:	August 3, 2018
Sampled By:	MA	Tested By:	ECV
Date Sampled:	April 24, 2018	Date Tested:	August 2, 2018

SUMMARY OF TEST RESULTS

Test Method:	AASHTO T-99 Met	nod A	
Maximum Dry Density, pcf:	94.0		
Optimum Moisture, %:	19.5		
Passing 3.5" Sieve, %:	100	AASHTO T-89 (LL):	N/A
Passing No. 4 Sieve, %:	100	AASHTO T-90 (PI):	N/A
Passing No. 200 Sieve, %:	53.7	AASHTO M-145 (Class,):	N/A



Sampled according to AASHTO T 002. Proctor plotted by software package.

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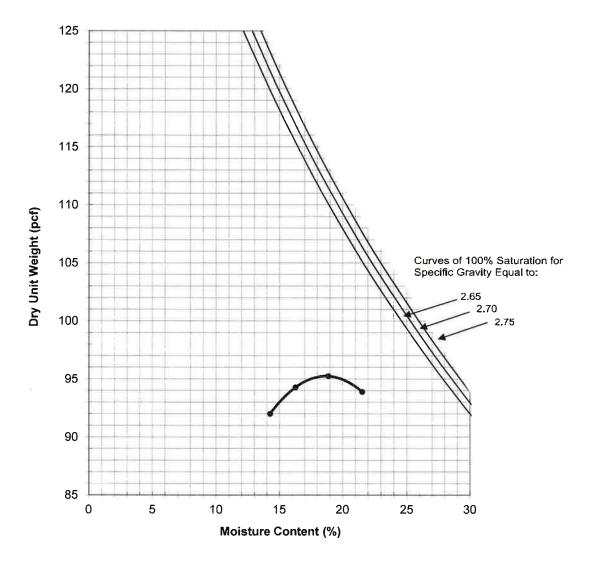


Project Number: 810.1500214.00 Lab Sample No.: 18-P1297 Work Order No.: 81060

Client:	Angelo's Materials		
Project Name:	Cell 16		
Sample Location:	Cell 16Section 1 Lift 2-2	Area Covered:	N/A
Intended Use:	Other		
Material Description:	Clay	Date Plotted:	July 23, 2018
Sampled By:	MA	Tested By:	ECV
Date Sampled:	April 24, 2018	Date Tested:	July 20, 2018
	SUMMARY OF TEST F	RESULTS	-
	Test Method:	AASHTO T-99 Met	hod A
	Maximum Dry Density, pcf:	95.3	

laximum Dry	Density, pct:	95.3	
Optimum	Moisture, %:	18.7	

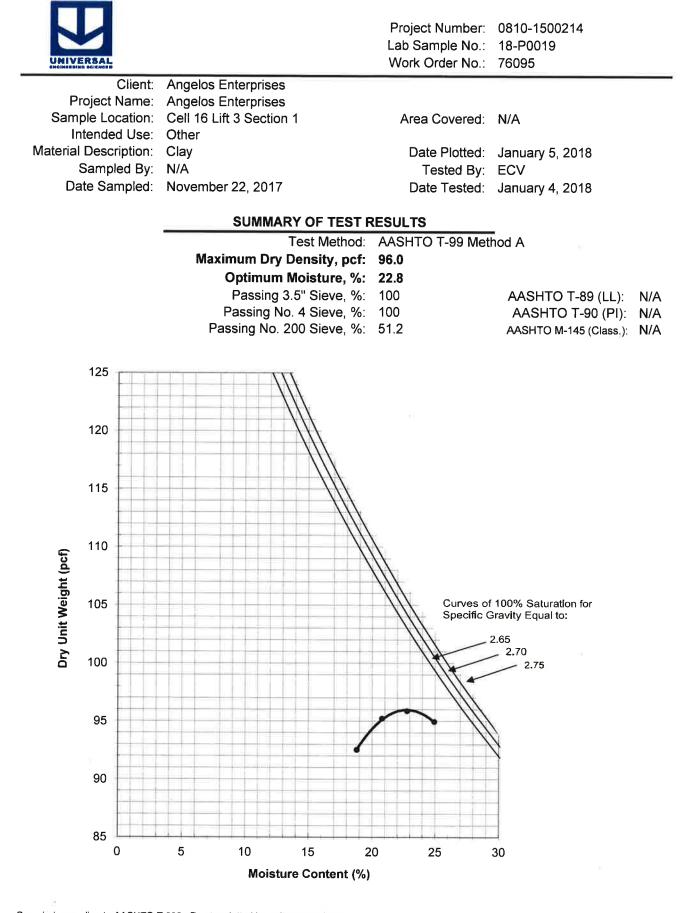
• • • • • • • • • • • • • • • • • • • •			
Passing 3.5" Sieve, %:	100	AASHTO T-89 (LL):	N/A
Passing No. 4 Sieve, %:	100	AASHTO T-90 (PI):	N/A
Passing No. 200 Sieve, %:	52.3	AASHTO M-145 (Class.)	N/A



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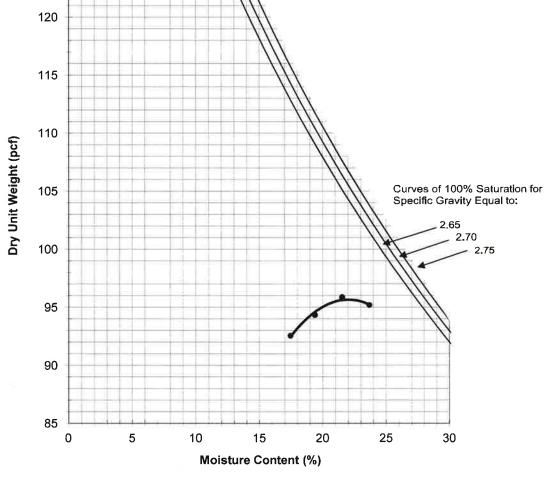
					Lab	-	r: 0810-1500214 .: 17-P1941 .: 75966
	oject Name	e: Angelos	Materials I	Enterprises Enterprises			
	ple Location	n: Cell 16 :	# 5 Lift 1 Se	ection 3-1	A	rea Covered	d: N/A
	Description					Date Plotted	d: January 2, 2018
	Sampled B					Tested By	y: ECV
Da	ite Sample	: Novemb	per 16, 201	7		Date Tester	d: December 29, 2017
			SUMMA	ARY OF TES	T RESUL	TS	
						HTO T-99 M	lethod A
		Ма		y Density, p		2	
			-	n Moisture,			
				3.5" Sieve, No. 4 Sieve,			AASHTO T-89 (LL): N/A AASHTO T-90 (PI): N/A
		F		. 200 Sieve,			AASHTO M-145 (Class.); N/A
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	90						
	85 — 0	5	10	15	20	25	30
	85	5	10 Mois	15 ture Content	20	25	30





Project Number: 0810-1500214 Lab Sample No.: 18-P0028 Work Order No.: 76095

Client:	Angelos Enterprises			
Project Name:	Angelos Enterprises			
Sample Location:	Cell 16 Lift 1 Section 1-2	Area Covered:	N/A	
Intended Use:	Other			
Material Description:	Clay	Date Plotted:	January 9, 2018	
Sampled By:	N/A	Tested By:	ECV	
Date Sampled:	November 22, 2017	Date Tested:	January 8, 2018	
	SUMMARY OF TEST F	RESULTS	_	
	Test Method:	AASHTO T-99 Met	hod A	
	Maximum Dry Density, pcf:	95.7		
	Optimum Moisture, % :	22.0		
	Passing 3.5" Sieve, %:	100	AASHTO T-89 (LL): 4	‡ 7
	Passing No. 4 Sieve, %:	100	AASHTO T-90 (PI): 4	11
	Passing No. 200 Sieve, %:	51.9	AASHTO M-145 (Class.): A	۹-7-5
405				
125				
120				
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Project Number: 0810-1500214 Lab Sample No.: 17-P1893 Work Order No.: 76095

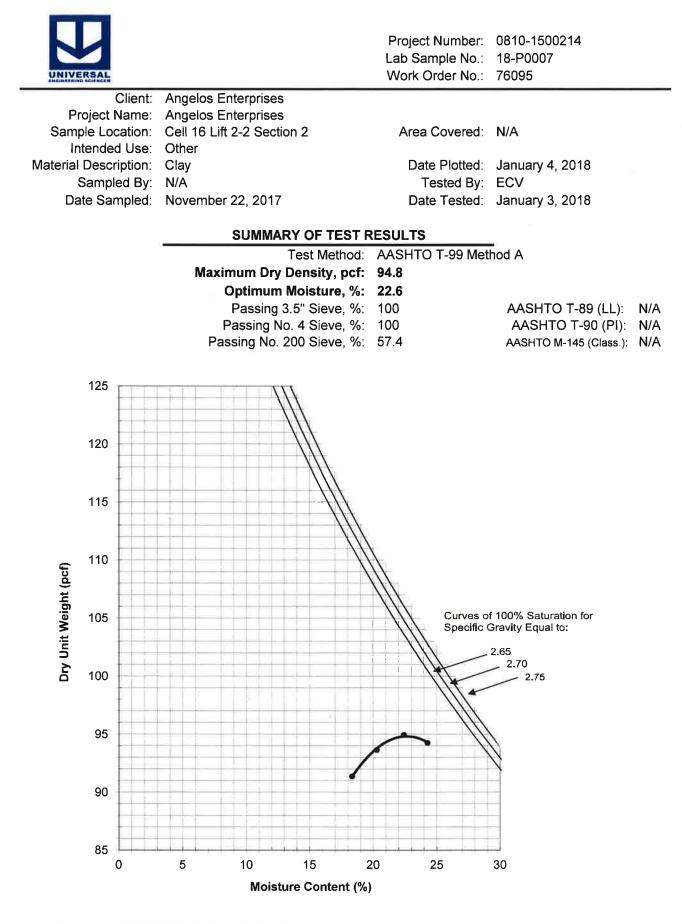
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Dry Unit Weight (pcf)	05						f 100% Saturation for Gravity Equal to:
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11	5						
12	20					3	
12	25			Ħ			
			Optimum Passing Passing N	Density, pcf Density, pcf Moisture, % 3.5" Sieve, % o. 4 Sieve, % 200 Sieve, %	95.6 23.2 100 100	TO T-99 Met	AASHTO T-89 (LL): N/A AASHTO T-90 (PI): N/A AASHTO M-145 (Class.): N/A
			SUMMA	RY OF TEST			
		Novembe	r 22, 2017		۵	-	December 20, 2017
Material De		Clay N/A			C	Date Plotted: Tested By:	December 21, 2017 ECV
Sample	Location: ded Use:	Cell 16 Lif			Ar	ea Covered:	N/A
Projo		Angelos E Angelos E					



Project Number: 0810-1500214 Lab Sample No.: 17-P1870 Work Order No.: N/A

CI	lient:	Angelos E	nterprises	s			
Project Na		Angelos E					
Sample Loca		Cell 16 Lif	t 3 Sectio	n 2-2	Are	ea Covered:	N/A
Intended	Use:	Other					
Material Descrip	tion:	Clay			C	Date Plotted:	December 18, 2017
Sampled	d By:	MA				Tested By:	ECV
Date Samp	pled:	November	22, 2017	7	C	Date Tested:	December 15, 2017
						-	
			SUMMA	RY OF TEST			hod A
		Mavi		Density, pcf:		10 1-33 Met	
			-	Moisture, %:			
			-	3.5" Sieve, %:			AASHTO T-89 (LL): N/A
		1		10. 4 Sieve, %:			AASHTO T-90 (PI): N/A
				200 Sieve, %:			AASHTO M-145 (Class.): N/A
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			WOIST	ture Content (%)		

Sampled according to AASHTO T 002. Proctor plotted by software package. This report shall not be reproduced, except in full, without the written approval of Universal Engineering Sciences, Inc. -



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Project Number:0810-1500214Lab Sample No.:17-P1869Work Order No.:N/A

	Client	Angelos Er	torprisos				
Dr	roject Name						
	ple Location	-			٨٢	ea Covered:	N/A
	ntended Use		1 Section 2		AI	ea Covereu.	N/A
					r		December 19, 2017
	Description	-			L	Date Plotted:	,
	Sampled By		00 0047		-	Tested By:	
Da	ate Sampled	November	22, 2017		L	Jate Tested:	December 15, 2017
						T O	
			SUMMARY				bod A
						ITO T-99 Met	nod A
			num Dry Do				
			Optimum M				
			Passing 3.5				AASHTO T-89 (LL): N/A
			assing No.				AASHTO T-90 (PI): N/A
		Pas	sing No. 20	0 Sieve, %	: 52.7		AASHTO M-145 (Class.): N/A
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Dry Unit Weight (pcf)					///		
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			Moisture	e Content (6)		

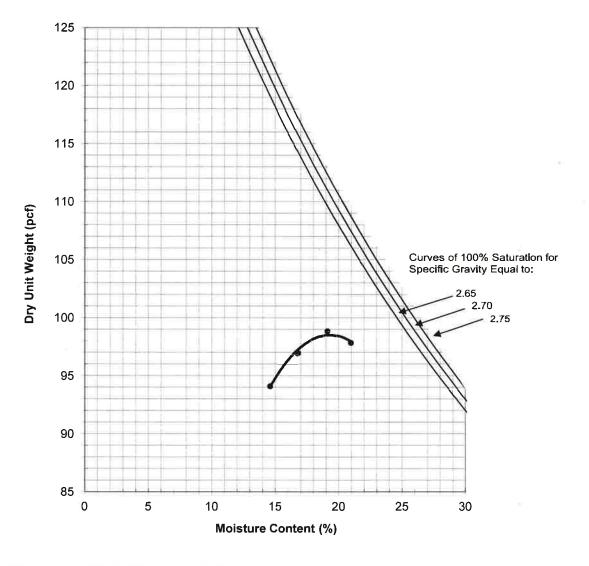


 Project Number:
 810.1500214.00

 Lab Sample No.:
 18-P1403

 Work Order No.:
 81060

Client:	Angelo's Materials		,	
Project Name:	Cell 16			
Sample Location:	Cell 16 Lift 3 Location 3-1	Area Covered:	N/A	
Intended Use:	Other			
Material Description:	Clay	Date Plotted:	August 9, 2018	
Sampled By:	MA	Tested By:	ECV	
Date Sampled:	April 24, 2018	Date Tested:	August 8, 2018	
	SUMMARY OF TEST F	RESULTS	_	
	Test Method:	AASHTO T-99 Met	hod A	
	Maximum Dry Density, pcf:	98.5		
	Optimum Moisture , %:	19.3		
	Passing 3.5" Sieve, %:	100	AASHTO T-89 (LL):	N/A
	Passing No. 4 Sieve, %:	100	AASHTO T-90 (PI):	N/A
	Passing No. 200 Sieve, %:	54.3	AASHTO M-145 (Class.):	N/A
	9		· · · ·	



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Project Number: 810.1500214.00 Lab Sample No.: 18-P1364 Work Order No.: 81060

30

ENGINEEATH			WORK OFGET NO.:	01000	
		Angelo's Materials			
	roject Name:	Cell 16			
		Cell 16 Section 1 Lift 2-1	Area Covered:	N/A	
	ntended Use:				
	Description:	-		August 3, 2018	
	Sampled By:	MA	Tested By:		
Da	ate Sampled:	April 24, 2018	Date Tested:	August 2, 2018	
		SUMMARY OF TEST F	RESULTS	_	
		Test Method:	AASHTO T-99 Met	hod A	
		Maximum Dry Density, pcf:	94.0		
		Optimum Moisture, %:	19.5		
		Passing 3.5" Sieve, %:	100	AASHTO T-89 (LL):	N/A
		Passing No. 4 Sieve, %:	100	AASHTO T-90 (PI):	N/A
		Passing No. 200 Sieve, %:	53.7	AASHTO M-145 (Class.):	N/A
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15

Moisture Content (%)

20

25

10



Angelo's Materials's

LOCATIONS: Atlanta Daytona Beach Fort Myers Fort Pierce Gainesville Jacksonville •Miami Ocala Orlando (Headquarters) Palm Coast Panama City Pensacola Rockledge Sarasota Tampa Tifton

Project: Enterprise Class 3 Cell 16

Client:

ſ

REPORT ON TRIAXIAL PERMEABILITY AND PERCENT PASSING NO. 200 SIEVE (ASTM D-5084 and ASTM C-117) (AASHTO T-11)

 Date Tested:
 5/22/2018
 Tested By:
 DL

 Date Sampled:
 11/22/2017
 Sampled By:
 MA

 Sample Ran At:
 Permeability:

		Sample Ran At:		Permeability:			
Sample Location	Percent Passing No. 200 Sieve	Moisture Content (%)	Dry Unit Weight (pcf)	K (cm/s)	K (ft/d)		
Berm	55.30	24.00	99.60	1.59 X 10 ⁻⁹	4.51 X 10 ⁻⁶		
Header Trench	53.60	23.00	97.20	4.45 X 10 ⁻⁹	1.26 X 10 ⁻⁵		
				3			

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Angelo's Materials's

LOCATIONS: Atlanta Daytona Beach Fort Myers Fort Pierce Gainesville Jacksonville

 Jackso
#Miam

Miami	
Ocala	

Orlando (Headquarters)

- Palm Coast
- Panama City
- Pensacola
 Rockledge
- *Sarasota
- Tampa
 Tifton

Enterprise Class 3 Cell 16

REPORT ON TRIAXIAL PERMEABILITY AND PERCENT PASSING NO. 200 SIEVE (ASTM D-5084 and ASTM C-117) (AASHTO T-11)

Date Tested: 5/23/17-8/10/18 Date Sampled: 11/7/2017-4/24/2018

Client:

Project:

Tested By: _____ Sampled By: _____

DL

		Sample	Ran At:	Perme	ability:
Sample Location	Percent Passing No. 200 Sieve	Moisture Content (%)	Dry Unit Weight (pcf)	K (cm/s)	K (ft/d)
Section 1 L1 A	57.00	20.50	98.10	1.41 x 10 ⁻⁸	3.99 X 10 ⁻⁵
Section 1 L1 B	51.90	22.00	95.70	6.01 X 10 ⁻⁹	1.70 X 10 ⁻⁵
Section 1 L2 A	53.70	19.50	94.00	6.05 X 10 ⁻⁹	1.72 X 10 ⁻⁵
Section 1 L2 B	52.30	18.70	95.30	3.36 X 10 ⁻⁹	9.52 X 10 ⁻⁶
Section 1 L3 A	49.30	19.70	101.20	2.24 X 10 ⁻⁹	6.35 X 10 ⁻⁶
Section 1 L3 B	51.20	22.80	96.00	2.47 X 10 ⁻⁹	7.00 X 10 ⁻⁶
Section 2 L1 A	54.30	20.00	94.00	5.2 X 10 ⁻⁹	1.47 X 10 ⁻⁵
Section 2 L1 B	52.70	20.30	94.20	2.06 X 10 ⁻⁹	5.84 X 10 ⁻⁶
Section 2 L2 A	57.40	22.60	94.80	1.85 X 10 ⁻⁹	5.24 X 10 ⁻⁶
Section 2 L2 B	54.20	23.20	95.60	1.63 X 10 ⁻⁹	4.62 X 10 ⁻⁶
Section 2 L3 A	49.90	18.50	98.50	2.22 X 10 ⁻⁹	6.29 X 10 ⁻⁶
Section 2 L3 B	58.20	22.60	96.80	2.73 X 10 ⁻⁹	7.74 X 10 ⁻⁶
Section 3 L1 A	53.10	18.00	99.90	4.43 X 10 ⁻⁹	1.26 X 10 ⁻⁵
Section 3 L1 B	55.20	22.00	94.30	2.6 X 10 ⁻⁹	7.37 X 10 ⁻⁶
Section 3 L2 A	49.80	19.00	97.10	2.28 X 10 ⁻⁹	6.46 X 10 ⁻⁶
Section 3 L2 B	53.40	22.00	93.80	5.37 X 10 ⁻⁹	1.52 X 10 ⁻⁵
Section 3 L3 A	55.60	24.00	94.00	5.02 X 10 ⁻⁹	1.42 X 10 ⁻⁵
Section 3 L3 B	54.30	19.30	98.50	3.51 X 10 ⁻⁹	9.95 X 10 ⁻⁶

9802 Palm River Road • Tampa, FL 33619 • (813) 740-8506 www.UniversalEngineering.com



Page 14 of 16

9802 Palm River Road, Tampa, FL 33619 - P: 813.740.8506 - F: 813.740.8706

In-Place Density Test Report

Client: Angelo's Materials 41111 Enterprise Road Dade City, FL 33525

Project: Enterprise Class III Landfill Cell 6 Pasco County, FL UES Technician: Mario Arroyo Date Tested: 4/24/2018

Type of Test:

Field:ASTM D-2937 Drive Cylinder MethodLaboratory:ASTM D1557 Modified Proctor

Reference Datum: 0 = Top of Fill

Area Tested: Section 1 -3

Material: Fill

The tests below meet the 95% minimum compaction requirement.

Test No.	Location of Test	Range	Maximum Density (pcf)	Optimum Moisture (%)	Field Dry Density (pcf)	Field Moisture (%)	Soil Compaction (%)	Pass or Fail
1	Section 1 Lift 1 A	6 inch	98.1	21.0	96.4	17.8	98	Pass
2	Section 1 Lift 2 A	6 inch	95.7	22.0	93.6	16.4	98	Pass
3	Section 1 Lift 3 A	6 inch	94.0	20.0	95.1	18.1	101	Pass
4	Section 2 Lift 1 A	6 inch	95.3	19.0	92.6	18.9	97	Pass
5	Section 2 Lift 2 A	6 inch	95.6	23.0	92.5	19.6	97	Pass
6	Section 2 Lift 3 A	6 inch	96.8	23.0	92.7	20.5	96	Pass
7	Section 3 Lift 1 A	6 inch	99.9	18.0	95.8	17.1	96	Pass
8	Section 3 Lift 2 A	6 inch	97.1	19.0	92.1	19.5	95	Pass
9	Section 3 Lift 3 A	6 inch	98.5	19.0	97.2	16.4	99	Pass

To establish a mutual protection to Universal's clients, the Public and ourselves, all reports are submitted as confidential property of our clients and authorization for publication of statements, conclusions or extracts from or regarding Universal's reports is reserved pending our written approval.

UNIVERSAL ENGINEERING SCIENCES Consultants In: Geotechnical Engineering • Environmental Sciences Geophysical Services • Materials Testing • Threshold Inspection Building Code Administration, Compliance Inspection & Plan Review UES Project No: 0810.1500214.0000 Workorder No: 81060-2 Report Date: 07/12/2018

9802 Palm River Road, Tampa, FL 33619 - P: 813.740.8506 - F: 813.740.8706

In-Place Density Test Report

Client: Angelo's Materials 41111 Enterprise Road Dade City, FL 33525

Project: Enterprise Class III Landfill Cell 6 Pasco County, FL

Area Tested: Section 1-3 B

UES Technician: Mario Arroyo Date Tested: 04/24/2018

Type of Test:

Field:ASTM D-2937 Drive Cylinder MethodLaboratory:ASTM D1557 Modified Proctor

Material: Clay Reference Datum: 0 = Top of Fill

The tests below meet the 95% minimum compaction requirement.

Test No.	Location of Test	Range	Maximum Density (pcf)	Optimum Moisture (%)	Field Dry Density (pcf)	Field Moisture (%)	Soil Compaction (%)	Pass or Fail
10	Section 1 Lift 1 B	6 inch	95.7	20.5	93.1	17.3	97	Pass
11	Section 1 Lift 2 B	6 inch	95.3	18.7	91.4	16.5	96	Pass
12	Section 1 Lift 3 B	6 inch	96.0	22.8	94.2	17.9	98	Pass
13	Section 2 Lift 1 B	6 inch	94.2	20.3	92.4	18.1	98	Pass
14	Section 2 Lift 2 B	6 inch	95.6	23.2	95.1	19.9	99	Pass
15	Section 2 Lift 3 B	6 inch	96.8	22.6	91.7	18.7	95	Pass
16	Section 3 Lift 1 B	6 inch	94.3	22.0	92.6	18.4	98	Pass
17	Section 3 Lift 2 B	6 inch	93.8	22.0	91.1	18.1	97	Pass
18	Section 3 Lift 3 B	6 inch	98.5	19.3	97.3	16.2	99	Pass

To establish a mutual protection to Universal's clients, the Public and ourselves, all reports are submitted as confidential property of our clients and authorization for publication of statements, conclusions or extracts from or regarding Universal's reports is reserved pending our written approval.

UNIVERSAL ENGINEERING SCIENCES Consultants In: Geotechnical Engineering • Environmental Sciences Geophysical Services • Materials Testing • Threshold Inspection Building Code Administration, Compliance Inspection & Plan Review UES Project No: 0810.1500214.0000 Workorder No: 81060-3 Report Date: 07/12/2018

9802 Palm River Road, Tampa, FL 33619 - P: 813.740.8506 - F: 813.740.8706

In-Place Density Test Report

Client: Angelo's Materials 41111 Enterprise Road Dade City, FL 33525

Project: Enterprise Class III Landfill Cell 6 Pasco County, FL

Material: Clay

Reference Datum: 0 = Top of Fill

Area Tested: Header and Berm

UES Technician: Mario Arroyo Date Tested: 04/24/2018

Type of Test:

Field:ASTM D-2937 Drive Cylinder MethodLaboratory:ASTM D1557 Modified Proctor

The tests below meet the 95% minimum compaction requirement.

Test No.	Location of Test	Range	Maximum Density (pcf)	Optimum Moisture (%)	Field Dry Density (pcf)	Field Moisture (%)	Soil Compaction (%)	Pass or Fail
19	Header trench	6 inch	97.2	23.0	94.4	18.9	97	Pass
20	Berm	6 inch	99.6	23.0	95.7	19.1	96	Pass

Attachment E

Pickett and Associates, Inc. Tolerance Correspondence

----- Forwarded message ------From: **Jeff Young** <<u>jyoung@pickettusa.com</u>> Date: Tue, Oct 30, 2018 at 11:22 AM Subject: Enterprise topo and imagery To: John Arnold <<u>john.phillip.arnold@gmail.com</u>>

John,

As the topo survey was performed using aerial lidar and imagery I am not surprised there are differences between our elevation values and your ground surveyed shots, especially if the area in question is vegetated. By Florida Standards of Practice regulations, an aerial survey's stated plus or minus tolerances encompass a minimum of 90% of the difference between photogrammetrically measured values and any ground truth of all well-identified features. Any spot elevations on paved surfaces may be measured to an estimated vertical positional accuracy of \pm -0.25'. Elevations in areas where the ground is obscured (either by vegetation, shadow or other structures) do not have to follow these stated accuracies and no accuracy statement must be supplied. Ultimately the aerial mapping should be used for preliminary design work only and should not replace an actual field survey.

Let me know if I can be of further assistance.



T. Jeffrey Young, PSM, CP | Pickett and Associates, Inc.

O: 863.533.9095 x 506 | C: 863.670.9607 |

475 S. First Avenue. Bartow, FL 33830 | http://www.PickettUSA.com



--John Arnold, P.E. Ph. (813) 477-1719